

**Trenco** 818 Soundside Rd Edenton, NC 27932

Re: J0820-3992 Precision/Lot 47 Summerlin/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15217445 thru E15217460

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



December 16,2020

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job		Truss			Truss Type				Qty	Ply	Precision	/Lot 47 Summ	erlin/Harnet	t		E4504	7445
.10820-3992		FT1			Floor Suppo	orted Gable			1	1						E1521	7445
00020 0002					l looi ouppe						Job Refer	ence (optiona	I)				
Comtech, Inc,	Fayettev	rille, NC - 28	3314,					ID:F	QVbfLGVT	8.330 s Oct T1IOnQ611	7 2020 Mi pZG_z1SF	Tek Industries 7-69ym04b1c	s, Inc. Wed S2BVV6yPcl	Dec 16 08: R8stLrqRk	59:28 202 AeZRarcw	0 Page fSry8RC	1 Qj
0-1-8																0- <mark>1</mark> -8	
																Scale =	1:34.8
										3:	x6 FP =						
1	2	3	4	5	6	7	8	9	10	11	12 13	14	15	16	17	18	
																	38 4-2-0
36	35	<del>×××××××××</del> 34	33	32 31	30	29	28	27	26	25	24	23	22	21	20	19	
3x4 =			:	3x6 FP =												3x4 =	

I			20-11-0			Ι
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.06 BC 0.01 WB 0.03 Matrix-R	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defi L/d - n/a 999 - n/a 999 19 n/a n/a	<b>PLATES</b> MT20 Weight: 87 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI WEBS 2x4 SI	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied c	ectly applied or 6-0-0 or 10-0-0 or 10-0-0 oc bracing.	oc purlins,

20-11-0

OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 20-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 36, 19, 35, 34, 33, 31, 30, 29, 28, 27, 26, 25, 24, 23, 22, 21, 20

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.



818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type		Qty	Ply	Precision/Lot 47 Summ	erlin/Harnett	E15217446
J0820-3992	ET2	GABLE		1	1	Job Deference (entional	<b>`</b>	E15217440
Comtech, Inc, Fa	ayetteville, NC - 28314,		ID:P	8. OVbfLGVTT1I	330 s Oct	7 2020 MiTek Industries	) , Inc. Wed Dec 16 08:59 hA26fh8z.JvNP5t0ar4ON	):29 2020 Page 1 0hi3GfC?Hv8RQi
0-1 <mark>1</mark> 8								0- <u>1</u> -18
								Scale = 1:28.0
							3x6 FP =	
1 2	3 4	5	6 7	8	9	10 11	12 13	14 15
30 29	28 27	26 25	24 23	22	21	20 19	18	17 16
3x4 =		3x6 FP =						3x4 =
1-4-0 1-4-0	2-8-0   4-0-0   1-4-0   1-4-0	5-4-0 6-8-0 1-4-0 1-4-0	<u>7-5-0</u> <u>8-0-0</u> <u>8-5-4</u> <u>9-4-1</u> 0-9-0 0-7-0 0-5-4 0-10-1	0 9-5-8 10-8-0 2 1 1-2-8 0-1-8	12	2-0-0   13-4-0   -4-0   1-4-0	14-8-0 16-0-0 1-4-0 1-4-0	<u></u>
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 CSI 1.00 TC 1.00 BC YES WB	. DEF 0.06 Vert 0.01 Vert 0.03 Horz	L. in (LL) n/a (CT) n/a (CT) 0.00	(loc) - - 16	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES GI MT20 24	RIP .4/190
BCDL 5.0	Code IRC2015/TP	12014 Mat	rix-R				Weight: 71 lb	FT = 20%F, 11%E

L	υI	ИE	BE	R-
_	•••			

Ξ

TOP CHORD	2x4 SP No.1(flat)
BOT CHORD	2x4 SP No.1(flat)
WEBS	2x4 SP No.3(flat)
OTHERS	2x4 SP No.3(flat)

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 71 lb

REACTIONS. All bearings 16-10-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 30, 16, 29, 28, 27, 25, 24, 23, 22, 21, 20, 19, 18, 17

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job		Truss		Truss Type					Qty	Ply	Precis	sion/Lot 47 S	Summerlir	/Harnett		<b>F</b> 16	017447
J0820-3992		ET3		GABLE					1	1						EID	21/44/
											Job R	eference (op	tional)				
Comtech, Inc,	Fayettevi	lle, NC - 2831	4,					ID:PQVbf	ہ LGVTT1	3.330 s Oc IIOnQ611p	t 7 2020 DZG_z1S	) MiTek Indu F7-aLW9E0	stries, Inc 2bfNmA26	:. Wed Deo Sfh8zJyNP	c 16 08:59 5t?7r4UN0	/:29 2020 Pa 0Yj3GfC?Hy8	ge 1 RQi
0 <sub>1</sub> 18																-1 <sub>1</sub>	8
																Scale	e = 1:23.1
1	2 2	7 3	4	_	5	28	6	7		8	29	9		10		11 12	Ŧ
		•							•		•	• •					26
			****			*****			~ ×××××	*****	*	~~~~~~	*****		*****		I <mark>6</mark>
24	23	22	2	1	20		19	1	8	<u>********</u> 1	<u>x x x x x x</u> 7	16	*****	15		14 13	
6x6 =	2x6	2x6	2	x6	2x6	II	2x6	2	x6	2	x6	2x6	П	2x6		6x6 =	
																2x6	
⊢ <u>1-4-0</u> 1-4-0	-+	2-8-0 1-4-0	<u>4-0-0</u> 1-4-0	<u>5-4-0</u> 1-4-0		<u>6-8-0</u> 1-4-0		8-0-0 1-4-0	+	9-4-0 1-4-0	+ <u>1(</u> + 1	0-8-0 4-0	<u>11-10-0</u> 1-2-0	12-0 <sub>0</sub> 0-2-0	<u>13-4-0</u> 1-4-0	13-11-0 0-7-0	
LOADING (psf)		SPACING	- 2-0-	0	CSI.			DEFL.	i	n (loc)	l/defl	L/d		PLATES	GI	RIP	
TCLL 40.0 TCDL 10.0		Plate Grip Lumber D0	DOL 1.0 DL 1.0	0	IC BC	0.09 0.01		Vert(LL) Vert(CT)	n/ n/	a - a -	n/a n/a	999 999		M120	24	4/190	
BCLL 0.0 BCDL 5.0		Rep Stress Code IRC	s Incr N 2015/TPI2014	C	WB Matri	0.04 x-R		Horz(CT)	0.0	0 13	n/a	n/a		Weight: 7	78 lb	FT = 20%F	, 11%E
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4 OTHERS 2x4	4 SP No. 4 SP No. 4 SP No. 4 SP No. 4 SP No.	1(flat) 1(flat) 3(flat) 3(flat)						BRACING TOP CHO BOT CHO	<b>3-</b> )RD )RD	Structu except Rigid c	ral wood end ver eiling di	d sheathing ticals. rectly applie	directly a	applied or )-0 oc brad	6-0-0 oc cing.	purlins,	

