

Trenco 818 Soundside Rd Edenton, NC 27932

# Re: 2000411-2000411A JOHNSON RESIDENCE

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I41573855 thru I41573913

My license renewal date for the state of North Carolina is December 31, 2020.

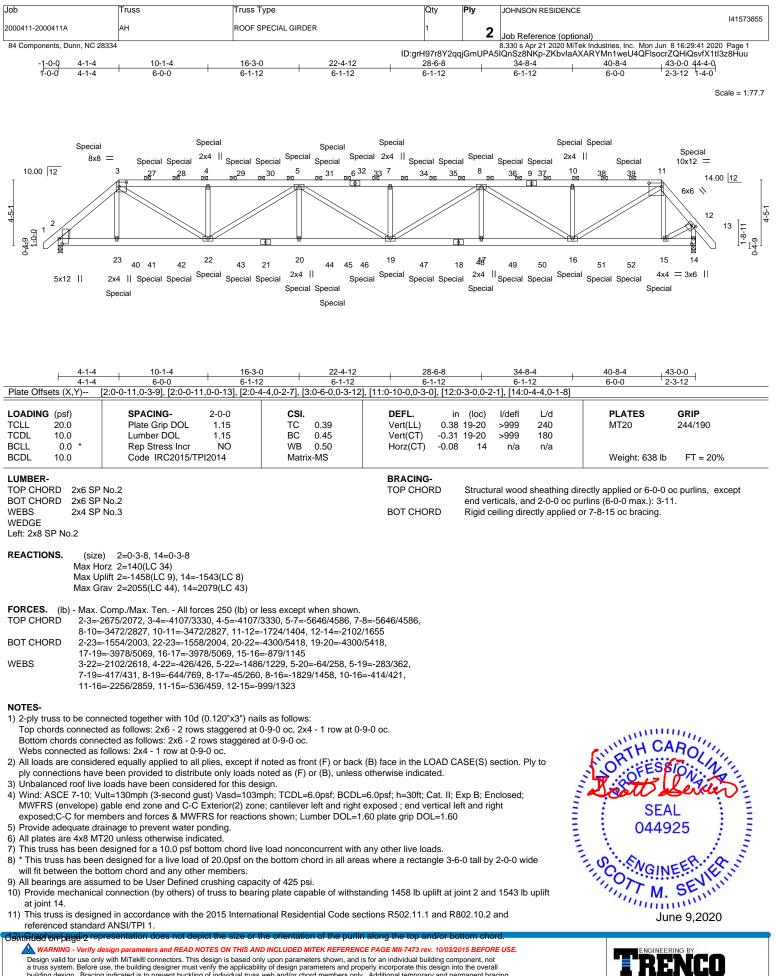
North Carolina COA: C-0844



June 9,2020

Sevier, Scott

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



balling read to day only main table contractions. This designer must end with your parameters and property incorporate this design into the overall building designer must weith the applicability design parameters and property incorporate this design into the overall building designer must be and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE
					141573855
2000411-2000411A	AH	ROOF SPECIAL GIRDER	1	2	Inh Deference (antional)
				-	Job Reference (optional)
04 Componente Dunn NC 202	24				9 220 a Apr 21 2020 MiTak Industrias, Inc. Man Jun. 9 16:20:41 2020, Dags 2

ID:grH97r8Y2qqjGmUPA5IQnSz8NKp-ZKbvIaAXARYMn1weU4QFIsocrZQHiQsvfX1tl3z8Huu

#### NOTES-

13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 226 lb down and 285 lb up at 4-1-4, 100 lb down and 129 lb up at 6-3-6, 100 lb down and 129 lb up at 10-3-6, 100 lb down and 129 lb up at 12-3-6, 100 lb down and 129 lb up at 14-3-6, 100 lb down and 129 lb up at 12-3-6, 100 lb down and 129 lb up at 12-3-6, 100 lb down and 129 lb up at 12-3-6, 100 lb down and 129 lb up at 120 lb up at 20-7-8, 100 lb down and 129 lb up at 22-7-8, 100 lb down and 129 lb up at 22-7-8, 100 lb down and 129 lb up at 22-7-8, 100 lb down and 129 lb up at 22-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 32-7-8, 100 lb down and 129 lb up at 40-8-4 on top chord, and 31 lb down and 24 lb up at 12-3-6, 32 lb down and 41 lb up at 14-3-6, 32 lb down and 41 lb up at 18-3-6, 32 lb down and 41 lb up at 18-3-6, 32 lb down and 41 lb up at 18-3-6, 32 lb down and 41 lb up at 18-7-8, 32 lb down and 41 lb up at 22-7-8, 32 lb down and 41 lb up at 22-7-8, 32 lb down and 41 lb up at 24-7-8, 32 lb down and 41 lb up at 24-7-8, 32 lb down and 41 lb up at 24-7-8, 32 lb down and 41 lb up at 32-7-8, 32 lb down and 41 lb up at 32-7-8, 32 lb down and 41 lb up at 32-7-8, 32 lb down and 41 lb up at 32-7-8, 32 lb down and 41 lb up at 32-7-8, 32 lb down and 41 lb up at 32-7-8, 32 lb down and 41 lb up at 32-7-8, 32 lb do

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

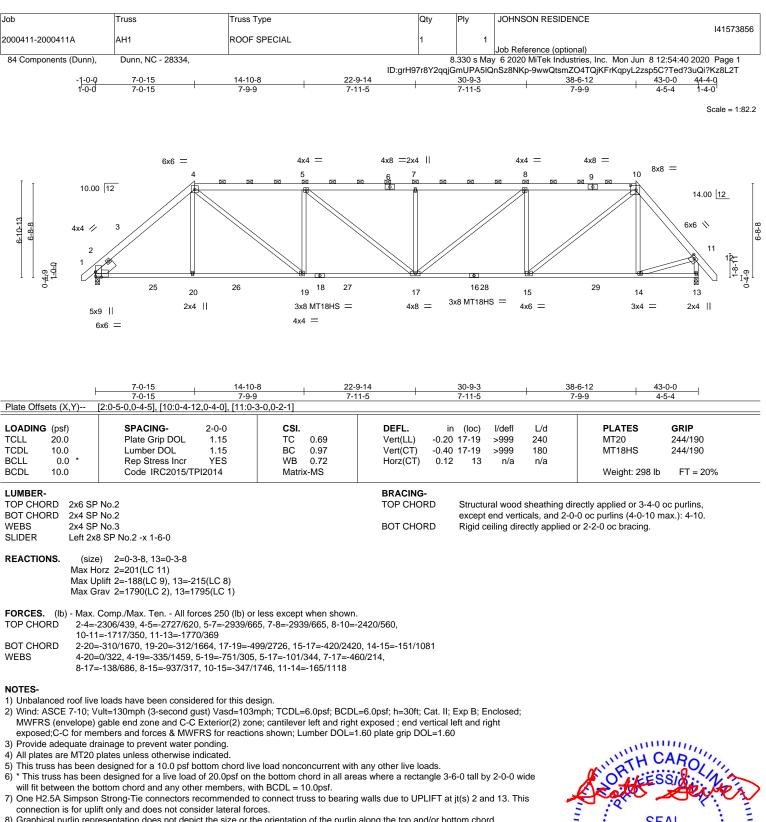
Uniform Loads (plf)

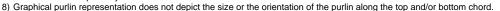
Vert: 1-3=-60, 3-11=-60, 11-12=-60, 12-13=-60, 14-24=-20

Concentrated Loads (lb)

Vert: 11=-0(F) 21=-4(F) 23=-6(F) 22=-4(F) 3=-2(F) 4=-0(F) 5=-0(F) 20=-4(F) 7=-0(F) 19=-4(F) 17=-4(F) 8=-0(F) 10=-0(F) 10=-0(F) 15=-0(F) 25=-0(F) 28=-0(F) 29=-0(F) 30=-0(F) 31=-0(F) 31=-0(F) 32=-0(F) 33=-0(F) 35=-0(F) 35



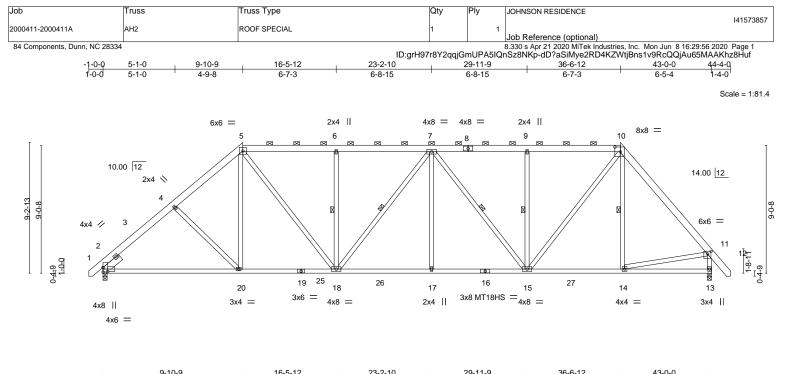






818 Soundside Road Edenton, NC 27932

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS ON TIPS REPREVED PAGE MIT-14/3 reference of the second secon fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



1	9-10-9	16-5-12	23-2-10	29-11-9	36-6-12	43-0-0	
I	9-10-9	6-7-3	6-8-15	6-8-15	6-7-3	6-5-4	1
Plate Offsets (X,Y)	[2:0-4-0,0-3-5], [10:0-4-12,0-4-0], [11:0-	·3-0,0-2-4]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.55 BC 0.93 WB 0.61 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/de -0.15 18-20 >99 -0.29 20-23 >99 0.10 13 n/	9 240 9 180	PLATES MT20 MT18HS Weight: 336 lb	<b>GRIP</b> 244/190 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SF BOT CHORD 2x4 SF WEBS 2x4 SF SLIDER Left 2x	P No.2	1	BRACING TOP CHO BOT CHO WEBS	RD Structural wo except end v	erticals, and 2-0-0 directly applied of	ectly applied or 3-10-1 0 oc purlins (4-11-1 m r 2-2-0 oc bracing. 18, 7-18, 7-15, 9-15, 7	ax.): 5-10.
Max H Max U Max G FORCES. (Ib) - Max. TOP CHORD 2-4=- 9-10= BOT CHORD 2-20= WEBS 5-20=	e) 2=0-3-8, 13=0-3-8 lorz 2=262(LC 11) lplift 2=-129(LC 9), 13=-152(LC 8) Grav 2=1766(LC 1), 13=1809(LC 2) Comp./Max. Ten All forces 250 (lb) o -2211/451, 4-5=-2060/471, 5-6=-2113/5 =-1889/472, 10-11=-1839/365, 11-13=-1 =-328/1581, 18-20=-284/1566, 17-18=-3 =-47/330, 5-18=-252/950, 6-18=-430/19 =-600/120, 9-15=-429/201, 10-15=-270/	27, 6-7=-2112/526, 7-9=-1 762/372 666/2248, 15-17=-366/224 3, 7-18=-263/104, 7-17=0/	1889/472, 18, 14-15=-129/11:	23			
<ol> <li>Wind: ASCE 7-10; W MWFRS (envelope) exposed;C-C for me</li> <li>Provide adequate dr</li> <li>All plates are MT20</li> <li>This truss has been</li> </ol>	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=103 gable end zone and C-C Exterior(2) zo mbers and forces & MWFRS for reaction rainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom chord lin or designed for a line load of 20 opsf on	mph; TCDL=6.0psf; BCDI ne; cantilever left and righ ns shown; Lumber DOL= re load nonconcurrent with	it exposed ; end ve 1.60 plate grip DO h any other live loa	ertical left and right L=1.60 ads.	,	Zunnin H CA	ROL

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) All bearings are assumed to be User Defined crushing capacity of 425 psi.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 2 and 152 lb uplift at joint 13.

9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

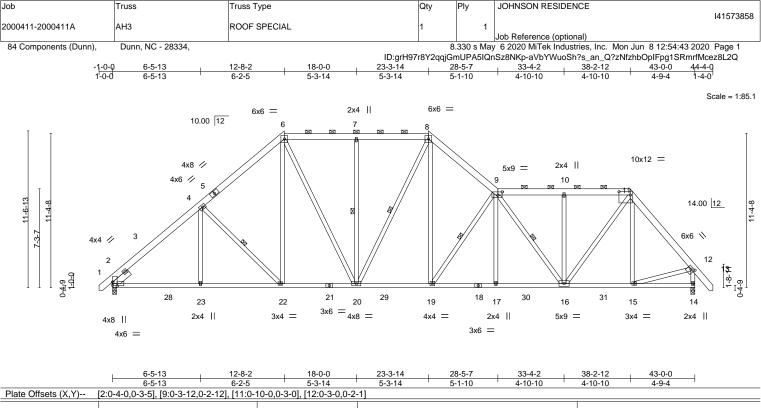
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

 $\cap$ EESS Summer. SEAL 044925 minin June 9,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSIVTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.58 BC 0.83 WB 0.59 Matrix-MS	Vert(LL) -0.14	n (loc) I/defi L/d 19-20 >999 240 19-20 >999 180 14 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 372 lb         FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x4 SP WEBS 2x4 SP SLIDER Left 2x0	No.2		BRACING- TOP CHORD BOT CHORD WEBS	except end verticals, and 2-0 Rigid ceiling directly applied	rectly applied or 3-9-10 oc purlins, I-0 oc purlins (5-3-11 max.): 6-8, 9-11. or 10-0-0 oc bracing. I-22, 7-20, 8-20, 9-19, 9-16
Max U Max G	e) 2=0-3-8, 14=0-3-8 orz 2=319(LC 11) plift 2=-132(LC 12), 14=-213(LC 13) rav 2=1766(LC 1), 14=1795(LC 1)				

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2230/417, 4-6=-1915/491, 6-7=-1669/487, 7-8=-1669/487, 8-9=-2236/553,

9-10=-1859/438, 10-11=-1859/438, 11-12=-1709/344, 12-14=-1754/363

 
 BOT CHORD
 2-23=-240/1690, 22-23=-240/1690, 20-22=-182/1425, 19-20=-132/1688, 17-19=-299/2396, 16-17=-300/2393, 15-16=-58/1036

 WEBS
 4-22=-379/246, 6-22=-101/447, 6-20=-157/639, 7-20=-349/165, 8-19=-203/1135,

WEBS 4-22=-379/246, 6-22=-101/447, 6-20=-157/639, 7-20=-349/165, 8-19=-203/1135, 9-19=-1191/327, 9-16=-926/170, 10-16=-322/164, 11-16=-261/1425, 12-15=-74/1035

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.

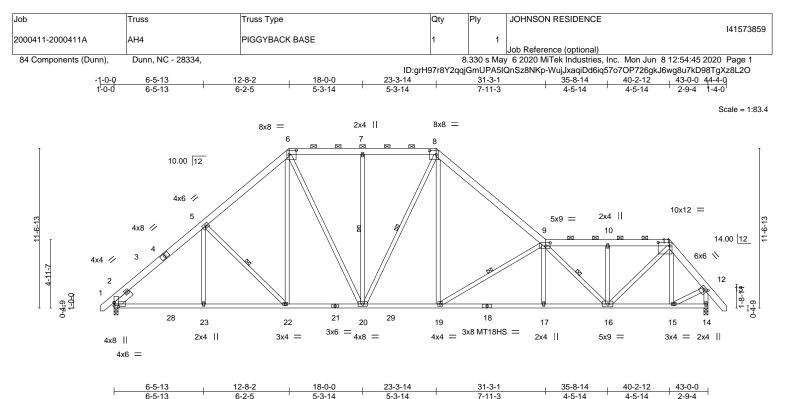
7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	0-0-10	0-2-5	3-3-14	J-J-1 <del>4</del>	1-11-5	7-3-17		2-3-4
Plate Offsets (X,Y)	[2:0-4-0,0-3-5], [6:0-6-0,0-3-	12], [8:0-5-12,0-3-8],	9:0-3-12,0-2-12	2], [11:0-10-0,0-3-0	], [12:0-3-0,0-2-1]			
OADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	Plate Grip DOL Lumber DOL	2-0-0 <b>CS</b> 1.15 TC 1.15 BC YES WE	0.58 0.94	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.17 17-19 >999 -0.40 17-19 >999 0.13 14 n/a	L/d 240 180 n/a	PLATES MT20 MT18HS Weight: 354 lb	<b>GRIP</b> 244/190 244/190 FT = 20%
								FT = 20%
	P No.2 P No.2			BRACING- TOP CHOR		0	ctly applied or 3-9-10 oc purlins (4-10-1 m	
	P No.3 x6 SP No.2 -x 1-6-0			BOT CHOR	D Rigid ceiling dire 2-2-0 oc bracing 8-5-2 oc bracing	g: 17-19	10-0-0 oc bracing,	Except:
REACTIONS. (siz	ze) 2=0-3-8, 14=0-3-8			WEBS	1 Row at midpt	, 5-2	2, 7-20, 8-20, 9-19,	9-16

TIONS. (size) 2=0-3-8, 14=0-3-8 Max Horz 2=320(LC 11) Max Uplift 2=-131(LC 12), 14=-213(LC 13) Max Grav 2=1766(LC 1), 14=1795(LC 1)

 
 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-5=-2225/419, 5-6=-1926/495, 6-7=-1608/485, 7-8=-1608/485, 8-9=-2310/521, 9-10=-2230/468, 10-11=-2230/468, 11-12=-1501/308, 12-14=-1779/357

 BOT CHORD
 2-23=-211/1664, 22-23=-211/1664, 20-22=-148/1382, 19-20=-92/1649, 17-19=-479/3194, 16-17=-481/3191, 15-16=-65/928

 WEBS
 5-22=-403/252, 6-22=-103/471, 6-20=-157/621, 7-20=-328/172, 8-19=-146/1046,

- 9-19=-1777/452, 9-17=0/260, 9-16=-1373/260, 10-16=-264/157, 11-16=-334/1860, 11-15=-425/92, 12-15=-86/1034
- 1

