

RE: J0520-2116 Weaver / 2 Adcock Farm / Harnett Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Lot/Block:	Project Name:	J0520-2116
Address:		
City:		

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2009/TPI2007 Wind Code: ASCE 7-05 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.3 Wind Speed: 100 mph Floor Load: N/A psf

This package includes 18 individual, dated Truss Design Drawings and 0 Additional Drawings.

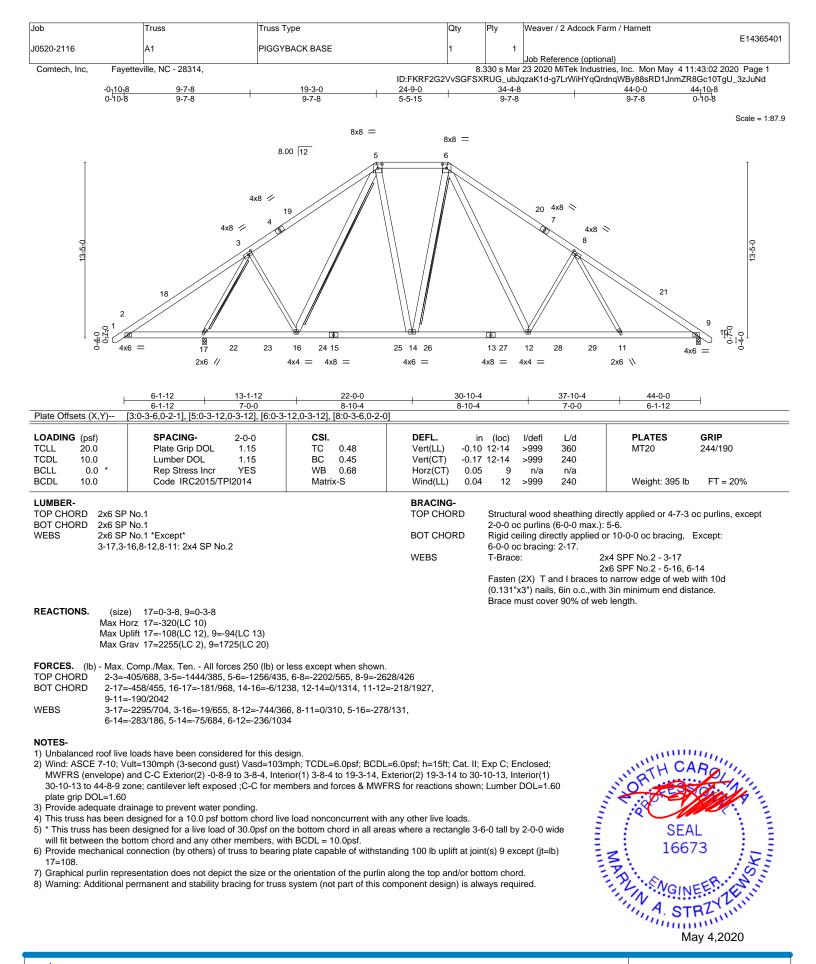
No.	Seal#	Truss Name	Date
1	E14365401	A1	5/25/2020
2	E14365402	A1SE	5/25/2020
3	E14365403	A2	5/25/2020
4	E14365404	A3	5/25/2020
5	E14365405	A4	5/25/2020
6	E14365406	A5	5/25/2020
7	E14365407	A6	5/25/2020
8	E14365408	A7	5/25/2020
9	E14365409	A8	5/25/2020
10	E14365410	A9	5/25/2020
11	E14365411	A9GE	5/25/2020
12	E14365412	B1	5/25/2020
13	E14365413	B1GE	5/25/2020
14	E14365414	C1	5/25/2020
15	E14365415	C2	5/25/2020
16	E14365416	C3	5/25/2020
17	E14365417	PB	5/25/2020
18	E14365418	PBGE	5/25/2020

The truss drawing(s) referenced above have been prepared by

into the overall building design per ANSI/TPI 1, Chapter 2.

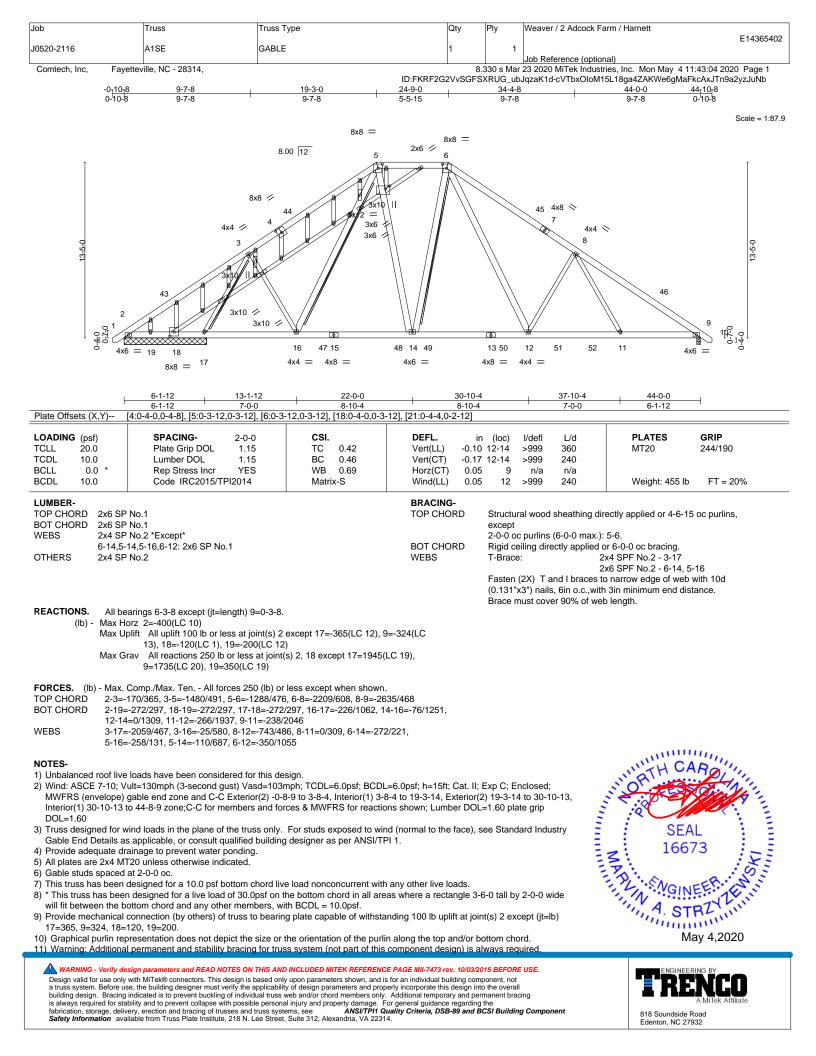
Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville. Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of North Carolina is December 31, 2020. North Carolina COA: C-0844 IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs

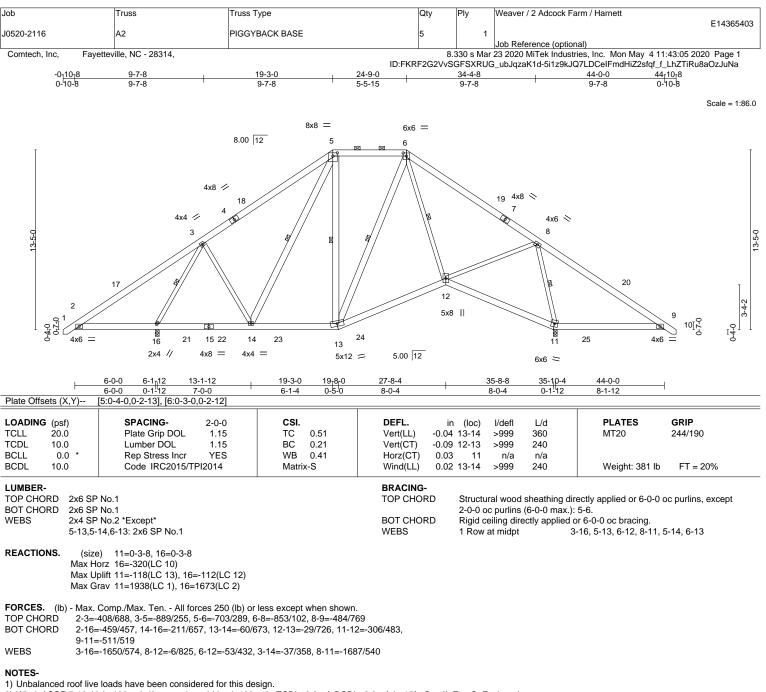




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 19-3-0, Exterior(2) 19-3-0 to 30-11-10, Interior(1) 30-11-10 to 44-8-9 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

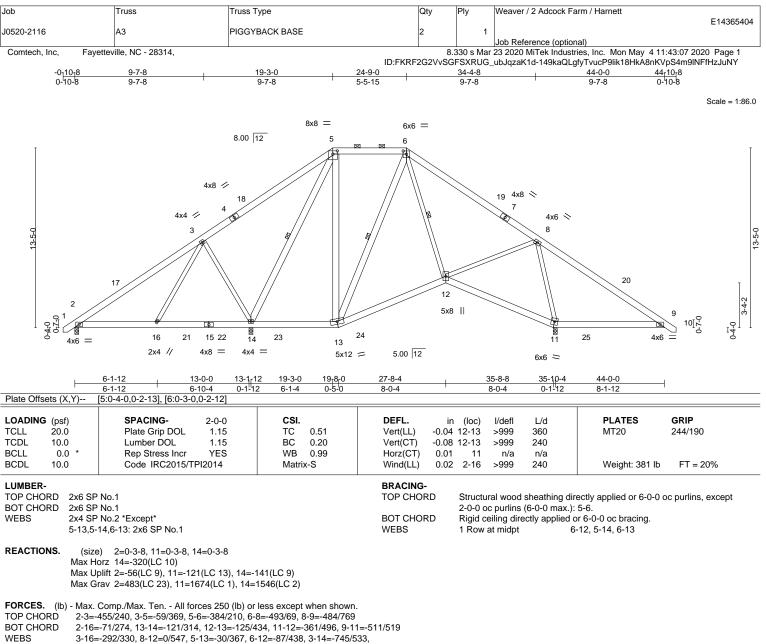
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=118, 16=112.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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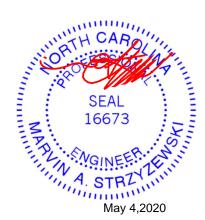


8-11=-1400/466. 5-14=-859/143. 6-13=-255/25

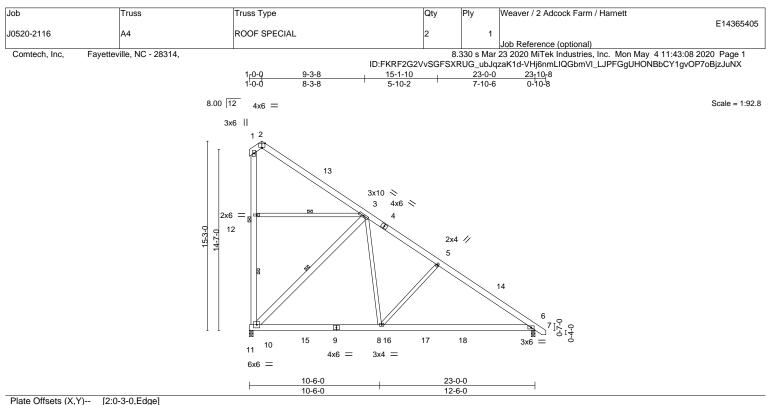
NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 19-3-0, Exterior(2) 19-3-0 to 30-11-10, Interior(1) 30-11-10 to 44-8-9 zone; cantilever right exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown;
- Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 11=121, 14=141.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







		1					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	n (loc)	l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.29	Vert(LL) -0.13	3 6-8	>999 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.56	Vert(CT) -0.28	3 6-8	>970 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.45	Horz(CT) 0.02	2 6	n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.03	3 6-8	>999 240	Weight: 219 lb	FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP			BRACING- TOP CHORD BOT CHORD	except e	al wood sheathing dir and verticals. viling directly applied o	rectly applied or 6-0-0 c	oc purlins,
	10: 2x6 SP No.1		WEBS JOINTS	1 Row a	0 7 11	0-12, 3-10, 3-12	

REACTIONS. (size) 10=0-3-8, 6=0-3-8 Max Horz 10=-482(LC 13) Max Uplift 10=-236(LC 13) Max Grav 10=1177(LC 20), 6=1071(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-5=-1052/0, 5-6=-1298/0, 10-12=-259/166, 1-12=-259/166

BOT CHORD 8-10=0/743, 6-8=0/987

WEBS 5-8=-424/234, 3-10=-1116/335, 3-8=-26/850

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 5-4-13, Interior(1) 5-4-13 to 23-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=236.