REACTIONS. All bearings 13-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 24, 13, 23, 22, 21, 20, 19, 18, 17, 16, 15, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
- Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard 1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

- Uniform Loads (plf)
- Vert: 13-24=-10, 1-12=-100

Concentrated Loads (lb) Vert: 4=-48 7=-48 10=-48 27=-48 28=-48 29=-48



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



1						16-7-8						I
LOADING         (psf           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	f) 0 0 0 0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.00 1.00 YES Pl2014	CSI. TC BC WB Matri	0.06 0.01 0.03 x-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 16	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 70 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP   2x4 SP   2x4 SP	No.1(flat) No.1(flat) No.3(flat)				BRACING- TOP CHOR BOT CHOR	D D	Structu except Rigid ce	ral wood end verti eiling dire	sheathing di icals. ectly applied	irectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,

16-7-8

OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 16-7-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 30, 16, 29, 28, 27, 25, 24, 23, 22, 21, 20, 19, 18, 17

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) All plates are 1.5x3 MT20 unless otherwise indicated.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Gable requires continuous bottom chord bearing.

4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

5) Gable studs spaced at 1-4-0 oc.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



818 Soundside Road Edenton, NC 27932



ł		000	10 + 12	20110	
		9-3-0	' 1-1-12 '	10-6-4	1
LOADIN TCLL TCDL BCLL BCDL	IG (psf) 40.0 10.0 0.0 5.0	SPACING- 1-7-3 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI.         DEFL.           TC         0.32         Vert(LL)           BC         0.61         Vert(CT)           WB         0.51         Horz(CT)           Matrix-S         Horz(CT)         Horz(CT)	in (loc) I/defi L/d PLATES -0.34 18-19 >728 480 MT20 -0.47 18-19 >528 360 M18SHS 0.07 14 n/a n/a Weight: 1	<b>GRIP</b> 244/190 244/190 07 lb FT = 20%F, 11%E
LUMBE TOP CH BOT CH WEBS	<b>R-</b> ORD 2x4 SF ORD 2x4 SF 2x4 SF	2 2400F 2.0E(flat) 2 2400F 2.0E(flat) 2 No.3(flat)	BRACING TOP CHOI BOT CHO	RD Structural wood sheathing directly applied or 6 except end verticals. RD Rigid ceiling directly applied or 10-0-0 oc brac	6-0-0 oc purlins, ing.
REACTI	ONS. (siz Max G	e) 24=0-3-0, 14=0-3-0 Grav 24=904(LC 1), 14=904(LC 1)			

20-11-0

10-4-12

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

0-3-0

 TOPOLOG
 Components
 Components

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) All plates are 3x4 MT20 unless otherwise indicated.

4) Plates checked for a plus or minus 1 degree rotation about its center.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.







		•	16-10-8			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge]		10-10-0			
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.45 BC 0.83 WB 0.62 Matrix-S	DEFL.         ir           Vert(LL)         -0.25           Vert(CT)         -0.35           Horz(CT)         0.06	n (loc) l/defl L/d 5 12-13 >791 480 5 12-13 >569 360 5 10 n/a n/a	<b>PLATES</b> MT20 Weight: 82 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	<sup>9</sup> No.1(flat) <sup>9</sup> No.1(flat) <sup>9</sup> No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing direct except end verticals. Rigid ceiling directly applied or	otly applied or 6-0-0 10-0-0 oc bracing.	oc purlins,

REACTIONS.	(size)	16=0-3-8, 10=0-3-0
	Max Grav	16=908(LC 1), 10=908(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-3175/0, 3-4=-3175/0, 4-5=-3602/0, 5-6=-3175/0, 6-8=-3175/0

BOT CHORD 14-16=0/1977, 13-14=0/3602, 12-13=0/3602, 11-12=0/3602, 10-11=0/1977

WEBS 2-16=-2120/0, 2-14=0/1293, 3-14=-268/21, 8-10=-2120/0, 8-11=0/1293, 6-11=-268/21, 5-11=-781/0, 4-14=-781/0

# NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



818 Soundside Road Edenton, NC 27932



				17-3-0			
				17-3-0			1
Plate 0	Offsets (X,Y)	[1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,	Edge]				
LOAD TCLL TCDL BCLL BCDL	ING (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.46 BC 0.87 WB 0.64 Matrix-S	DEFL.         ir           Vert(LL)         -0.27           Vert(CT)         -0.38           Horz(CT)         0.07	n (loc) I/defi L/d / 12-13 >743 480 9 12-13 >536 360 / 10 n/a n/a	<b>PLATES</b> MT20 Weight: 84 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMB TOP C BOT C WEBS	ER- HORD 2x4 SP HORD 2x4 SP 2x4 SP	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing diru except end verticals. Rigid ceiling directly applied o	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,
REAC	TIONS. (size	e) 16=Mechanical 10=0-3-0					

Max Grav 16=935(LC 1), 10=929(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-3282/0, 3-4=-3282/0, 4-5=-3772/0, 5-6=-3282/0, 6-8=-3282/0