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

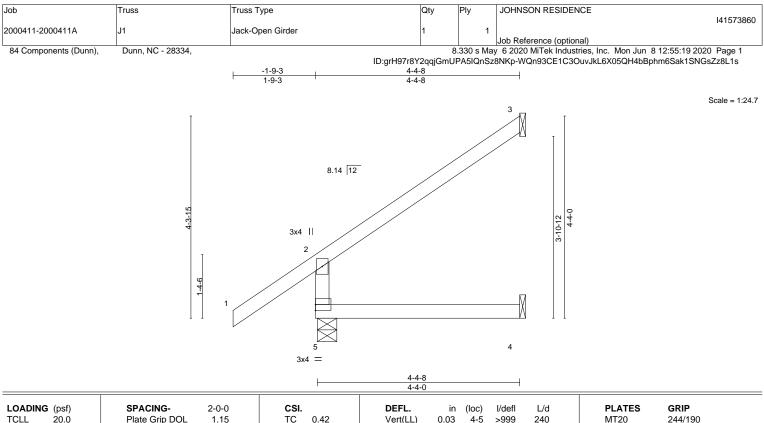
6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







LOADING	(p3i)	SI AGING-	2-0-0	0.01.					(100)	i/uen	L/u	ILAILO	UNI
TCLL	20.0	Plate Grip DOL	1.15	TC	0.42	V	ert(LL)	0.03	4-5	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.29	V	ert(CT)	-0.03	4-5	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	н	lorz(CT)	-0.04	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-MR							Weight: 19 lb	FT = 20%
		•											

## LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-4-8 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-5-0, 3=Mechanical, 4=Mechanical Max Horz 5=137(LC 12) Max Uplift 5=-10(LC 12), 3=-90(LC 12), 4=-1(LC 12) Max Grav 5=308(LC 1), 3=116(LC 19), 4=77(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-264/127

### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

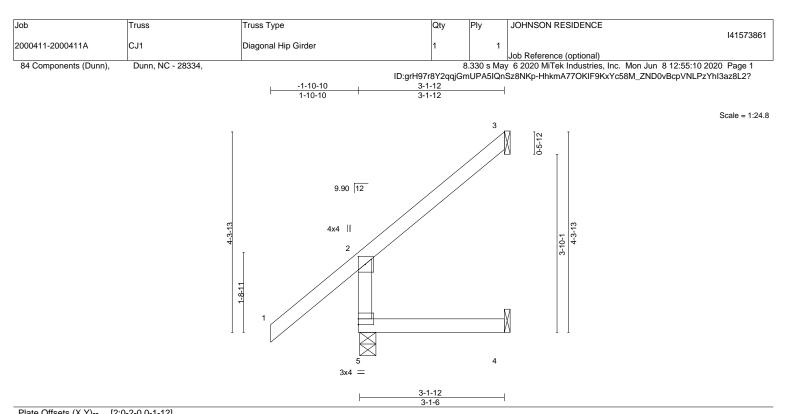
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 90 lb uplift at joint 3 and 1 lb uplift at joint 4.

6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.







OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.52	Vert(LL) 0.01 4-5 >999 240	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.28	Vert(CT) -0.01 4-5 >999 180	
BCLL 0.0 *	Rep Stress Incr NO	WB 0.00	Horz(CT) -0.04 3 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MR		Weight: 17 lb FT = 20%

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 TOP CHORD

Structural wood sheathing directly applied or 3-1-12 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS. (size) 5=0-4-3, 3=Mechanical, 4=Mechanical

Max Horz 5=121(LC 12)

Max Uplift 3=-82(LC 12), 4=-17(LC 12) Max Grav 5=282(LC 1), 3=74(LC 19), 4=53(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right

exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

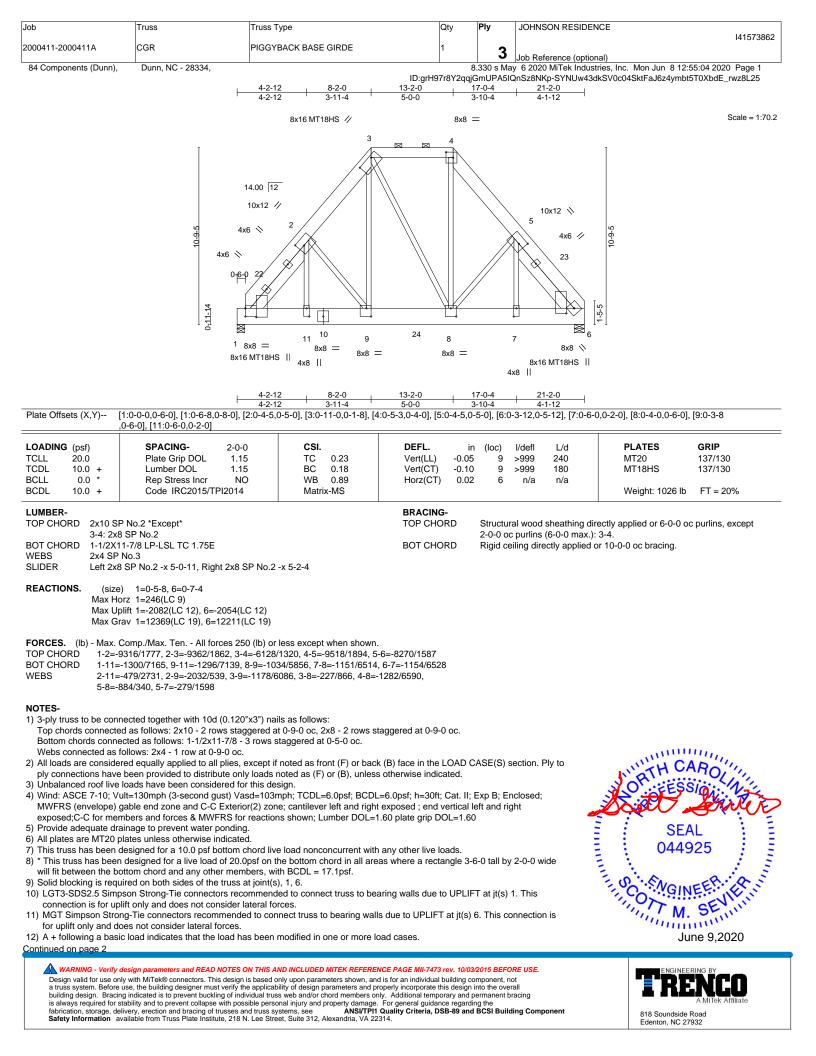
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 82 lb uplift at joint 3 and 17 lb uplift at joint 4.







Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE	
			4	-		141573862
2000411-2000411A	CGR	PIGGYBACK BASE GIRDE	1	3	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,				ay 6 2020 MiTek Industries, Inc. Mon Jun 8 1 QnSz8NKp-SYNUw43dkSV0c04SktFaJ6z4ym	
NOTES-	0 7 40 44 40 47 40	10 00 04 00 00 04 05 00 00 00 00 00	0 10			-
	6, 7, 10, 11, 16, 17, 18, 7 ey are correct for the inte	19, 20, 21, 22, 23, 24, 25, 26, 29, 30, 33, 34 ended use of this truss.	, 39, 40, 41, 42, 43	, 44 has/r	have been modified. Building designer must	review
14) Graphical purlin repre	sentation does not depi	ct the size or the orientation of the purlin alo	ng the top and/or b	ottom cho	ord.	
<ol> <li>LGT3 Hurricane ties</li> <li>MGT Huricane tie mu</li> </ol>		line below the truss.				
LOAD CASE(S) Standa						
		=1.15, Plate Increase=1.15				
Uniform Loads (plf)	74					
Trapezoidal Loads (plf	74, 3-4=-74, 19-23=-74, <i>*</i> )					
		), 4=-94(F=-20)-to-23=-314(F=-240) ab. Attic Storage: Lumber Increase=1.15, F	late Increase-1 15			
Uniform Loads (plf)	(balanced) + 0.75 Onini	ab. Alle Storage. Lumber increase=1.13, F				
Vert: 14-22=-6 Trapezoidal Loads (plf		I-9=-879(F=-844), 9-24=-909(F=-844), 6-24	=-879(F=-844)			
		), 4=-82(F=-17)-to-23=-274(F=-210)				
,	Attic Without Storage: Lu	Imber Increase=1.25, Plate Increase=1.25				
Uniform Loads (plf) Vert: 14-22=-3	34, 3-4=-34, 19-23=-34, <sup>2</sup>	I-6=-778(F=-724)				
Trapezoidal Loads (plf		) $4 - 40(E - 15)$ to 22 - 214(E - 180)				
		), 4=-49(F=-15)-to-23=-214(F=-180) umber Increase=1.60, Plate Increase=1.60				
Uniform Loads (plf)	× 2 4- 46 10 22- 64 ·	6- 970/E- 945)				
Horz: 3-14=30	64, 3-4=-46, 19-23=-64, <i>*</i> ), 4-19=-30	-0=-079(F=-043)				
Trapezoidal Loads (plf		), 4=-82(F=-18)-to-23=-275(F=-210)				
		umber Increase= $1.60$ , Plate Increase= $1.60$				
Uniform Loads (plf)	64, 3-4=-46, 19-23=-64, <sup>2</sup>	L-6879(F845)				
Horz: 3-14=30	, 4-19=-30	1-0013(10+3)				
Trapezoidal Loads (plf		), 4=-82(F=-18)-to-23=-275(F=-210)				
		: Lumber Increase=1.60, Plate Increase=1.6	60			
Uniform Loads (plf)	-49, 3-4=-7, 19-23=-24, <sup>-</sup>	1-6700(F666)				
Horz: 3-14=1		1 0 - 7 00(1 - 000)				
Trapezoidal Loads (p	,	4), 4=-37(F=-14)-to-23=-189(F=-166)				
		nt: Lumber Increase=1.60, Plate Increase=1	.60			
Uniform Loads (plf)	-24, 3-4=-7, 19-23=-49, 1	1-6700(F666)				
Horz: 3-14=-		1 0 - 7 00(1 - 000)				
Trapezoidal Loads (p Vert: 22=-18		4), 4=-63(F=-14)-to-23=-215(F=-166)				
16) Dead + 0.6 MWFRS	- ( / (	Parallel: Lumber Increase=1.60, Plate Incre	ase=1.60			
Uniform Loads (plf)	-7, 3-4=-23, 19-23=-23, 1	1-6584(F550)				
Horz: 3-14=-	28, 4-19=11	0- 00+(1 - 000)				
Trapezoidal Loads (p Vert: 22=-14		1), 4=-35(F=-11)-to-23=-160(F=-137)				
17) Dead + 0.6 MWFRS		Parallel: Lumber Increase=1.60, Plate Incre	ease=1.60			
Uniform Loads (plf) Vert: 14-22=	-23, 3-4=-23, 19-23=-7, 1	1-6=-584(F=-550)				
Horz: 3-14=-	11, 4-19=28					
Trapezoidal Loads (p Vert: 22=-16		1), 4=-18(F=-11)-to-23=-143(F=-137)				
18) Dead + Uninhabitable		ncrease=1.25, Plate Increase=1.25				
Uniform Loads (plf) Vert: 14-22=	-34 3-4=-34 19-23=-34	1-9=-517(F=-483), 9-24=-557(F=-483), 6-2	4=-517(F=-483)			
Trapezoidal Loads (p	lf)					
		0), 4=-44(F=-10)-to-23=-154(F=-120) Attic Storage + 0.75(0.6 MWFRS Wind (Neg	a. Int) Left): Lumbe	r Increase	e=1.60. Plate	
Increase=1.60			g, _o			
Uniform Loads (plf) Vert: 14-22=	-76 3-4=-44 19-23=-56	1-9=-1016(F=-982), 9-24=-1046(F=-982), 6	3-24=-1016(F=-982	)		
Horz: 3-14=1	1, 4-19=8			,		
Trapezoidal Loads (p Vert: 22=-32		0), 4=-77(F=-20)-to-23=-300(F=-244)				
20) Dead + 0.75 Roof Liv		Attic Storage + 0.75(0.6 MWFRS Wind (Neg	g. Int) Right): Lumb	er Increas	se=1.60,	
Plate Increase=1.60 Uniform Loads (plf)						
Vert: 14-22=		1-9=-1016(F=-982), 9-24=-1046(F=-982), 6	6-24=-1016(F=-982	)		
-=Horz: 3-14 Trapezoidal Loads (p						
		0), 4=-96(F=-20)-to-23=-320(F=-244)				



Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE	
2000411-2000411A	CGR	PIGGYBACK BASE GIRDE	1	<b>_</b> _		l41573862
84 Components (Dunn),	Dunn, NC - 28334,		o	330 s May	Job Reference (optional) y 6 2020 MiTek Industries, Inc. Mon Jun 8 12:55:04 20	20 Page 3
64 Components (Dunin),	Dunn, NC - 20334,				QnSz8NKp-SYNUw43dkSV0c04SktFaJ6z4ymbt5T0XbdE	
Uniform Loads (plf) Vert: 14-22=-4 Horz: 3-14=-2	(bal.) + 0.75 Uninhab. Attic \$ 14, 3-4=-56, 19-23=-56, 1-9=- 1, 4-19=8	Storage + 0.75(0.6 MWFRS Wind (Neg. 929(F=-895), 9-24=-959(F=-895), 6-24	, ,	∟umber In	crease=1.60, Plate Increase=1.60	
22) Dead + 0.75 Roof Live Uniform Loads (plf)	F=-223)-to-3=-62(F=-19), 4= (bal.) + 0.75 Uninhab. Attic \$ 56, 3-4=-56, 19-23=-44, 1-9=- , 4-19=21	-75(F=-19)-to-23=-279(F=-223) Storage + 0.75(0.6 MWFRS Wind (Neg. 929(F=-895), 9-24=-959(F=-895), 6-24		Lumber li	ncrease=1.60, Plate Increase=1.60	
Vert: 22=-279 23) 1st Dead + Roof Live ( Uniform Loads (plf) Vert: 14-22=-7	(F=-223)-to-3=-75(F=-19), 4= unbalanced): Lumber Increas 74, 3-4=-74, 19-23=-34, 1-6=-	-62(F=-19)-to-23=-266(F=-223) se=1.15, Plate Increase=1.15 999(F=-965)				
24) 2nd Dead + Roof Live Uniform Loads (plf) Vert: 14-22=-3	(F=-240)-to-3=-94(F=-20), 4= (unbalanced): Lumber Increa 34, 3-4=-74, 19-23=-74, 1-6=-	-54(F=-20)-to-23=-274(F=-240) se=1.15, Plate Increase=1.15 999(F=-965)				
25) 3rd Dead + 0.75 Roof Uniform Loads (plf)	, F=-240)-to-3=-54(F=-20), 4= Live (unbalanced) + 0.75 Uni 54, 3-4=-64, 19-23=-34, 1-9≕	-94(F=-20)-to-23=-314(F=-240) nhab. Attic Storage: Lumber Increase= :879(F=-844), 9-24=-909(F=-844), 6-24		e=1.15		
Vert: 22=-274 26) 4th Dead + 0.75 Roof I	(F=-210)-to-3=-82(F=-18), 4=	-52(F=-17)-to-23=-244(F=-210) nhab. Attic Storage: Lumber Increase=1	1.15, Plate Increas	e=1.15		
Trapezoidal Loads (plf	)	879(F=-844), 9-24=-909(F=-844), 6-24 -82(F=-17)-to-23=-274(F=-210)	=-879(F=-844)			
Uniform Loads (plf)	64, 3-4=-46, 19-23=-64, 1-6=- ), 4-19=-30	e 1: Lumber Increase=1.60, Plate Incre 879(F=-845)	ease=1.60			
Vert: 22=-275 30) Reversal: Dead + 0.6 ( Uniform Loads (plf) Vert: 14-22=-6 Horz: 3-14=30	(F=-210)-to-3=-82(F=-18), 4= C-C Wind (Neg. Internal) Cas 64, 3-4=-46, 19-23=-64, 1-6=- 0, 4-19=-30	-82(F=-18)-to-23=-275(F=-210) e 2: Lumber Increase=1.60, Plate Incre 879(F=-845)	ease=1.60			
33) Reversal: Dead + 0.6 N Uniform Loads (plf)	(F=-210)-to-3=-82(F=-18), 4=	-82(F=-18)-to-23=-275(F=-210) Left: Lumber Increase=1.60, Plate Incr 200(F=-666)	ease=1.60			
Horz: 3-14=15 Trapezoidal Loads (plf	5, 4-19=11 )	37(F=-14)-to-23=-189(F=-166)				
Uniform Loads (plf)	24, 3-4=-7, 19-23=-49, 1-6=-7	Right: Lumber Increase=1.60, Plate Inc 00(F=-666)	crease=1.60			
	(F=-166)-to-3=-37(F=-14), 4=	-63(F=-14)-to-23=-215(F=-166) 1st Parallel: Lumber Increase=1.60, Pl	ate Increase=1.60			
Horz: 3-14=-2 Trapezoidal Loads (plf		84(F=-550) -35(F=-11)-to-23=-160(F=-137)				
Uniform Loads (plf) Vert: 14-22=-2 Horz: 3-14=-1	23, 3-4=-23, 19-23=-7, 1-6=-5 1, 4-19=28	2nd Parallel: Lumber Increase=1.60, P 84(F=-550)	Plate Increase=1.60	)		
41) Reversal: Dead + 0.75 Increase=1.60, Plate Ir	(F=-137)-to-3=-35(F=-11), 4= Roof Live (bal.) + 0.75 Uninh	-18(F=-11)-to-23=-143(F=-137) nab. Attic Storage + 0.75(0.6 MWFRS V	Vind (Neg. Int) Left	): Lumbe	r	
Uniform Loads (plf) Vert: 14-22=-7 Horz: 3-14=11 Trapezoidal Loads (plf	, 4-19=8	1016(F=-982), 9-24=-1046(F=-982), 6-	24=-1016(F=-982)			
Vert: 22=-320	(F=-244)-to-3=-96(F=-20), 4=	-77(F=-20)-to-23=-300(F=-244)				