Job	Truss	Truss Type	Qty	Ply	Weaver / 2 Adcock Farm	/ Harnett	
J0520-2116	A5	ROOF SPECIAL	1	1			E14365406
					Job Reference (optional)		(0.00.000 D. (
Comtech, Inc, Faye	tteville, NC - 28314,	ID:EKRE2			· 23 2020 MiTek Industries, JqzaK1d-zTGU_5MxAZjd7v		
		1 _c 0-0 9-3-8 15-1-10	02110010	23-0-0	23 ₁ 10 ₁ 8	2.00mm Diq mbyra	
		1 ¹ 0-0 8-3-8 5-10-2		7-10-6	0-10-8		
	8.00 1	² 4x6 =					Scale = 1:93.4
	3.	6					
	38	1 2					
	0 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	15 3 4x6 ≈ 4 4 10 12 17 11 109 4x12	2x4 5	1	6 6 7 [0 0 14 8 8x16 4x12		
		6x6 =			¥X12		
		8x16 _ 10-6-0 11-10-8	20-10-0	D	23-0-0		
		10-6-0 1-4-8	8-11-8		2-2-0		
Plate Offsets (X,Y) [2	2:0-3-0,Edge], [6:0-5-0,Edge],	8:0-4-12,0-1-12], [9:0-4-12,0-1-8], [10:1-0-8,0-	4-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 COLL 10.0	SPACING- 2-0- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE	5 TC 0.37 Vert(L 5 BC 0.49 Vert(C 6 WB 0.45 Horz(L) -0.12 CT) -0.30 CT) 0.08	0 6-10 3 6	l/defl L/d >999 360 >887 240 n/a n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S Wind(LL) 0.05	5 6-10	>999 240	Weight: 245 lb	FT = 20%
6-11: 2x WEBS 2x4 SP	No.1 *Except* 6 SP No.1 No.2 *Except* 2: 2x6 SP No.1		HORD	except Rigid ce 1 Row a	al wood sheathing directl and verticals. illing directly applied or 10 at midpt 12-14 at Jt(s): 14		4 oc purlins,
Max Ho Max Up	6=0-3-8, 12=0-3-8 rz 12=-481(LC 13) lift 12=-233(LC 13) av 6=979(LC 20), 12=1127(LC	20)					
TOP CHORD 3-5=-1 BOT CHORD 10-12=	Comp./Max. Ten All forces 25 079/0, 5-6=-1350/0, 12-14=-25 e0/827, 6-10=0/1044 432/206, 3-12=-1146/302, 3-1						
2) Wind: ASCE 7-10; Vu MWFRS (envelope) a		sd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=1 -13, Interior(1) 5-4-13 to 23-8-9 zone;C-C for					

reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=233.





Job	Truce		04.	Ply	Weaver / 2 Adcock Fa	arm / Harpott	
Jod J0520-2116	Truss A6	Truss Type ROOF SPECIAL	Qty 1		Weaver / Z AOCOCK Fa	ann / Haifiell	E14365407
	Atteville, NC - 28314,			2	Job Reference (option	nal) ries, Inc. Mon May 411	·43·10 2020 Page 1
Comtech, Inc, Faye	enteville, NC - 28314,	1 <u>-0-0 9-3-8</u> 1-0-0 8-3-8				ries, Inc. Mon May 411 NZxtrUI37kQqHklvMjU?I	
	8.00 1	470 —					Scale = 1:93.4
	0-2×6 9-2-1 9-1 9-1 9-1 9-1 9-1 9-1 9-1 9-1 9-1 9		3x10 × 3 4x6 × 4 2x4 4 5 11 109 4x12	× 11	6 7 [- 8 8x16 4x12		
		10-6-0	8x16 11-10-8 20-10-0		23-0-0		
Plate Offsets (X,Y)	[2:0-3-0.Edae]. [6:0-5-0.Edae].	10-6-0 B:0-4-12,0-1-12], [9:0-4-12,0-1-12	1-4-8 8-11-8		2-2-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 3-6- Plate Grip DOL 1.1: Lumber DOL 1.1: Rep Stress Incr NK Code IRC2015/TPI2014	CSI. TC 0.37 BC 0.47	DEFL. in Vert(LL) -0.11 Vert(CT) -0.27 Horz(CT) 0.07	6	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 490 lb	GRIP 244/190 FT = 20%
6-11: 2: WEBS 2x4 SP 1-12,3- REACTIONS. (size Max H Max U	P No.1 *Except* k6 SP No.1 No.2 *Except* 12: 2x6 SP No.1 e) 6=0-3-8, 12=0-3-8 prz 12=-842(LC 13) plift 12=-407(LC 13)		BRACING- TOP CHORD BOT CHORD WEBS JOINTS	(Switche Rigid ce 1 Row a	ed from sheeted: Spa iling directly applied o		
FORCES. (lb) - Max. TOP CHORD 1-2=- 1-14= BOT CHORD 10-12) 0 (Ib) or less except when shown 89/0, 5-6=-2362/0, 12-14=-448/2					
 Top chords connected Bottom chords connected Bottom chords connected as All loads are conside ply connections have Unbalanced roof live Wind: ASCE 7-10; V MWFRS (envelope) reactions shown; Lui This truss has been will fit between the b Provide mechanical 12=407. 	follows: 2x4 - 1 row at 0-9-0 oc, red equally applied to all plies, been provided to distribute onl loads have been considered fo ult=130mph (3-second gust) Va and C-C Exterior(2) 0-4-4 to 5-4 mber DOL=1.60 plate grip DOL= designed for a 10.0 psf bottom on the designed for a live load of 30.0 bottom chord and any other mem connection (by others) of truss to	ered at 0-9-0 oc. taggered at 0-9-0 oc, 2x6 - 2 row 2x6 - 2 rows staggered at 0-9-0 c except if noted as front (F) or bac vecept if noted as (F) or (B), unless this design. sd=103mph; TCDL=6.0psf; BCDL -13, Interior(1) 5-4-13 to 23-8-9 z -1.60 hord live load nonconcurrent with psf on the bottom chord in all are	bc. k (B) face in the LOAD C. s otherwise indicated. ==6.0psf; h=15ft; Cat. II; E cone;C-C for members an n any other live loads. bas where a rectangle 3-6 anding 100 lb uplift at join	Exp C; Er d forces i-0 tall by t(s) exce	nclosed; & MWFRS for 2-0-0 wide	SEA 166 A ST	

May 4,2020

CO

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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ENGINEERING BY

