



: 2:_residential eng\2020 Structural Projects\2001-010199 - Whittenton Builders - Turlington residence\2001-010199.DMG Saved by: Johnny Last Plot Date:6/26/2020



PLANS DESIGNED TO THE NC STATE CODE, 2018 EDITION





FIRST FLOOR PLAN

1/4" = 1'-0" CLG. HGT. = 10'-0" (U.N.O.)

*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. *Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.			
TYNDALL ENGINEERING & DESIGN, P.A.	+ 419 773-1200 = # 919 773-9466 250 Shipwesh Drive = Gerner = North Caroline = 27329 www.tyndellengineering.com		
Client: WHITTENTON BUILDERS	Plan: TURLINGTON RESIDENCE		
FIRST	FIRST FLOOR PLAN		
Project #: 2001-01 Date: 6/8/20 Drawn/Design IJE DWG. Checke PAT Scale: SEE PL No. Date: 1 2 3 4	$\begin{tabular}{ c c c c c } \hline \hline Project #: \\ \hline 2001-010199 \\ \hline \hline Date: \\ \hline 6/8/20 \\ \hline Drawn/Design By: \\ \hline IJE \\ \hline DWG. Checked By: \\ PAT \\ \hline Scale: \\ SEE PLAN \\ \hline \hline SEE PLAN \\ \hline \hline SEE PLAN \\ \hline \hline \hline $ $ $ $ $ $ $ $ $ $ $ $ $ $ $ $$		
3 3 of 11			

HEATED SF First Floor 2856 TOTAL HEATED 2856 UNHEATED SF Front Porch Screened Porch Side Porch 467 527 TOTAL UNHEATED 1058

64

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TYNDALL ENGINEERING & DESIGN, P A 1919 775-1200 - F 919 775-1200 - F 919 775-946 280 Shipwash Drive - Garner - North Caroline - 2752
Client: WHITTENTON BUILDERS Plan: TURLINGTON RESIDENCE
SECOND FLOOR PLAN
Project #: 2001-010199 Date: 6/8/20 Drawn/Design By: IJE DWG. Checked By: PAT Scale: SEE PLAN REVISIONS 1
4

FOUNDATION PLAN

1/4" = 1'-0"

*ALL LUMBER TO BE #2 SYP, UNO

- STRUCTURAL NOTES: 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST
- REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- 2) IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- 3) ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (I.E. ILEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) 4) ALL LOAD BEARING EXTERIOR WINDOW HEADERS WITH MAXIMUM SPAN OF 5'-6" SHOULD BE A (2) $2\times10 \text{ w/}(1)$ 2x4 KING STUD AND (1) 2x4 JACK STUD NAILED TOGETHER w/ (2) 10d @ 8" O.C. PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6", OTHERWISE REFER TO TABLE R502.5(1).
- 5) ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLE R502.5(1) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND
- EXTERIOR LOAD CONDITIONS (UNO) 6) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR
- CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT. 7) ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO) 8) ALL EXTERIOR LUMBER TO BE #2 SYP PT
- 9) ALL CONCRETE, fc = 3000 PSI MIN.
- 10) PRESUMPTIVE BEARING CAPACITY = 2000 PSF 11) 1/2"¢ ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPÁCED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION
- AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.) 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF
- THE 2018 IRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE
- CONTINUOUSLY ANCHORED TO THE FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF
- 120 MPH OR LESS. 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS
- $\langle 1 \rangle$ REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC. 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR
- $\langle 2 \rangle$ 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C.

WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)

 $\langle 3 \rangle$ 3/8" wood structural panel (wsp) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS

AT INTERMEDIATE SUPPORTS

- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS
- PRESCRIBED IN SECTION R602.10.3 (UNO) 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS
- (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A
- MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.
- 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN 67% OF WALL HEIGHT – 30" ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT. – 48" FOR OPENINGS GREATER THAN 85% OF WALL HEIGHT
- $\langle 4 \rangle$ sheath interior & exterior
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW. $\langle 5 \rangle$ MINIMUM 800# HOLD-DOWN DEVICE

BWL 2

DESIGN LOADS

_						
		LIVE LOAD DEAD LOAD		DEFLE	CTION	
		()	()	LL	TL	
	FLOOR (primary)	40	10	L/360	L/240	
	FLOOR (secondary)	40	10	L/360	L/240	
	ATTIC (w/ storage)	20	10	L/240	L/180	
	ATTIC (no access)	10	5	L/240	L/180	
	EXTERNAL BALCONY	40	10	L/360	L/240	
	ROOF	20	10	L/240	L/180	
	ROOF TRUSS	20	20	L/240	L/180	
	WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
	SEISMIC	BASED ON SEISMIC ZONES A, B & C				

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- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
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 5 MINIMUM 800# HOLD-DOWN DEVICE

2:13 PM

*ALL LUMBER TO BE #2 SYP, UNO ALL WALLS TO BE 4" THICK

*Engineers seal does not means, methods, techn procedures or safety p *Any deviations or discr to be brought to the im Tyndall Engineering & do so will void Tyndall P.A. liability. *Please review these doo Tyndall Engineering & interpret that all discrete recommendations, etc. presented in these deemed acceptable one set of the set of the set of the set of the deemed acceptable one set of the set of the set of the set of the deemed acceptable one set of the set of the set of the set of the deemed acceptable one set of the set of the set of the set of the deemed acceptable one set of the set of the set of the set of the set of the deemed acceptable one set of the set of the set of the set of the	*Engineers seal does not include construction means, methods, techniques, sequences, procedures or safety precaution. *Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability. *Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.		
TYNDALL ENGINEERING & DESIGN, P.A.	+ 416 778-1300 = F 919 778-1308 280 Shipwash Drive = Gerner = Nerth Carolina = 27839 www.tyndellengineering.com		
Glient: WHITTENTON BUILDERS	Plan: TURLINGTON RESIDENCE		
2ND FLOOR HEADER	2ND FLOOR HEADER 2ND FLR. CLG. FRAMING		
$ \begin{array}{c c} \hline \hline 1 \\ \hline 2001-010 \\ \hline \hline 2001-010 \\ \hline \hline 0 \\ \hline 0 $	$ \begin{array}{r} \underline{rioject \#} \\ 2001-010199 \\ \hline \underline{Date:} \\ 6/8/20 \\ \hline \underline{Drawn/Design By:} \\ IJE \\ \hline \underline{DWG. Checked By:} \\ PAT \\ \hline \underline{Scale:} \\ SEE PLAN \\ \hline \hline \underline{SEE PLAN} \\ \hline \hline \underline{REVISIONS} \\ \hline \underline{No. Date: Remarks} \\ \hline \underline{1} \\ \hline \underline{2} \\ \hline \underline{3} \\ \hline \underline{4} \\ \hline \end{array} $		
S	S 3 7 of 11		

*ALL LUMBER TO BE #2 SYP, UNO BUILDER MAY USE ROOF TRUSSES. TRUSS DESIGN, LAYOUT, AND ENGINEERING TO BE PROVIDED BY TRUSS MANUFACTURER

	*Engineers seal does no means, methods, techn procedures or safety p *Any deviations or disc to be brought to the in Tyndall Engineering & do so will void Tyndall P.A. liability. *Please review these do Tyndall Engineering & interpret that all dime recommendations, etc. presented in these deemed acceptable on the safety of the safety of the generation of the safety of the safety of the safety of the safety of the safety of the safety of the safet	t include construction iques, sequences, recaution. repancies on plans are immediate attention of a Design, P.A. Failure to Engineering & Design, cuments carefully. a Design, P.A. will nsions, documents were are construction begins.	
	TYNDALL ENGINEERING & DESIGN, P.A.	+ 919 773-1300 = 1: 919 773-1300 = 1: 919 773-9488 250 Shipwash Drive = Clarnar = Nach Genelina = 27339 www.syndallenginaering.com	
	Client: WHITTENTON BUILDERS	Plan: TURLINGTON RESIDENCE	
	ROOF PLAN		
	Project #: 2001-010199 Date: 6/8/20 Drawn/Design By: IJE DWG. Checked By: PAT Scale: SEE PLAN No. Date: REVISIONS A A J A J Sheet Number SG4		