## Continued on page 4



Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE
					141573862
2000411-2000411A	CGR	PIGGYBACK BASE GIRDE	1	2	
				3	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	.330 s May	/ 6 2020 MiTek Industries, Inc. Mon Jun 8 12:55:04 2020 Page 4
		ID:grH9	7r8Y2qqjQ	GmUPA5IC	nSz8NKp-SYNUw43dkSV0c04SktFaJ6z4ymbt5T0XbdE_rwz8L25

LOAD CASE(S)

42) Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 14-22=-56, 3-4=-44, 19-23=-76, 1-9=-1016(F=-982), 9-24=-1046(F=-982), 6-24=-1016(F=-982)

Horz: 3-14=-8, 4-19=-11

Trapezoidal Loads (plf)

Vert: 22=-300(F=-244)-to-3=-77(F=-20), 4=-96(F=-20)-to-23=-320(F=-244)

43) Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Úninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 14-22=-44, 3-4=-56, 19-23=-56, 1-9=-929(F=-895), 9-24=-959(F=-895), 6-24=-929(F=-895)

Horz: 3-14=-21, 4-19=8 Trapezoidal Loads (plf)

Vert: 22=-266(F=-223)-to-3=-62(F=-19), 4=-75(F=-19)-to-23=-279(F=-223)

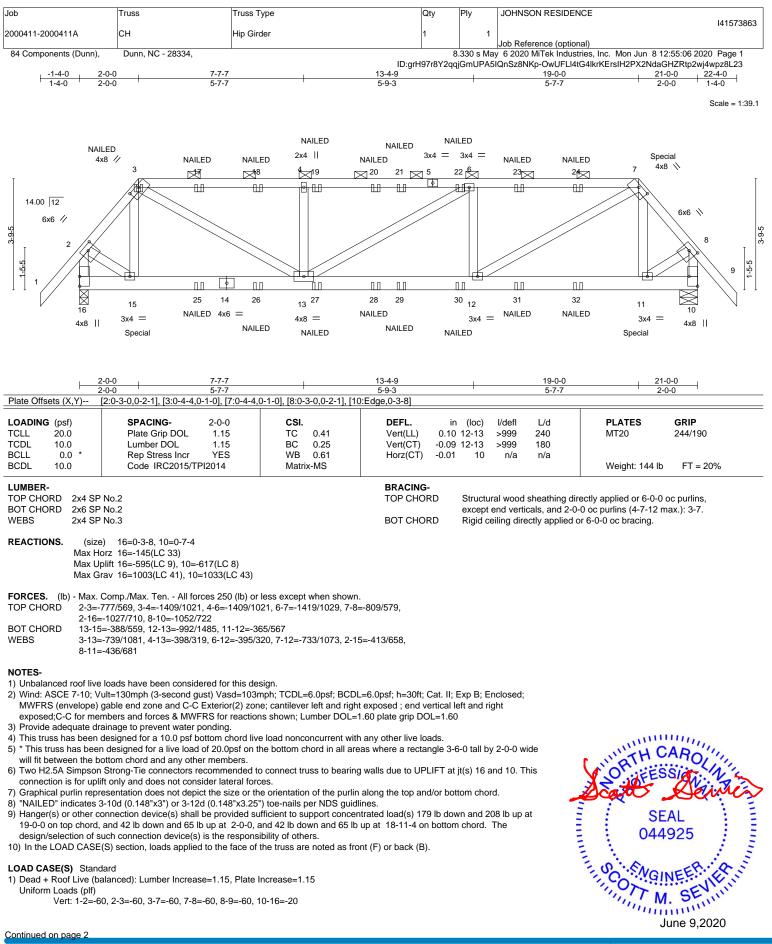
44) Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 14-22=-56, 3-4=-56, 19-23=-44, 1-9=-929(F=-895), 9-24=-959(F=-895), 6-24=-929(F=-895)

Horz: 3-14=-8, 4-19=21 Trapezoidal Loads (plf)

Vert: 22=-279(F=-223)-to-3=-75(F=-19), 4=-62(F=-19)-to-23=-266(F=-223)





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Edenton, NC 27932

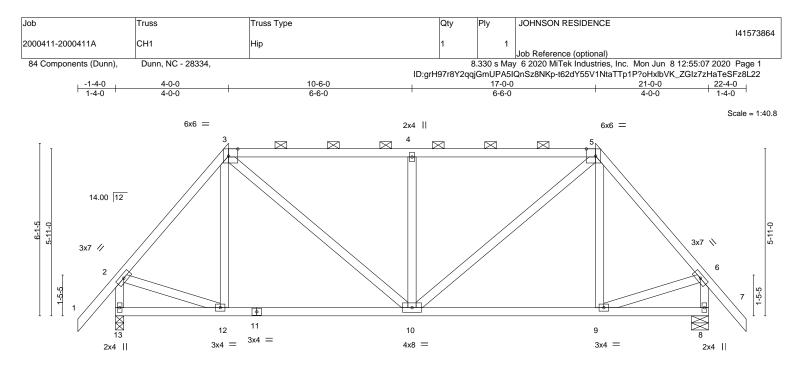
Ī,	Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE
						I41573863
	2000411-2000411A	СН	Hip Girder	1	1	
						Job Reference (optional)
	84 Components (Dunn),	Dunn, NC - 28334,		8	.330 s May	/ 6 2020 MiTek Industries, Inc. Mon Jun 8 12:55:06 2020 Page 2

ID:grH97r8Y2qqjGmUPA5IQnSz8NKp-OwUFLI4tG4lkrKErsIH2PX2NdaGHZRtp2wj4wpz8L23

# LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 15=9(F) 11=9(F) 25=4(F) 26=4(F) 27=4(F) 28=4(F) 29=4(F) 30=4(F) 31=4(F) 32=4(F)





ŀ	4-0-0 4-0-0	<u>10-6-0</u> 6-6-0		<u>17-0-0</u> 6-6-0	<u>  21-0-0</u> 4-0-0	
Plate Offsets (X,Y)	[3:0-3-15,0-3-0], [5:0-3-15,0-3-0]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.61 BC 0.40 WB 0.29 Matrix-MS	Vert(CT)	in (loc) l/defl L/d -0.04 10-12 >999 240 -0.08 10-12 >999 180 0.01 8 n/a n/a	PLATES MT20 Weight: 138 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP	P No.2		BRACING- TOP CHORE BOT CHORE	except end verticals, and	g directly applied or 6-0-0 l 2-0-0 oc purlins (5-8-15 n lied or 6-0-0 oc bracing.	
Max U Max G	e) 13=0-3-8, 8=0-7-4 orz 13=203(LC 11) plift 13=-64(LC 9), 8=-64(LC 8) irav 13=917(LC 1), 8=917(LC 1) Comp./Max. Ten All forces 250 (lb) c	r less excent when shown				
TOP CHORD 2-3=- 6-8=-	799/190, 3-4=-813/253, 4-5=-813/253, 892/229					
	2=-128/470, 9-10=-57/470 =-138/476, 4-10=-458/204, 5-10=-138/4	76, 2-12=-83/480, 6-9=-85	5/480			
<ul> <li>2) Wind: ASCE 7-10; V MWFRS (envelope) exposed;C-C for me</li> <li>3) Provide adequate dr</li> <li>4) This truss has been</li> </ul>	e loads have been considered for this d (ult=130mph (3-second gust) Vasd=10; gable end zone and C-C Exterior(2) zo mbers and forces & MWFRS for reactive rainage to prevent water ponding. designed for a 10.0 psf bottom chord lin n designed for a live load of 20.0psf on	Bmph; TCDL=6.0psf; BCDL one; cantilever left and righ ons shown; Lumber DOL= ve load nonconcurrent with	t exposed ; end ver 1.60 plate grip DOL n any other live load	tical left and right =1.60 Is.		

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.

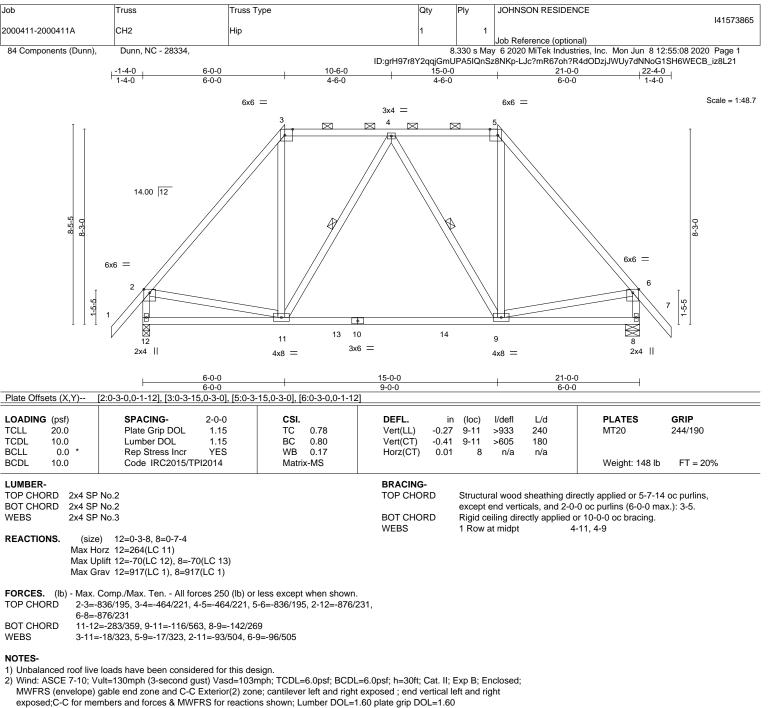
One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This
connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

A MiTek A 818 Soundside Road Edenton, NC 27932



3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

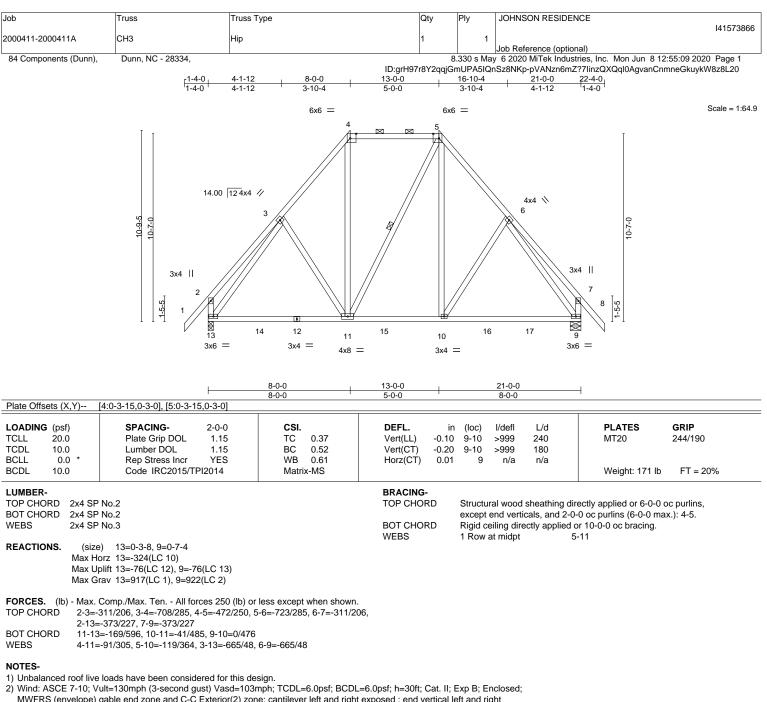
6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12. This connection is for uplift only and does not consider lateral forces.

One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This
connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







WWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS (or reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Provide degraded to a provent water coordinate

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

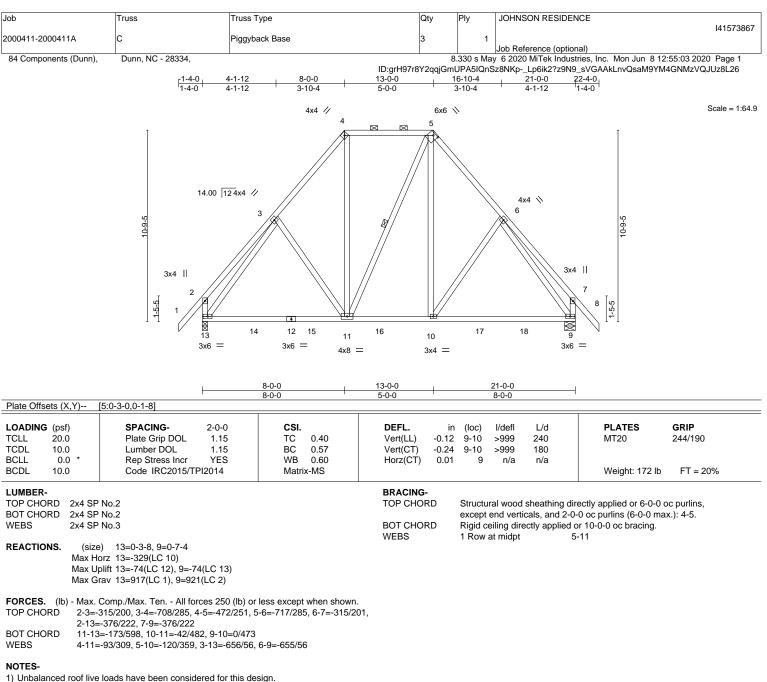
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 9. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

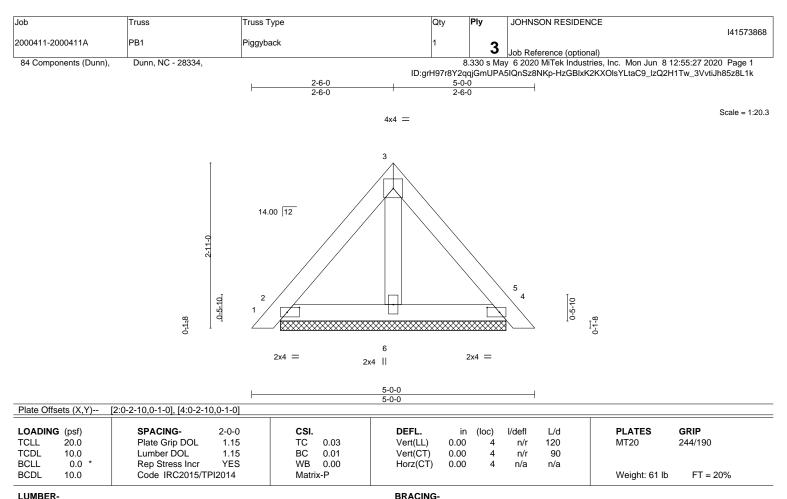
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 9. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

OTHERS 2x4 SP No.3

REACTIONS. (size) 2=3-11-12, 4=3-11-12, 6=3-11-12 Max Horz 2=70(LC 11)

Max Uplift 2=-32(LC 13), 4=-30(LC 13) Max Grav 2=120(LC 1), 4=120(LC 1), 6=121(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) 3-ply truss to be connected together with 10d (0.120"x3") nails as follows:

Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

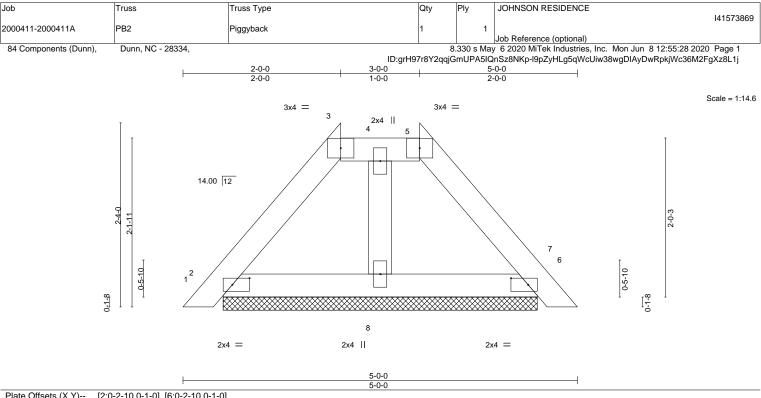
9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



OADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in	(loc) l/defl	L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.04	Vert(LL) 0.00	6 n/r	120	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) 0.00	6 n/r	90	
CLL 0.0 *	Rep Stress Incr YES	WB 0.01	Horz(CT) 0.00	6 n/a	n/a	
CDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 19 lb FT = 20%

BOT CHORD

2-0-0 oc purlins: 3-5.

#### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

**REACTIONS.** (size) 2=3-11-12, 6=3-11-12, 8=3-11-12

Max Horz 2=-52(LC 10) Max Uplift 2=-25(LC 12), 6=-30(LC 13)

Max Grav 2=121(LC 1), 6=121(LC 1), 8=116(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

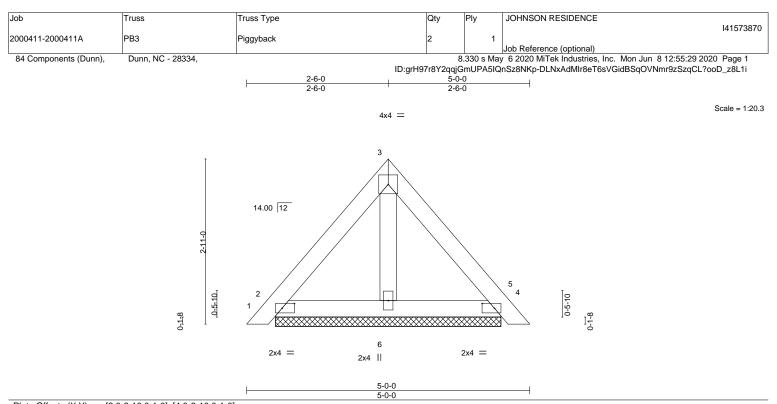
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 5-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.





LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	тс	0.10	Vert(LL)	0.00	5	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	5	n/r	90		
BCLL 0.0	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matri	x-P						Weight: 20 lb	FT = 20%

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS.