BOT CHORD 14-16=0/2030, 13-14=0/3772, 12-13=0/3772, 11-12=0/3772, 10-11=0/2028

WEBS 2-16=-2184/0, 2-14=0/1351, 3-14=-278/14, 8-10=-2175/0, 8-11=0/1354, 6-11=-280/14, 5-11=-843/0, 4-14=-843/0

# NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.









Plate Offsets (X,Y)         [2:0-1-8,Edge], [3:0           LOADING (psf)         SPACING-	2-0-0 CSI.	1-8,Edge] DEFL.	:- (l) l/d- <b>6</b> l /d	
LOADING (psf) SPACING-	2-0-0 <b>CSI.</b>	DEFL.	;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;;	
TCLL         40.0         Plate Grip D           TCDL         10.0         Lumber DO           BCLL         0.0         Rep Stress           BCDL         5.0         Code IRC2	L 1.00 BC Incr YES WB 015/TPI2014 Matrix	0.44 Vert(LL) -0. 0.59 Vert(CT) -0. 0.45 Horz(CT) 0. -S	in (ioc) i/defi L/d 13 12-13 >999 480 21 12-13 >770 360 03 12 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 87 lb         FT = 20%F, 1
LUMBER- TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) WEBS 2x4 SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing direct except end verticals. Rigid ceiling directly applied or 6	lly applied or 6-0-0 oc purlins, 6-0-0 oc bracing.

REACTIONS. (size) 20=0-3-0, 17=0-3-8, 12=0-3-0 Max Uplift 20=-42(LC 4) Max Grav 20=179(LC 3), 17=1084(LC 1), 12=707(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-4=0/352, 4-5=0/357, 5-6=-2076/0, 6-7=-2076/0, 7-8=-2186/0, 8-10=-2186/0

BOT CHORD 15-17=0/1236, 14-15=0/2186, 13-14=0/2186, 12-13=0/1472

WEBS 3-17=-491/0, 10-12=-1577/0, 10-13=0/814, 5-17=-1661/0, 5-15=0/916, 7-15=-476/115

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 20.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 **MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



2-4-14	1-4-14 0-1-8		13	-7-12		
Plate Offsets (X,Y)	[1:Edge,0-1-8], [7:0-1-8,Edge], [13:0-1-	8,Edge]				
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.67 BC 0.61 WB 0.48 Matrix-S	DEFL.         ii           Vert(LL)         -0.14           Vert(CT)         -0.22           Horz(CT)         0.03	n (loc) l/defl L/d 4 12-13 >999 480 2 12-13 >735 360 3 12 n/a n/a	<b>PLATES</b> MT20 Weight: 92 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	│ <sup>●</sup> No.1(flat) <sup>●</sup> No.3(flat) <sup>●</sup> No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire except end verticals. Rigid ceiling directly applied or	ectly applied or 6-0-0 r 10-0-0 oc bracing.	oc purlins,

15

3x10 =

14

17-7-0

13

3x4 =

REACTIONS. (size) 20=0-3-0, 17=0-3-8, 12=0-3-0 Max Grav 20=530(LC 3), 17=1662(LC 1), 12=675(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

11

译

4x8 =

16

3x6 FP =

18

3-9-12 3-11-4

- TOP CHORD 2-3=-553/0, 3-4=0/873, 4-5=0/823, 5-6=-1815/0, 6-7=-1815/0, 7-8=-2002/0,
- 8-10=-2002/0
- BOT CHORD 19-20=0/553, 18-19=0/553, 17-18=0/553, 15-17=0/901, 14-15=0/2002, 13-14=0/2002, 12-13=0/1391
- WEBS 4-17=0/312, 2-20=-882/0, 3-17=-1710/0, 10-12=-1490/0, 10-13=0/697, 5-17=-1772/0, 5-15=0/992, 7-15=-532/0

#### NOTES-

24

19

2-4-14

3x6 =

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
- Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 915 lb down at 2-1-12 on top
- chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 12-20=-10, 1-11=-100

Concentrated Loads (lb)

Vert: 23=-835(B)



X

3x6 =





1				13	-9-8						1
				13	-9-8						1
Plate Off	sets (X,Y)	[4:0-1-8,Edge], [9:0-1-8,E	[dge]	I							
LOADIN	G (psf)	SPACING-	2-0-0 <b>CS</b>		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00 TC	0.44	Vert(LL)	-0.16	10-11	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00 BC	0.74	Vert(CT)	-0.20	8-9	>796	360		
BCLL	0.0	Rep Stress Incr	YES WB	0.46	Horz(CT)	0.03	8	n/a	n/a		
BCDL	5.0	Code IRC2015/TP	PI2014 Mat	rix-S	- (- )					Weight: 67 lb	FT = 20%F, 11%E
LUMBER	3-		·	i.	BRACING-						
TOP CH	ORD 2x4 SP	No.1(flat)			TOP CHOP	RD	Structu	ral wood	sheathing dire	ectly applied or 6-0-0	oc purlins,
BOT CH	ORD 2x4 SP	No.1(flat)					except	end verti	cals.		•
WEBS	2x4 SF	P No.3(flat)			BOT CHOP	RD	Rigid ce	eiling dire	ectly applied o	r 10-0-0 oc bracing.	
							<b>J</b> • •	<b>J</b>		· · · · · · · · · · · · · · · · · · ·	
REACTION	ONS. (size	e) 12=0-3-8, 8=0-3-0									
	Max G	arav 12=739(LC 1), 8=739	9(LC 1)								

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2333/0, 3-4=-2333/0, 4-5=-2367/0, 5-6=-2367/0

BOT CHORD 11-12=0/1555, 10-11=0/2367, 9-10=0/2367, 8-9=0/1551

WEBS 2-12=-1667/0, 2-11=0/840, 3-11=-263/71, 6-8=-1662/0, 6-9=0/938, 5-9=-262/0, 4-11=-422/240

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job	Truss	Truss Type	Qty	Ply	Precision/Lot 47 Summerlin/Harnett
					E15217455
J0820-3992	F6-GR	Floor	1	1	
					Job Reference (optional)
Comtech, Inc, Fayette	/ille, NC - 28314,		8.	330 s Oct	7 2020 MiTek Industries, Inc. Wed Dec 16 08:59:36 2020 Page 1
· · · ·		ID:P	QVbfLGV	T1IOnQ6	11pZG_z1SF7-thRoiph2jv22SkjVtla0BZg49fGpW3Qlgrs4kNy8RQb
0-1-8					

0-10-8 0-10-8	2-6-0	0-9-8 2-0-8	0-9-8	0 <sub>1</sub> 78
	I	1 11 1	1 1	Scale = 1:27.2