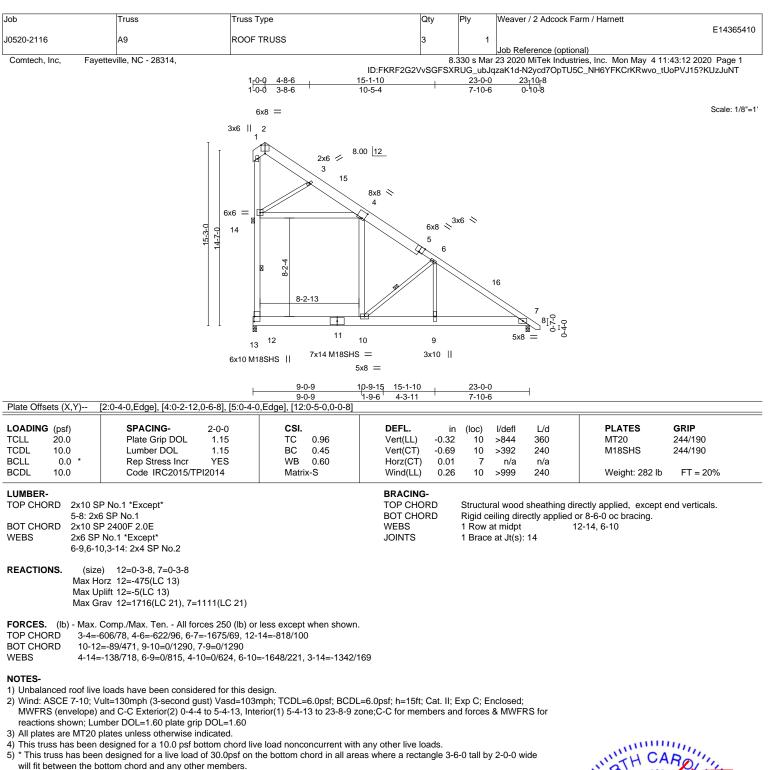
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<pre>turn to turn to t</pre>			1 ₋ 0-0 9-3-8 1-0-0 8-3-8					
Automatical and a state of the state of		8.00 11	_	0.10.2				Scale - 1:03 /
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arife II 108-0 123-00 23-00 Plate Offsets (X,Y)- [20-3-0.Edge], [80-0-12.0:1-12], [10-0.0:4-0] (DADING 6: 4-0 Colspan="2">Colspan="2"Colsp			13 12		4			
Image: The original sector of the s			6x8 =					
Plate Offsets (X,Y)- [2:0-3-0.Edge], [6:0-5-0.Edge], [6:0-4-12.0-1-12], [9:0-4-12.0-1-12], [1:0-1-0-0.0-4-0] LoADING (ps) TCLL SPACING- 148 Grip DOL 1.15 1.15 CSI. TC DEFL. 10:0 in (loc) Idell 10:0 PLATES 244/190 GRIP MT20 CLD 10:0 Rep Stress inc NO Rep Stress inc NO Rep Stress inc NO BCLL 0.0 Rep Stress inc NO WB 0.79 Horz(CT) 0.09 6 n/a BCDL 10:0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.06 6.10 >999 240 Weight: 490 lb FT = 20% LUMBER- TOP CHORD 2x10 SP No.1 Excentions Switched from sheeted: Spacing > 2-8-0). Switched from sheeted: Spacing > 2-8-0). <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>								
TCLL 20.0 Plate Grip DOL 1.15 TC 0.46 Vert(C1) 0.13 6-10 398 360 MT20 244/190 BCLL 0.0 * Rep Stress Incr NO WB 0.79 Horz(CT) 0.03 6-10 358 240 Weight: 490 lb FT = 20% LUMEC Code IRC015/TPI2014 WB 0.79 Horz(CT) 0.03 6-10 399 240 Weight: 490 lb FT = 20% LUMEC TOP CHORD 2x10 SP No.1 BOT CHORD 2x10 SP No.1 BOT CHORD 2x00 oc purlins (6-0-0 mx.), except and verticals (Switched from sheeted: Spacing 2-8-0). Rigid caling directly applied or 10-0-0 oc bracing. WEBS 2x14 SP No.2 Except* WESS 1 Row at midpt 12-14 T1-2_3-122.202 (LC 13) Max Upit 12_3-122.302 (D 30-35-22940, 0-5-23860, 12-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-544/352, 1-14=-546/353 SOT CHORD 2x0 bit 0.10/12=0/178, 57 SOT CHORD Xer Ar	Plate Offsets (X,Y) [2:0-3-0,Edge], [6:0-5-0,Edge], [-		
LUMBER- TOP CHORD 2x6 SP No.1 BOT CHORD 2x10 SP No.1*Except* 6-11:2x6 SP No.1 BOT CHORD 2x10 SP No.1*Except* 6-11:2x6 SP No.1 WEBS 2x4 SP No.2*Except* H12,3-12:2x6 SP No.1 Reactions: (size) 6=0-3-8, 12=0-3-8 Max Horz 12=-1022(LC 13) Max Cgrut 21=-4052(LC 13) Max Grav 6=2081(LC 20), 12=2394(LC 20) FORCES. (b) - Max. Comp./Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 10-12=0/1758, 6-10-0/2219 WEBS 5-10=-919/438, 3-12=-2436(641, 3-10=0/1857 NOTES 1) 2-p19/truss to be connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Botom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works Connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works Connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Works (envelope) and C-0 Exterior(2) -0.44 to 54-13, 1.54 to 23-8-9 zone; C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 pelf or 10.0 p5 botom chord live load nonconcurrent with any other live loads. 5) This trues has been designed for a 10.0 p5 botom chord live load nonconcurrent with any other live loads. 6) This trues has been designed for a 10.0 p5 botom chord live load nonconcurrent with any other l	TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNO	TC 0.46 BC 0.58 WB 0.79	Vert(LL) -0.13 Vert(CT) -0.33 Horz(CT) 0.03	3 6-10 2 6-10 9 6	>999 360 >835 240 n/a n/a	MT20	244/190
TOP CHORD 2x6 SP No.1 TOP CHORD 2-0-0 cp unlins (6-0-0 max), except end verticals (Switched from sheeted: Spacing 2-8-0). BOT CHORD 2x10 SP No.1*Except* BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2x4 SP No.2*Except* BOT CHORD New at midpt 12-14 Int: 2x5 SP No.1 JOINTS 1 Brace at Jt(s): 2, 1, 14 12-14 REACTIONS (size) 6-0-3-8, 12-0-3-8 Max Horz 12=-1022(LC 13) Max Uplint 12=-496(LC 13) Max Grav 6-2081(LC 20), 12-22394(LC 20) 1 Brace at Jt(s): 2, 1, 14 12-14 FORCESS. (b) -Max. Comp.Max. TenAll forces 250 (b) or less except when shown. TOP CHORD 1-2=-404/270, 2-3=-460/130, 3-5=-2294/0, 5-6=-2868/0, 12-14=-544/352, 1-14=-544/352, 1-14=-544/353. 1-14=-543/353 BOT CHORD 1-2=-404/270, 2-3=-460/130, 3-5=-2294/0, 5-6=-2868/0, 12-14=-544/352, 1-14=-544/352, 1-14=-544/353. 1-2=-404/270, 2-3=-460/130, 3-5=-2294/0, 5-6=-2868/0, 12-14=-544/352, 1-14=-544/352, 1-14=-544/353. BOT CHORD 1-2=-404/270, 2-3=-460/130, 3-5=-2294/0, 5-6=-2868/0, 12-14==544/352, 1-14=-544/353. 1-12=-10=194/38, 3-12=-2436/641, 3-10=0/1857 NOTES 1) 2-ply truss to be connected together with 10d (0.131*X3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. 2 2) All loads are considered do gallows: 2x10 - 2 rows staggered at 0-9-0 oc. 2 2 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>								
Max Grav 6=2081(LC 20), 12=2394(LC 20) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-404/270, 2-3=-460/130, 3-5=-2294/0, 5-6=-2868/0, 12-14=-544/352, 1-14=-545/353 BOT CHORD 10-12=0/1758, 6-10=0/2219 WEBS 5-10=-919/438, 3-12=-2436/641, 3-10=0/1857 NOTES- 1) 2-ply truss to be connected together with 10d (0.131*x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x6 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 5-4-13, Interior(1) 5-4-13 to 23-8-9 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 5) This truss has been designed for a 1.00 pls bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a 1.00 pls bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a 1.00 pls bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a 1.00 pls bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a 1.00 pls bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a live	TOP CHORD 2x6 SP BOT CHORD 2x10 SF 6-11: 2x WEBS 2x4 SP 1-12,3-1 REACTIONS. (size Max Ho	P No.1 *Except* 6 SP No.1 No.2 *Except* 12: 2x6 SP No.1) 6=0-3-8, 12=0-3-8 prz 12=-1022(LC 13)		TOP CHORD BOT CHORD WEBS	(Switche Rigid ce 1 Row a	ed from sheeted: Spacing iling directly applied or 1 it midpt 12-1	g > 2-8-0). 0-0-0 oc bracing.	
 FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-404/270, 2-3=-460/130, 3-5=-2294/0, 5-6=-2868/0, 12-14=-544/352, 1-14=-545/353 BOT CHORD 10-12=-01758, 6-10=0/2219 WEBS 5-10=-919/438, 3-12=-2436/641, 3-10=0/1857 NOTES- 1) 2-ply truss to be connected together with 10d (0.131*x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to taistribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 5) This truss has been designed for a 10.0 psf bottom chord live load on nonconcurrent with any other live loads. 6) * This truss has been designed for a 10.0 psf bottom chord live load on nonconcurrent with any other live loads. 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 			C 20)					
 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 5-4-13, Interior(1) 5-4-13 to 23-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a 10.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 	FORCES. (lb) - Max. 0 TOP CHORD 1-2=-4 1-14=- BOT CHORD 10-12=	Comp./Max. Ten All forces 25 104/270, 2-3=-460/130, 3-5=-22 -545/353 =0/1758, 6-10=0/2219	0 (Ib) or less except when shown. 94/0, 5-6=-2868/0, 12-14=-544/35					
 Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 5-4-13, Interior(1) 5-4-13 to 23-8-9 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6) * This truss has been designed for a ive load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 								
	 Top chords connecte Bottom chords conner Webs connected as f 2) All loads are consided ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; Vu MWFRS (envelope) a reactions shown; Lun 5) This truss has been c 6) * This truss has been to will fit between the bo 7) Provide mechanical c 12=495. 	d as follows: 2x6 - 2 rows stagg cted as follows: 2x10 - 2 rows st ollows: 2x4 - 1 row at 0-9-0 oc, red equally applied to all plies, been provided to distribute onl loads have been considered fo ult=130mph (3-second gust) Va and C-C Exterior(2) 0-4-4 to 5-4 hoer DOL=1.60 plate grip DOL= designed for a 10.0 psf bottom of designed for a live load of 30.0 ottom chord and any other mem connection (by others) of truss to	ered at 0-9-0 oc. taggered at 0-9-0 oc, 2x6 - 2 row 2x6 - 2 rows staggered at 0-9-0 o except if noted as front (F) or back / loads noted as (F) or (B), unless this design. sd=103mph; TCDL=6.0psf; BCDL -13, Interior(1) 5-4-13 to 23-8-9 zt 1.60 hord live load nonconcurrent with psf on the bottom chord in all are bers, with BCDL = 10.0psf. b bearing plate capable of withsta	AC. (B) face in the LOAD (s otherwise indicated. =6.0psf; h=15ft; Cat. II; one;C-C for members a any other live loads. as where a rectangle 3 inding 100 lb uplift at join	CASE(S) s Exp C; Er and forces a -6-0 tall by int(s) excep	nclosed; & MWFRS for 2-0-0 wide		• -