(size) 2=3-11-12, 4=3-11-12, 6=3-11-12 Max Horz 2=70(LC 11)

Max Uplift 2=-32(LC 13), 4=-30(LC 13) Max Grav 2=120(LC 1), 4=120(LC 1), 6=121(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members. 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

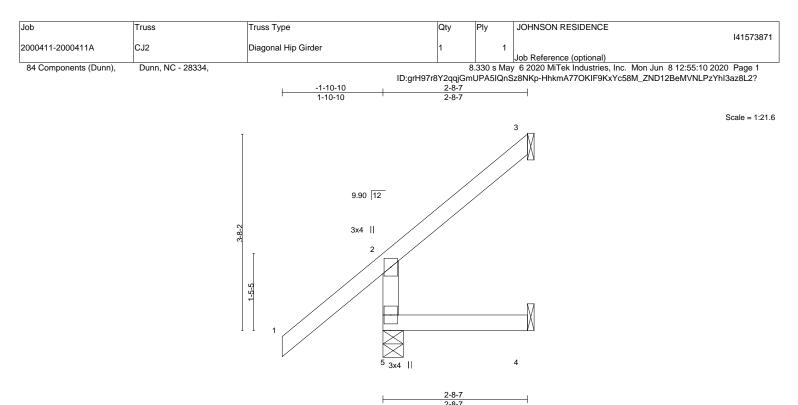


Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 1003/2016 Sec. 1005/2016 Sec. Design valid for use only with MITER & connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





		Z-0-1										
LOADIN	G (psf)	SPACING- 2-	-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1	1.15	TC	0.45	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL 1	1.15	BC	0.18	Vert(CT)	-0.01	4-5	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	-0.02	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI20	14	Matri	x-MR						Weight: 15 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-8-7 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-4-9, 3=Mechanical, 4=Mechanical Max Horz 5=112(LC 12) Max Uplift 5=-4(LC 12), 3=-64(LC 12), 4=-12(LC 12) Max Grav 5=271(LC 1), 3=54(LC 19), 4=43(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

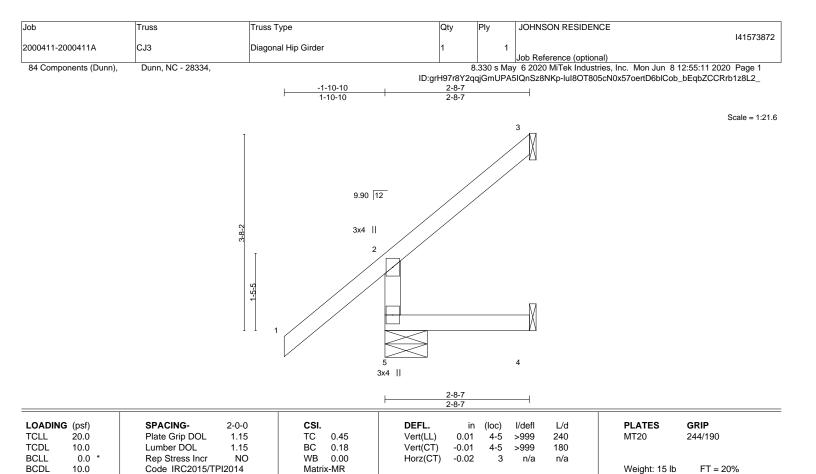
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3 and 12 lb uplift at joint 4.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.







1 1 11	мве	P-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-8-7 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 5=0-9-8, 4=Mechanical Max Horz 5=112(LC 12) Max Uplift 3=-64(LC 12), 5=-4(LC 12), 4=-12(LC 12) Max Grav 3=54(LC 19), 5=271(LC 1), 4=43(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

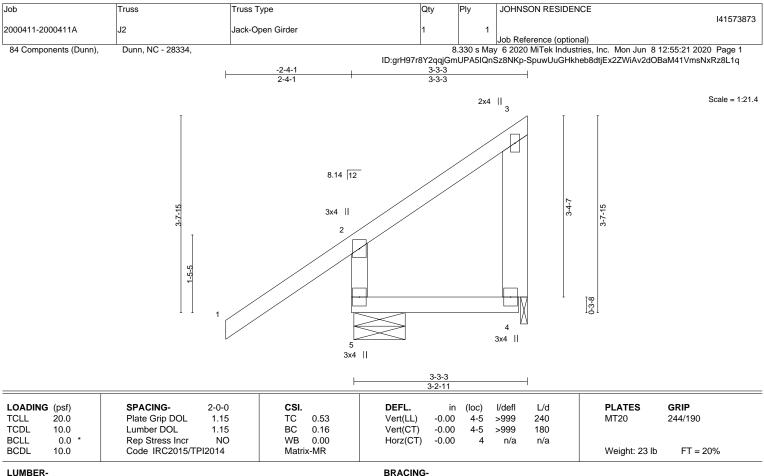
6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3 and 12 lb uplift at joint 4.

8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.



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2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x6 SP No.2 \*Except\* WEBS 2-5: 2x4 SP No.3

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-3-3 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 5=0-11-7, 4=0-1-8 Max Horz 5=146(LC 9) Max Uplift 5=-67(LC 12), 4=-62(LC 9) Max Grav 5=329(LC 1), 4=94(LC 10)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-287/222

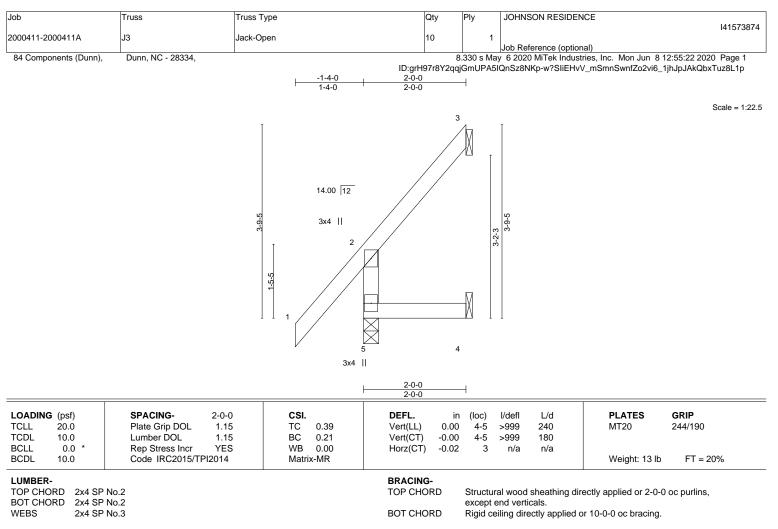
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.



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🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 1003/2016 Sec. 1005/2016 Sec. Design valid for use only with MITER & connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=124(LC 12) Max Uplift 3=-80(LC 12), 4=-32(LC 12)

Max Grav 5=198(LC 1), 3=53(LC 12), 4=39(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

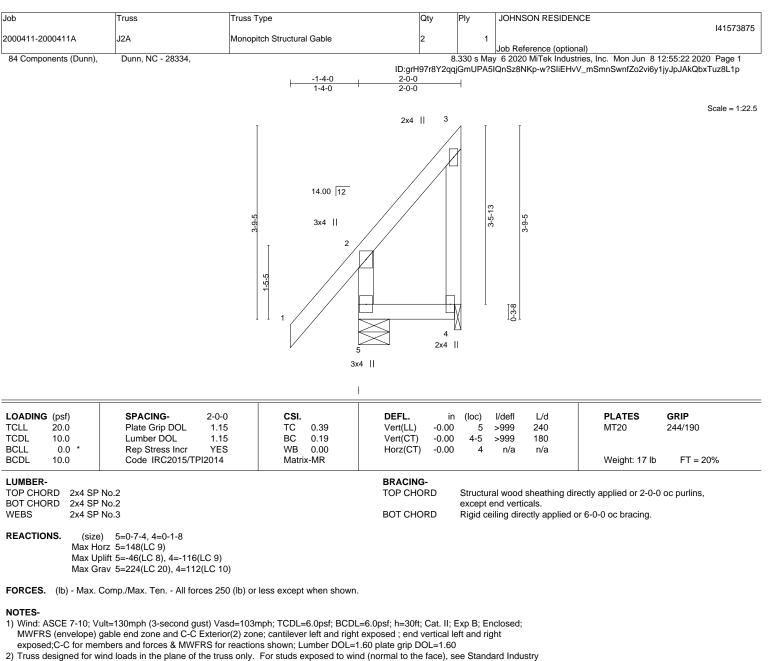
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 3 and 32 lb uplift at joint 4.



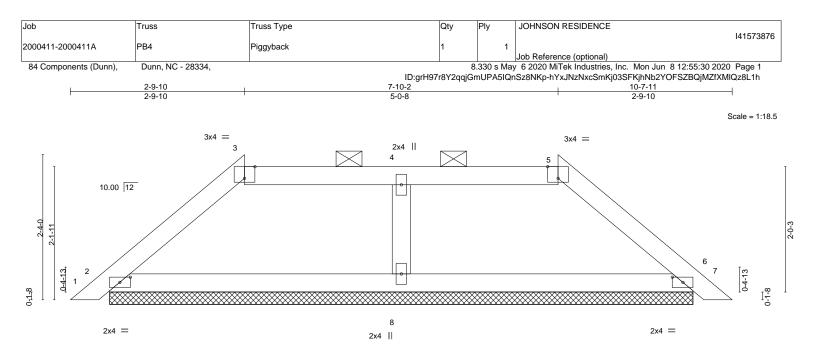




- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.







			10-1-11			
Plate Offsets (X,Y)	[2:0-2-1,0-1-0], [3:0-2-0,Edge], [5:0-2-0	,Edge], [6:0-2-1,0-1-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Den Otrace Jack	CSI. TC 0.10 BC 0.21	Vert(LL) 0.0 Vert(CT) 0.0	0 7 n/	r 120 r 90	PLATES         GRIP           MT20         244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.04 Matrix-S	Horz(CT) 0.0	0 6 n/a	a n/a	Weight: 34 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP OTHERS 2x4 SP	No.2		BRACING- TOP CHORD BOT CHORD	2-0-0 oc purli	ns (6-0-0 max.)	irectly applied or 6-0-0 oc purlins, except ): 3-5. or 10-0-0 oc bracing.

10-7-11

REACTIONS. (size) 2=9-4-9, 6=9-4-9, 8=9-4-9

Max Horz 2=51(LC 11) Max Uplift 2=-45(LC 12), 6=-45(LC 13), 8=-19(LC 9)

Max Grav 2=250(LC 1), 6=250(LC 1), 8=299(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

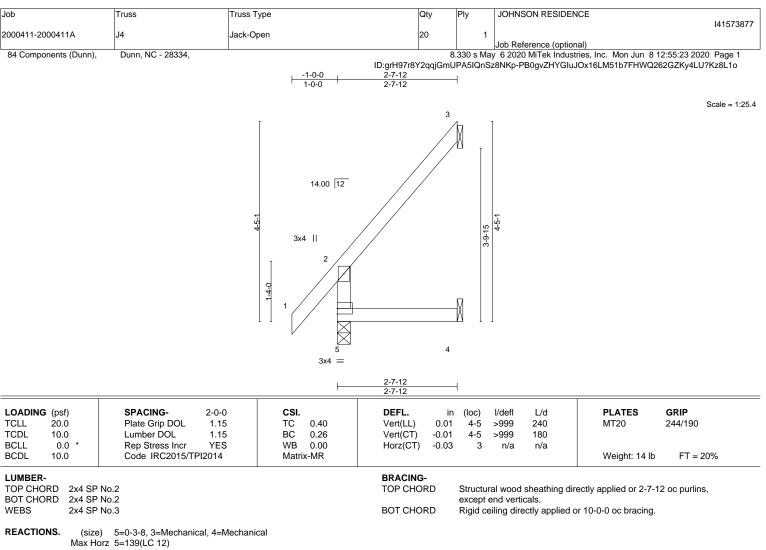
7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 6, and 8. This connection is for uplift only and does not consider lateral forces.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







Max Uplift 3=-105(LC 12), 4=-27(LC 12) Max Grav 5=182(LC 1), 3=86(LC 19), 4=46(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

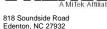
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

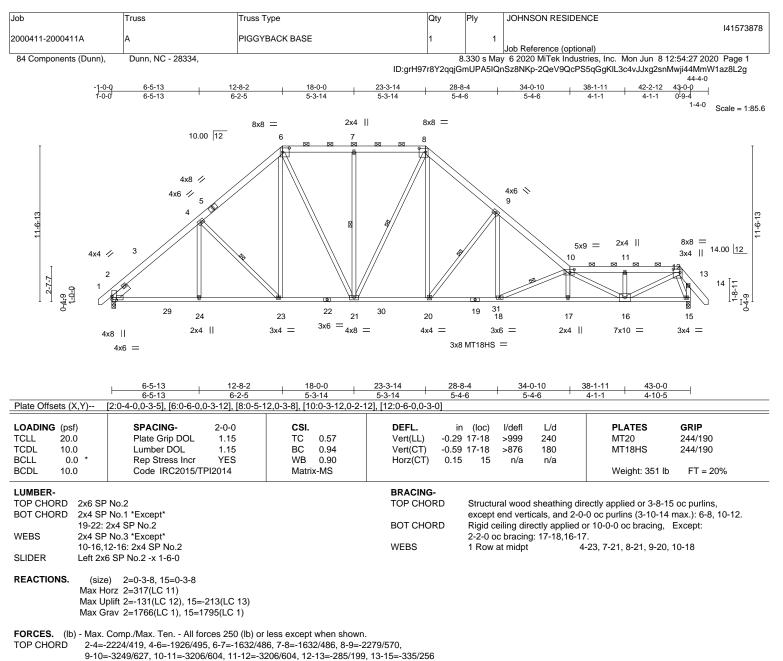
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 3 and 27 lb uplift at joint 4.







 
 BOT CHORD
 2-24=-183/1690, 23-24=-183/1690, 21-23=-118/1401, 20-21=-71/1651, 18-20=-307/2474, 17-18=-851/5254, 16-17=-855/5250, 15-16=-30/477

 WEBS
 4-23=-404/252, 6-23=-104/470, 6-21=-158/629, 7-21=-339/165, 8-20=-234/1202, 9-20=-1333/380, 9-18=-172/1293, 10-18=-3014/602, 10-16=-233/409, 11-16=-273/129, 12-16=-535/3113, 12-15=-1488/204

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-7 to 44-2-5 zone; cantilever left and right exposed; c-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.
 All plates are MT20 plates unless otherwise indicated.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

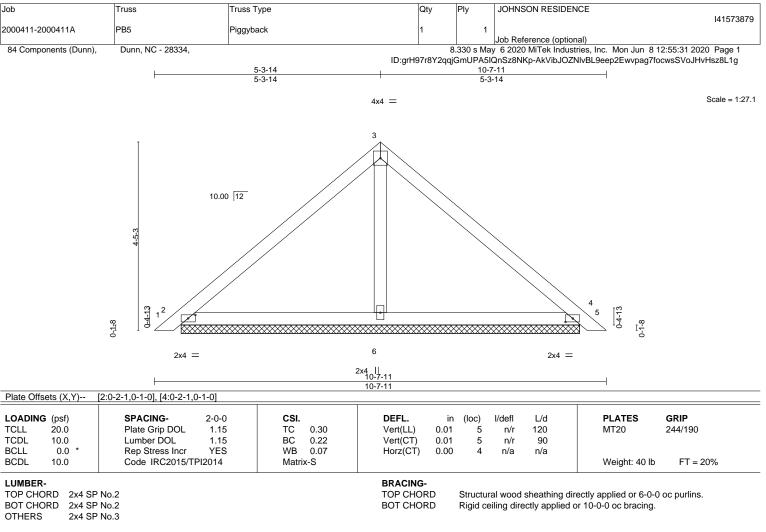
6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 15. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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**REACTIONS.** (size) 2=9-4-9, 4=9-4-9, 6=9-4-9

Max Horz 2=-105(LC 10) Max Uplift 2=-40(LC 12), 4=-53(LC 13), 6=-8(LC 12) Max Grav 2=223(LC 1), 4=223(LC 1), 6=354(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

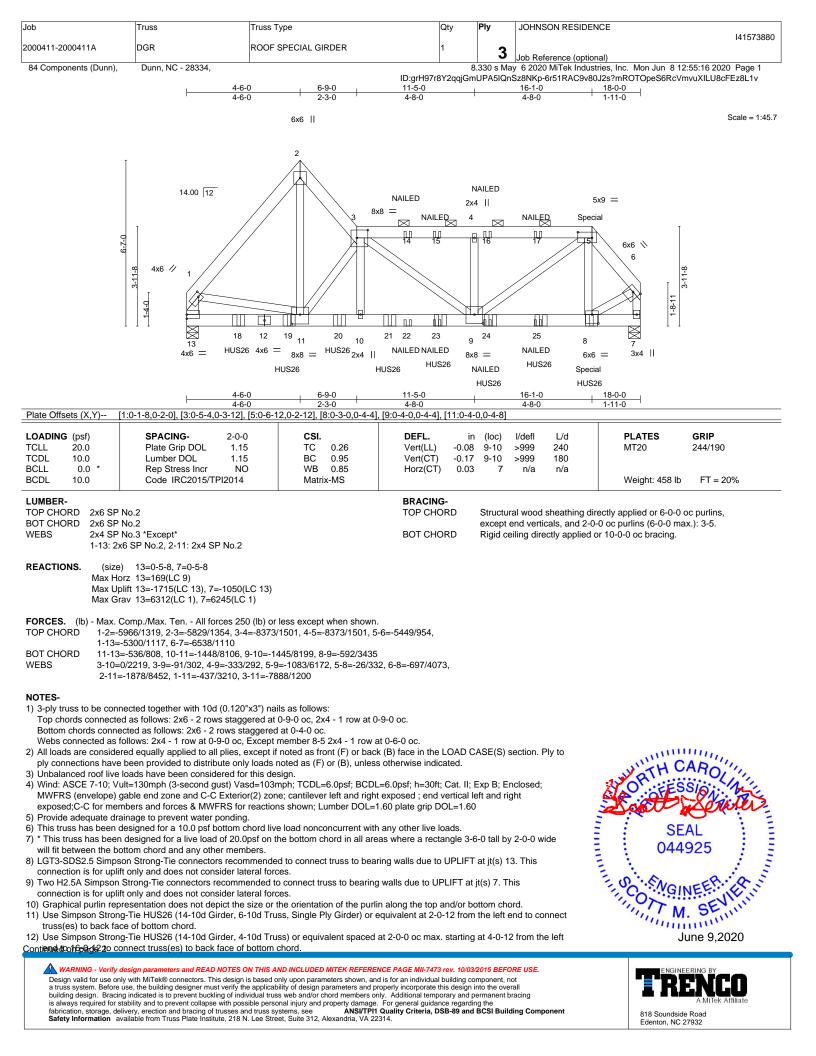
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 4, and 6. This connection is for uplift only and does not consider lateral forces.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE			
					I41573880			
2000411-2000411A	DGR	ROOF SPECIAL GIRDER	1	2				
				J	Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,		8	.330 s Ma	/ 6 2020 MiTek Industries, Inc. Mon Jun 8 12:55:16 2020 Page 2			
		ID:grH97r	ID:grH97r8Y2qqjGmUPA5IQnSz8NKp-6r51RAC9v80J2s?mROTOpeS6RcVmvuXILU8cFEz8L1v					

NOTES-

13) Fill all nail holes where hanger is in contact with lumber.