2-0-	-0 2-	<sup>1</sup> ۲8	7-4-8		8-10-4				16-7-8		
2-0-	-0 0-	-1-8	5-3-0	1	1-5-12				7-9-4		1
Plate Offsets (X,	,Y) [4:(	0-3-4,Edge], [6:0-1-8,E	dge], [7:0-1-8	Edge]							
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0		SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.00 1.00 NO Pl2014	<b>CSI.</b> TC 0.55 BC 0.79 WB 0.54 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.16 -0.22 0.04	(loc)   15 > 14-15 > 12	l/defl >999 >767 n/a	L/d 480 360 n/a	<b>PLATES</b> MT20 Weight: 88 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2 REACTIONS.	2x4 SP No 2x4 SP No 2x4 SP No (size) Max Grav	o.1(flat) o.1(flat) o.3(flat) 18=0-3-8, 12=0-3-0 v 18=1745(LC 1), 12=8	609(LC 4)		BRACIN TOP CHO BOT CHO	<b>G-</b> DRD DRD	Structural except er Rigid ceili 6-0-0 oc b	l wood s nd vertic ing direc bracing:	sheathing direct als. ctly applied or 18-19.	xtly applied or 6-0-0 10-0-0 oc bracing,	oc purlins, Except:

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 4-5=-2781/0, 5-6=-2777/0, 6-7=-2852/0, 7-8=-2675/0, 8-10=-2675/0

BOT CHORD 16-18=0/1924, 15-16=0/2852, 14-15=0/2852, 13-14=0/2852, 12-13=0/1732

WEBS 3-18=-856/0, 4-18=-1987/0, 4-16=0/939, 10-12=-1857/0, 10-13=0/1018, 7-13=-575/72, 6-16=-466/209

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 818 lb down at 2-9-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 12-19=-10, 1-11=-100

Concentrated Loads (lb)

Vert: 23=-755(F)



818 Soundside Road Edenton, NC 27932



	i i i i i i i i i i i i i i i i i i i	5-4-8	6-10-4	1	13-8-8		
		5-4-8	1-5-12	1	6-10-4		1
Plate (	Offsets (X,Y)	[1:Edge,0-1-8], [4:0-1-8,Edge], [9:0-1-8,	Edge]				
LOAD TCLL TCDL BCLL BCDL	ING (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.42 BC 0.72 WB 0.45 Matrix-S	<b>DEFL.</b> i Vert(LL) -0.1! Vert(CT) -0.2! Horz(CT) 0.03	n (loc) l/defl L/d 5 10-11 >999 480 ) 8-9 >809 360 3 8 n/a n/a	<b>PLATES</b> MT20 Weight: 68 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMB TOP C BOT C WEBS	ER- CHORD 2x4 SF CHORD 2x4 SF CHORD 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o	rectly applied or 6-0-0 or 10-0-0 oc bracing.	oc purlins,
REAC	TIONS. (size Max G	e) 12=Mechanical, 8=0-3-0 irav 12=740(LC 1), 8=734(LC 1)					
FORC	<b>ES.</b> (lb) - Max.	Comp./Max. Ten All forces 250 (lb) or	less except when shown.				

TOP CHORD 2-3=-2310/0, 3-4=-2310/0, 4-5=-2341/0, 5-6=-2341/0

BOT CHORD 11-12=0/1546, 10-11=0/2341, 9-10=0/2341, 8-9=0/1540

WEBS 2-12=-1664/0, 2-11=0/824, 3-11=-260/68, 6-8=-1650/0, 6-9=0/920, 5-9=-257/0, 4-11=-411/241

#### NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Plates checked for a plus or minus 1 degree rotation about its center.

3) Refer to girder(s) for truss to truss connections.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MaNSTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Edenton, NC 27932



#### NOTES-

1) Plates checked for a plus or minus 1 degree rotation about its center.

2) Refer to girder(s) for truss to truss connections.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

4) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 660 lb down at 1-4-4, and 660

Ib down at 2-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf) Vert: 5-8=-8, 1-4=-80 Concentrated Loads (lb) Vert: 3=-660(B) 2=-660(B)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MI lek Aff 818 Soundside Road Edenton, NC 27932



			<u>16-7-8</u> 16-7-8			
Plate Offsets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,Edge]					
LOADING         (psf)           TCLL         40.0           TCDL         10.0           BCLL         0.0           BCDL         5.0	SPACING-2-0-0Plate Grip DOL1.00Lumber DOL1.00Rep Stress IncrYESCodeIRC2015/TPI2014	<b>CSI.</b> TC 0.44 BC 0.81 WB 0.60 Matrix-S	DEFL.         ir           Vert(LL)         -0.24           Vert(CT)         -0.33           Horz(CT)         0.06	1 (loc) l/defl L/d 12-13 >824 480 12-13 >593 360 10 n/a n/a	<b>PLATES</b> MT20 Weight: 81 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing direct except end verticals. Rigid ceiling directly applied or	ctly applied or 6-0-0 10-0-0 oc bracing.	oc purlins,

REACTIONS.	(size)	16=0-3-8, 10=0-3-0
	Max Grav	16=894(LC 1), 10=894(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-3105/0, 3-4=-3105/0, 4-5=-3492/0, 5-6=-3105/0, 6-8=-3105/0

BOT CHORD 14-16=0/1943, 13-14=0/3492, 12-13=0/3492, 11-12=0/3492, 10-11=0/1943

WEBS 2-16=-2084/0, 2-14=0/1254, 3-14=-262/25, 8-10=-2084/0, 8-11=0/1254, 6-11=-262/25, 5-11=-740/0, 4-14=-740/0

# NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.







14

13

3x6 =

16

3x10 =

15

⊢	2-0-0	2 <sub>1</sub> 1 <u>8</u> 0-1-8	7-4-8		8-10-4	<u>16-7-8</u> 7-9-4		
Plate Off	sets (X,Y)	[6:0-1-8,Edge], [7:0-1-8,E	Edge]					
LOADIN TCLL TCDL BCLL BCDL	G (psf) 40.0 10.0 0.0 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TI	2-0-0 1.00 1.00 YES Pl2014	<b>CSI.</b> TC 0.39 BC 0.65 WB 0.48 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl L/d -0.15 14-15 >999 480 -0.20 14-15 >848 360 0.04 12 n/a n/a	<b>PLATES</b> MT20 Weight: 85 lb	<b>GRIP</b> 244/190 FT = 20%F, 11%E
LUMBER TOP CH BOT CH WEBS	<b>R-</b> ORD 2x4 SF ORD 2x4 SF 2x4 SF	P No.1(flat) P No.1(flat) P No.3(flat)			BRACING TOP CHC BOT CHC	- RD Structural wood sheathing dir except end verticals. RD Rigid ceiling directly applied o 6-0-0 oc bracing: 18-19.	ectly applied or 6-0-0 or 10-0-0 oc bracing,	oc purlins, Except:

