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T						REVISIONS	DATE	BY
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VOT TO BE REPRODUCED								
CEILING OUTLET	,-O, ,6					Copyright Frank Betz All Rights Reserved. T Reproduction, Copying Licensing, or any other use of these drawings thereof, or the plans	Associates, he Duplicat , Sale, Ren r distributi s, any porti depicted h	, Inc. tion, tal, ion or ion ereon
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CEILING OUTLET	, , , , , , ,	54'-O´				D		* * * *
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1 100 1 100 1 100 1 100 5'-1'' 2-4 5 0 1 1 1 1 1 1 1 1 1 1 1 1 1	3,-6,		5'-6''				M M C	
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LAUND. SINK L 2-4 POCKET \$\$ DOOR	, 5, , , , , , , , , , , , , , , , , ,					SST F	THF	- - -
		•	-					
	LASTER- (EE DET. 3, , , HT. 1 d	V 2						
FPL.	; ~ ~ !	t .				INC	E PARKW/	GIA 30144 38-717-30
		★ 14′−6			1	T E S,	GE BUSBE SUITE 190	N, GEORC 38 81
2'-4'')					550 GEOR	KENNESA) 70-431-08
2 x 6 WALL- BRACE AS REQD.			.			S S O		
≻	ć	N •					P-	
	č U L)				BY: <i>PLD</i> CK:	KKW	
2''		↓ ,,0-,∠1		SQUARE FOOTAGE FIRST FLOOR 2745 SQ, FT, SECOND FLOOR 1133 SQ, FT,		DATE: <i>6-29-05</i>		
		5		TOTAL 3878 SQ. FT. OPT. BONUS RM. 649 SQ. FT. TOTAL 4527 SQ. FT.		SHEET:		٦
7'-8''	FIRST	Ļ FLC)) (R PLAN		~		
/	1⁄4″ = 1′-0″					of: 6		



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NORTHFIELD

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- 4. BUILDER MUST CHECK, VERIFY, AND APPROVE ALL DIMENSIONS, LENGTHS, FRAMING CONDITIONS, AND LOADING METHODS BEFORE STARTING CONSTRUCTION. 5. ROOF BRACING METHODS MUST BE DISCUSSED WITH A
- TECHNICAL REPRESENTATIVE OF JOIST MANUFACTURER O INSURE ADEQUATE MATERIAL IS PROVIDED. 6. FILL AND CONNECT DOUBLE JOISTS. INTERIOR BEARINGS MUST BE LEVEL WITH EXTERIOR
- BEARINGS. 8. 8d NAILS ARE REQUIRED FOR PROPER JOIST ATTACH-MENT. DO NOT USE 16d NAILS. TAKE CARE NOT TO SPLIT FLANGES.
- 9. LOCATE PROBABLE PLUMBING DROPS BEFORE CONSTRUCTION BEGINS. SHIFT JOISTS TO ALLOW DROP OR BUILD PLUMBING WALL ABOVE. 10. REVIEW OF THIS LAYOUT BY A PROFESSIONAL ENGINEER TO INSURE STRUCTURAL INTEGRITY IS STRONGLY ENCOURAGED.

	Bi	EAM SCHEDULE	
$\langle A \rangle$	NOT USED		
$\langle B \rangle$	NOT USED		
$\langle \mathcal{C} \rangle$	NOT USED		
$\langle \mathcal{O} \rangle$	NOT USED		



FRAMING PLAN NOTES STANDAD CONSTRUCTION FRANTICAL USING STANDAD CONSTRUCTION FRANTICAL INTERVATIONAL EGEORGIA THEY CONFORM TO THE INTERVATIONAL EVENTS AND THEY CONSTRUCTIONS IN LOCAL CODES & GEOLGICAL CONDITIONS, RAVE BE REQUIRED TO THESE PLANS. 2.ALL WORK SHOULD BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE LOCAL CODES, REGULATIONS, AND HUD MPS, THE BUILDER SHOULD VERIFY ALL CONSTRUCTION ON THE FLATES PLANS. 2.ALL WORK SHOULD BE DERFORMED IN ACCORDANCE WITH COLSE INFE LOCAL CODES, REGULATIONS, AND HUD MPS, THE BUILDED TO INSURE COMPLIANCE WITH CODE AND STRUCTURAL ENGINEERS AND CONSTRUCTION WITH ALLS PRIOR TO USING THE FRAMING MATERIAL BIS PRIOR TO USING THE FRAMING MATERIAL BIS PRIOR TO USING THE FRAMING PLANS AND MATERIAL LIST THAT DO NOT MATCH. 3.THE FOOLOWING GUIDELINES ARE PROVIDED TO FEATHAIN GUIDELINES AND THE FRAMING PLANS AND MATERIAL LIST THAT DO NOT MATCH. 4.THE FOLLOWING GUIDELINES AND THEIR ROUTH CLEAS SHOULD AN THE SOLUTIED A STRUCT ON THE STRUMENES WILL RESULT IN A FRAMING PLAN AND MATERIAL LIST THAT DO NOT MATCH. 5.ALL PECES ANE LABBLED AS TO THEIR ROUTH CLEAS SHOULD EAST ON THE STRUMENES WILL RESULT IN A FRAMING PLANS AND MATERIAL LIST THAT DO NOT MATCH. 5.ALL PECES AND CHEDERS AND STRUESS OF AND CLEASE SHOULD AS INDICATED ON PHANS. 4.ALL BEEMS AND CHEDEES SHOULD HAVE A MUNICAMED EMENSION STRUESS OF ANT AND MATERIAL LIST THAT DO NOT MATCH. 5.ALL HEADERS AND CHEDERS AND EXTENSION FOR THESE OF ANT AND MATERIAL LIST THAT DO NOT MATCH. 6.ALL FRANCH AND ALLOWABLE BEAMS SHOULD HAVE A MUNICAMED EMENSION STRUESS OF ANT AND AND ALLOWABLE BEAMS SHOULD HEAD AND ANT AND ALLOWABLE BEAMS SHOULD HEAD AND ANT AND ALLOWABLE BEAMON STRUESS OF ANT AND ANT AND ANT AND ANT AND AND AND AND AND AND AND AND A FRAMING PLAN NOTES