14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

 15) LGT2 Hurricane ties must have three studs in line below the truss.
 16) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 185 lb down and 213 lb up at 16-1-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

### LOAD CASE(S) Standard

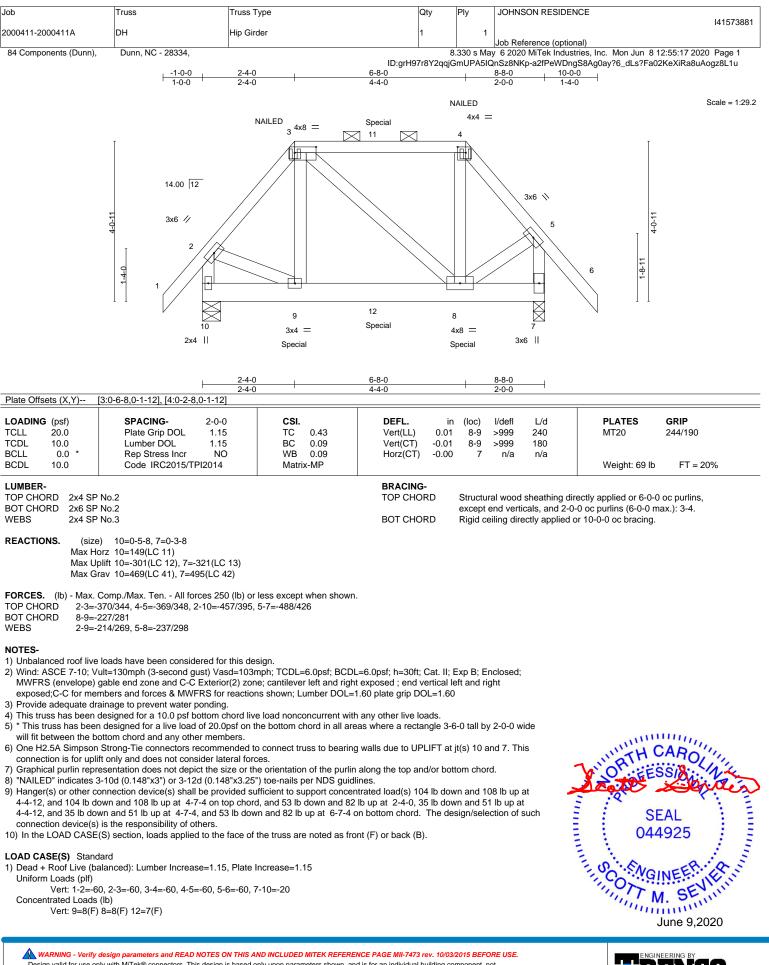
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-5=-60, 5-6=-60, 7-13=-20 Concentrated Loads (lb)

Vert: 8=-1387(F=9, B=-1396) 18=-1401(B) 19=-1396(B) 20=-1396(B) 21=-1396(B) 22=4(F) 23=-1392(F=4, B=-1396) 24=-1392(F=4, B=-1396) 25=-1392(F=4, B=-1392) 25=-1392(F=5, B=-1392) 25=-1392(F=5, B=-1392) 25=-1392(F=5, B=-1392) 25=-139 B=-1396)



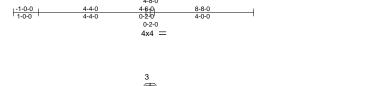


Design valid for use only using indicated is to prevent building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

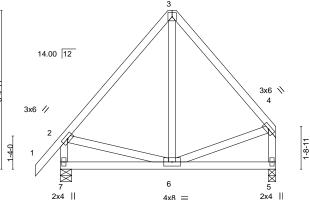
818 Soundside Road Edenton, NC 27932

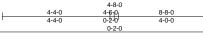


#### 8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 12:55:18 2020 Page 1 ID:grH97r8Y2qqjGmUPA5IQnSz8NKp-2EDnssDPRmG1HA98ZpVsu3YP5PNNN?TapodjK7z8L1t 480



Scale = 1:46.4





LOADIN	u /	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.51	Vert(LL)	-0.01	6-7	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.17	Vert(CT)	-0.02	6-7	>999	180		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.06	Horz(CT)	-0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-MP						Weight: 60 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=0-5-8, 5=0-3-8 Max Horz 7=196(LC 9) Max Uplift 7=-42(LC 13), 5=-45(LC 12) Max Grav 7=408(LC 1), 5=330(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-285/101, 3-4=-258/98, 2-7=-374/164, 4-5=-300/113

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

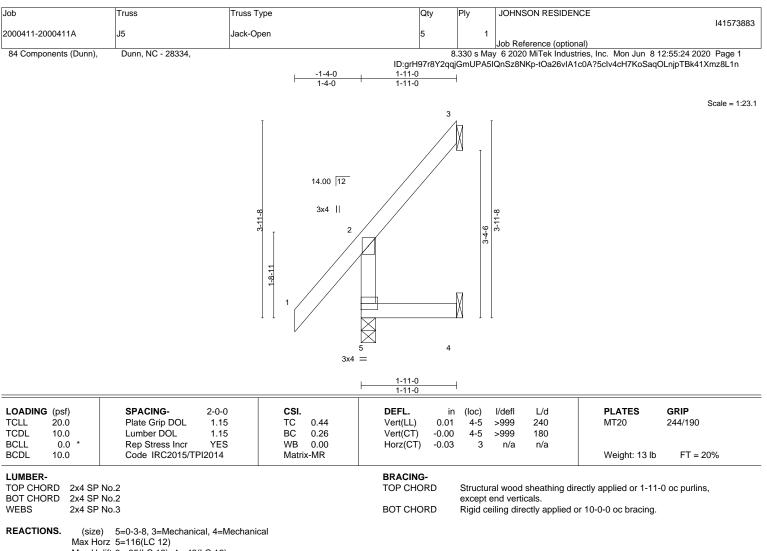
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.





<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



Max Uplift 3=-85(LC 12), 4=-43(LC 12) Max Grav 5=196(LC 1), 3=53(LC 19), 4=47(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

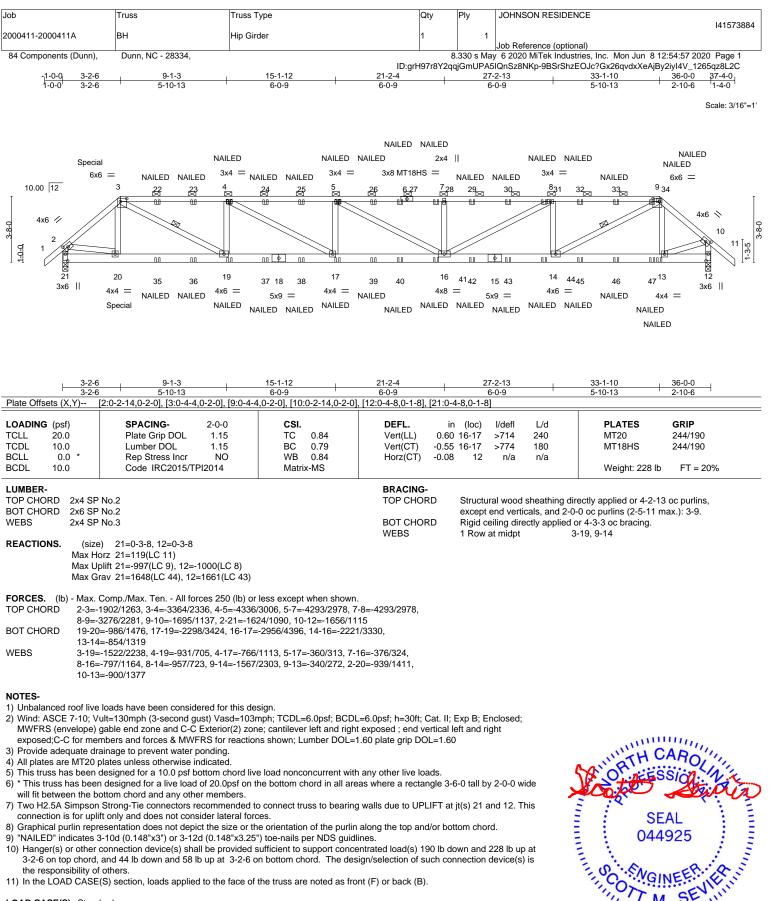
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 3 and 43 lb uplift at joint 4.







11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

#### Continued on page 2

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

minim

June 9,2020



Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE
					I41573884
2000411-2000411A	BH	Hip Girder	1	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	.330 s May	/ 6 2020 MiTek Industries, Inc. Mon Jun 8 12:54:57 2020 Page 2

8.330 s May 6 2020 MiTek Industries, Inc. Mon Jun 8 12:54:57 2020 Page 2 ID:grH97r8Y2qqjGmUPA5IQnSz8NKp-9BSrShzEOJc?Gx26qvdxXeAjBy2iyl4V\_1265qz8L2C

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

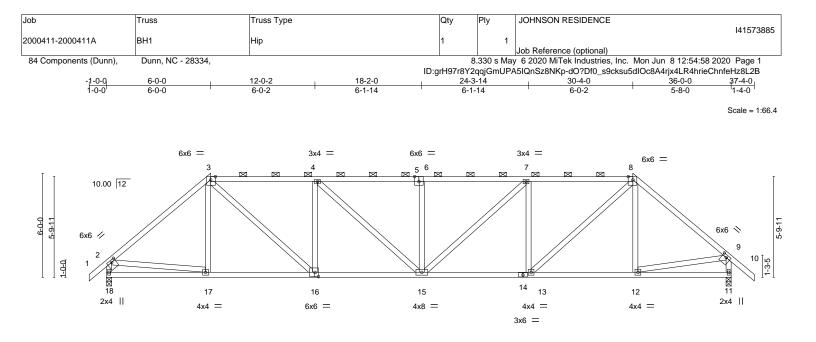
Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 3-9=-60, 9-10=-60, 10-11=-60, 12-21=-20

Concentrated Loads (lb)

Vert: 20=4(B) 19=1(B) 17=1(B) 13=2(B) 35=1(B) 36=1(B) 37=1(B) 38=1(B) 39=1(B) 40=1(B) 41=1(B) 42=1(B) 43=1(B) 44=1(B) 45=1(B) 46=1(B) 47=1(B) 46=1(B) 47=1(B) 46=1(B) 46=1(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





F	6-0-0 6-0-0	12-0-2 6-0-2	18-2-0	24-3-1		<u>30-4-0</u> 6-0-2	36-0- 5-8-	
Plate Offsets (X,Y)			[5:0-0-0,0-1-12], [6:0-1-12]		-			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/	1.15 T 1.15 E YES V	C 0.64 V 8C 0.59 V	EFL. in ert(LL) -0.13 ert(CT) -0.26 1 orz(CT) 0.07	(loc) l/defl 15 >999 15-16 >999 11 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 221 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2x4 S WEBS 2x4 S REACTIONS. (si Max Max	P No.2 P No.2 P No.3 ze) 18=0-3-8, 11=0-3-8 Horz 18=176(LC 11) Uplift 18=-148(LC 9), 11= Grav 18=1496(LC 1), 11=	=-152(LC 8)	T		except end verti	cals, and 2-0-0	ttly applied or 3-10-3 oc purlins (3-6-1 ma 9-8-7 oc bracing.	
TOP CHORD         2-3           8-9         8-9           BOT CHORD         17-           WEBS         3-1	=-1673/345, 2-18=-1441/3 18=-219/395, 16-17=-237	97, 4-6=-2342/546, 6-7=- 340, 9-11=-1467/353 //1273, 15-16=-368/2089 247, 4-15=-92/365, 6-15=	-2342/546, 7-8=-2060/493 , 13-15=-319/2058, 12-13 =-360/167, 7-15=-96/403,	,				

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

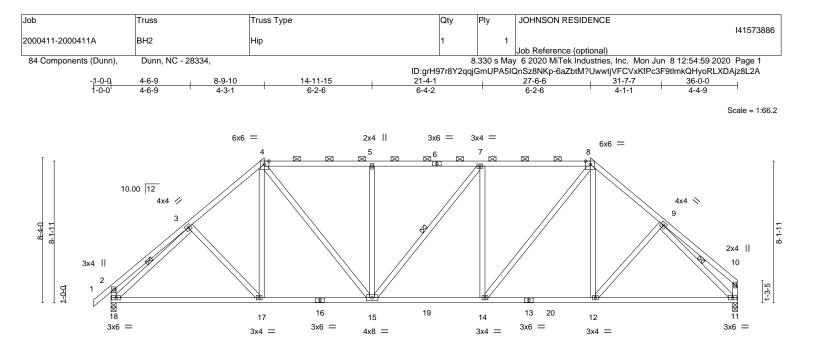
6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 18 and 11. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



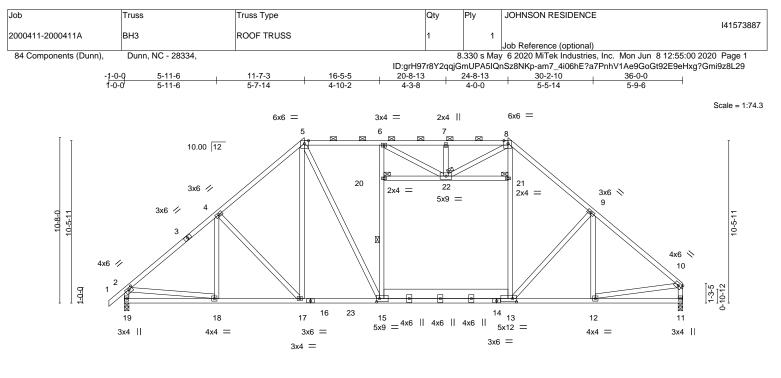


F	8-9-10	14-11-15	21-4-1	27-6-6	36-0-0	
	8-9-10	6-2-6	6-4-2	6-2-6	8-5-10	
Plate Offsets (X,Y)	[4:0-3-4,Edge], [8:0-3-4,Edge]	1	1			_
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.51 BC 0.69 WB 0.56 Matrix-MS		(loc) l/defl L/d 17-18 >999 240 17-18 >999 180 11 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 242 lb         FT = 20%	
Max Ho Max Up	No.2		BRACING- TOP CHORD BOT CHORD WEBS	except end verticals, and 2-0- Rigid ceiling directly applied of	ectly applied or 4-4-8 oc purlins, 0 oc purlins (3-11-15 max.): 4-8. r 10-0-0 oc bracing. -15, 3-18, 9-11	
TOP CHORD 2-3=- 8-9=- BOT CHORD 17-18 11-12 WEBS 4-17=	Comp./Max. Ten All forces 250 (lb) of 394/151, 3-4=-1637/398, 4-5=-1623/44 1602/394, 9-10=-261/109, 2-18=-397/1 =-259/1225, 15-17=-208/1213, 14-15= -206/1155 -40/301, 4-15=-200/711, 5-15=-413/18 -43/258, 3-18=-1429/250, 9-11=-1522/	3, 5-7=-1622/442, 7-8=-16 71 219/1629, 12-14=-118/11 4, 7-14=-464/229, 8-14=-2	90,			
<ol> <li>Wind: ASCE 7-10; V MWFRS (envelope) exposed;C-C for mer</li> <li>Provide adequate dr.</li> <li>This truss has been will fit between the build for the</li></ol>	loads have been considered for this de ult=130mph (3-second gust) Vasd=103 gable end zone and C-C Exterior(2) zon mbers and forces & MWFRS for reactio ainage to prevent water ponding. designed for a 10.0 psf bottom chord lin n designed for a live load of 20.0psf on ottom chord and any other members, w Strong-Tie connectors recommended for for only and does not consider lateral for esentation does not depict the size or the	mph; TCDL=6.0psf; BCDL ne; cantilever left and right ns shown; Lumber DOL=1 re load nonconcurrent with the bottom chord in all are ith BCDL = 10.0psf. o connect truss to bearing ces.	t exposed ; end vertical le 1.60 plate grip DOL=1.60 n any other live loads. eas where a rectangle 3-6 g walls due to UPLIFT at j	ft and right -0 tall by 2-0-0 wide t(s) 18 and 11. This	SEAL	A