REACTIONS. (size) 18=0-3-8, 12=0-3-0 Max Grav 18=1021(LC 1), 12=778(LC 4)

涿

3x10 =

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 4-5=-2489/0, 5-6=-2489/0, 6-7=-2635/0, 7-8=-2522/0, 8-10=-2522/0

BOT CHORD 16-18=0/1591, 15-16=0/2635, 14-15=0/2635, 13-14=0/2635, 12-13=0/1653

WEBS 4-18=-1734/0, 4-16=0/998, 10-12=-1772/0, 10-13=0/938, 7-13=-478/177, 6-16=-561/100

17

3x6 FP =

NOTES-

20

19

3x4 =

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 1.5x3 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.

Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



X

3x6 =

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 Most/TP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

A MITEK Affiliate A MITEK Affiliate 818 Soundside Road Edenton, NC 27932





**Trenco** 818 Soundside Rd Edenton, NC 27932

Re: J0820-3991 Precision/Lot 47 Summerlin/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E15217423 thru E15217444

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



December 16,2020

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MaNSTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



December 16,2020



2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ff; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 33-0-15 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13, Interior(1) 20-4-13 to 33-0-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.











Edenton, NC 27932

December 16,2020



#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 21, 22, 17, 16, 15, 12 except (jt=lb) 23=104, 14=103.







				10-6-0					21-0-0			
				10-6-0					10-6-0			
Plate Off	sets (X,Y)	[2:0-0-6,0-0-2], [6:	0-0-6,0-0-2]									
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip I	DOL 1.15	TC	0.11	Vert(LL)	-0.06	6-9	>999	360	MT20	244/190
TCDL	10.0	Lumber DC	L 1.15	BC	0.36	Vert(CT)	-0.12	6-9	>999	240		
BCLL	0.0 *	Rep Stress	Incr YES	WB	0.24	Horz(CT)	0.02	6	n/a	n/a		
BCDL	10.0	Code IRC2	015/TPI2014	Matri	k-S	Wind(LL)	-0.01	6-9	>999	240	Weight: 143 lb	FT = 20%
											-	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 6=0-3-8, 2=0-3-8 Max Horz 2=-185(LC 10) Max Uplift 6=-61(LC 13), 2=-61(LC 12) Max Grav 6=902(LC 1), 2=902(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1127/271, 3-4=-872/237, 4-5=-872/237, 5-6=-1127/272
- BOT CHORD 2-9=-103/913, 6-9=-119/878

WEBS 4-9=-104/674, 5-9=-372/219, 3-9=-372/219

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 10-6-0, Exterior(2) 10-6-0 to 14-10-13, Interior(1) 14-10-13 to 22-0-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **MISI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Precision/Lot 47 Summerlin/Harnett	
					E	15217430
J0820-3991	B3-2PLY	FINK	1	2		
				<b>Z</b>	Job Reference (optional)	
Comtech, Inc, Fayette	ville, NC - 28314,		. 8.	330 s Oct	7 2020 MiTek Industries, Inc. Wed Dec 16 08:59:32 2020 F	Page 2
-		ID:PQ\	/bfLGVTT	1IOnQ611	DZG z1SF7-1HJXpk6heIgOXHntQhm1AjVR12ykaCiAlEutbc	y8RQf

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 7=-1249(B) 6=-1249(B) 9=-1249(B) 10=-1249(B) 11=-1249(B) 12=-1249(B) 13=-1249(B) 14=-1249(B) 15=-1249(B) 16=-1251(B)





- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 11 except (jt=lb) 14=125, 10=125.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems. see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

#### 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Precision/Lot 47 Summerlin/Harnett	
10000 0001						E15217432
J0820-3991	C2-2PLY	COMMON GIRDER	1	2	lob Reference (optional)	
Comtech, Inc, Fayett	eville, NC - 28314,		8	.330 s Oct	7 2020 MiTek Industries, Inc. Wed Dec 16 08:59:34 202	0 Page 2
		ID:PQVbf	LGVTT1IC	nQ611pZ0	G_z1SF7-zgRIEQ7y9vw6nbxGY6pVF8bo8sf926PTDYNz	gVy8RQd

#### LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 5=-1249(B) 6=-1249(B) 7=-1249(B) 8=-1249(B) 9=-1249(B) 10=-1249(B)





	' '						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.14 BC 0.12 WB 0.00 Matrix-P	DEFL.         i           Vert(LL)         -0.0           Vert(CT)         -0.0           Horz(CT)         0.0           Wind(LL)         0.0	n (loc) I 2-4 2 2-4 ) 2 2-4	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES         GRIP           MT20         244/190           Weight: 17 lb         FT = 20%
LUMBER-			BRACING-				

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x6 SP No.1
OTHERS	2x4 SP No 2

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=83(LC 8) Max Uplift 2=-152(LC 8), 4=-80(LC 8) Max Grav 2=240(LC 1), 4=131(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

ł

- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=152.



Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.14	Vert(LL) -0.01	2-4	>999	360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT) -0.02	2-4	>999	240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00		n/a	n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.02	2-4	>999	240	Weight: 16 lb FT = 20%
LUMBER-		1	BRACING-				

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=60(LC 8) Max Uplift 2=-107(LC 8), 4=-54(LC 8)

Max Grav 2=240(LC 1), 4=131(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Interior(1) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

ł

- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=107.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 4-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



	5-0-0 5-0-0									
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.26	Vert(LL)	-0.02	2-4	>999	360	MT20	244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.20	Vert(CT)	-0.05	2-4	>999	240			
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00		n/a	n/a			
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.05	2-4	>999	240	Weight: 20 lb	FT = 20%	
LUMBER-	·	·	BRACING-							

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x6 SP No.1 WEBS

REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=71(LC 8)

Max Uplift 2=-118(LC 8), 4=-73(LC 8)

Max Grav 2=277(LC 1), 4=174(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=118.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MaNS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.



