

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2000570-2000570A Clinton

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I42125682 thru I42125700

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



July 22,2020

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





July 22,2020

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANS/TPH1</u> Quality Criteria, DSB-89 and BCSI Building Component
 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



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		1		3	32-0-0					1	
		Γ		3	32-0-0					1	
Plate Offsets ((X,Y)	[1:0-0-0,0-1-1], [1:0-1-9,0-4-2	2], [19:0-1-9,0-4-2], [19	:0-0-0,0-1-1], [28	:0-0-0,0-1-12],	[29:0-2	-0,0-1-4]	, [29:0-1-'	12,0-0-0]		
LOADING (ps TCLL 20 TCDL 10	sf)).0).0	SPACING- 2- Plate Grip DOL 1 Lumber DOL 1	0-0 CSI. 1.15 TC 1.15 BC	0.10 0.04	DEFL. Vert(LL) Vert(CT)	in -0.00 -0.01	(loc) 20 20	l/defl n/r n/r	L/d 120 90	PLATES MT20	GRIP 197/144
BCLL 0 BCDL 10).0 *).0	Rep Stress Incr Y Code IRC2015/TPI20	YES WB 14 Matri	0.13 ix-S	Horz(CT)	0.01	19	n/a	n/a	Weight: 218 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD OTHERS	2x4 SF 2x4 SF 2x4 SF	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2 P No.3			BRACING TOP CHOI BOT CHOI WEBS	RD RD	Structur Rigid ce 1 Row a	al wood s iling direc t midpt	heathing dire tly applied or 10	ctly applied or 6-0-0 o 10-0-0 oc bracing. -29, 9-30, 11-27	c purlins.

WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 32-0-0.

(lb) - Max Horz 1=-244(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 30, 32, 33, 34, 35, 36, 37, 27, 26, 25, 24, 23, 22, 21, 19 Max Grav All reactions 250 lb or less at joint(s) 1, 29, 30, 32, 33, 34, 35, 36, 37, 27, 26, 25, 24, 23, 22, 21.19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.









		1				32-0-0					1	
		I				32-0-0					1	
Plate Off	sets (X,Y)	[2:0-1-9,0-4-2], [2:0-0-0,0)-1-1], [12:0-2	-0,Edge], [22:	0-0-0,0-1-1]	, [22:0-1-9,0-4-2]						
	G (psf)	SPACING- Plate Grip DOI	2-0-0	CSI.	0.11	DEFL.	in -0.00	(loc)	l/defl	L/d 120	PLATES MT20	GRIP
TCDL BCLL	10.0 0.0 *	Lumber DOL Rep Stress Incr	1.15 YES	BC WB	0.05 0.13	Vert(CT) Horz(CT)	-0.00 -0.01 0.01	23 22	n/r n/a	90 n/a	WI 20	13// 144
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 232 lb	FT = 20%
LUMBER	-					BRACING-						

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
OTHERS	2x4 SP No.3
WEDGE	

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 11-33, 13-31, 10-34, 14-30

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 32-0-0. (lb) -

Max Horz 2=-248(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 34, 36, 37, 38, 39, 40, 41, 30, 29, 28, 27, 26, 25, 24, 22 Max Grav All reactions 250 lb or less at joint(s) 2, 33, 31, 34, 36, 37, 38, 39, 40, 41, 30, 29, 28, 27, 26, 25.24.22

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-279/178

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



818 Soundside Road Edenton, NC 27932









- TOP CHORD 2-3=-1202/195, 3-4=-1220/294, 6-7=-1223/294, 7-8=-1203/195
- BOT CHORD 2-14=-122/1078, 10-14=-51/787, 8-10=-79/970
- WEBS 6-10=-137/561, 7-10=-301/215, 4-14=-136/558, 3-14=-301/215, 4-6=-675/212

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.

6) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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	L				20-0-0						
					20-0-0						
Plate Offsets (X,Y)	[2:Edge,0-1-1], [2:0-1-9,0)-4-2], [12:0-1-	9,0-4-2], [12:	Edge,0-1-1],	[21:0-2-0,0-1-4], [21:0-0-0),0-1-12], [22:0-1	-12,0-0-0]		
LOADING (psf)	SPACING-	2-0-0	CSI.	0.10	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Lumber DOL Rep Stress Incr	1.15 1.15 YES	BC WB	0.10 0.03 0.09	Vert(LL) Vert(CT) Horz(CT)	-0.00 -0.01 0.00	13 13 12	n/r n/r n/a	120 90 n/a	M120	197/144
BCDL 10.0	Code IRC2015/TI	PI2014	Matrix	x-S						Weight: 114 lb	FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S	P No.2 or 2x4 SPF No.2 P No.2 or 2x4 SPF No.2				BRACING- TOP CHOF BOT CHOF	D D	Structu Rigid c	ral wood eiling dire	sheathing dir	rectly applied or 6-0-0 or 10-0-0 or 10-0-0 or bracing.	oc purlins.

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 OTHERS
 2x4 SP No.3

 WEDGE
 V

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 2=-162(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 22, 23, 17, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 2, 18, 19, 20, 22, 23, 17, 16, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=140(LC 11) Max Uplift 2=-102(LC 12), 6=-1

Max Uplift 2=-102(LC 12), 6=-102(LC 13) Max Grav 2=753(LC 1), 6=752(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-980/197, 3-4=-740/155, 4-5=-740/155, 5-6=-980/197
- BOT CHORD 2-8=-130/809, 6-8=-88/793

WEBS 4-8=-40/474, 5-8=-280/166, 3-8=-280/165

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.







					17-0-0 17-0-0						
Plate Offsets (X,Y)	[2:0-1-9,0-4-2], [2:0-0-0,0-1	-1], [7:0-2-0),Edge], [12:0-	0-0,0-1-1], [12:0-1-9,0-4-2]						
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	-0.00	13	n/r	120	MT20	197/144
CDL 10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	-0.00	13	n/r	90		
CLL 0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	12	n/a	n/a		
CDL 10.0	Code IRC2015/TPI2	2014	Matrix	-S						Weight: 97 lb	FT = 20%
UMBER-					BRACING-					1	
OP CHORD 2x4 SI	P No.2 or 2x4 SPF No.2				TOP CHOP	D	Structu	ral wood	sheathing di	rectly applied or 6-0-0	oc purlins.

BOT CHORD

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD OTHERS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 17-0-0.

(lb) - Max Horz 2=-140(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 19, 20, 22, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 2, 18, 17, 19, 20, 22, 16, 15, 14, 12