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





	5-11-6 11-7 5-11-6 5-7-		24-8-13	30-2-10	36-0-0	4
Plate Offsets (X,Y)	[2:0-2-12,0-1-8], [5:0-3-4,Edge], [8:0-3-4			0014	000	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.75 BC 0.93 WB 0.93 Matrix-MS	Vert(LL) -0.52 Vert(CT) -0.91 Horz(CT) 0.03	(loc) I/defi L/d 15-17 >819 240 15-17 >471 180 11 n/a n/a 13-15 275 360		<b>GRIP</b> 244/190 FT = 20%
BOT CHORD         2x4 S         13-15           WEBS         2x4 S         5-17,           REACTIONS.         (si	P No.2 P DSS *Except* : 2x8 SP No.2 P No.3 *Except* 6-15,8-13: 2x4 SP No.1 ze) 19=0-3-8, 11=0-3-8	· · · · ·	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Structural wood sheathing dii except end verticals, and 2-0 Rigid ceiling directly applied of 2-2-0 oc bracing: 12-13. 1 Row at midpt 1 1 Brace at Jt(s): 20, 21, 22	-0 oc purlins (3-4-5 max.)	: 5-8.
Max Max FORCES. (Ib) - Max TOP CHORD 2-4: 8-9: BOT CHORD 18- WEBS 4-1: 13-2	Horz 19=281(LC 9) Uplift 19=-15(LC 12) Grav 19=1635(LC 1), 11=1614(LC 1) C. Comp./Max. Ten All forces 250 (lb) or -1935/192, 4-5=-1796/262, 5-6=-1485/22 -1902/213, 9-10=-1890/136, 2-19=-1567 19=-300/445, 17-18=-97/1473, 15-17=-45 7=-306/224, 5-17=-119/512, 5-15=-90/50 21=-42/637, 8-21=0/662, 9-13=-224/277, 2=-171/593, 7-22=-312/136	27, 6-7=-1682/290, 7-8=-16 /214, 10-11=-1527/122 /1313, 13-15=0/1448, 12-1 I, 15-20=-425/210, 6-20=-3	3=-25/1387 334/219,			

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

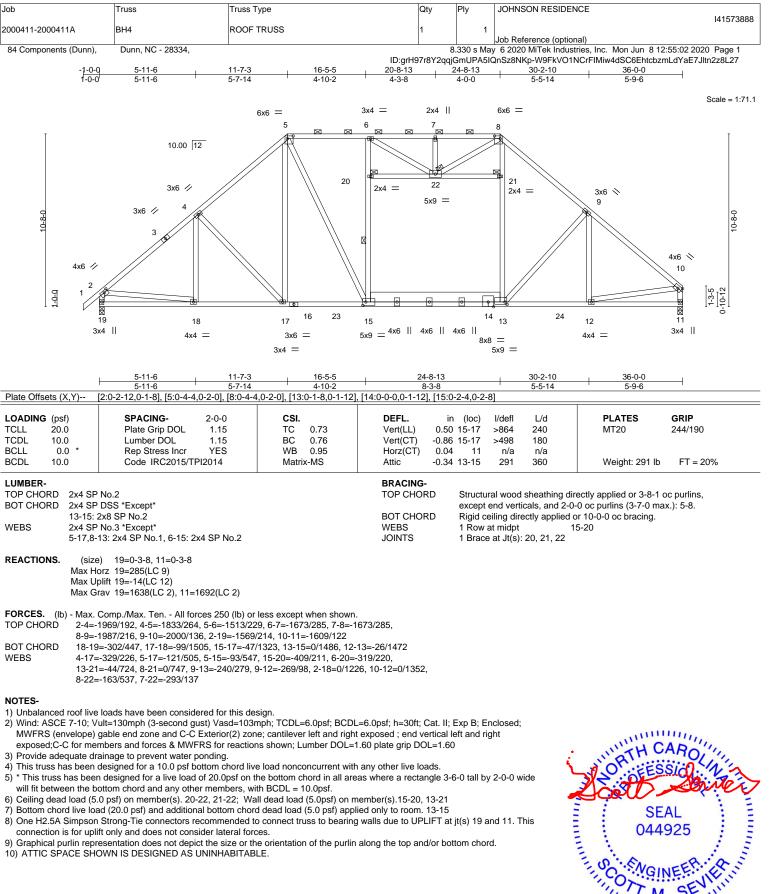
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Ceiling dead load (5.0 psf) on member(s). 20-22, 21-22; Wall dead load (5.0 psf) on member(s). 15-20, 13-21
- 7) Bottom chord live load (20.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 13-15
- 8) One H2.5A Simpson Strong Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19. This connection is for uplift only and does not consider lateral forces.

Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

SEAL 044925 MGINEEP, HERMIN June 9,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSIVTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

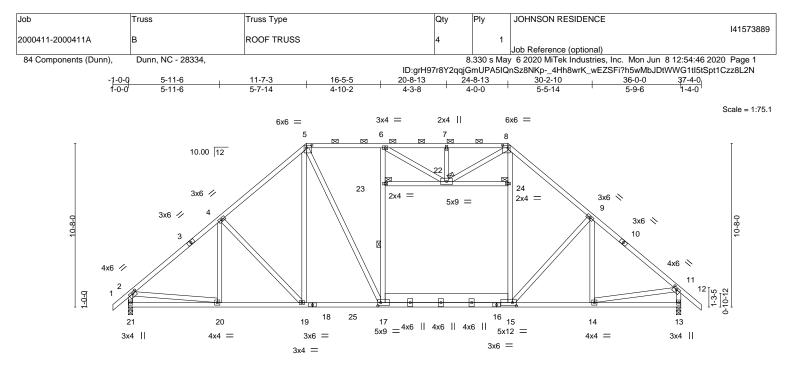
10) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

mann June 9,2020

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid for dise only with with every connectors. This design is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road

Edenton, NC 27932



	<u>5-11-6</u> <u>11-7-</u> 5-11-6 <u>5-7-1</u>		24-8-13	30-2-10	36-0-0	4
Plate Offsets (X,Y)-	- [2:0-2-12,0-1-8], [5:0-4-4,0-2-0], [8:0-4-	4,0-2-0], [11:0-2-12,0-1-8],	[15:0-3-0,0-2-8], [17:0-2			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.74 BC 0.93 WB 0.98 Matrix-MS	Vert(LL) -0.52 Vert(CT) -0.89 Horz(CT) 0.03	n (loc) l/defl L/d 2 17-19 >827 240 0 17-19 >479 180 8 13 n/a n/a 0 15-17 275 360	PLATES MT20 Weight: 294 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2x4 15- WEBS 2x4	SP No.2 SP DSS *Except* 17: 2x8 SP No.2 I SP No.3 *Except* 9,8-15: 2x4 SP No.1, 6-17: 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Structural wood sheathing except end verticals, and 2 Rigid ceiling directly applie 2-2-0 oc bracing: 14-15. 1 Row at midpt 1 Brace at Jt(s): 22, 23, 24	2-0-0 oc purlins (3-4-13 m d or 10-0-0 oc bracing, 17-23	nax.): 5-8.
Ma Ma	(size) 21=0-3-8, 13=0-3-8 IX Horz 21=295(LC 11) IX Uplift 21=-15(LC 12) IX Grav 21=1633(LC 1), 13=1704(LC 1)					
TOP CHORD 2	lax. Comp./Max. Ten All forces 250 (lb) o -4=-1933/191, 4-5=-1794/263, 5-6=-1457/2 -9=-1895/214. 9-11=-1886/139, 2-21=-1568	29, 6-7=-1617/285, 7-8=-16				
BOT CHORD 2 WEBS 4	-19=-329/2462, 19-20=-87/1489, 17-19=-38 -19=-329/226, 5-19=-121/528, 5-17=-93/48 -22=-163/537, 15-24=-41/643, 8-24=0/667.	5/1302, 15-17=0/1425, 14-1 6, 17-23=-408/211, 6-23=-3	318/220,			

### 8-22=-163/537, 15-24=-41/643, 8-24=0/667, 9-15=-233/276, 9-14=-295/99, 2-20=0/1194, 11-14=0/1239, 7-22=-293/137

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Ceiling dead load (5.0 psf) on member(s). 22-23, 22-24; Wall dead load (5.0 psf) on member(s). 17-23, 15-24
- 7) Bottom chord live load (20.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 15-17
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 21 and 13. This connection is for uplift only and does not consider lateral forces.

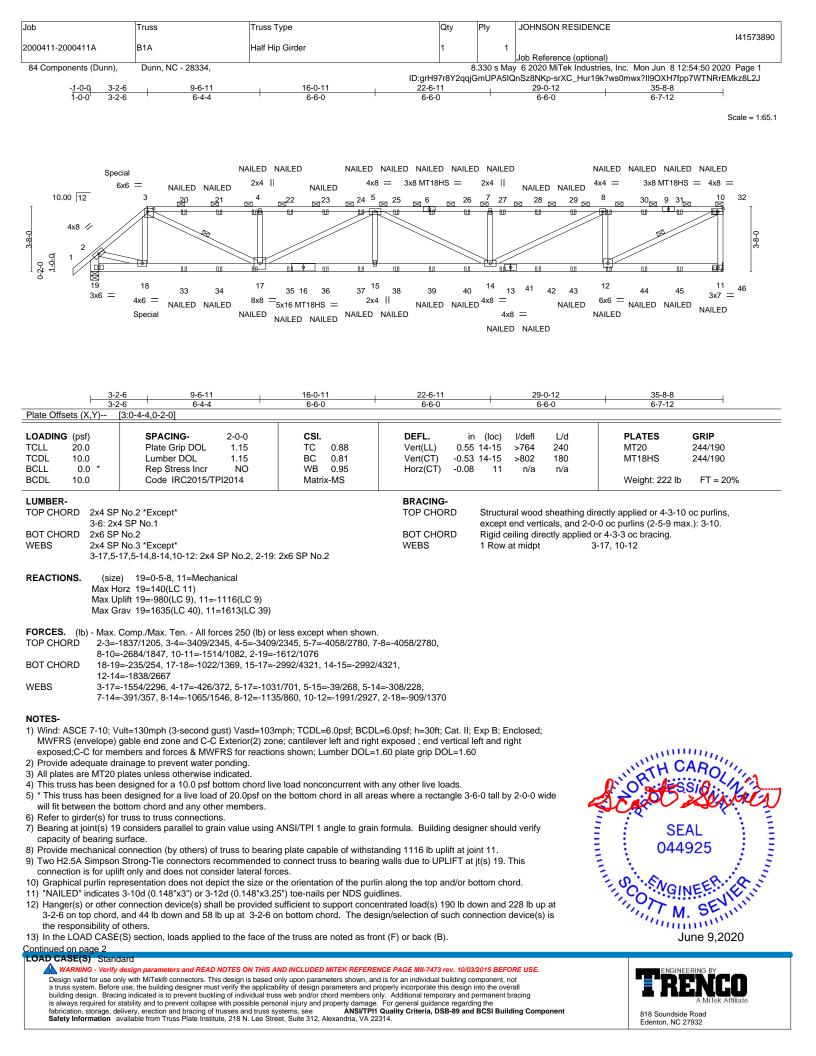
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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Job	Truss	Truss Type	Qty	Ply	JOHNSON RESIDENCE
					141573890
2000411-2000411A	B1A	Half Hip Girder	1	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	.330 s May	/ 6 2020 MiTek Industries, Inc. Mon Jun 8 12:54:50 2020 Page 2

ID:grH97r8Y2qqjGmUPA5lQnSz8NKp-srXC\_Hur19k?ws0mwx?ll9OXH7fpp7WTNRrEMkz8L2J

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

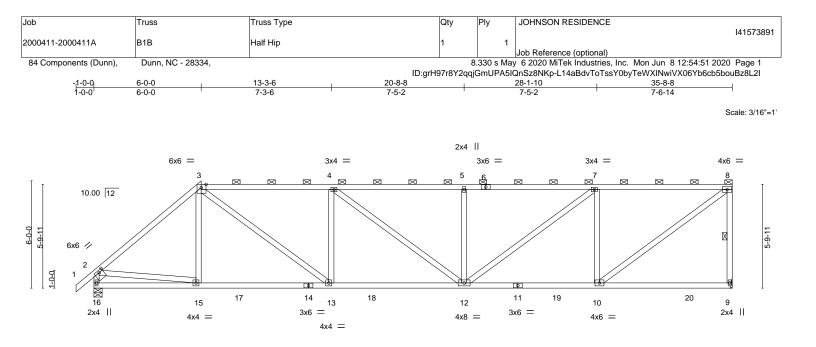
Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 3-10=-60, 11-19=-20

Concentrated Loads (lb)

Vert: 18=4(F) 17=1(F) 12=1(F) 32=-0(F) 33=1(F) 34=1(F) 35=-3(F) 36=-3(F) 37=-3(F) 38=-3(F) 39=-3(F) 40=-3(F) 41=-3(F) 42=-3(F) 43=1(F) 44=1(F) 45=1(F) 46=-2(F)

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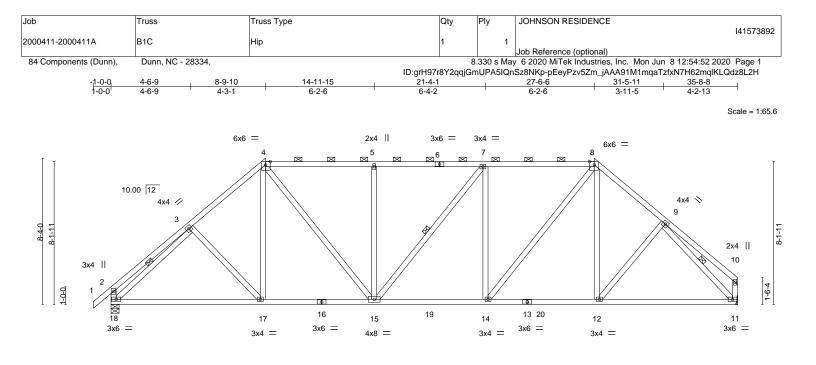
L		3-3-6	20-8-8	28-1-10	35-8-8
	6-0-0	7-3-6	7-5-2	7-5-2	7-6-14
Plate Offsets (X,Y)	[2:0-2-12,0-1-8], [3:0-3-4,Edge]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.85 BC 0.74 WB 0.80 Matrix-MS	Vert(LL) -0.13	n (loc) l/defl L/d 12-13 >999 240 12-13 >999 180 5 9 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 212 lb         FT = 20%
			BRACING- TOP CHORD BOT CHORD WEBS	except end verticals, and 2-0- Rigid ceiling directly applied c	ectly applied or 3-10-15 oc purlins, -0 oc purlins (2-11-6 max.): 3-8. or 7-10-7 oc bracing. -9
Max L	e) 9=Mechanical, 16=0-5-8 łorz 16=222(LC 9) Jplift 9=-267(LC 9), 16=-159(LC 9) Grav 9=1416(LC 1), 16=1487(LC 1				
TOP CHORD 2-3= 8-9=	Comp./Max. Ten All forces 250 -1758/346, 3-4=-2190/514, 4-5=-2 -1348/331, 2-16=-1435/335	71/528, 5-7=-2271/528, 7-8=-	1581/393,		
WEBS 3-13	6=-431/549, 13-15=-405/1269, 12- =-278/1197, 4-13=-575/260, 5-12= =-413/1939, 2-15=-216/1069				
<ol> <li>Wind: ASCE 7-10; MWFRS (envelope) exposed;C-C for me</li> <li>Provide adequate d</li> <li>This truss has been</li> <li>* This truss has been</li> <li>will fit between the b</li> <li>Refer to girder(s) fo</li> <li>Provide mechanical</li> <li>One H2.5A Simpson</li> </ol>	e loads have been considered for t /ult=130mph (3-second gust) Vasc gable end zone and C-C Exterior( mbers and forces & MWFRS for rr rainage to prevent water ponding. designed for a 10.0 psf bottom ch in designed for a live load of 20.0p pottom chord and any other member r truss to truss connections. connection (by others) of truss to in Strong-Tie connectors recommer lift only and does not consider late	=103mph; TCDL=6.0psf; BCD ) zone; cantilever left and rigit actions shown; Lumber DOL= rd live load nonconcurrent wit f on the bottom chord in all ar rs, with BCDL = 10.0psf. earing plate capable of withst ded to connect truss to bearin	ht exposed ; end vertical =1.60 plate grip DOL=1.60 th any other live loads. reas where a rectangle 3- tanding 267 lb uplift at join	eft and right ) 6-0 tall by 2-0-0 wide nt 9.	ORTH CARO

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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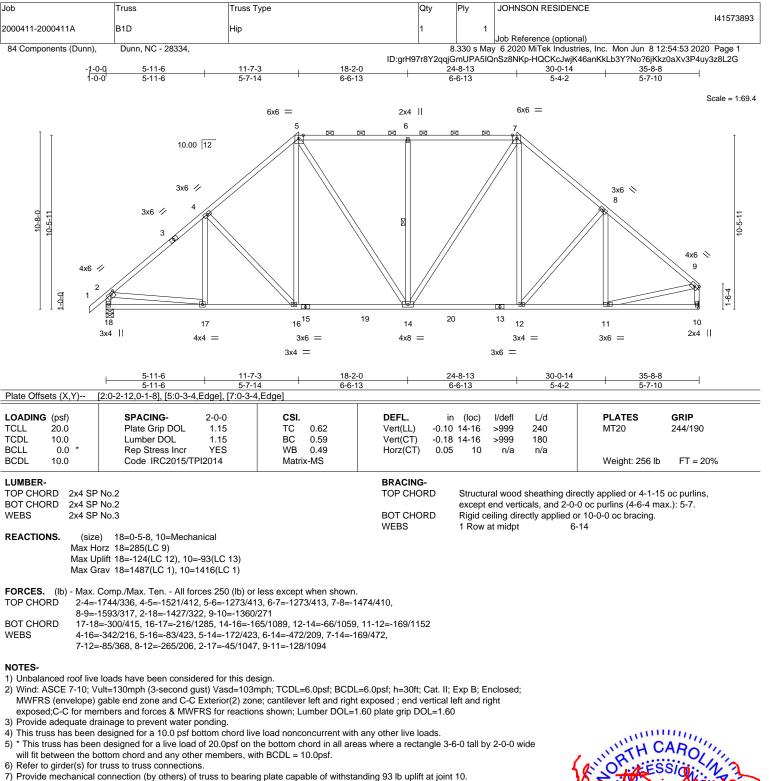