	L		5-0-0					10-0-0			
			5-0-0	1				5-0-0		I	
Plate Offsets (X,Y)	[2:0-1-6,Edge], [6:0-1-6,E	dge]									
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.	0.10 \	Vert(LL)	-0.01	8	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.	0.11 \	Vert(CT)	-0.01	8	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.	D.05 H	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10.0	Code IRC2015/TF	912014	Matrix-S	s v	Wind(LL)	0.01	8	>999	240	Weight: 63 lb	FT = 20%
LUMBER-				E	BRACING-						
TOP CHORD 2x6 S	SP No.1			Т	TOP CHOP	RD	Structu	ral wood	sheathing di	rectly applied or 6-0-0	oc purlins.
BOT CHORD 2x6 S	SP No.1			В	BOT CHOF	RD	Rigid c	eiling dire	ectly applied	or 10-0-0 oc bracing.	

BOT CHORD 2x6 SP No.1 2x4 SP No.2 WFBS OTHERS 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 6=0-3-0 Max Horz 2=-61(LC 13) Max Uplift 2=-119(LC 9), 6=-119(LC 8) Max Grav 2=461(LC 1), 6=461(LC 1)

4-9=-325/196

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-523/590, 3-4=-461/615, 4-5=-461/615, 5-6=-523/590

BOT CHORD 2-10=-425/413, 9-10=-425/413, 8-9=-425/413, 6-8=-425/413

WEBS NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=119, 6=119.



Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 Most/TP11 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



		<u>5-0-0</u> 5-0-0		<u> </u>			
Plate Offsets (X,Y)	[2:0-1-6,Edge], [4:0-1-6,Edge]						
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in	(loc) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.10	Vert(LL) -0.00	4-6 >999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) -0.01	4-6 >999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT) 0.00	4 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.01	4-6 >999	240	Weight: 60 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x6 S	SP No.1		TOP CHORD	Structural wood	sheathing direct	ly applied or 6-0-0	oc purlins.
BOT CHORD 2x6 S	SP No.1		BOT CHORD	Rigid ceiling dir	ectly applied or 1	0-0-0 oc bracing.	

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2

 REACTIONS.
 (size)
 2=0-3-0, 4=0-3-0

(size) 2=0-3-0, 4=0-3-0 Max Horz 2=-39(LC 10) Max Uplift 2=-91(LC 9), 4=-91(LC 8) Max Grav 2=461(LC 1), 4=461(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-524/470, 3-4=-524/470

BOT CHORD 2-6=-341/404, 4-6=-341/404

WEBS 3-6=-286/228

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Interior(1) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.













MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 4-10-9, Interior(1) 4-10-9 to 6-9-0, Exterior(2) 6-9-0 to 11-1-13, Interior(1) 11-1-13 to 13-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







#### 2x4 ⁄

2x4 📎

			5-6-0	
Plate Offsets (X,Y)	[2:0-2-0,Edge]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.07 BC 0.19 WB 0.00	<b>DEFL.</b> in (loc) I/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a n/a	PLATES         GRIP           MT20         244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 16 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP	2 No.1 2 No.1		BRACING- TOP CHORD Structural wood sheathing dir BOT CHORD Rigid ceiling directly applied o	ectly applied or 5-6-0 oc purlins. r 10-0-0 oc bracing.

5-6-0

**REACTIONS.** (size) 1=5-6-0, 3=5-6-0 Max Horz 1=-36(LC 8)

Max Horz 1=-36(LC 8) Max Uplift 1=-10(LC 12), 3=-10(LC 13) Max Grav 1=181(LC 1), 3=181(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



818 Soundside Road Edenton, NC 27932



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.





<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



**REACTIONS.** (size) 1=6-6-0, 3=6-6-0, 4=6-6-0

Max Horz 1=-44(LC 8)

Max Uplift 1=-19(LC 12), 3=-23(LC 13)

Max Grav 1=120(LC 1), 3=120(LC 1), 4=202(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



818 Soundside Road Edenton, NC 27932



2x4 🥢

2x4 📎

2-4-7           2-4-7           Plate Offsets (X,Y)           [2:0-2-0,Edge]											
	SPACING- 2-0-0	291	DEEL in (loc) l/defl L/d								
	Plate Grin DOI 115	TC 0.01	Vert(11) n/a - n/a 999	MT20 244/190							
CDL 10.0	Lumber DOL 1.15	BC 0.02	Vert(CT) n/a - n/a 999								
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a								
3CDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 6 lb FT = 20%							

REACTIONS. (size) 1=2-4-7, 3=2-4-7

Max Horz 1=-11(LC 8) Max Uplift 1=-3(LC 12), 3=-3(LC 13)

Max Grav 1=56(LC 1), 3=56(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Interior(1) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.