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





10-8-8

13-4-0

6-8-0

2-7-8

	2-7-8	4-0-8		4-0-8		2-7-8	
Plate Offsets (X,Y)	[1:0-8-0,0-1-6], [5:0-8-0,0-1-6], [6:0-4-8,	0-1-8], [8:0-4-8,0-1-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.25 BC 0.80 WB 0.51 Matrix-MS	DEFL. in Vert(LL) -0.06 Vert(CT) -0.12 Horz(CT) 0.03	(loc) l/defl 7-8 >999 7-8 >999 5 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 248 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP	No.2 No.2 No.3		BRACING- TOP CHORD BOT CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling dire	sheathing directly (6-0-0 max.): 2-4. ctly applied or 10	/ applied or 6-0-0 o)-0-0 oc bracing.	c purlins, except
REACTIONS. (size Max He Max U Max G	e) 1=0-3-8, 5=0-3-8 brz 1=-39(LC 10) blift 1=-642(LC 12), 5=-664(LC 13) rav 1=4868(LC 1), 5=5074(LC 1)						
FORCES. (lb) - Max. TOP CHORD 1-2=- BOT CHORD 1-8=- WEBS 2-8=-	Comp./Max. Ten All forces 250 (lb) or 7783/1094, 2-3=-9755/1407, 3-4=-9755 925/6682, 7-8=-905/6475, 6-7=-871/637 240/2316, 2-7=-506/3549, 4-7=-522/366	less except when shown. /1407, 4-5=-7666/1079 /1, 5-6=-889/6571 61, 4-6=-230/2233					
NOTES- 1) 3-ply truss to be con Top chords connected Bottom chords connect Bottom chords conne Webs connected as rows staggered at 0- 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; V MWFRS (envelope) DOL=1.60 plate grip 5) Provide adequate dr. 6) This truss has been will fit between the b 8) Two H2.5A Simpson connection is for upli 9) Graphical purtin repr 10) Hanger(s) or other 2-7-8, 78 lb down and and 168 lb up at 10- 114 lb up at 10-11- device(s) is the res; Continued on page 2 LOAD CASE(S) Stant	nected together with 10d (0.120"x3") na ed as follows: 2x6 - 2 rows staggered at acted as follows: 2x6 - 3 rows staggered at follows: 2x4 - 2 rows staggered at 0-4-0 5-0 oc, member 7-4 2x4 - 1 row at 0-9-0 red equally applied to all plies, except if a been provided to distribute only loads loads have been considered for this de ult=130mph (3-second gust) Vasd=103i gable end zone and C-C Exterior(2) zor DOL=1.60 ainage to prevent water ponding. designed for a 10.0 psf bottom chord live totom chord and any other members. Strong-Tie connectors recommended t ft only and does not depict the size or tt connection device(s) shall be provided sind for D-8-8 on top chord, and 1261 lb down and up at 2-11-4, 17 lb down at 4-9-8, 1260 11-4, 17 lb down at 8-6-8, 1260 lb dow 4, and 1266 lb down and 158 lb up at 4 ponsibility of others.	ils as follows: 0-4-0 oc. d at 0-4-0 oc. oc, Except member 7-2 2 0 oc. i noted as front (F) or bacl noted as (F) or (B), unless sign. mph; TCDL=6.0psf; BCDL he;C-C for members and f e load nonconcurrent with the bottom chord in all are o connect truss to bearing ces. he orientation of the purlin sufficient to support conce 7 lb up at 6-9-8, and 78 lb nd 163 lb up at 0-11-4, 18 0 lb down and 164 lb up at n and 164 lb up at 8-11-4 12-11-4 on bottom chord.	2x4 - 1 row at 0-9-0 oc, m k (B) face in the LOAD C s otherwise indicated. L=6.0psf; h=30ft; Cat. II; F forces & MWFRS for read n any other live loads. eas where a rectangle 3-6 g walls due to UPLIFT at along the top and/or bott entrated load(s) 107 lb do b down and 67 lb up at 8 8 lb down at 2-7-8, 17 lb at 4-11-4, 17 lb down at 4, 35 lb down at 10-6-8, a The design/selection of	ASE(S) section. F Exp B; Enclosed; tions shown; Lurr tions shown; Lurr ti(s) 1 and 5. This tom chord. wn and 168 lb up 6-8, and 107 lb d down at 2-9-8, 12 6-9-8, 1260 lb do and 1260 lb down such connection	2 Ply to hber de at own 260 wn and	SE 044	AROLINA SIGNAL 925 VEER.HAININ SEVILINI NU 22,2020
WARNING - Verify Design valid for use of a truss system. Befor building design. Brac is always required for fabrication, storage, of Safety Information	design parameters and READ NOTES ON THIS AN only with MITek® connectors. This design is based e use, the building designer must verify the applica- ing indicated is to prevent buckling of individual tri stability and to prevent collapse with possible per lelivery, rection and bracing of trusses and truss a available from Truss Plate Institute, 2670 Crain His	ID INCLUDED MITEK REFERENC only upon parameters shown, ar ability of design parameters and p uss web and/or chord members c sonal injury and property damage systems, see ANSUTPI ghway, Suite 203 Waldorf, MD 20	CE PAGE MII-7473 rev. 5/19/202 nd is for an individual building or properly incorporate this design only. Additional temporary and J e. For general guidance regard I Quality Criteria, DSB-89 and 0601	to BEFORE USE. component, not into the overall commanent bracing ing the I BCSI Building Com	ponent	818 Soundside Edenton, NC 2	ERING BY ENCLO A MiTek Attiliate Road 7932