F	<u>8-9-10</u>	14-11-15	<u>21-4-1</u> 6-4-2	27-6-6	35-8-8
Plate Offsets (X,Y)	[4:0-3-4,Edge], [8:0-3-4,Edge]	6-2-6	0-4-2	0-2-0	8-2-2
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	<b>CSI.</b> TC 0.51 BC 0.69 WB 0.57 Matrix-MS	Vert(LL) -0.14	(loc) I/defl L/d 17-18 >999 240 17-18 >999 180 11 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 241 lb         FT = 20%
Max Ho Max Up	No.2		BRACING- TOP CHORD BOT CHORD WEBS		directly applied or 4-4-13 oc purlins, 2-0-0 oc purlins (4-0-4 max.): 4-8. ed or 10-0-0 oc bracing. 7-15, 3-18, 9-11
TOP CHORD 2-3=-: 8-9=- BOT CHORD 17-18 11-12 WEBS 4-17=	Comp./Max. Ten All forces 250 (lb) or 393/151, 3-4=-1620/394, 4-5=-1602/439 1545/387, 2-18=-396/171 =-263/1213, 15-17=-211/1200, 14-15=- =-198/1079 -40/302, 4-15=-199/696, 5-15=-413/184 -1415/247, 9-11=-1516/287	9, 5-7=-1600/437, 7-8=-16 222/1598, 12-14=-122/11	00/436, 50,		
<ol> <li>Wind: ASCE 7-10; Vi MWFRS (envelope) (exposed;C-C for mer</li> <li>Provide adequate dra</li> <li>This truss has been</li> <li>This truss has been</li> <li>* This truss has been</li> <li>Refer to girder(s) for</li> <li>Provide mechanical distribution</li> <li>One H2.5A Simpson connection is for upli</li> </ol>	loads have been considered for this de ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Exterior(2) zor ainage to prevent water ponding. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on t ottom chord and any other members, wi truss to truss connections. connection (by others) of truss to bearin Strong-Tie connectors recommended t ft only and does not consider lateral for esentation does not depict the size or th	mph; TCDL=6.0psf; BCDL he; cantilever left and right ns shown; Lumber DOL=1 e load nonconcurrent with the bottom chord in all are ith BCDL = 10.0psf. hg plate capable of withsta o connect truss to bearing ces.	exposed ; end vertical le .60 plate grip DOL=1.60 any other live loads. as where a rectangle 3-6 nding 75 lb uplift at joint walls due to UPLIFT at	-0 tall by 2-0-0 wide	SEAL 044925

inin. M. SEVIET SCO

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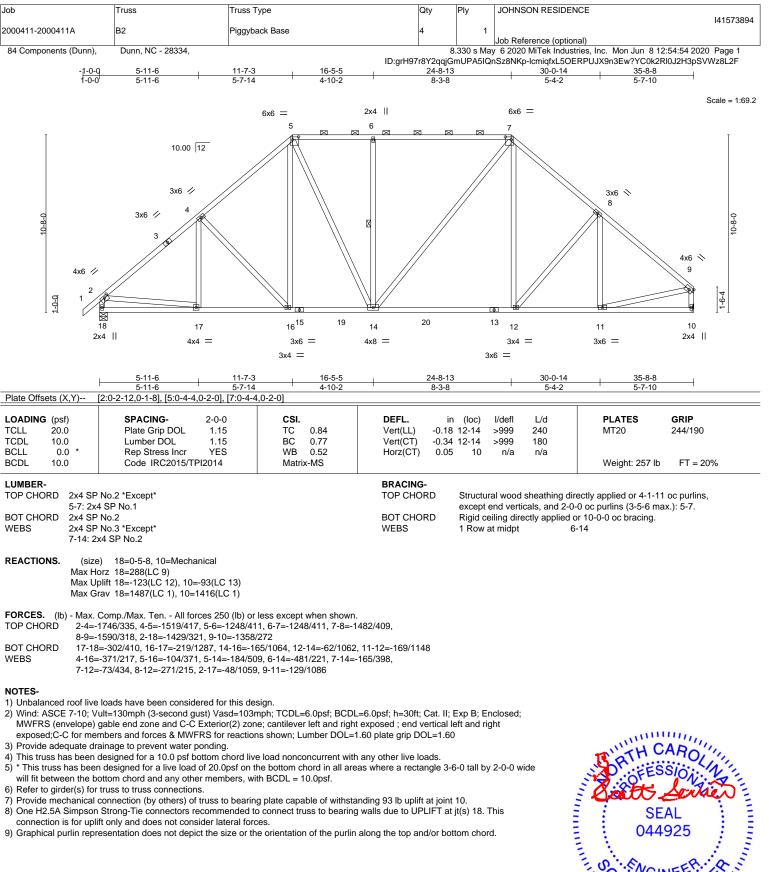
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 to upint at joint 10.
   One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 18. This
- connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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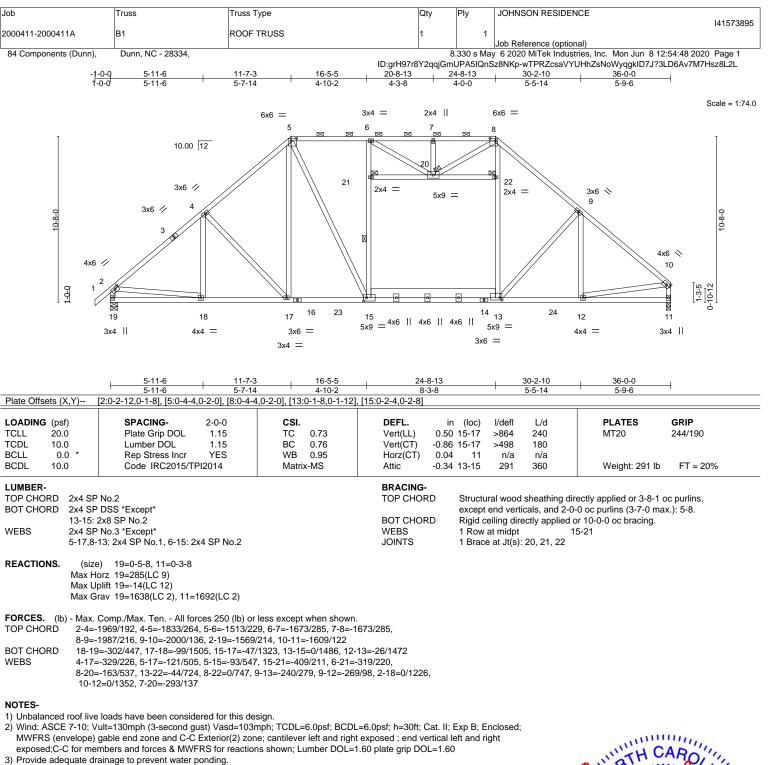
Edenton, NC 27932





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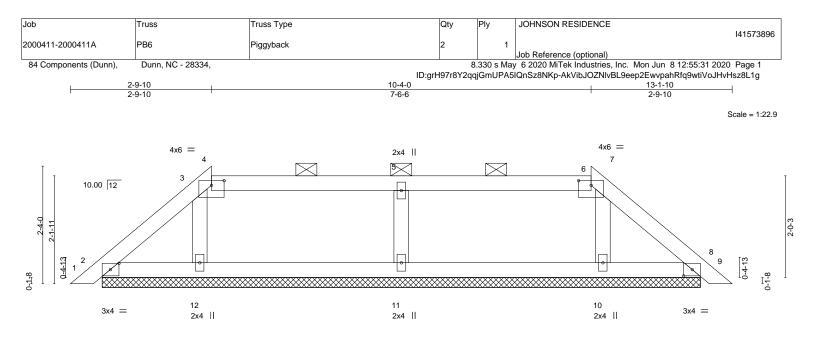


- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Ceiling dead load (5.0 psf) on member(s). 20-21, 20-22; Wall dead load (5.0 psf) on member(s).15-21, 13-22
- 7) Bottom chord live load (20.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 13-15
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19 and 11. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



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			13-1-10 13-1-10						
Plate Offsets (X,Y)	[2:0-2-1,0-1-8], [3:0-1-12,0-1-7], [4:0-3-0	,0-1-2], [4:0-0-0,0-1-2], [6:		-3-0,0-1	1-2], [7:0	)-1-12,0-	1-7], [8:0-2-1	1,0-1-8]	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.21 BC 0.12 WB 0.05 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 8 8 8	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 46 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP OTHERS 2x4 SP	No.2		BRACING- TOP CHOR BOT CHOR		2-0-0 o	c purlins	(6-0-0 max.)	irectly applied or 6-0-0 ): 4-6. or 10-0-0 oc bracing.	) oc purlins, except
(lb) - Max H Max U	earings 11-10-7. orz 2=51(LC 11) plift All uplift 100 lb or less at joint(s) 2, rav All reactions 250 lb or less at joint(		853(LC 23)						

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## WEBS

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5-11=-268/152

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

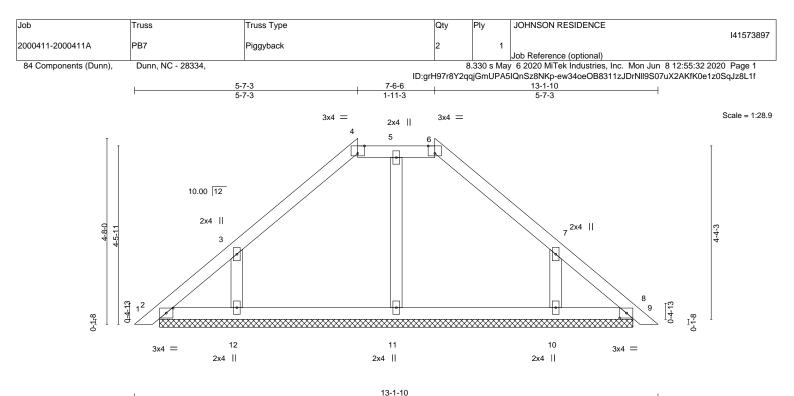
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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<sup>7)</sup> n/a



13-1-10 Plate Offsets (X,Y)--[2:0-2-1,0-1-8], [4:0-2-0, Edge], [6:0-2-0, Edge], [8:0-2-1, 0-1-8] DEFL. GRIP LOADING (psf) SPACING-2-0-0 CSI. in (loc) l/defl L/d PLATES TCLL 20.0 Plate Grip DOL 1.15 тс 0.13 Vert(LL) 0.00 120 MT20 244/190 8 n/r TCDL 10.0 Lumber DOL 1.15 BC 0.13 Vert(CT) 0.00 8 n/r 90 BCLL 0.0 Rep Stress Incr YES WВ 0.05 Horz(CT) 8 0.00 n/a n/a Code IRC2015/TPI2014 BCDL Matrix-S Weight: 52 lb FT = 20% 10.0 LUMBER-BRACING-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except

BOT CHORD

2-0-0 oc purlins (6-0-0 max.): 4-6.

Rigid ceiling directly applied or 10-0-0 oc bracing

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

**REACTIONS.** All bearings 11-10-7.

(lb) - Max Horz 2=-108(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8 except 12=-131(LC 12), 10=-129(LC 13) Max Grav All reactions 250 lb or less at joint(s) 2, 8, 11 except 12=283(LC 19), 10=281(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

7) n/a

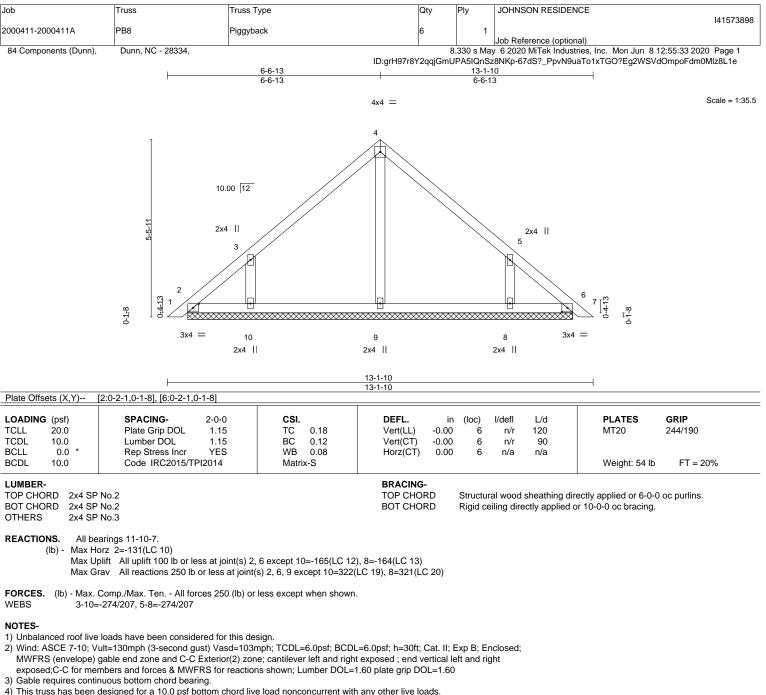
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

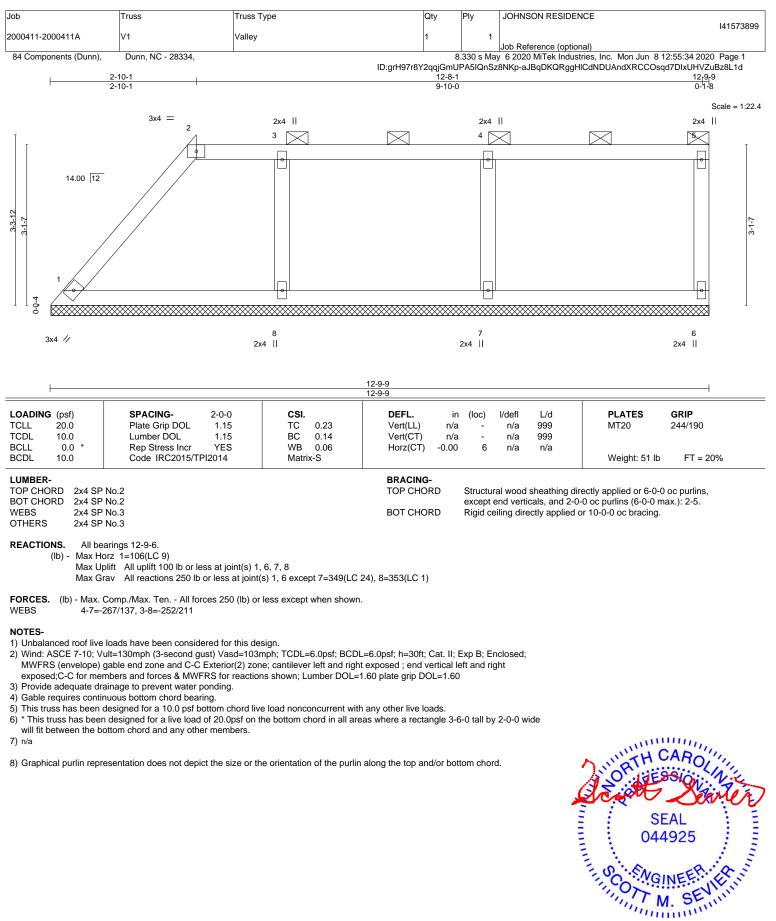
6) n/a

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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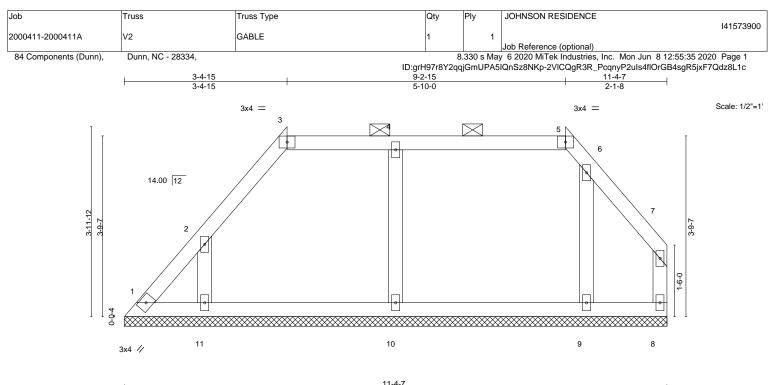




June 9,2020

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		1	11-4-7				I	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc		L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.19 BC 0.12	Vert(LL) Vert(CT)	n/a n/a	n/a n/a	999 999	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.07 Matrix-S	Horz(CT) -	-0.00	8 n/a	n/a	Weight: 51 lb	FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS OTHERS 2x4 SP No.3

REACTIONS. All bearings 11-4-7.

(lb) -Max Horz 1=106(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 1, 8, 10, 9 except 11=-102(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 8, 9 except 10=328(LC 23), 11=258(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) n/a

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

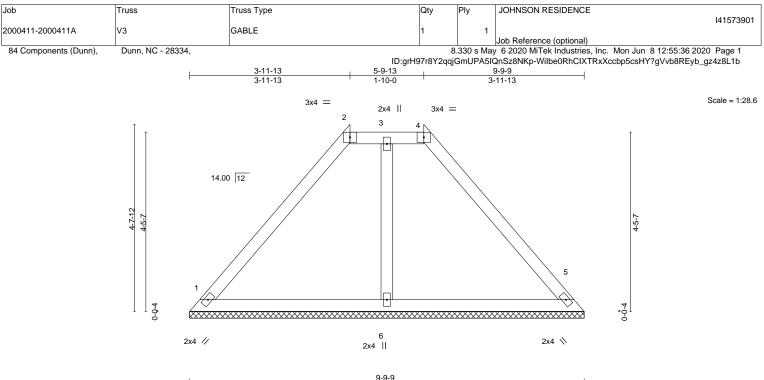


except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-5.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





			9-9-9			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.22	Vert(LL)	n/a -	n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.21	Vert(CT)	n/a -	n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02	Horz(CT)	0.00 5	n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	. ,			Weight: 40 lb FT = 20%
		1				<u> </u>
LUMBER-			BRACING-			

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. 1=9-9-9, 5=9-9-9, 6=9-9-9 (size) Max Horz 1=108(LC 9)

Max Uplift 1=-72(LC 13), 5=-77(LC 13)

Max Grav 1=280(LC 1), 5=280(LC 1), 6=236(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-2=-275/175, 4-5=-275/175

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6)

will fit between the bottom chord and any other members.