- I JOIST NOTES
- 1. THE I JOIST ARE SPECIFIED USING THE APA STANDARD PRI 400, PERFORMANCE STANDARD FOR APA EWS I JOIST. FOLLOW MANUFACTURERS GUIDELINES FOR THE PROPER INSTALLATION OF JOIST, HANGERS, SQUASH BLOCKS AND DECKING. 2. ALL RIM BOARD TO BE APA RIM BOARD – VERTICAL LOAD TRANSFER = 4400 PLF MAX. AMOUNT REQUIRED IS LISTED AS TOTAL
- LINEAR FOOTAGE. 3. THIS I JOIST LAYOUT MUST BE REVIEWED BY A TECHNICAL REPRESENTATIVE OF THE ACTUAL JOIST MANUFACTURER SUPPLYING THE CONSTRUCTION MATERIALS. SOME ADJUSTMENTS MAY BE REQUIRED
- DUE TO VARIATIONS IN PRODUCTS.
 4. BUILDER MUST CHECK, VERIFY, AND APPROVE ALL DIMENSIONS, LENGTHS, FRAMING CONDITIONS, AND LOADING METHODS BEFORE STARTING CONSTRUCTION. 5. ROOF BRACING METHODS MUST BE DISCUSSED WITH A
- TECHNICAL REPRESENTATIVE OF JOIST MANUFACTURER TO INSURE ADEQUATE MATERIAL IS PROVIDED. 6. FILL AND CONNECT DOUBLE JOISTS. 7. INTERIOR BEARINGS MUST BE LEVEL WITH EXTERIOR
- BEARINGS. 8. 8d NAILS ARE REQUIRED FOR PROPER JOIST ATTACH-MENT. DO NOT USE 16d NAILS. TAKE CARE NOT TO SPLIT FLANGES. 9. LOCATE PROBABLE PLUMBING DROPS BEFORE
- CONSTRUCTION BEGINS. SHIFT JOISTS TO ALLOW DROP OR BUILD PLUMBING WALL ABOVE. 10. REVIEW OF THIS LAYOUT BY A PROFESSIONAL ENGINEER TO INSURE STRUCTURAL INTEGRITY IS STRONGLY ENCOURAGED.

	BEAM SCHEDULE
$\langle E \rangle$	2 - 1 3/4" x 11 7/8" x 12' L VL FLUSH
$\langle \mathcal{F} \rangle$	2 - 1 3/4" x 11 7/8" x 10° L VL FL USH
$\langle G \rangle$	2 - 2 × 10 × 8' - DROPPED
$\langle H \rangle$	2 - 1 3/4" x 9 1/4" x 8" L VL DROPPED
$\langle I \rangle$	2 - 1 3/4'' x 9 1/4'' x 8' L VL FLUSH AT 9'-O'' A.F.F.
$\langle \mathcal{I} \rangle$	3 - 1 3/4'' x 14'' x 20' L VL FLUSH
$\langle k \rangle$	2 - 1 3/4'' x 11 7/8'' x 14' L VL, - FLUSH
$\langle L \rangle$	2 - 1 3/4" x 11 7/8" x 24" L VL FLUSH
	2 - 1 3/4'' x 11 7/8'' x 8' L VL FLUSH
$\langle \! \! \! \! \! \rangle $	2 - 1 3/4" x 16" x 16" LVL FLUSH - EXTEND INTO WALL ABOVE
$\langle \mathcal{O} \rangle$	2 - 1 3/4'' x 11 7/8'' x 14' L VL, - FLUSH
$\langle P \rangle$	2 - 1 3/4" x 11 7/8" x 12' L VL HEADER AT 9'-2 7/8" A.F.F.
$\langle \mathcal{Q} \rangle$	2 - 1 3/4" x 11 7/8" x 10° L VL DROPPED
$\langle \mathcal{R} \rangle$	2 – 1 3/4'' x 9 1/4'' x 10' L VL. – HEADER AT 8'–0 7/8'' A.F.F.

QUANTITIES	
I × 11 7/8'' PRI-20 × 2'	01
I × 11 7/8'' PRI-20 × 4'	01
I × 11 7/8'' PRI-20 × 6'	02
I × 11 7/8'' PRI-20 × 8'	02
I × 11 7/8'' PRI-20 × 10'	10
I × 11 7/8'' PRI-20 × 12'	01
I × 11 7/8'' PRI-20 × 14'	17
I × 11 7/8'' PRI-20 × 16'	01
I × 11 7/8'' PRI-20 × 20'	06
I × 11 7/8'' PRI-20 × 22'	01
I × 11 7/8'' PRI-20 × 24'	01
I × 14'' PRI-40 × 16'	01
I × 14'' PRI-40 × 20'	12
I × 16'' PRI-50 × 4'	02
I × 16'' PRI-50 × 20'	02
I × 16'' PRI-50 × 24'	33
11 7/8" RIM BD. – L.F.	100
14'' RIM BD L.F.	80
16'' RIM BD L.F.	92

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2ND. FLOOR FRAMING PLAN			ITE NUKITIELU MANUK
FRANK BETZ Associates, inc.	DESIGNERS OF CUSTOM AND STOCK HOMEPLANS	3550 GEORGE BUSBEE PARKWAY SUITE 190	KENNESAW, GEORGIA 30144 770-431-0888 888-717-3003
BY: XLS	CK: 4	DCF	
DATE: 6-24	9-05		
SHEET:			
	-2		

QUANTITIES	
2 x 6 x 10	10
2 x 6 x 14	01
2 x 6 x 16	26
2 x 8 x 10	69
2 x 8 x 12	09
2 x 8 x 14	37
2 x 8 x 16	110
2 x 8 x 18	05
2 x 10 x 10	06
2 x 10 x 12	02
2 x 10 x 14	07
2 x 10 x 16	07

	BEAM SCHEDULE
$\langle S \rangle$	3 - 2 × 10 × 14' - FLUSH
$\langle 7 \rangle$	3 – 1 3/4'' x 11 7/8'' x 18' L VL. – FLUSH
$\langle U \rangle$	2 - 1 3/4" x 11 7/8" x 14' L VL FLUSH
$\langle \nu \rangle$	3 - 1 3/4'' x 16'' x 22' L VL FLUSH
<i>u</i>	2 - 2 × 10 × 10' - FLUSH

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CLG. JOIST FRAMING PLAN	THE NORTHFIELD MANOR
FRANK BETZ ASSOCIATES, INC. Designers of custom and stock homeplans	3550 GEORGE BUSBEE PARKWAY SUITE 190 KENNESAW, GEORGIA 30144 770-431-0888 888-717-3003
BY: XLS CK:	DCF
DATE: 6-29-05	
SHEET:)