2 X 8 16" (

12:12

- \	DE", IN ADDITION	TO ALL LOCAL CODES AND F	REGULATIONS.					
2) DES	SIGN LOADS:		LIVE LOAD	DEAD LOAD	DEF	LECTION		ALT CANT
		ALL FLOORS	40	10	LL L/360	TL L/240		CJ CMU COL
		ATTIC (w/ walk up stairs) ATTIC (pull down access)	30 20	10 10	L/360 L/240	L/240 L/180		CONC
		EXTERNAL BALCONY	10 40 20	5 10 10	L/240 L/360	L/180 L/240	=	DBL DIA
	ŀ	ROOF TRUSS	20	20	L/240	L/180		DJ DR FA
		SEISMIC	87	SEISMIC ZONE	ES A, B & C	(E B)	_	EE FJ
Z) MINI			2000 BSE					FND FTG GALV
4) CON	NCRETE SHALL H	AVE A MINIMUM 28 DAY COM	PRESSIVE STRENG	TH OF 3000 PSI	AND A MAX	IMUM SLUMP	OF FIVE INCHES	HORIZ HT
UNL	LESS NOTED OTH	IERWISE. (U.N.O.)		S TO BE LESS -	THAN 4'_0"		C SUFFICIENT WALL	MANU
BRA THI	ACING. REFER TO CKNESS, SOIL T	PONDALANCED FILL AGAINST D SECTION R404 OF 2018 NC (PE, AND UNBALANCED BACKF	BUILDING CODE FO	DR BACKFILL LIM	ITATIONS BA	SED ON WALL	HEIGHT, WALL	
5) ALL ALL ALL ALL ALL	FRAMING LUMB FRAMING LUMB LVL LUMBER TO LSL LUMBER TO PSL LUMBER T	ER SHALL BE SYP #2 (Fb = ER EXPOSED TO THE ELEMENT O BE 1.75" WIDE NOMINAL EAC O BE 3.5" WIDE NOMINAL EAC O BE 3.5" WIDE NOMINAL FAC	800 PSI, BASED C S SHALL BE TREA CH SINGLE MEMBEI H SINGLE MEMBER H SINGLE MEMBER	ON 2x10) UNO. ATED MATERIAL. R AND Fb = 26 AND Fb = 232 AND Fb = 240	00 PSI, E = 25 PSI, E = 20 PSI F =	1.9M PSI (U.N 1.6M PSI (U.N 1.8M PSI (U.N	N.O.) .O.)	1) MAXI
7) ALL	LOAD BEARING	EXTERIOR HEADERS SHALL BE	E AT (2) 2x10. (U	I.N.O.) REFER TO) TABLE R60	2.7(1) & (2)	FOR JACK STUD	
B) ALL	STRUCTURAL S	TEEL W-SHAPES (I-BEAMS) S	HALL BE ASTM AS	992 GRADE 50.	- UNLLOO OF	LUNIOALLI N		
ALL ALL	SIEEL ANGLES, STEEL PIPE SH	ALL BE ASTM A53 GRADE B.	SHALL BE ASTM A	JD.				* THIS
9) STE PRO	EL BEAMS SHAL	L BE SUPPORTED AT EACH EI RING FROM BEAM SUPPORT T	ND WITH A MINIMU O FOUNDATION. BI	M BEARING LEN EAMS SHALL BE	GTH OF 3-1, ATTACHED	2" AND FULL	FLANGE WIDTH. PORT WITH TWO (2)	1113
LAG SOL	3 SCREWS (1/2" LE PLATES, AND	x 4" LONG). LATERAL SUPP THE SOLE PLATES ARE NAILE	URT IS CONSIDERE D OR BOLTED TO	D ADEQUATE PR THE BEAM FLAN	KOVIDED THE NGES © 48"	JOISTS ARE ⁻ O.C.	IOE NAILED TO THE	** FRON *** DECK
0) PRO THE	OVIDE ANCHOR E E END OF EACH	OLT PLACEMENT PER SECTION PLATE SECTION. ANCHOR BOL	I 403.1.6: 1/2"∅ / TS SHALL BE SPA	ANCHOR BOLTS	SPACED AT (0.C. FOR BAS	6'—0" O.C. AN SEMENTS. ANC	D PLACED 12" FROM HOR BOLT SHALL	2) DECK
EXT THE	TEND 7" INTO CO ERE SHALL BE A	NCRETE OR MASONRY. THE B MINIMUM TWO ANCHOR BOLTS	OLTS SHALL BE L S PER PLATE SEC	OCATED IN THE TION.	MIDDLE THIR	O OF THE WID	TH OF THE PLATE.	A. THE