May 4,2020

ENGINEERING BY EREENCED A Mi Tek Affiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Weaver / 2 Adcock Farm / H	Harnett	
J0520-2116	A8	ROOF TRUSS	1			lamon	E14365409
			'	2	Job Reference (optional)	- M M 444	10:10 0000 Dava 1
	 etteville, NC - 28314, 3	$1 \xrightarrow{10-0}_{1-0-0} \frac{4-8-7}{3-8-7} \qquad 1 \\ 6x8 = \\ 6 \parallel 2 \\ 1 \\ 7 \\ 6x6 = \\ 8 \\ 4 \\ 8 \\ 8 \\ 7 \\ 8 \\ 8 \\ 8 \\ 2x6 \neq 8.0 \\ 3 \\ 15 \\ 1 \\ 1 \\ 7 \\ 8 \\ 8 \\ 8 \\ 2x6 \neq 8.0 \\ 3 \\ 15 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ $		330 s Mar UG_ubJq: 23-0-0 7-10-6	23 2020 MiTek Industries, Ir zaK1d-N2ycd7OpTU5C_NH 23_10_8 0-10-8		
Plate Offsets (X,Y)	[2:0-4-0,Edge], [4:0-6-0,0-2-8],	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	0 9 48 = 2x6 0-9-15 15-1-10 1-9-6 4-3-11	23-0-0 7-10-6			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-8- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr Ni Code IRC2015/TPI2014	5 TC 0.73 5 BC 0.33	DEFL. in Vert(LL) -0.21 Vert(CT) -0.46 Horz(CT) 0.01 Wind(LL) 0.17	10 10 7	l/defl L/d >999 360 >587 240 n/a n/a >999 240	PLATES MT20 M18SHS Weight: 563 lb	GRIP 244/190 244/190 FT = 20%
BOT CHORD 5-8: 2x0 WEBS 2x10 SP 6-9,6-11 REACTIONS. (size Max He Max U	P No.1 *Except* 6 SP No.1 P 2400F 2.0E No.1 *Except* 0,3-14: 2x4 SP No.2 e) 12=0-3-8, 7=0-3-8 orz 12=-633(LC 13) plift 12=-7(LC 13) rav 12=2288(LC 21), 7=1482(L	C 21)	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	except e Rigid cei 1 Row at	al wood sheathing directly nd verticals. ling directly applied or 10- t midpt 12-14 at Jt(s): 14		oc purlins,
TOP CHORD 3-4=- BOT CHORD 10-12 WEBS 4-14= NOTES- 1) 2-ply truss to be con Top chords connecte Bottom chords connected Bottom chords connected as 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; V MWFRS (envelope) reactions shown; Lur 5) All plates are MT20 p 6) This truss has been ") * This truss has been will fit between the b 8) Ceiling dead load (11 9) Bottom chord live load	808/104, 4-6=-829/128, 6-7=-22 2=-119/629, 9-10=0/1720, 7-9=0 -184/957, 6-9=0/1086, 4-10=0/ nected together with 10d (0.131 ad as follows: 2x10 - 2 rows stag- ected as follows: 2x10 - 2 rows stag- ected as follows: 2x10 - 2 rows stag- red equally applied to all plies, a been provided to distribute on a loads have been considered for ult=130mph (3-second gust) V2 and C-C Exterior(2) 0-4-4 to 5-4 mber DOL=1.60 plate grip DOL- plates unless otherwise indicate designed for a 10.0 psf bottom n designed for a 10.0 psf bottom n designed for a live load of 30.0 ottom chord and any other men 0.0 psf) and additional bott I connection (by others) of truss	/1720 i33, 6-10=-2198/295, 3-14=-1790, i33, 6-10=-2198/295, 3-14=-1790, istaggered at 0-9-0 oc, 2x6 - 2 rows si istaggered at 0-9-0 oc. at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc except if noted as front (F) or back y loads noted as front (F) or back y loads noted as (F) or (B), unless t this design. sd=103mph; TCDL=6.0psf; BCDL -13, Interior(1) 5-4-13 to 23-8-9 zr -1.60 d. hord live load nonconcurrent with psf on the bottom chord in all are	/225 taggered at 0-9-0 oc. oc. < (B) face in the LOAD C, s otherwise indicated. =6.0psf; h=15ft; Cat. II; E one;C-C for members an any other live loads. as where a rectangle 3-6 (s).4-10 blied only to room. 10-12	Exp C; En d forces & -0 tall by	closed; & MWFRS for	A ST	• •