Job	Truss	Truss Type	Qty	Ply	Clinton
					142125692
2000570-2000570A	DH	Hip Girder	1	2	
				5	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	.330 s May	6 2020 MiTek Industries, Inc. Wed Jul 22 10:14:04 2020 Page 2

ID:3mk_e2i7TNdXP4sqpvd8YMyvb5y-?To0S5m9DZTIxkfNcZEgtc3o5BJ9iLMEVnQ0ySyvYB1

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-4=-60, 4-5=-60, 9-12=-20

Concentrated Loads (lb)

Vert: 8=-4(B) 7=-1265(F=-1260, B=-5) 2=-5(B) 3=-2(B) 4=-5(B) 6=-1260(F) 11=-1261(F) 15=-2(B) 16=-2(B) 17=-1265(F=-1260, B=-5) 18=-1265(F=-1260, B=-5) 19=-5(B) 20=-1260(F) 21=-9(B) 22=-1266(F)





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TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-7-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=81(LC 12)

Max Uplift 3=-41(LC 12), 2=-29(LC 12) Max Grav 3=66(LC 19), 2=192(LC 1), 4=46(LC 3)

 $\frac{1}{100} \frac{1}{100} \frac{1}$

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This connection is for uplift only and does not consider lateral forces.







		3-7-1									
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.20 BC 0.11 WB 0.00 Matrix-MP	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 4-7 >999 240 Vert(CT) -0.01 4-7 >999 180 Horz(CT) 0.00 3 n/a n/a	PLATES GRIP MT20 197/144 Weight: 14 lb FT = 20%							
LUMBER-			BRACING-								

LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-9, 4=Mechanical

Max Horz 2=80(LC 12)

Max Uplift 3=-45(LC 12), 2=-60(LC 8)

Max Grav 3=83(LC 1), 2=268(LC 1), 4=62(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 3) will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MILER KEERENCE PAGE MIL-7475 fev. or 19/2/2/0/ DEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-7-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.





















REACTIONS. (size) 1=7-8-2, 3=7-8-2, 4=7-8-2

Max Horz 1=-48(LC 8)

Max Uplift 1=-31(LC 12), 3=-38(LC 13)

Max Grav 1=138(LC 1), 3=138(LC 1), 4=256(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







	0-0 ₁ 7	2-10-8			4-11-0	1	6-11-8				9-10-0	
	0-0-7	2-10-1			2-0-8	I	2-0-8		1		2-10-8	I
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	x-S						Weight: 33 lb	FT = 20%
LUMBER-	RD 2x4 SP No	b. 3				BRACING- TOP CHOR	D	Structu	ral wood	sheathing di	rectly applied or 6-0-0	oc purlins, except

BOT CHORD

2-0-0 oc purlins (6-0-0 max.): 2-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

WEBS 2x4 SP No.3

REACTIONS. All bearings 9-9-2.

Max Horz 1=32(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6, 7

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 8, 6, 7

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

7) n/a

8) n/a

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







	0-0 <u>-</u> 7 0-0-7	2-11-0 2-10-9					5-10-0 2-11-0			
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2 Plate Grip DOL Lumber DOL	-0-0 CSI. 1.15 TC 1.15 BC	0.17	DEFL. Vert(LL) Vert(CT)	in n/a n/a	(loc) - -	l/defl n/a n/a	L/d 999 999	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code IRC2015/TPI20	YES WB 114 Matr	0.02 ix-P	Horz(CT)	0.00	3	n/a	n/a	Weight: 18 lb	FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD

2x4 SP No.3 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3

REACTIONS. (size) 1=5-9-2, 3=5-9-2, 4=5-9-2

Max Horz 1=-34(LC 10)

Max Uplift 1=-22(LC 12), 3=-27(LC 13)

Max Grav 1=99(LC 1), 3=99(LC 1), 4=183(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

CONTRACTION OF THE STATE WILLING THE EAL 44925 mm July 22,2020

Structural wood sheathing directly applied or 5-10-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