1) FOL	JNDATION DRAIN	AGE-DAMP PROOFING OR WAT	ERPROOFING PER	SECTION 405 AN	ND 406 OF N	IC BUILDING C	ODE.	
2) WAI WAI	LL AND ROOF CL	ADDING VALUES: ALL BE DESIGNED FOR 28.0 P	OUNDS PER SQUA	RE FOOT (LBS/	SQFT) OR GR	EATER POSITI	VE AND NEGATIVE PRESSURE.	B. 4 x
ROC 39. 36.	OF VALUES BOTH 0 LBS/SQFT FOF 0 LBS/SQFT FOF	R ROOF PITCHES 0/12 TO 1.5, R ROOF PITCHES 0/12 TO 1.5, R ROOF PITCHES 1.5/12 TO 6	/12 /12 /12	v3;				
18.(**M	0 LBS/SQFT FOR IEAN ROOF HEIGI	R ROOF PITCHES 6/12 TO 12/ HT 30'-0" OR LESS	/12					C. FOR
3) FOF	R ROOF SLOPES	FROM 2/12 THROUGH 4/12, E	BUILDER TO INSTA	LL 2 LAYERS OF	F 15# FELT P	APER.		
4) REF	FER TO SECTION	R602.3 FOR FRAMING OF ALL	WALLS OVER 10'	-0" IN HEIGHT.				
5) PR(6) LIPI	UVIDE CONTINUO	US SHEATHING PER SECTION 6	ONTINUOUSI Y AND	2018 NCRC.	FOUNDATION			
7) REF	FER TO TABLE N	1102.1 FOR PRESCRIPTIVE BUI	LDING ENVELOPE 1	THERMAL COMPO	NENT CRITER	IA.		
8) PSL	_ COLUMNS DESI	GNED WITH MAXIMUM HEIGHT	OF 9'-0" (U.N.O.)					D. 2 x
9) PR(OVIDE A MINIMUN	OF 500# UPLIFT & LATERAL	CONNECTION AT	TOP AND BOTTO	OM OF PORCH	I COLUMNS. (I	J.N.O.)	
נט) MA: 1) יידו	AIMUM MASONRY	TORS RESPONSIBILITY TO VER	ELU FOUR TIMES	NS AND SOLLARE	FOOTAGE P	NSIUN.	TRUCTION.	E. FOR
TYN	NDALL ENGINEERI	NG & DESIGN, PA IS NOT RES	SPONSIBLE FOR DI	MENSION OR SQL	UARE FOOTA	GE ERRORS ON	ICE CONSTRUCTION BEGINS.	
IMATE	FENESTRATION	GLAZED SKYLIGHT ^b FENESTRATION CEI	WOOD	MASS ALL WALL	FLOOR	BASEMENT ^{c,⊙} WALL	SLAB ^d CRAWL SPACE ^c R–VALUE WALL	
ONES 3	U-FACTOR ^{D,J}	J−FACTOR SHGC ^{b,k} R−\ 0.55 0.30 <u>38</u>	ALUE R-VALUE or 30 15 or	$E = \frac{R - VALUE}{5/13 \text{ or}}$	R-VALUE	R-VALUE	AND DEPTH R-VALUE 0 5/13	ST
4	0.35	0.55 0.30 2	ont 13 + <u>2.5</u> or 30 15 or	<u>5/10 cont</u> _h <u>5/13 or</u>	10	10 /15	10 10/15	
5	0.35	0.55 ND 38	ont ¹ 13 + <u>2.5</u> or <u>30</u> 19 [°] , or 13 -	$\frac{5}{10 \text{ cont}} = \frac{5/10 \text{ cont}}{13/17 \text{ or}}$	<u> </u>	10/15		
			ont or 15 +	<u>3 13/12.5 co</u>	nt ⁵⁰	<u>10/10</u>		
NO SCALE		JES ARE MINIMUMS. U-FACTORS AND SHGC E INSULATION, THE INSTALLED R-VALUE OF	ARE MAXIMUMS. WHEN INSU THE INSULATION SHALL NO	JLATION IS INSTALLED IN T BE LESS THAN THE R	A CAVITY WHICH	IS LESS THAN THE L IN THE TABLE.	ABEL OR DESIGN THICKNESS	2 × 6
	b. THE FE (SHGC	NESTRATION U-FACTOR COLUMN EXCLUDED :) COLUMN APPLIES TO ALL GLAZED FENESTR	SKYLIGHTS. THE SOLAR HEA ATION.	AT GAIN COEFFICIENT	DMF			
	d. <u>FOR M</u>	-15 CAVITY INSULATION AT THE INTERIOR OF NOULTHIC SLABS, INSULATION SHALL BE APP FOOTING OR A MAXIMIM OF 24" RELOW OP	THE BASEMENT WALL OR LIED FROM THE INSPECTION	CRAWL SPACE WALL. N GAP DOWNWARD TO THE	HE BOTTOM ISULATION			