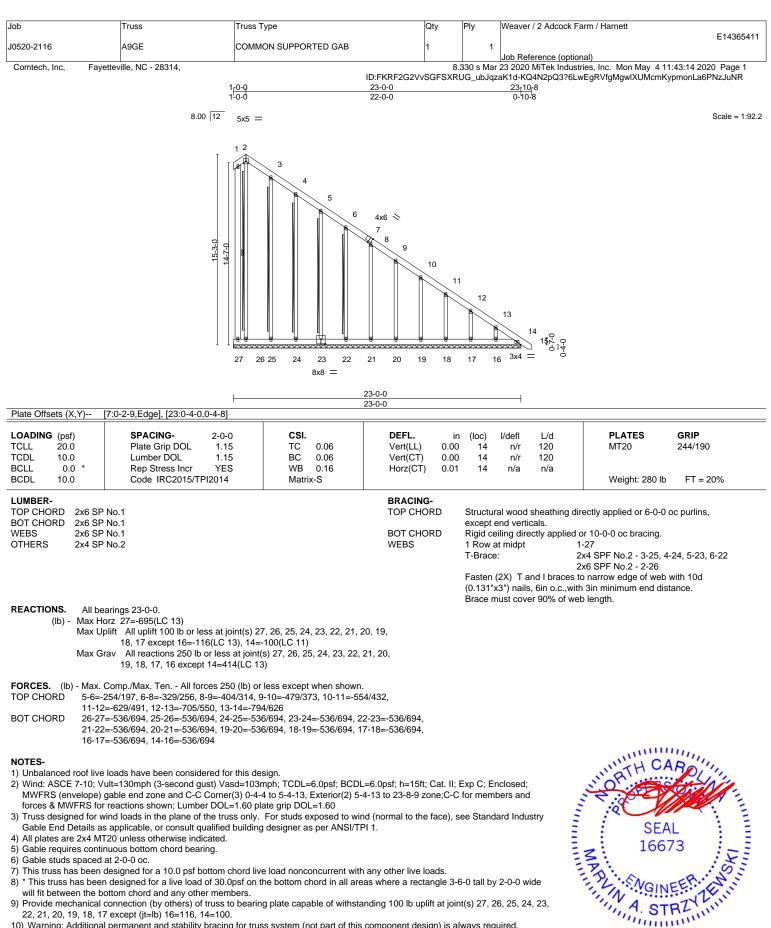
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- 6) Ceiling dead load (10.0 psf) on member(s). 4-14; Wall dead load (5.0psf) on member(s).4-10
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 10-12
- B) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12.
- 9) Attic room checked for L/360 deflection.
- Autic room cnecked for L/360 deflection





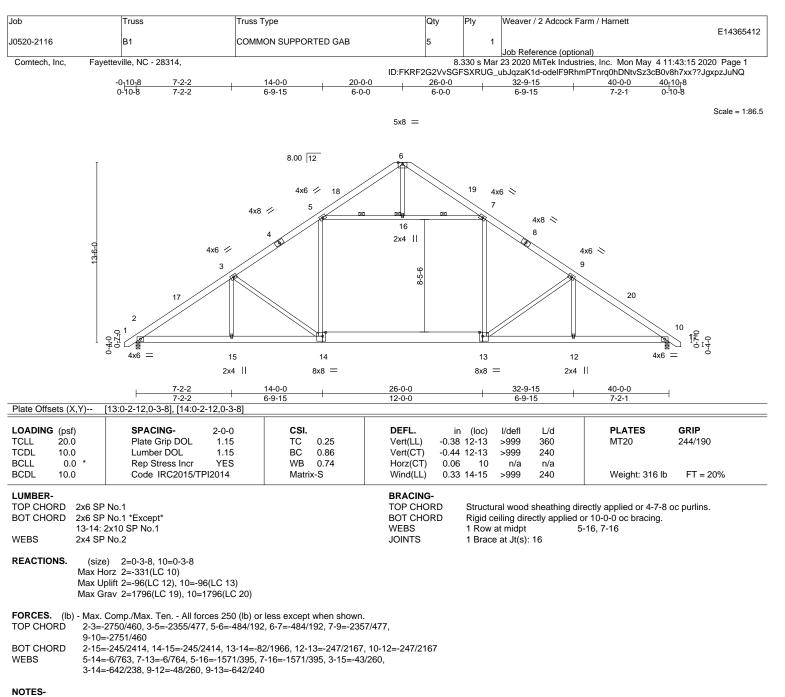


- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 27, 26, 25, 24, 23, 22, 21, 20, 19, 18, 17 except (jt=lb) 16=116, 14=100.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid for use only with with every connectors. This design is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-9 to 3-8-4, Interior(1) 3-8-4 to 20-0-0, Exterior(2) 20-0-0 to 24-4-13, Interior(1) 24-4-13 to 40-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