Impute:       Impute:       Impute:       Neal Baggett         GDH       Kerto-S LVL       1.750" X 14.000"       2-Ply - PASSED       Level: Level         Impute:       Level: Level       Impute:       Impute:       Impute:         Impute:       Impute:       Level: Level       Impute:       Impute:         Impute:       Impute:       Level: Level       Impute:       Impute:         Impute:       Impute:       Impute:       Impute:       Impute:       Impute:         GDH       Kerto-S LVL       1.750" X 14.000"       2-Ply - PASSED       Impute:       Impu	1'2" 3 1/2"
GDH Kerto-S LVL       1.750" X 14.000"       2-Ply - PASSED       Level Level         Image:	1 <sup>1</sup> / <sub>2</sub> "
ADH         Kerto-S LVL         1.750" X 14.000"         2-Ply - PASSED         Level: Level           Image: Second Sec	1'2" 1'2" 1'3 1/2"
1 SPF End Grain       2 SPF End Grain         1 SPF End Grain       2 SPF End Grain         1 SPF End Grain       16*10"         1 SPF End Grain       16*10"         1 SPF End Grain       16*10"         1 SPF End Grain       2 SPF End Grain         2 SPF End Grain       1 SPF End Grain         2 SPF End Grain       3 SPF End Grain         2 SPF End Grain </th <th>1'2" 1'2" 1'2" 1'2"</th>	1'2" 1'2" 1'2" 1'2"
1 SPF End Grain       2 SPF End Grain         16'10"       16'10"         11 SPF End Grain       2 SPF End Grain         16'10"       16'10"         Intitie Ply Analysis         Issten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c Maximum end distance not to exceed 6"         ipacity       0.0 %         ad       0.0 PLF         ald Limit per Fost       246.6 PLF         ald Limit per Fastener       81.9 1b.         ald Mode       V         Ige Distance       1 1/2"         End Distance       3"         ad Combination       1.00	1'2" 3 1/2"
1 SPF End Grain       2 SPF End Grain         16'10"       16'10"         Iulti-Ply Analysis         Isten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c Maximum end distance not to exceed 6"         pacity       0.0 %         ad       0.0 PLF         bid Limit per Foot       245.6 PLF         bid Limit per Fastener       81.9 lb.         bid Mode       IV         ge Distance       1 1/2"         n. End Distance       3"         ad Combination       1.00	3 1/2"
16'10"         Iulti-Ply Analysis         asten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c Maximum end distance not to exceed 6"         apacity         0.0 %         ad       0.0 PLF         ald Limit per Foot       245.6 PLF         ald Limit per Fastener       81.9 lb.         sid Mode       IV         Ige Distance       1 1/2"         n. End Distance       3"         ad Combination       1.00	f <b>3</b> 1/2"
Idition         Initial Plant Pla	
Aulti-Ply Analysis asten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c Maximum end distance not to exceed 6" upacity 0.0% ad 0.0 PLF ad Limit per Foot 245.6 PLF ald Limit per Fastener 81.9 lb. ald Mode IV Ige Distance 11/2" n. End Distance 3" ad Combination uration Factor 1.00	
Iulti-Ply Analysis         asten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c Maximum end distance not to exceed 6"         apacity       0.0 %         ad       0.0 PLF         ald Limit per Foot       245.6 PLF         ald Limit per Fastener       81.9 lb.         ald Mode       IV         Ige Distance       1 1/2"         n. End Distance       3"         ad Combination       1.00	
apacity 0.0 % ad 0.0 PLF eld Limit per Foot 245.6 PLF ald Limit per Fastener 81.9 lb. ald Mode IV lge Distance 11/2" n. End Distance 3" ad Combination tration Factor 1.00	
ad 0.0 PLF eld Limit per Foot 245.6 PLF eld Limit per Fastener 81.9 lb. eld Mode V ge Distance 11/2" 1. End Distance 3" ad Combination ration Factor 1.00	
lel Limit per Fastener 81.9 lb. eld Mode IV tge Distance 1 1/2" in. End Distance 3" vad Combination uration Factor 1.00	
ield ModeIVdge Distance1 1/2"in. End Distance3"bad Combination	
dge Distance1 1/2"in. End Distance3"vad Combinationuration Factor1.00	
n. End Distance 3" ad Combination rration Factor 1.00	
ad Combination Iration Factor 1.00	
I.00	

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road. Suite #639
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Interded application, and to verify the dimensions and loads. <b>Lumber</b> 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 11/27/2023	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633	Fayelteville, NC USA 28314 910-864-TRUS





Version 20.80.210 Powered by iStruct™

		Client	Data	12/16/2020	Dage 4 of 6
2		Droject:	Date.	Neal Baggett	Fage 4 01 0
1 1	isDesign	Address:	Input by:		
│ ↓	130631311	Address.	Project t	4.	
DMO		4 75011 X 0 0501		- Level: Level	
BM2	Kerto-S LVI	_ 1.750" X 9.250"	2-Ply - PASSED		
•	•	• •	• •	_= <b>●</b>	N/N/I I
				7	
	•	• •	• •		
	PF End Grain		2 SPF En	id Grain	
		617"			2 1/2"
		67			3 1/2
1		6'7"		1	
Mul4: DI-	v Apolycic				
wuiti-PI	y Analysis				
Fasten all	l plies using 2 rows of	f 10d Box nails (.128x3") at 12"	o.c Maximum end distance r	not to exceed 6"	
Capacity	0.0	D %			
Load	0.0	) PLF			
Yield Limit pe	er Foot 16 er Fastener 81				
Yield Mode	IV				
Edge Distan	ice 1 1	1/2"			
Min. End Dis	stance 3"				
Load Combin	nation				
Duration Fac	ctor 1.0	00			
<b> </b>					
Notes		chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	Comtech, Inc. 1001 S. Reilly Road, Suite #639
Calculated Struc structural adequ	ctured Designs is responsible only of the uacy of this component based on the	Handling & Installation 1. LVL beams must not be cut or drilled	Found	Metsä Wood 301 Merritt 7 Building, 2nd Floor	Fayetteville, NC USA
design criteria responsibility of	and loadings shown. It is the the customer and/or the contractor to	2. Refer to manufacturer's product information regarding installation requirements. multi-nly		Norwalk, CT 06851	28314 910-864-TRUS
ensure the co application, and t	omponent suitability of the intended to verify the dimensions and loads.	fastening details, beam strength values, and code approvals		(800) 622-5850 www.metsawood.com/us	
Lumber	conditions unless noted otherwise	<ol> <li>Damaged Beams must not be used</li> <li>Design assumes top edge is laterally restrained</li> </ol>		ICC-ES: ESR-3633	
2. LVL not to be	be treated with fire retardant or corrosive	5. Provide lateral support at bearing points to avoid lateral displacement and rotation	This design is valid until 11/27/2023		соттесн
L			1113 ucayn 13 vanu unur 11/21/2023		