FRAMING PLAN NOTES 1. THESE FRAMING PLANS WERE DESIGNED USING STANDARD CONSTRUCTION PRACTICES IN ATLANTA, GEORGIA. THEY CONFORM TO THE INTERNATIONAL BUILDING CODE AND TWO FAMILY DWELLINGS. DUE TO VARIATIONS IN LOCAL CODES & GEOLOGICAL CONDITIONS, REVISIONS MAY BE REQUIRED TO THESE PLANS. 2. ALL WORK SHOULD EF PERFORMED IN ACCORDANCE WITH ALL APPLICABLE LOCAL CODES REGULATIONS, AND HUD MPS. THE BUILDER SHOULD VERIFY ALL CONDITIONS BEFORE BEGINNING CONSTRUCTION, CONSULT WITH LOCAL STRUCTURAL INTEGRITY. 3. DESIGN LOADS: FLOOR: 40LBS LIVE LOAD 10LBS DEAD LOAD 10LBS DEAD LOAD 20LBS LIVE LOAD 10LBS DEAD LOAD BEDROM: 30LBS LIVE LOAD 10LBS DEAD LOAD BEDROM: 30LBS LIVE LOAD 10LBS DEAD LOAD CHING: 20LBS LIVE LOAD 10LBS DEAD LOAD BEDROM: 30LBS LIVE LOAD 10LBS DEAD LOAD BEDROM: 30LBS LIVE LOAD 10LBS DEAD LOAD CHING: 20LBS LIVE LOAD 10LBS DEAD LOAD BEDROM: 30LBS LIVE LOAD 10LBS DEAD LOAD CHING: 20LBS LIVE LOAD 10LBS DEAD LOAD STHE FOLLOWING GUIDELINES ARE PROVIDED TO EXPLAIN HOW THE QUANITIES LISTED ON THE FRAMING PLANS AND MATERIAL LIST WHERE DETERMINED, FAILURE TO COMPLY WITH THESE GUIDELINES WILL RESULT IN A FRAMING PLAN AND MATERIAL LIST THAT DO NOT MATCH. 6. ANY LABELED ON FRAMING PLANS. 6. ANY LABELED ON FIRMING PLANS. 6. ANY LABELED ON FRAMING SCUT FROM A 16 PIECE. 7. ALL BEAMS ARE 2XIO #1 SYP, 2XI2 #1 SYP, LAMINATED VENEER LUMBER (LVL), OR GUU-LAMINATED BEAMS (GLB) AS INDICATED ON PLANS. 8. GUU-LAMINATED BEAMS (GLB) AS INDICATED ON PLANS. 8. GUI-LAMINA ALD WABLE BENDING STRESS OF 2400 PSI. 9. LAMINATED VENEER LUMBER SEAMS SHOULD HAVE A MINIMUM ALLOWABLE BENDING STRESS OF 2400 PSI. 9. LAMINATED VENEER LUMBER MEAMS SHOULD HAVE A MINIMUM ALLOWABLE BENDING STRESS OF 2400 PSI. 9. LAMINATED VENEER LUMBER MEAMS SHOULD HAVE A MINIMUM ALLOWABLE BENDING STRESS OF 2400 PSI. 9. LAL ELOOR JOIST ARE 2XIO #2 SYP < FRAMING PLAN NOTES ALL 2 X 10 X 16' AND 2 X 10 X 18' FLOOR JOIST TO BE # 1 SYP. 11. ALL BAND MATERIAL IS 2X10 #2 SYP. 12. 1X4 CROSS-BRIDGING SHOULD BE USED AT MID POINT OF SPAN OR 8'-0" O.C. MAXIMUM IN ALL FLOORS. 13. ALL CEILING JOISTS ARE 2X8'S #2 SPF AT 16" O.C. EXCEPT AS NOTED ON PLANS. ALL TRAY CEILING JOISTS ARE RAFTER TO KNEEWALL IN LENGTH. SEE TRAY DETAIL SHOWN ON PLANS. 14. ALL RAFTERS ARE 2X8'S #3 SPF AT 16" O.C. EXCEPT AS NOTED ON PLANS. 15. ALL RAFTERS ARE 2X8'S #3 SPF AT 16" O.C. EXCEPT AS NOTED ON PLANS. 16. ALL RAFTERS SHOULD BE BRACED AS CLOSE TO MID-SPAN AS POSSIBLE WITH NO SPAN EXCEEDING 13'-0". 17. ALL RAFTERS OVER A VAULTED ROOM ARE 2 x 10'S #2 SPF AT 16" O.C. EXCEPT AS NOTED ON PLANS. 18. ALL CEILING JOISTS AND RAFTER BRACING TO BEAR ON LOAD BEARING WALLS DESIGNED TO CARRY LOAD THRU ALL LEVELS AND TERMINATE AT FOUNDATION AND BE SUPPORTED BY THICKENED SLAB GRADE BEAM OR FOOTING DESIGNED TO CARRY LOAD.

QUANTI TIES	
2 x 6 x 10	19.
2 x 6 x 16	06
2 x 8 x 10	37
2 x 8 x 12	44
2 x 8 x 14	50
2 x 8 x 16	97
2 x 8 x 18	11
2 x 8 x 20	04
2 x 8 x 22	16
2 x 8 x 24	26
2 x 8 x 26	05
2 x 8 x 28	03
2 x 10 x 10	07
2 x 10 x 12	12
2 x 10 x 14	07
2 x 10 x 16	11
2 x 10 x 18	04
2 x 10 x 20	03
2 x 10 x 22	03
2 x 10 x 26	04

2 x 12 x 16

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ROOF FRAMING PLAN THE NORTHFIELD MANOR	HOMEPLANS	Leagned for rodays Markel ®	www.Irankbetz.com
	ROOF FRAMING PLAN		IHE NOR INFIELD MANOR
C I A T E S, INC. C I A T E S, INC. CUSTOM AND STOCK HOMEPLANS 550 GEORGE BUSBEE PARKWAY SUITE 190 KENNESAW, GEORGIA 30144	RANK BETZ SSOCIATES, INC.	3550 GEORGE BUSBEE PARKWAY SUITE 190	KENNESAW, GEORGIA 30144 770-431-0888 888-717-3003
FRA ASSO BESIGNERS OF			

BY: XLS CK: DCF
DATE: 6-29-05
SHEET:

First Floor System Layout Scale: 3/16" = 1'-0"

			ENGINEERED WOOD				
	<u>Architectural Drawings</u> <u>Prepared By:</u>	Enter Architect Info (or erase this text)	Original Plan Date: Enter Original Plan Date	Latest Revision: Enter Latest Revision Date	TECHNICAL SUPPORT	Rock Hill, SC - (803) 323-1650 Columbia, SC - (803)-788-8950	Locust, NC - (704) 888-4411 Monroe, NC - (704) 289-8441
			1	1		# MyBMC #	
	MINGS					Date BCC	
	AD CUMN					Release D	5/6/202
	BR					Drawn By	
	COMMENTS	System Layout					
Wall Legend Bearing Wall Non-Load Bearing Wall	DATE	K ##/##/## Original	1			1	1
SALES PRESENTATION DRAWING This layout and associated materials list has been prepared based on project plans and/or information provided to BMC by the builder. It remains the responsibility of the builder, architect, engineer of record, or other responsible persons to review this information to assure that it is appropriate, accurate,complete and complies with applicable building codes.	REV. BY	· ·	' ' S 1	he of	eet		1