	e. <u>DELETE</u>	EXTEND TO THE BOTTOM OF THE FOUNDATIO TO THE REQUIRED SLAB EDGE R-VALUES FO D	N WALL OR 24", WHICHEVE OR HEATED SLABS.	<u>R IS LESS.</u> R–5 SHALL	BE			
	f. BASEME g. OR INS	NT WALL INSULATION IS NOT REQUIRED IN W ULATION SUFFICIENT TO FILL THE FRAMING C	ARM-HUMID LOCATIONS AS AVITY. R-19 MINIMUM.	DEFINED BY FIGURE N1	1101.7 AND TABLE	<u>1101.7</u> .		
	h. THE FI SHEAT INSUL	NATURE IS CAVITY INSULATION, THE SECO THING, "15+3" MEANS R-15 CAVITY INSULATI ATING SHEATHING IS NOT REQUIRED WHERE T	NU VALUE IS CONTINUOUS ON. PLUS R-3 INSULATED THE STRUCTURAL SHEATHIN	SHEATHING. IF STRUCTU	MEANS R-13 CAV JRAL SHEATHING CO RAL SHEATHING CO	DVERS 25% OR LESS	OF THE EXTERIOR.	
	<u>OF TH</u> INSUL i. FOR MA	E CATERIUR, SHALL BE SUPPLEMENTED WITH ATTON PLUS R-2.5 SHEATHING. SS WALLS, THE SECOND R-VALUE APPLIES T	WHEN MORE THAN HALF TH	HE INSULATION IS ON TH	2.0 MEANS R-13	WALL.		
	j. IN ADDI PERMIT K. IN ADD	TION TO THE EXEMPTION IN SECTION N1102.3 TED TO BE SUBSTITUTED FOR MINIMUM CODE TION TO THE EXEMPTION IN SECTION N1102.	COMPLIANT FENESTRATION 3.3. A MAXIMUM OF TWO G	PRODUCT ASSEMBLIES	WITHOUT ASSEMBLIE	<u>s having a U-Fact</u> S having a shgc n	ON ING GREATER THAN 0.55 SHALL BE	
	<u>I.</u> <u>R–30 S</u> <u>AT THE</u> <u>OF THE</u>	IED TO BE SUBSTITUTED FOR MINIMOM CODE HALL BE DEEMED TO SATISFY THE CEILING II EAVES. OTHERWISE R-38 INSULATION IS RE ATTIC ROOF DECK.	COMPLIANT FENESTRATION NSULATION REQUIREMENT W QUIRED WHERE ADEQUATE	VHEREVER THE FULL HEIG CLEARANCE EXISTS OR	WITHOUT PENALTY. GHT OF UNCOMPRE INSULATION MUST E	- <u>SSED R-30 INSULATI</u> EXTEND TO EITHER TI	ON EXTENDS OVER THE WALL TOP PLATE HE INSULATION BAFFLE OR WITHIN 1 INCH	NO S
	<u>m.</u> <u>TABLE v</u> n. <u>R – 19 F</u> <u>AND IN</u> :	VALUE REQUIRED EXCEPT FOR ROOF EDGE WH IBERGLASS BATTS COMPRESSED AND INSTALL STALLED IN A 2X4 WALL IS NOT DEEMED TO IT WALL WEETING THE ANY	HERE THE SPACE IS LIMITED ED IN A NOMINAL 2 6 1 COMPLY.	D BY THE PITCH OF THE FRAMING CAVITY IS DEEM	E ROOF; THERE THE MED TO COMPLY. F	EINSULATION MUST F	TILL THE SPACE UP TO THE AIR BAFFLE. TED R-19 OR HIGHER COMPRESSED	
	O. BASEMEN	WALL MEETING THE MINIMUM MASS WALL	SI LOIFIC MEAT CONTENT RI	LOUINEMENT MAY USE T	TIL MIASS WALL R-	VALUE AS (HE MININ		
	Q. FT. OF CRAWL	. SPACE / 150 = 17.83 SQ.	FT. OF REQ'D VE	NTILATION WITHO	OUT CROSS V	ENTILATION		
2675 SG	αι Γι ΟΓ VEINIIL	-OR-	$L_{\rm IV} = 40$	LINIS NEW DI				