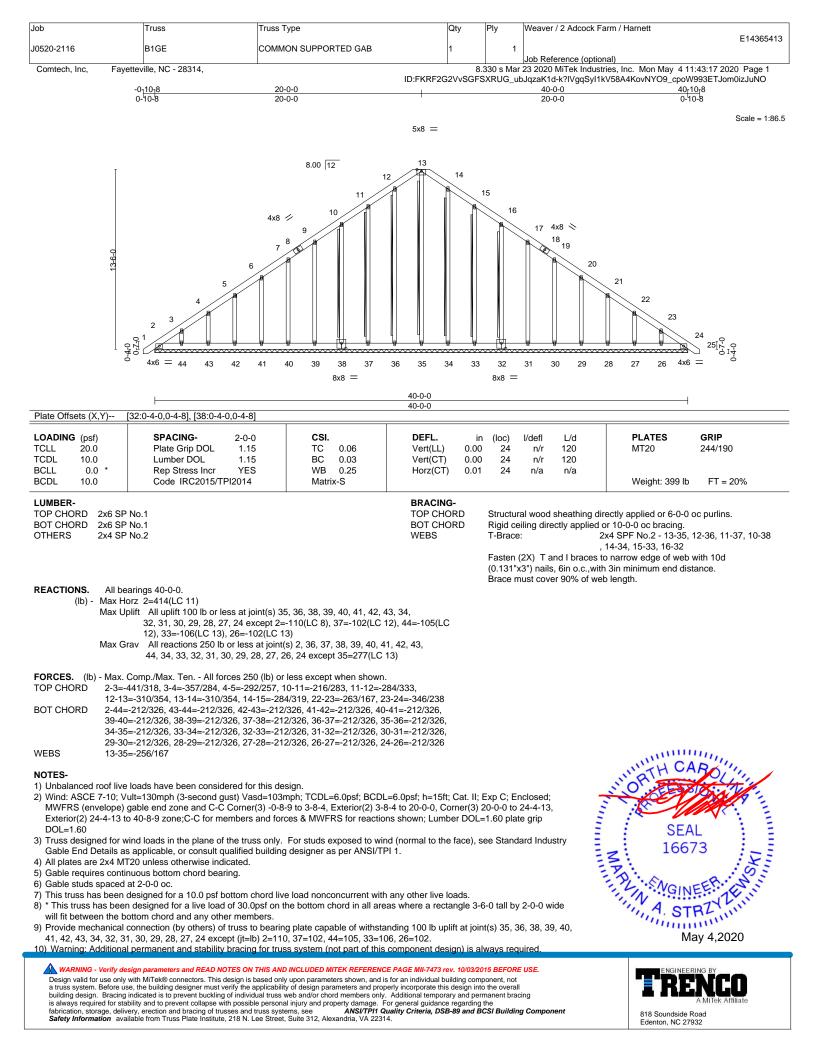
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

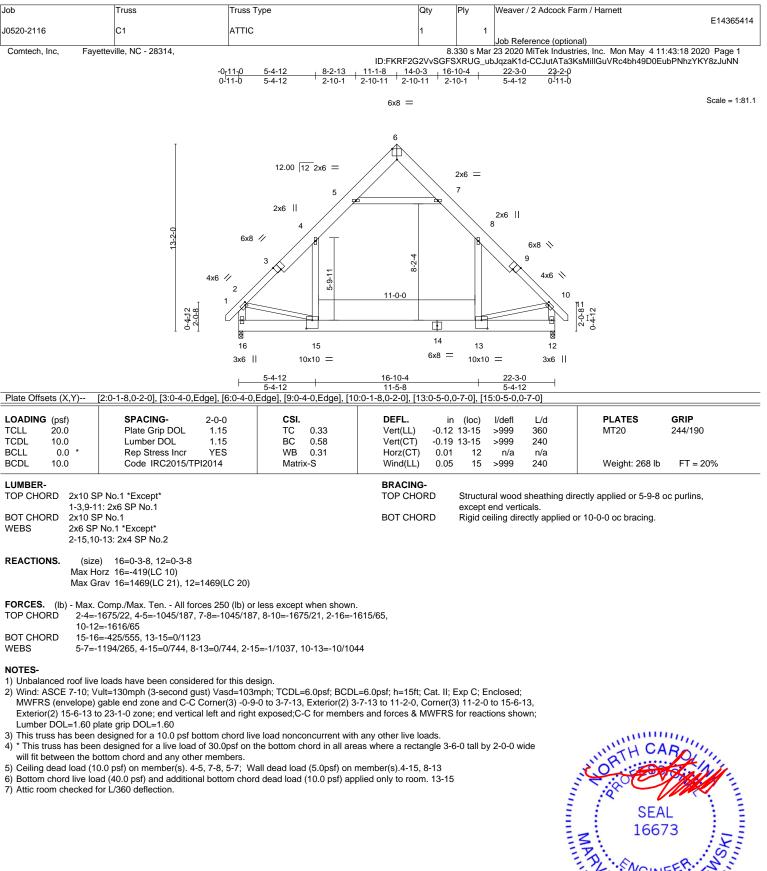
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.