Verity capacity and fastening requirements of hangers and

Minimum nail sizes:

DON' drill flange

connectors

			ENGINEERED WOOD				®
	rchitectural Drawings Prepared By:	inter Architect Info (or erase this text)	Vriginal Plan Date: Enter Original Plan Date	atest Revision: Enter Latest Revision Date		Коск ПІІІ, ЭС - (803) 323-1650 Columbia, SC - (803)-788-8950	Locust, NC - (704) 888-4411 Monroe, NC - (704) 289-8441
	D CUMMINGS					Release Date BCC # MyBMC #	5/6/2021
	BRA			UEVA		Drawn By	
	COMMENTS	ginal System Layout					
Wall Legend Bearing Wall Non-Load Bearing Wall Non-Load Bearing Wall SALES PRESENTATION DRAWING This layout and associated materials list has been prepared based on project plans and/or information provided to BMC by the builder. It remains the responsibility of the builder, architect, engineer of record, or other responsible persons to review this information to assure that it is appropriate	REV. BY DATE	- XXX ##/##/## Oric	, , , , ,		· ·	•	1

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		C	Client:				Da	ate:	5/5/20)21				Pa	age 1 of 1
Tic	Decign	F	Project:				IN	put by:	D						
15	Design	4	Address:				JC	ob inam	e: Deva	ne					
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Beam E	LP-LVL 2	900FD-2.	0E 1./	/50" X	11.8/5	2-Piy	- PASS	ED	Level. Le						
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								Υ							
			1					\mathbb{N}							
														$\Lambda \Lambda$	1
LP				LP						P				IXIXI	11 7/8"
	C. T. TALL	- Mine	and the second						- 124-		dala a	11.114	1	$\langle V \rangle$	
1 SPF												2 SPF			
/ <u>/ </u>					10'8"										/2"
<u> </u>					10'8"									1 10	_
					100								I		
Member Inf	formation						Reaction	ns UN	PATTE	RNED I	b (Uplif	t)			
Туре:	Girder		Applicatio	n: F	loor		Brg	Liv	е	Dead	Snov	v	Wind	Const	
Plies:	2		Design Me	ethod: A	ASD		1	45	4	768	(D	0	0	
Moisture Cond	lition: Dry		Building C	Code: I	BC/IRC 2015		2	103	9	989	(D	0	0	
Deflection LL:	480		Load Shar	ring: N	No										
Deflection 1L:	240		Deck:	Г	Not Checked										
Importance:	Normal	0°F													
Temperature:	Temp <= 10	0'F					Bearings								
							Bearing	l enat	h (an Rea	act D/L lb	Total			omb
								3 500"		лар. пес 23%		1014			JIID.
							2 905	3.500		20%	100/404	2029	1		
Analysis Re	sults						2-365	3.300		3970 8	0097 1009	2020	L	D+L	
Analysis	Actual	Location A	Allowed	Capacity	Comb.	Case	7								
Moment	5675 ft-lb	7'4" 1	19902 ft-lb	0.285 (29%	6) D+L	L									
Shear	1891 lb	9'5 3/8" 7	′897 lb	0.239 (24%	6) D+L	L									
LL Defl inch	0.055 (L/2243)	6'1 3/8" 0).255 (L/480)	0.210 (21%	6) L	L									
TL Defl inch	0.107 (L/1149)	5'10 1/2" 0).510 (L/240)	0.210 (21%	%) D+L	L									
Desian Not	es						1								
1 Provide late	eral support to prev	ent rotation at	end bearings	and at inte	rior bearings w	vhen	1								
required by	code for seismic d	lesign.		0"											
3 Girders are	designed to be su	pported on the	bottom edge	only											
4 Multiple plie	es must be fastene	d together as p	per manufactu	irer's details	s.										
5 Top loads n	nust be supported	equally by all p	lies.												
6 Top braced	at bearings.														
	Load Type	I	ocation Tr	rih Width	Side	Dead 0.9		1 Sn	ow 1 15	Wind	1.6 Con	st 1 25	Commer	its	
1	Liniform	L			Ton	100 PLF		=		01	1.0 0011 PIF	0 PI F	Ceiling Io	iete	
י ר	Point		7_4_0		Top	564 lb	1/03	`		01	0.16		Beam E B	isis	
Z	Point Boaring Longt	ь	038		юр	J04 ID	1495 1	J	u u		aro	u u	Deam F D	iy z	
	Solf Moight	11	0-3-0												
	Sell Weight					12 PLF									
									Manufac	turer Info		F	MC/Locust	l umber C	ompany
Notes This component and	alysis is based on the	loads,							Louisiana	a-Pacific Co	orp	3	12 E. Main	Street, No	rth Carolina
geometry and other of and listed in this rep	conditions as entered by th port. The user is response	e user ible to							414 Unio	n Street, S	uite 2000	2	8127 04-888-441	1	
ensure the accuracy of the actual conditions	of the input and the applicat of the structure for which	bility to this							(888) 820)-0325	2		01000111		
product listed.	1. This analysis is valid only	Desifie							APA: PR	orp.com -L280, ICC	-ES: ESR-2	403,			
Corp. 414 Union St Su	ite 2000, Nashville, TN 372	19				Thi	is design is valid	until	LADBS:	RR-25783,	Florida: FL	15228	B	MC	7