2675 SG 7.83 SG				NTILATION WITH	CROSS VENT	LATION		٨
2675 SC 7.83 SC 2675 SC .78 SO	Q. FT. OF CRAWL	SPACE / 1500 = 1.78 SQ.	FT. OF REQ'D VEN					
2675 SG 7.83 SG 2675 SG .78 SQ. VENT I	2. FT. OF CRAWL FT. OF VENTILA	SPACE / 1500 = 1.78 SQ. ATION REQ'D / 0.45 SQ.FT. PE ROM THOSE SHOWN ON PLAN, HOWEVER VEN ON AT ALL POINTS AND TO PREVENT DEAD	FT. OF REQ'D VER R VENT = 4 VEI TS SHALL BE PLACED TO AIR POCKETS					· · · ·
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2675 SC 17.83 SC 2675 SC 1.78 SQ. VENT I PROVID THE TO GROUN OF TH ORE F RAINW, WALLS WHEN EXTERI	2. FT. OF CRAWL . FT. OF VENTILA LOCATIONS MAY VARY F DE ADEQUATE VENTILATION OTAL AREA OF VENTILATION OTAL AREA OF VENTILATION OUNDATION VENT SHALL STATER ENTRY WHEN THE MAY BE CONSTRUCTED THE BOTTOM OF THE FC IOR GRADE.	SPACE / 1500 = 1.78 SQ. ATION REQ'D / 0.45 SQ.FT. PE ROM THOSE SHOWN ON PLAN, HOWEVER VEN DON AT ALL POINTS AND TO PREVENT DEAD / ION OPENINGS MAY BE REDUCED TO 1/1500 QUIRED OPENINGS ARE PLACED SO AS TO PI ISTALLATION OF OPERABLE LOUVERS SHALL I BE WITHIN 3 FEET OF EACH CORNER OF TH CRAWL SPACE IS BUILT ON A SLOPED SITE, WITHOUT WALL VENT OPENINGS. VENT DAMS JUNDATION VENT OPENING IS LESS THAN 4 I	FT. OF REQ'D VER TR VENT = 4 VER TS SHALL BE PLACED TO ANR POCKETS. OF THE CRAWL SPACE ROVIDE CROSS VENTILATION NOT BE PROHIBITED. E BUILDING. TO PREVENT THE UPHILL FOUNDATION SHALL BE PROVIDED NCHES ABOVE THE FINISHE	D			FLOOR JOISTS (PER PLAN)	
2675 SC 17.83 SC 2675 SC 1.78 SQ. VENT I PROVIE THE TT GROUN OF TH ONE F RAINW. WALLS WHEN EXTERI	Q. FT. OF CRAWL FT. OF VENTILA LOCATIONS MAY VARY F DE ADEQUATE VENTILATION OTAL AREA OF VENTILATION DAREA WHERE THE RE IE CRAWL SPACE. THE IN OUNDATION VENT SHALL ATER ENTRY WHEN THE CONSTRUCTED THE BOTTOM OF THE FO IOR GRADE.	SPACE / 1500 = 1.78 SQ. ATION REQ'D / 0.45 SQ.FT. PE ROM THOSE SHOWN ON PLAN, HOWEVER VEN DN AT ALL POINTS AND TO PREVENT DEAD / TON OPENINGS MAY BE REDUCED TO 1/1500 QUIRED OPENINGS ARE PLACED SO AS TO PH ISTALLATION OF OPERABLE LOUVERS SHALL I BE WITHIN 3 FEET OF EACH CORNER OF TH CRAWL SPACE IS BUILT ON A SLOPED SITE, WITHOUT WALL VENT OPENING IS LESS THAN 4 I REQUIRE FULL COVERAGE GROUND VAPOR R CRAWI SPACE VE	FT. OF REQ'D VER TR VENT = 4 VER TS SHALL BE PLACED TO AIR POCKETS. OF THE CRAWL SPACE ROWIDE CROSS VENTILATION OF BE PROHIBITED. E BUILDING. TO PREVENT THE UPHILL FOUNDATION SHALL BE PROVIDED NCHES ABOVE THE FINISHE ETARDERS.				FLOOR JOISTS (PER PLAN)	
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GHT OF DECK SUPPORT POSTS AS FOLLOWS:

POST SIZE	MAX. POST HEIGHT**
4 × 4	8'-0"
6 × 6	20'-0"
***	OVER 20'-0"

BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET AY BE LOCATED AT DIFFERENT LEVELS. FOOTING TO BOTTOM OF GIRDER

POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT. BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF

DOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4) LATERAL BRACING IS NOT REQUIRED.

(NEE BRACES MAY BE PROVIDED ON EACH COLUMN IN ECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST INT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED POST AND GIRDER WITH ONE 5/8" HOT DIPPED GALVANIZED EACH END OF THE BRACE.

IDING DECKS WITHOUT KNEE BRACES OR DIAGONAL , LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE ACCORDANCE WITH THE FOLLOWING:

ZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
	48 SQ. FT.	4'-0"	2'-6"	1'-0"
	120 SQ. FT.	6'-0"	3'-6"	1'-8"

IAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO ENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8" # HOT GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER. NT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.

PANEL CONNECTIONS				
REQUIRED (CONNECTION			
@ PANEL EDGES	@ INTERMEDIATE SUPPORTS			
6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12"O.C.			
5d COOLER NAIL** @ 7" O.C.	5d COOLER NAIL** @ 7" O.C.			
6d COMMON NAILS @ 6" 0.C.	6d COMMON NAILS @ 12" O.C.			