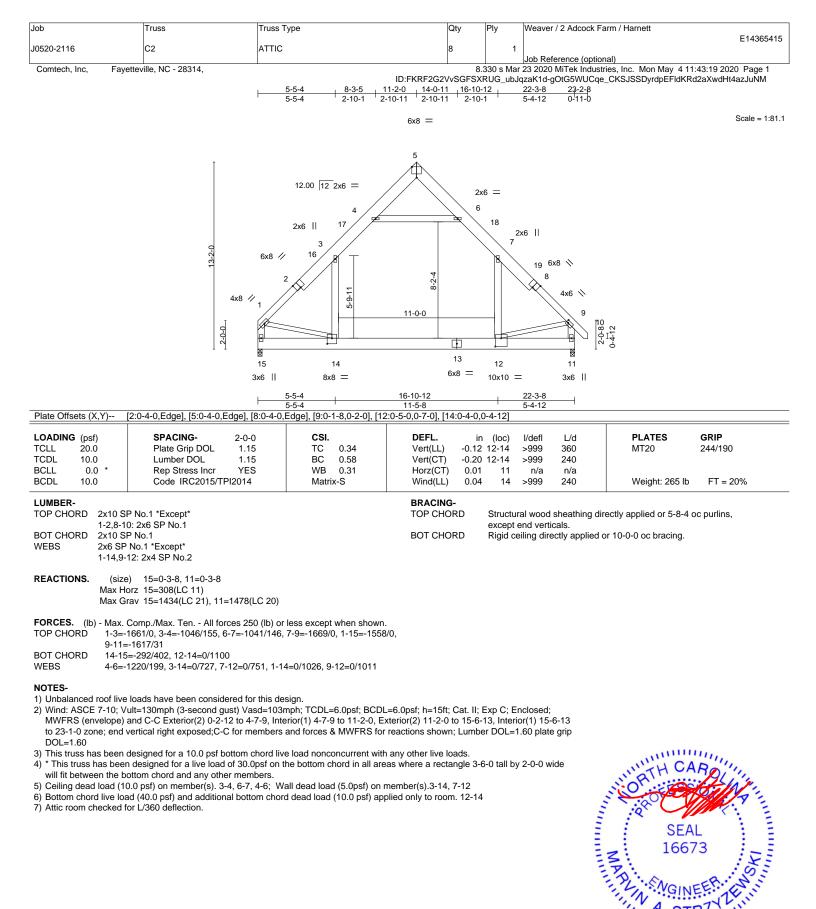
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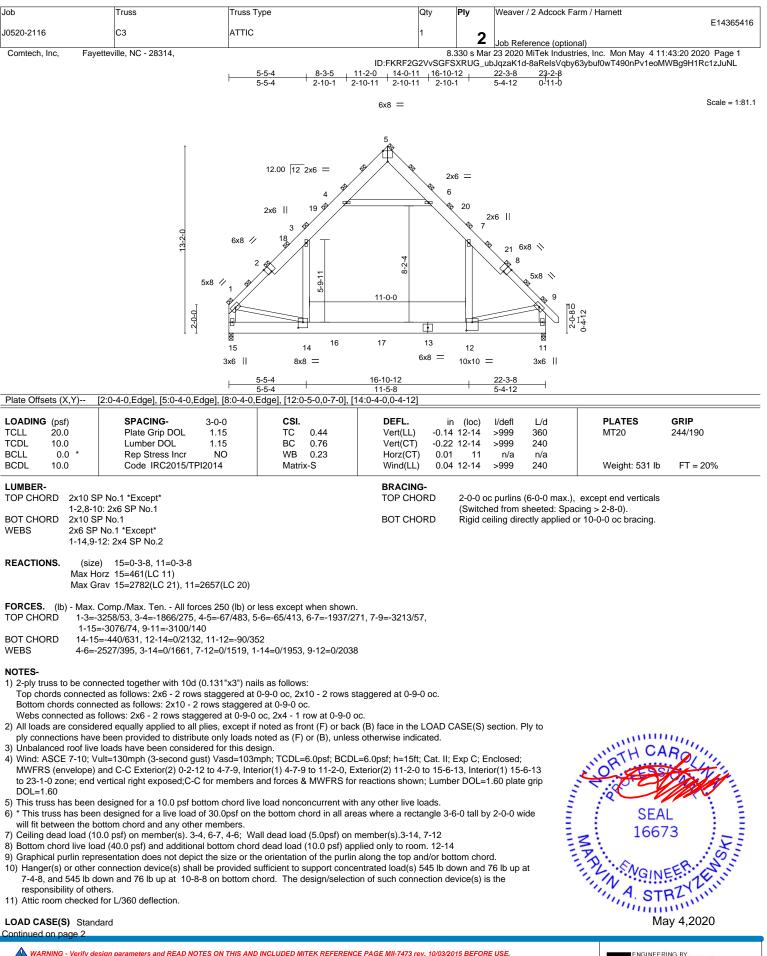




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May 4,2020





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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Weaver / 2 Adcock Farm / Harnett
					E14365416
J0520-2116	C3	ATTIC	1	2	
				_	Job Reference (optional)
Comtech, Inc, Fayettevi	lle, NC - 28314,		8.3	330 s Mar	23 2020 MiTek Industries, Inc. Mon May 4 11:43:20 2020 Page 2

on May dustrie ID:FKRF2G2VvSGFSXRUG_ubJqzaK1d-8aRelsVqby63ybuf0wT490nPv1eoMWBg9H1Rc1zJuNL

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

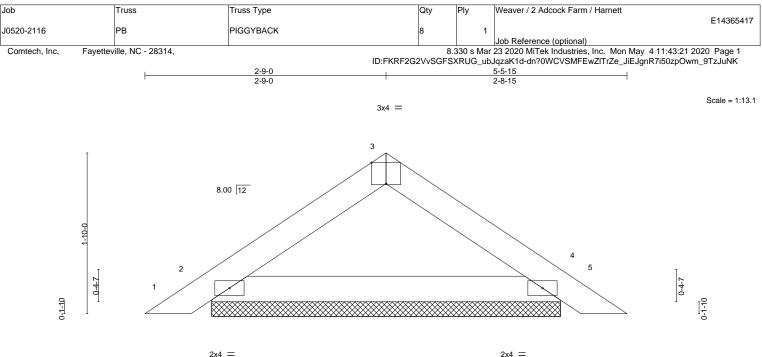
Uniform Loads (plf)

Vert: 14-15=-30, 12-14=-60, 11-12=-30, 1-3=-90, 3-4=-120, 4-5=-90, 5-6=-90, 6-7=-120, 7-9=-90, 9-10=-90, 4-6=-30 Drag: 3-14=-15, 7-12=-15

Concentrated Loads (lb)

Vert: 16=-300(B) 17=-300(B)





CLL ŽO.Ó			DEFL. in (loc) I/defl L/d	PLATES GRIP
OLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) 0.00 4 n/r 120	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.14	Vert(CT) 0.00 5 n/r 120	
CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 4 n/a n/a	
CDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 16 lb FT = 20%

5-5-15

REACTIONS. (size) 2=3-11-11, 4=3-11-11 Max Horz 2=-40(LC 10) Max Uplift 2=-16(LC 12), 4=-16(LC 13) Max Grav 2=189(LC 1), 4=189(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

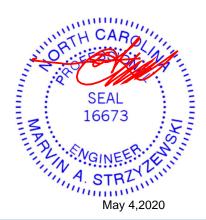
3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

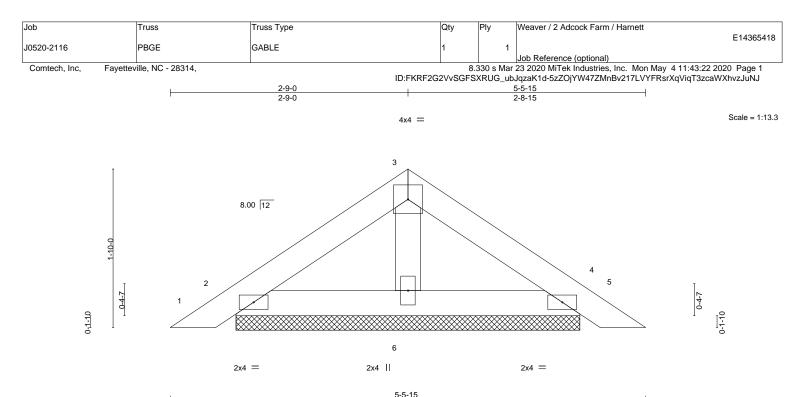
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







	5-5-15									1		
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.05	Vert(LL)	0.00	4	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	5	n/r	120	-	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-P						Weight: 17 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-5-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=3-11-11, 4=3-11-11, 6=3-11-11 Max Horz 2=50(LC 11) Max Uplift 2=-45(LC 12), 4=-52(LC 13) Max Grav 2=119(LC 1), 4=119(LC 1), 6=139(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone;C-C for members and forces & MWFRS for reactions shown; Lumber

- DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.