	•		Client:				Da	ate:	5/5/20	21			Page 1 of 1
is	Design		Address:				in Jo	put by: bb Name	e: Devar	ie			
-				75011 1/			Pr	oject #:					
Beam F	LP-LVL 2	900FD-2	2.0E 1.	.750" X '	11.875"	2-Piy	- PASS	ED	Level. Le	vei			
		2											
		2											
				_ 1									
													\mathbf{M} 1
LP				LP	-								11 7/8
	and the second	- Aller											
1 SPF								2	SPF				
				8'3 1/2"								,	3 1/2"
1				8'3 1/2"					1	•			
Member In	formation						Reaction		DATTE		Inlift)		
Туре:	Girder		Applicati	on: F	loor		Brg	Live	e	Dead	Snow	Wind	Const
Plies:	2		Design N	/lethod: A	SD		1	149	3	588	0	0	0
Moisture Cond	dition: Dry		Building	Code: IE	3C/IRC 2015		2	149	3	588	0	0	0
Deflection TL:	400 240		Deck	anng. N N	o ot Checked								
Importance:	Normal												
Temperature:	Temp <= 10	00°F											
							Bearings	5					
							Bearing	Lengt	h C	ap. React [D/L lb T	otal Ld. Case	Ld. Comb.
							1 - SPF	3.500"		40% 588 /	1493 2	2081 L	D+L
Analysis Re	sults						2 - SPF	3.500"	4	40% 588 /	1493 2	2081 L	D+L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case]						
Moment	3850 ft-lb	4'1 3/4"	19902 ft-lb	0.193 (19%) D+L	L							
Shear	1469 lb	1'2 5/8"	7897 lb	0.186 (19%) D+L	L							
LL Defl inch	0.039 (L/2418)	4'1 13/16"	0.196 (L/480)) 0.200 (20%) L	L							
	0.054 (L/1734)	41113/16	0.392 (L/240) 0.140 (14%) D+L	L	4						
1 Provide late	eral support to pre	vent rotation a	at end bearing	s and at interi	or bearings w	/hen	1						
required by	code for seismic	design.			o. Doarnigo n								
2 Dead Load 3 Girders are	Deflection: Instan	t = 0.015", Lo inported on th	ng Term = 0.0 ne bottom eda	23" e only									
4 Multiple plie	es must be fastene	ed together as	s per manufac	turer's details.									
5 Top loads n	nust be supported	equally by all	plies.										
7 Bottom bra	ced at bearings.												
ID	Load Type		Location	Frib Width	Side	Dead 0.9	Live	1 Sno	ow 1.15	Wind 1.6	Const. 1	.25 Comment	s
1	Uniform				Тор	50 PLF	200 PLF	=	0 PLF	0 PLF	0 F	PLF Floor Load	
2	Uniform				Тор	80 PLF	160 PLF	=	0 PLF	0 PLF	0 F	PLF Roof Load	
	Self Weight					12 PLF							
Notes									Manufac	turer Info		BMC/Locust L	umber Company
This component and geometry and other of	alysis is based on the	loads,							Louisiana	-Pacific Corp	2000	312 E. Main S	treet, North Carolina
and listed in this re ensure the accuracy of	port. The user is respon- of the input and the applica-	sible to ability to							414 Unio Nashville	n Street, Suite 2 , TN 37219	2000	704-888-4411	
the actual conditions component is intended product listed	or the structure for wh t. This analysis is valid only	ich this / for the							(888) 820 www.lpcc	rp.com			
Copyright 2019 All rig Corp. 414 Union St Su	hts reserved by Louisiana ite 2000, Nashville, TN 372	Pacific 219				Thi	s desian is valid	until	apa: PR- Ladbs: F	L280, ICC-ES: RR-25783, Flor	ESR-2403, da: FL15228	D	VIC
						10/	31/2021					Ð	

			Client				D	ata:	E/E/DO	04			Dc	ac 1 of 1
			Dilent. Project:				Da In	ale.	5/5/202	21			Fa	ige i oi i
	Design		Δddress [.]				 Ic	bu Nam	e [.] Devan	<u>_</u>				
		,					Pr	roiect #		5				
Boom U		000Eh 2		750" V	0.250"	2 DIV	DACCE		Level: Lev	el				
Dealli П		.900FD-2	.02 1.	/50 A	9.230	2-Piy -	PASSE	U.						
		2												
				1										
														\uparrow
LP						P							IVIVI	
	Coltan.		-	-	Strange	•		- 20	- The second				MA	9 1/4
		alan and a start of the start	and the second										<u> </u>	<u> </u>
1 SPF										2 SPF				
1				7'2'						1			13 1	/2"
1				7'2						1				
Member Inf	ormation		1				Reaction	ns UN	PATTER	NED lb (l	Jplift)			
Туре:	Girder		Applicatio	n: F	loor		Brg	Liv	e [Dead	Snow	Wind	Const	
Plies:	2		Design M	ethod: A	SD		1	322	:5	1646	0	0	0	
Moisture Cond	lition: Dry		Building C	Code: IE	3C/IRC 2015		2	322	5	1646	0	0	0	
Deflection LL:	480		Load Sha	ring: N	0									
Deflection TL:	240		Deck:	N	ot Checked									
Importance:	Normal													
Temperature:	Temp <= 10)0°F					- ·							
							Bearings	5						
							Bearing	Lengt	ih Ca	ap. React D	0/Llb T	otal Ld. Ca	ise Ld. Co	mb.
							1 - SPF	3.500"	' 9	4% 1646 /	3225 4	871 L	D+L	
							2 - SPF	3.500"	' 9	4% 1646 /	3225 4	871 L	D+L	
Analysis Re	sults						1							
Analysis	Actual	Location A	Allowed	Capacity	Comb.	Case								
Moment	7646 ft-lb	3'7"	12416 ft-lb	0.616 (62%) D+L	L								
Shear	3511 lb	1' (6151 lb	0.571 (57%) D+L	L								
LL Defl inch	0.107 (L/753)	3'7 1/16"	0.168 (L/480)	0.640 (64%) L	L								
TL Defl inch	0.161 (L/499)	3'7 1/16" (0.335 (L/240)	0.480 (48%) D+L	L]							
Design Not	es													
1 Provide late	ral support to pre-	vent rotation at	end bearings	and at interi	ior bearings v	vhen	1							
required by	code for seismic (design. t = 0.055" I on	a Torm = 0.09	0 "										
3 Girders are	designed to be su	ipported on the	e bottom edge	only										
4 Multiple plie	s must be fastene	ed together as	per manufactu	irer's details.										
5 Top loads n	nust be supported	equally by all p	olies.											
6 Top braced	at bearings.													
	Load Type		_ocation Tr	ib Width	Side	Dead 0.9	Live '	1 Sno	ow 1.15	Wind 1.6	Const. 1	.25 Comm	ients	
1	Uniform	•			Тор	150 PLF	600 PLF	F	0 PLF	0 PLF	0 F	PLF Floor L	.oad	
2	Uniform				Тор	300 PLF	300 PI I	F	0 PLF	0 PLF	0 F	PLF Roof/C	eilina Load	
L	Self Weight				100		00012		0121	01 Ei	01		olinig Loud	
	Sell Weight					9 FLF								
									Manufact	urer Info		BMC/Locu	ist Lumber Co	ompany
Notes This component and	lvsis is based on the	loads.							Louisiana-	Pacific Corp		312 E. Ma	in Street, Nor	th Carolina
geometry and other of and listed in this ren	onditions as entered by the	he user sible to							414 Union	Street, Suite 2	000	28127	411	
ensure the accuracy of the actual conditions	f the input and the applica of the structure for wh	ability to ich this							(888) 820-	0325		, 54-000-4		
product listed.	. mis analysis is valid only	Desifie							www.lpcor APA: PR-L	p.com 280, ICC-ES:	ESR-2403.			
Copyright 2019 All rig Corp. 414 Union St Su	nts reserved by Louisiana ite 2000, Nashville, TN 372	Pacific 219				Thi	s design is valid	l until	LADBS: R	R-25783, Flori	da: FL15228		BMC	-
						10/	31/2021		1					