		С	lient:				Da	ite:	12/16/202	0			Page 5 of 6
Í.		P	roject:				Inp	out by:	Neal Bagg	jett			
IS	Design	A	ddress:				Jol	b Name:	Lot 47 SU	MMERLIN			
BM3	Kerto-S LV	L 1	.750"	X 9.2	250"	2-Ply -	PASSE		evel: Level				
						3	3						
	2												
				1_									
•		•		•		•	•		•				MM
	and the second second				at the seaso				it in				9
							in a superior superior	Sector Sector					
1 SPF E	End Grain			6171			2 S	PF End	Grain				
ļ				6'7"									3 1/2"
				07					I				
Member In	formation						Reaction	s UNP	ATTERN	ED lb (U	plift)		
Туре:	Girder		Applicatio	on:	Floor		Brg	Live	Dea	ad S	Snow	Wind	Const
Plies:	2		Design M	lethod:	ASD	2045	1	968	191	13	1172	0	0
Moisture Con	dition: Dry		Building (	Code:	IBC/IRC 2	2015	2	968	191	13	1172	0	0
Deflection TL:	360		Deck:	anng.	Not Chec	ked							
Importance:	Normal - II												
Temperature:	Temp <= 100°F												
							Bearings						
							Bearing	Length	Cap.	React D/	Llb T	otal Ld. Cas	se Ld. Comb.
							1 - SPF	3.000"	39%	1913 / 1	605 3	518 L	D+0.75(L+S)
Analvsis Re	sults						Grain						
Analysis	Actual Lo	ocation A	llowed	Capacit	y Comb	o. Case	2 - SPF	3.000"	39%	1913 / 1	605 3	518 L	D+0.75(L+S)
Moment	5149 ft-lb	3'3 1/2" 14	4423 ft-lb	0.357 (3	6%) D+0.7	5(L+S) L	Grain						
Unbraced	5149 ft-lb	3'3 1/2" 10	0370 ft-lb	0.497 (5	0%) D+0.7	5(L+S) L							
Shear	2494 lb	11 1/2" 79	943 lb	0.314 (3	1%) D+0.7	5(L+S) L							
LL Defl inch	0.044 (L/1707)	3'3 1/2" 0.	.155 (L/480)	0.280 (2	8%) 0.75(L	_+S) L							
TL Defl inch	0.096 (L/779)	3'3 1/2" 0.	.207 (L/360)	0.460 (4	6%) D+0.7	5(L+S) L	ļ						
Design Not	tes		( 100-011) -1	401 1	4								
to exceed 6	5". 6".	d Box nails	(.128x3°) ai	t 12° 0.C. I	viaximum e	nd distance not							
2 Refer to las	st page of calculations f	or fastener	s required fo	or specified	d loads.								
4 Top loads r	nust be supported equa	ally by all pl	lies.	e only.									
5 Top braced	l at bearings.												
6 Bottom bra 7 Lateral sler	iced at bearings. nderness ratio based or	n sinale plv	width.										
ID	Load Type		ocation T	rib Width	Side	Dead 0.9	Live 1	Snov	v 1.15 V	Vind 1.6	Const. 1.	.25 Comme	ents
1	Uniform				Тор	98 PLF	294 PLF	:	0 PLF	0 PLF	0 P	PLF F2	
2	Uniform				Тор	120 PLF	0 PLF	:	0 PLF	0 PLF	0 F	PLF WALL	
3	Uniform				Тор	356 PLF	0 PLF	35	56 PLF	0 PLF	0 P	PLF A2	
	Self Weight					7 PLF							
Notes		chemicals	3		6.	For flat roofs provide p	roper drainage to	prevent	Manufacture	Info		Comtech, Inc. 1001 S. Reilly R	toad, Suite #639
Calculated Structured structural adequacy	Designs is responsible only of the of this component based on the	Handling	& Installation	<b>n</b> or drilled		ponding			Metsä Wood 301 Merritt 7 F	Buildina. 2nd	Floor	Fayetteville, NC USA	
design criteria and responsibility of the ensure the composi-	d loadings shown. It is the customer and/or the contractor to pent suitability of the intended	2. Refer to regarding	o manufacturer's installation i	s product i requirements,	nformation multi-ply				Norwalk, CT 0	6851 0		28314 910-864-TRUS	
application, and to ver	rify the dimensions and loads.	tastening approvals 3. Damaged	details, beam str Beams must not	rengtn values, be used	and code				www.metsawc	od.com/us			
1. Dry service condit 2. LVL not to be tree	tions, unless noted otherwise ated with fire retardant or corrective	4. Design as 5. Provide la	ssumes top edge is ateral support at	s laterally restra bearing points	ained s to avoid				100-E9: E9R-	-3033			тесн
		lateral dis	placement and rot	lation		This design is valid	until 11/27/202	3					

	ont	Data: 12/16/2020	Dogo 6 of 6
	ent.	Date. 12/10/2020	Fage 6 01 6
TisDesign ad	dress:	lob Name: Lot 47 SLIMMERLIN	
	uress.	Project #:	
BM3 Kerto-SLVL 1.	750" X 9.250" 2-Ply	PASSED	
	• •		N/N/I I
	• •	• • <u> </u>	
1 SPF End Grain		2 SPF End Grain	
/ /	6'7"	ł	3 1/2"
		,	5 1/2
1	6'7"	1	
Multi-Ply Analysis			
Fasten all plies using 2 rows of 10d Box	k nails (.128x3") at 12" o.c Maximur	n end distance not to exceed 6"	
Capacity 0.0 %			
LOAD U.U PLF Vield Limit per Foot 163.7 PLF			
Yield Limit per Fastener 81.9 lb.			
Yield Mode IV			
Edge Distance 1 1/2"			
Min. End Distance 3"			
Duration Eactor 1.00			
		Manufacturer Info	Comtech, Inc.
Notes chemicals Calculated Structured Designs is responsible only of the Handling &	6. For flat roots provide ponding	Metsä Wood	1001 S. Reilly Road, Suite #639 Fayetteville, NC
structural adequacy of this component based on the 1. LVL beams design criteria and loadings shown. It is the 2 Bofer to	must not be cut or drilled manufacturer's product information	301 Merritt 7 Building, 2nd Floor	USA 28314
responsibility of the customer and/or the contractor to ensure the component suitability of the intended	installation requirements, multi-ply etails, beam strength values, and code	Norwalk, CT 06851 (800) 622-5850	910-864-TRUS
application, and to verify the dimensions and loads. Lumber 3. Damaged B	learns must not be used	www.metsawood.com/us	
Dry service conditions, unless noted otherwise     Design assu     Design assu     Design assu     S. Provide late	umes top edge is laterally restrained eral support at bearing points to avoid	100-L3. ESR-3033	Comtecul
2. LVL not to be treated with fire retardant or corrosive lateral displa	acement and rotation This design is va	id until 11/27/2023	