			Client:				Da	ate:	5/5/2021					Pa	ge 1 of 1
	- •		Project:				In	put by:							
IS	Design		Address:				Jo	b Name	: Devane						
							Pr	oject #:							
Beam J	LP-LVL 29	900Fb-2	.0E 1	.750" X	14.000"	3-Ply	- PASS	ED	_evel: Level						
			1)				2 ↓							
LP	(mitte	LP		LP	att a gar	- Contra	LP		*:	LP	1914 P	17		\mathbb{M}	1'2"
1 SPF												2 SP	F		
/					19'7"								-	5 1/4	
/					19'7"								\rightarrow		
I					107								I		
Momborin	formation						Postion		DATTEDN		Inlift)				
	Girder		Applicat	ion [.] F	loor		Brg	Live		ad	Snow		Wind	Const	
Plies:	3		Design I	Method: A	ASD		1	797	·	328	0		0	001131	
Moisture Cond	dition: Dry		Building	Code: I	BC/IRC 2015		2	1843	2	352	0		0	0	
Deflection LL:	480		Load Sh	aring: Y	⁄es										
Deflection TL:	240		Deck:	Ν	lot Checked										
Importance:	Normal														
Temperature:	Temp <= 10	0°F					Booring								
							Dearings	•		Deast	V/L II.	Tatal			un la
							Bearing	Lengtr	i Cap	 React L 4000 	//L ID	Iotal	Ld. Case	Ed. Co	mb.
							1 - SPF	3.500"	349	6 1828 / 0050/	/ 797	2625	L .	D+L	
Analysis Re	sults						2 - SPF	3.500	54%	0 23527	1643	4195	L	D+L	
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	7								
Moment	21584 ft-lb	13'7"	42165 ft-lb	0.512 (51%	6) D+L	L									
Shear	3991 lb	18'2 1/4"	13965 lb	0.286 (29%	6) D+L	L									
LL Defl inch	0.236 (L/972)	10'11 1/2"	0.478 (L/480) 0.490 (49%	6) L	L									
TL Defl inch	0.546 (L/421)	10'6 1/2"	0.956 (L/240) 0.570 (57%	6) D+L	L									
Desian Not	es						1								
 Provide late required by Dead Load Girders are Multiple plice 	eral support to prev code for seismic d Deflection: Instant designed to be sup es must be fastener	rent rotation a lesign. = 0.309", Lo pported on th d together as	at end bearing ng Term = 0.4 ne bottom edg per manufac	gs and at inter 164" le only. turer's details	ior bearings v	vhen									
6 Top must b	e laterally braced a	t a maximum	of 8'10 7/8"	D.C.											
7 Bottom bra	ced at bearings.														
ID	Load Type		Location	Trib Width	Side	Dead 0.9	Live ?	1 Sno	w 1.15	Wind 1.6	Const.	1.25	Commer	nts	
1	Uniform				Тор	125 PLF	0 PLI	=	0 PLF	0 PLF	C	PLF	Ceiling Jo	oists above	
2	Point		13-7-0		Тор	1320 lb	2640 II	C	0 lb	0 lb		0 lb	Point Loa	d from Abo	ove
	Bearing Lengtl	h	0-3-8												
	Self Weight					21 PLF									
Natas									Manufacture	er Info		В	MC/Locust	Lumber Co	ompany
NOTES This component and	alysis is based on the	loads,						ŀ	Louisiana-Pa	cific Corp		3	12 E. Main	Street, Nor	th Carol
geometry and other of and listed in this rep	conditions as entered by the port. The user is responsi of the input and the application	e user ible to bility to							414 Union S Nashville, Th	treet, Suite 2 37219	000	28	5127)4-888-441	1	
the actual conditions component is intender	of the structure for which d. This analysis is valid only	this for the							(888) 820-03	25 25		\vdash			
product listed. Copyright 2019 All rig Corp. 414 Union St Su	hts reserved by Louisiana ite 2000, Nashville, TN 372	Pacific 19				Th 10/	is design is valid /31/2021	until	APA: PR-L28 LADBS: RR-	30, ICC-ES: 25783, Flori	ESR-2403 da: FL1522	, 28	B	MC	

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	•	Cli	ient:				Dat	te:	5/5/2021				Page 1 of 1
		Pr	oject:				Inp	ut by:					-
is	Design	Ad	ldress:				Job	Name	e: Devane				
							Pro	ject #:					
Beam K	LP-LVL 29	00Fb-2.0)E 1.	750" X	11.875"	2-Ply	- PASSE	ED	Level: Level				
						-							
									3				
		2											
					1								
LP	TO PERSON	LD			LP		LP		THE		LP		M
			1.1141									17 Manager	11 7/8"
1 SPF		2 SPI	F End Grain								3	SPF	· · ·
							1010 1/01						
	5'11"		1				12'3 1/2"						3 1/2"
1					18'2 1/2"							1	
Member Inf	ormation						Reaction				nlift)		
Type [.]	Girder	I	Applicatio	n. I	Floor		Bra				Snow	Wind	Const
Plies:	2		Design Me	ethod:	ASD		1	420	a 20	iu (0	0	0
Moisture Cond	ition: Dry		Building C	ode:	IBC/IRC 2015		2	6334	4 428	37	0	0	0
Deflection LL:	480		Load Sha	ring:	No		3	2704	4 183	30	0	0	0
Deflection TL:	240		Deck:	l	Not Checked		Ů	2.0			0	0	Ū
Importance:	Normal												
Temperature:	Temp <= 100°F	F					D						
							Bearings						
							Bearing	Length	h Cap.	React D/	Llb To	tal Ld. Case	Ld. Comb.
							1 - SPF 3	3.500"	33%	236 / 1	479 17	16 L_ 74)	D+L(D+L)
Analysis Re	sults						2 - SPF 4	4.250"	97%	4366 / 6	452 108	19 LL	D+L
Analysis	Actual I	Location Al	lowed	Canacity	Comb	Case	End						
Neg Moment	-11906 ft-lb	5'11" 19	902 ft-lb	0.598 (609	%) D+L	LL	Grain	0 500"	070/	4005 / 0	700 45	07 1	D.I
Pos Moment	10739 ft-lb	13' 3/16" 19	902 ft-lb	0.540 (549	%) D+L	L	3 - SPF	3.500	87%	1805/2	45	27 _L	D+L
Shear	5383 lb	6'10 7/8" 78	97 lb	0.682 (689	%) D+L	- LL							
LL Defl inch	0.184 (L/786) 1	2'4 5/16" 0.3	302 (L/480)	0.610 (619	%) L	_L							
TL Defl inch	0.302 (L/479)	12'4 5/8" 0.6	603 (L/240)	0.500 (50%	%) D+L	_L							
Design Not	96						1						
1 Provide late	ral support to preven	nt rotation at e	nd bearings	and at inte	rior bearings w	hen	4						
required by	code for seismic des	sign.	5		5								
2 Dead Load	Deflection: Instant =	0.118", Long	Term = 0.17	7" oply									
4 Multiple plie	s must be fastened t	ogether as pe	er manufactu	irer's detail:	S.								
5 Top loads m	ust be supported eq	ually by all pli	es.										
6 Tie-down co	onnection required at	bearing 1 for	r uplift 774 lb	o (Combina	tion D+L, Load	Case							
7 Top must be	e laterally braced at a	a maximum of	10'8 1/4" o.	C.									
8 Bottom mus	t be laterally braced	at a maximun	n of 9'3 5/8"	0.C.									
ID	Load Type	Lo	cation Tr	ib Width	Side	Dead 0.9	Live 1	Sno	ow 1.15 V	Vind 1.6	Const. 1.2	5 Comment	s
1	Uniform				Тор	70 PLF	280 PLF		0 PLF	0 PLF	0 PL	.F Floor Load	l
2	Uniform				Тор	150 PLF	0 PLF		0 PLF	0 PLF	0 PL	.F Ceiling Joi	sts
3	Uniform				Тор	120 PLF	240 PLF		0 PLF	0 PLF	0 PL	.F Roof Load	
	Self Weight					12 PLF							
	2												
									Mar. 6	1.6		DMC "	
Notes	lucio io bossi en tin t	de						ŀ	Manufacturer	into		BMC/Locust L 312 E. Main S	umber Company street, North Carolina
geometry and other of and listed in this ren	onditions as entered by the user is responsible	ser to							414 Union Str	eet, Suite 20	00	28127	
ensure the accuracy o the actual conditions	of the structure for which t	/ to this							Nashville, TN (888) 820-032	37219 5		/∪4-888-4411	
component is intended product listed.	. This analysis is valid only for	the							www.lpcorp.co	om LICC-ES: E	SR-2403		
Copyright 2019 All right Corp. 414 Union St Suit	nts reserved by Louisiana Pac te 2000, Nashville, TN 37219	sific				Thi	s design is valid u	until	LADBS: RR-2	5783, Florida	a: FL15228	P	MC
						10/	31/2021						
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	•		Client:				Da	te:	5/5/2021				Page 1 of 1
i.	•		Project:				Inp	out by:					
IS	Design		Address:				Jol	b Name:	: Devane				
Boam N		900Eb_2	0F 1	750" X /	16 000"	2_DIv			evel: Level				
Deallin		5001 0-2	.0 1.	730 A	10.000	2-F I y	- FA33						
		2											
					1								
											Calendary and the		π
LP			LP			LP							W
	Coltra .			-	Har Jag		-	and the	The second			-	1'4"
		and the second damage			California Con Con		Service A. Service	Section and					
1 SPF En	d Grain									2	SPF End G	rain	
1					14'1"							1	3 1/2"
1					14'1"								
Member Inf	ormation						Reaction	s UNF	PATTERN	ED lb (L	Jplift)		
Туре:	Girder		Applicatio	on: Fl	oor		Brg	Live	Dea	ad	Snow	Wind	Const
Plies:	2 lition: Dru		Design M	ethod: AS	SD CURC 2015		1	4155	27	89	0	0	0
Deflection LL:	480		Load Sha	ring: No	C/IRC 2015		2	4155	27	89	0	0	0
Deflection TL:	240		Deck:	N	ot Checked								
Importance:	Normal												
Temperature:	Temp <= 10	0°F											
							Bearings						
							Bearing	Length	Cap.	React D	/Llb To	tal Ld. Case	Ld. Comb.
							1 - SPF End	3.500"	76%	2789/-	4155 69	943 L	D+L
Analysis Re	sults						Grain						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF End	3.500"	76%	2789/	4155 69	943 L	D+L
Moment	22951 ft-lb	7' 1/2"	34636 ft-lb	0.663 (66%)	D+L	L	Grain						
Shear	5413 lb	12'6 3/8"	10640 lb	0.509 (51%)	D+L	L							
LL Defl inch	0.221 (L/741)	7' 9/16"	0.341 (L/480)	0.650 (65%)		L							
	0.369 (L/444)	7 9/10	0.082 (L/240)	0.540 (54%)	D+L	L	4						
Design Not	es	ont rotation a	t and boaring	and at interi		hon	4						
required by	code for seismic o	lesign.	t enu beanings		bi bearings w	/ileli							
2 Dead Load	Deflection: Instant	t = 0.148", Lor	ng Term = 0.22	2"									
4 Multiple plie	es must be fastene	d together as	per manufacti	urer's details.									
5 Top loads m	nust be supported	equally by all	plies.										
6 Top must be 7 Bottom brac	e laterally braced a ced at bearings.	at a maximum	of 6' o.c.										
ID	Load Type		Location T	rib Width	Side	Dead 0.9	Live 1	Snov	w 1.15 \	Wind 1.6	Const. 1.	25 Comments	3
1	Uniform				Тор	70 PLF	280 PLF		0 PLF	0 PLF	0 P	LF Floor Load	
2	Uniform				Тор	310 PLF	310 PLF		0 PLF	0 PLF	0 P	LF Roof Load	
	Self Weight					16 PLF							
Netra									Manufacture	r Info		BMC/Locust L	umber Company
Notes This component and	lysis is based on the	loads,						F	Louisiana-Pa	cific Corp		312 E. Main St	reet, North Carolina
geometry and other c and listed in this rep ensure the accuracy of	onditions as entered by th port. The user is respons f the input and the application	ible to bility to							414 Union Str Nashville, TN	reet, Suite 2 37219	000	20127 704-888-4411	
the actual conditions component is intended	of the structure for white This analysis is valid only	ch this for the							(888) 820-032 www.lpcorp.co	25 om			
Copyright 2019 All rig	hts reserved by Louisiana	Pacific					- design to the		APA: PR-L28 LADBS: RR-2	0, ICC-ES: E 25783, Florid	ESR-2403, la: FL15228		
Corp. 414 Union St Su	ite 2000, inasitville, TN 372	13				Thi 10/	s design is valid 31/2021	until				BI	VIG

			Client:				Da	ate:	5/5/2021				Page 1 o
	-		Project:				In	put by:					Ū.
İS	Design		Address:				Jo	b Name	e: Devane				
	_						Pr	oject #:					
Beam O	LP-LVL 2	900Fb-2	2.0E 1	.750"	X 11.875"	2-Ply	- PASS	ED	Level: Level				
					1								
					ĺ								
					\checkmark								
LP	Contration of the second	LP			LP				Per		LP	2	
			and a street			1.1					0.000		
1 SPF											2 SPF		
1					16'11"								13 1/2"
1					16'11"							1	
N/	·						Desstien						
Type:	Girder		Applicat	ion:	Floor		Brg	l ive	PALIERINE Dea	d D ID (Uplift d Snow	.) \	Wind	Const
Plies:	2		Design	Method:	ASD		1	430) 20	B 0	-	0	0
Moisture Cond	lition: Dry		Building	Code:	IBC/IRC 2015		2	370) 19	3 0		0	0
Deflection LL:	480		Load St	naring:	No Not Checked								
Deflection 1L:	240 Normal		Deck:		Not Checked								
Temperature:	Temp <= 100	J∘E											
iomperature.		51					Bearings	5					
							Bearing	Length	n Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	3.500"	12%	208 / 430	639	L	D+L
							2 - SPF	3.500"	11%	193 / 370	563	L	D+L
Analysis Re	sults			-									
Analysis	Actual	Location	Allowed	Capac	ity Comb.	Case							
Noment	4491 π-ID	7"10" 110 E/0"	19902 π-ID	0.226 (2	23%) D+L	L .							
Snear	024 ID	1 Z 0/0		0.079 (5%) D+L	L .							
TL Dell Inch	0.139 (L/1419)	0 10/10 91 2/9"	0.411 (L/460	0.340(24%) L	L .							
	0.195 (L/1012)	013/0	0.823 (L/240) 0.240 (2	24%) D+L	L	-						
Design Not	es	ant retation	at and bearing	na and at i	atavian kaavinaa v	uh e n	4						
required by	code for seismic d	ent rotation : esign.	at end bearing	gs and at i	nterior bearings v	wnen							
2 Dead Load	Deflection: Instant	= 0.056", Lo	ong Term = 0.0	084"									
3 Girders are	designed to be sup	oported on the	he bottom edg	ge only.	aile								
5 Top loads n	nust be supported e	equally by al	l plies.		ans.								
6 Top braced	at bearings.												
7 Bottom bra	ced at bearings.		Location	Trib \A/idt	h Sida	Dood 0.0		1 600	N/ 1 1 E \A	lind 1.6 Cons	+ 1 05	Common	
	Loau Type Point		7 10 0		Ton	200 lb	Live		0 lb		0.1.20	Stair Hoay	llS dor
I	Point	_	7-10-0		юр	200 ID	000 11	J	aro	aro	di U		lei
	Bearing Lengtr	1	0-3-8										
	Self Weight					12 PLF							
Notes								<u> </u>	Manufacturer	Info	BN	/IC/Locust I	Lumber Compan
This component and	alysis is based on the	loads,						F	Louisiana-Paci	fic Corp	31	2 E. Main \$	Street, North Car
geometry and other of and listed in this rep	conditions as entered by the port. The user is responsil of the input and the applicable	e user ble to illity to							414 Union Stre Nashville, TN 3	et, Suite 2000 7219	28	1∠7 4-888-4411	1
the actual conditions component is intender	of the structure for whic d. This analysis is valid only t	h this for the							(888) 820-0325	i n			
product listed. Copyright 2019 All rid	hts reserved by Louisiana F	Pacific							APA: PR-L280,	ICC-ES: ESR-24	03,		
Corp. 414 Union St Su	ite 2000, Nashville, TN 3721	9				Thi	is design is valid (31/2021	until	LADBS: RR-25	783, ⊢lorida: FL1	5228	B	VIC
						10/	0 112021						

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CSD

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CSD

Designer: Tony Huneycutt--ADH

FIRST FLOOR FRAMING

2362	LF	LPI201178	11-7/8" LI	PI 20Plus J	OISTS	3/28' 19/24'	15/22'	17/20'	3	7086
						26/18' 2/16'	16/14'	1/12'		
						7/10' 18/8' 2	/6' 9/4	' + 154' B	К	
14	PCS	LPRB118117812	1-1/8" x	11-7/8"	x 12' RIN	/I BOARD			43.2	604.8

7690.8

SECOND FLOOR FRAMING

620	LF	LPI201178	11-7/8" LPI 20Plus JOISTS	6/20' 3/16' 19	9/14' 13/10'	3	1860
				4/8' 2/4' 1/2'	+ 14' BLKG		
5	PCS	LPRB118117812	1-1/8" x 11-7/8" x 12' RIN	/I BOARD		43.2	216
314	LF	LPI2014	14" LPI 20Plus JOISTS	13/20' 1/16' 1	1/14' 1/12' +	3.3	1036.2
				12' BLKG			
4	PCS	LPRB1181412	1-1/8" x 14" x 12' RIM BC	ARD		51.6	206.4
24	LF	LPI3216	16" LPI 32Plus JOISTS	2/4' + 16' BLK	G	3.5	84
900	LF	LPI4216	16" LPI 42Plus JOISTS	38/24'		4.5	4050
6	PCS	LPRB1181612	1-1/8" x 16" x 12' RIM BC	ARD		58.8	352.8
32	LF	LPLVL16	1-3/4" x 16" LVL	2/16'		7.3	233.6
60	LF	LPLVL14	1-3/4" x 14" LVL	3/20'		6.4	384
264	LF	LPLVL1178	1-3/4" x 11-7/8" LVL	2/24' 4/18' 2/	/12' 12/10'	5.4	1425.6
84	LF	LPLVL914	1-3/4" x 9-1/4" LVL	2/10' 8/8'		4.2	352.8
38	PCS	IUS2.56/11.88	FACE MOUNT SINGLE JOI	ST HANGERS			
1	PCS	MIU5.12/11	FACE MOUNT DOUBLE JO	ISTS HANGERS			
5	PCS	IUS3.56/16	FACE MOUNT SINGLE JOI	ST HANGERS			
1	PCS	MIU5.12/16	FACE MOUNT DOUBLE JO	IST HANGERS			
2	PCS	152-999	HGUS412 FACE MOUNT D	OUBLE LVL HAI	NGERS		

10201.4