7) n/a

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



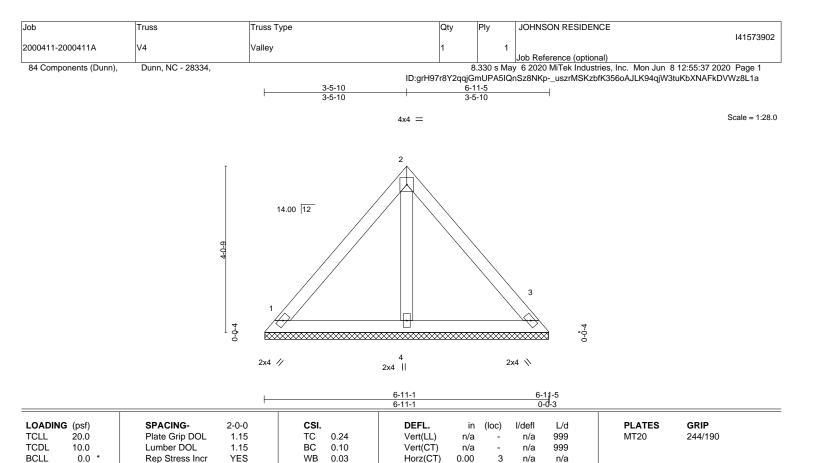
Structural wood sheathing directly applied or 6-0-0 oc purlins, except

2-0-0 oc purlins (6-0-0 max.): 2-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-
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BCDL

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

10.0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 30 lb

FT = 20%

REACTIONS. (size) 1=6-10-14, 3=6-10-14, 4=6-10-14 Max Horz 1=-95(LC 8) Max Uplift 1=-44(LC 13), 3=-35(LC 12)

Max Grav 1=158(LC 1), 3=158(LC 1), 4=194(LC 3)

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

Matrix-P

- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right
- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

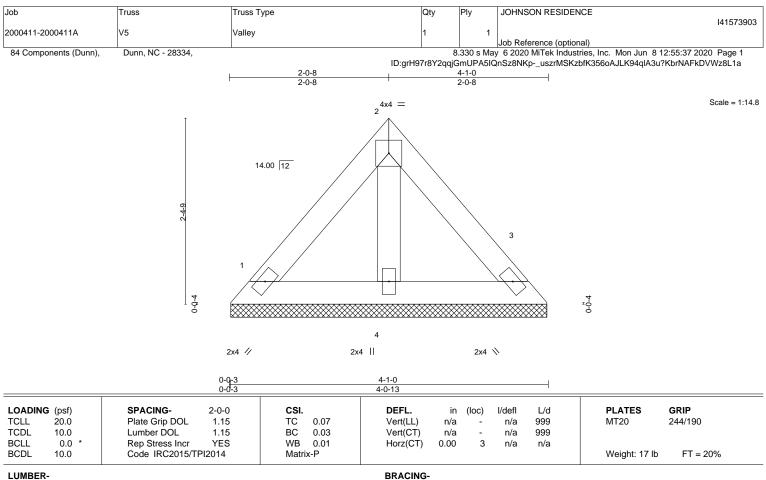
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

# SEAL 044925 VGINEERIE

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-1-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. 1=4-0-9, 3=4-0-9, 4=4-0-9 (size) Max Horz 1=-52(LC 8) Max Uplift 1=-24(LC 13), 3=-19(LC 12) Max Grav 1=87(LC 1), 3=87(LC 1), 4=106(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right
- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

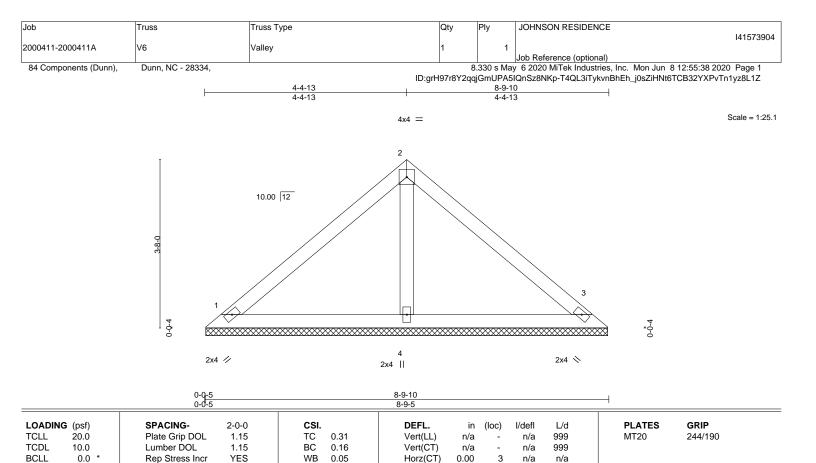
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BRACING-TOP CHORD

BOT CHORD

LUMBER	•
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BCDL

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

10.0

2x4 SP No.3 OTHERS

REACTIONS. 1=8-9-0, 3=8-9-0, 4=8-9-0 (size) Max Horz 1=82(LC 11) Max Uplift 1=-38(LC 13), 3=-49(LC 13) Max Grav 1=183(LC 1), 3=183(LC 1), 4=273(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

Matrix-P

- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right
- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

# VIIII III IIII SEAL 044925 mm June 9,2020

Weight: 33 lb

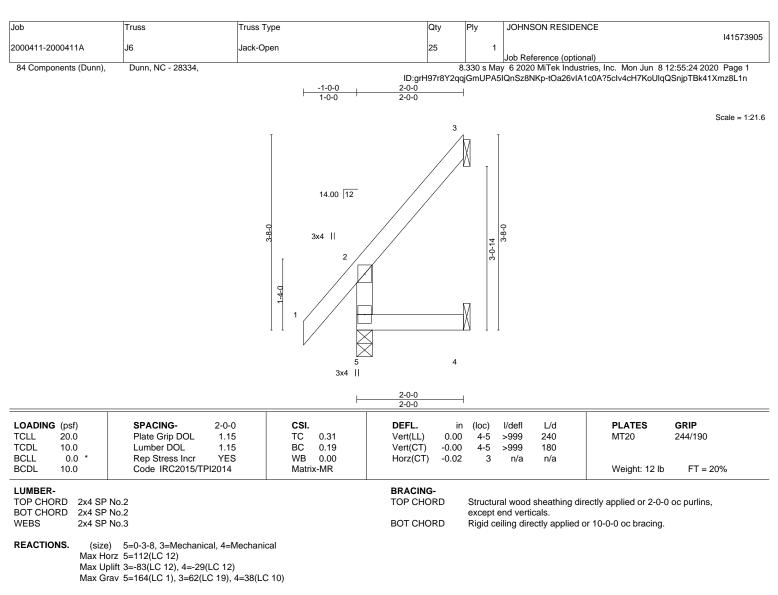
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

FT = 20%

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

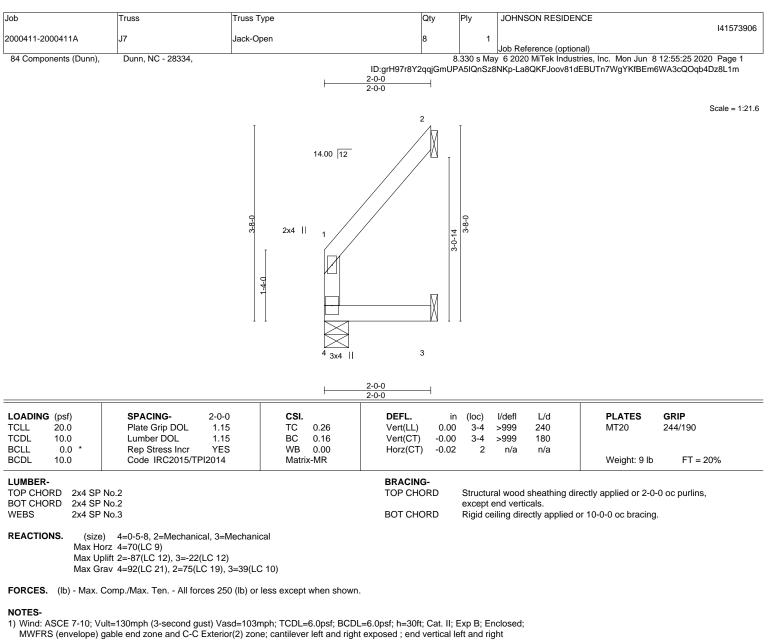
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 3 and 29 lb uplift at joint 4.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

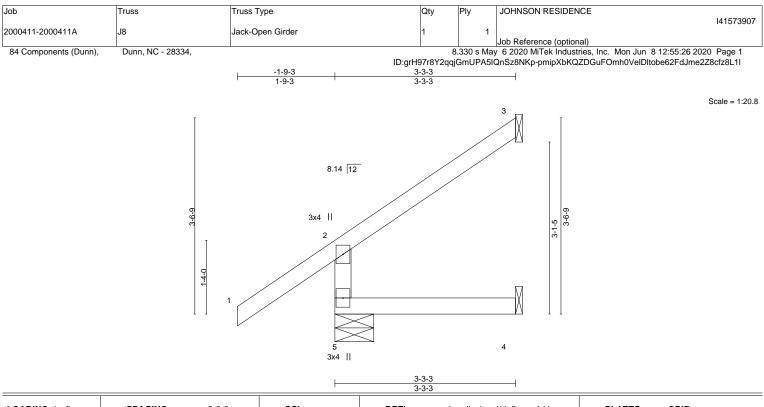
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 2 and 22 lb uplift at joint 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocllapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LOADING (psf)	SPACING- 2-0-0	<b>CSI.</b>	<b>DEFL.</b> in	(loc)	l/defl	L/d	PLATES         GRIP           MT20         244/190
TCLL 20.0	Plate Grip DOL 1.15	TC 0.35	Vert(LL) 0.01	4-5	>999	240	
TCDL 10.0	Lumber DOL 1.15	BC 0.18	Vert(CT) -0.01	4-5	>999	180	
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NO Code IRC2015/TPI2014	WB 0.00 Matrix-MR	Horz(CT) -0.02	3	n/a	n/a	Weight: 16 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD WEBS

2x4 SP No.3

REACTIONS. 5=0-8-7, 3=Mechanical, 4=Mechanical (size) Max Horz 5=108(LC 12) Max Uplift 5=-15(LC 12), 3=-66(LC 12), 4=-3(LC 12) Max Grav 5=273(LC 1), 3=77(LC 19), 4=55(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 3-3-3 oc purlins,

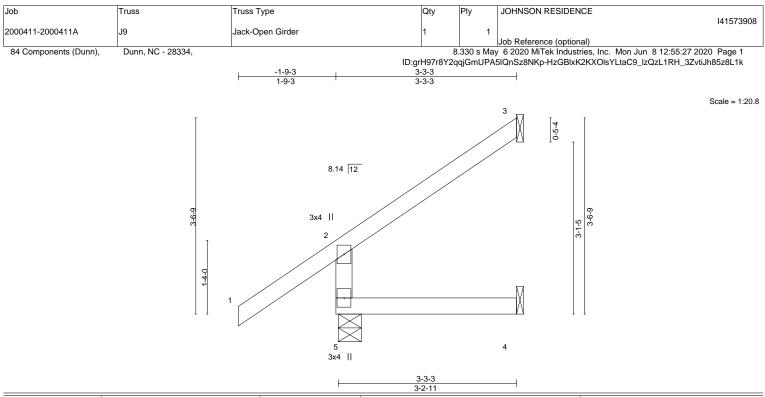
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REPERENCE PAGE MIL-14's rev. Invozens Derrore USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



<sup>5)</sup> Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 3 and 3 lb uplift at ioint 4.



-						02						
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.35	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	-0.01	4-5	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	-0.02	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-MR						Weight: 16 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

WEBS 2x4 SP No.3

REACTIONS. 5=0-5-0, 3=Mechanical, 4=Mechanical (size) Max Horz 5=108(LC 12) Max Uplift 5=-15(LC 12), 3=-66(LC 12), 4=-3(LC 12) Max Grav 5=273(LC 1), 3=77(LC 19), 4=55(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

<sup>6)</sup> One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 3-3-3 oc purlins,

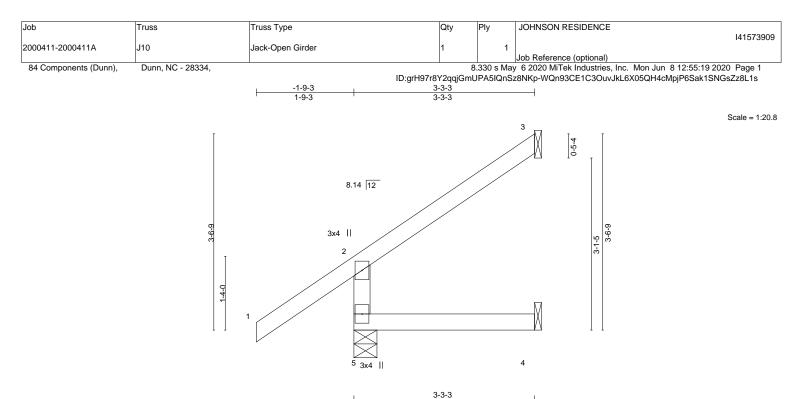
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REPERENCE PAGE MIL-14's rev. Invozens Derrore USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



<sup>5)</sup> Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 3 and 3 lb uplift at ioint 4.



						3-3-	.3		1			
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.35	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	-0.01	4-5	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	-0.02	3	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MR						Weight: 16 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS. (size) 5=0-5-0, 3=Mechanical, 4=Mechanical Max Horz 5=108(LC 12) Max Uplift 5=-15(LC 12), 3=-66(LC 12), 4=-3(LC 12) Max Grav 5=273(LC 1), 3=77(LC 19), 4=55(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 3 and 3 lb uplift at joint 4.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.



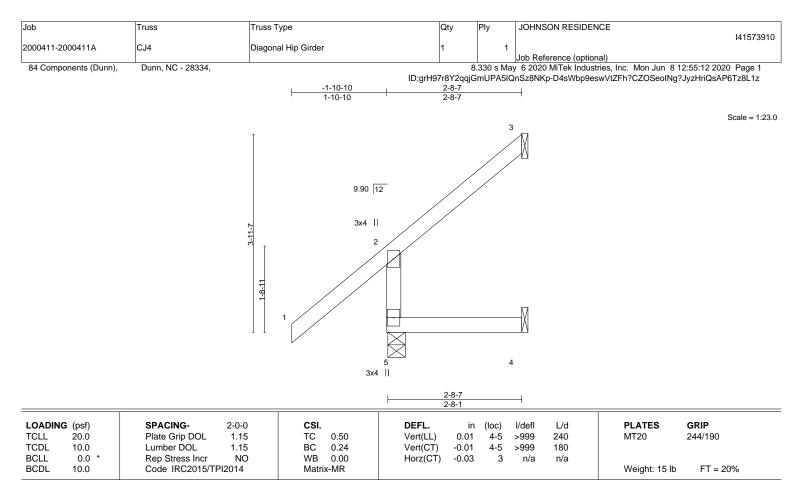
Structural wood sheathing directly applied or 3-3-3 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-8-7 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-4-3, 3=Mechanical, 4=Mechanical Max Horz 5=107(LC 12) Max Uplift 3=-70(LC 12), 4=-18(LC 12)

Max Grav 5=271(LC 1), 3=55(LC 19), 4=44(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

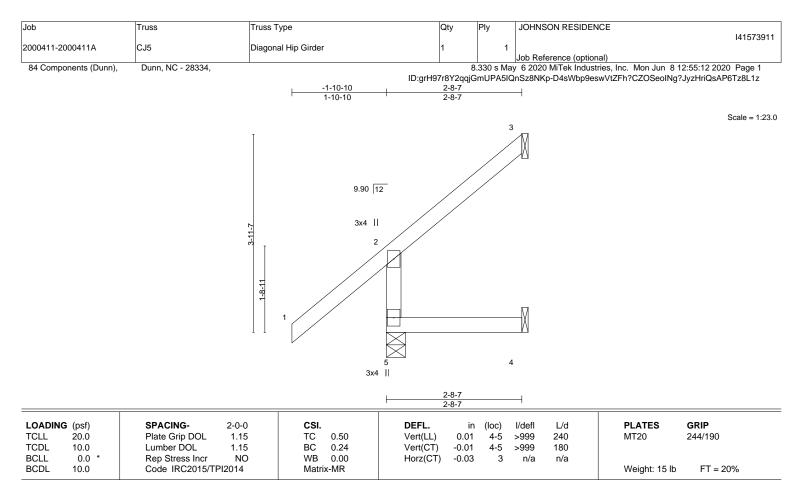
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 3 and 18 lb uplift at joint 4.



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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-8-7 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-4-9, 3=Mechanical, 4=Mechanical Max Horz 5=107(LC 12) Max Uplift 3=-70(LC 12), 4=-18(LC 12)

Max Grav 5=271(LC 1), 3=55(LC 19), 4=44(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

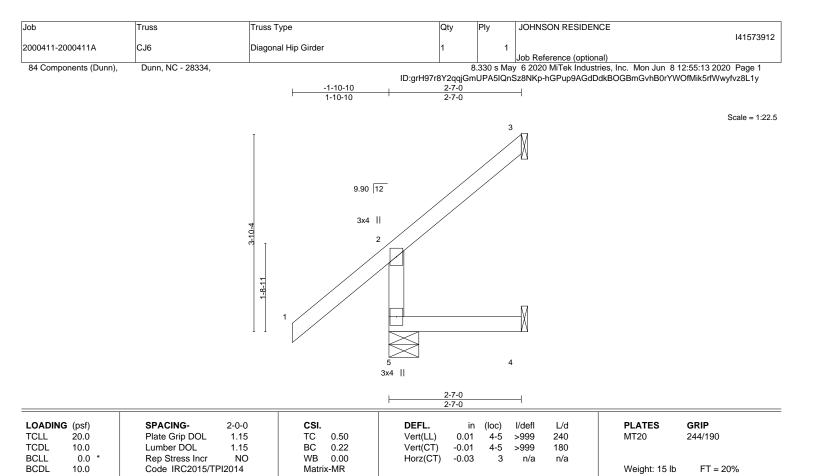
4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 3 and 18 lb uplift at joint 4.



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TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-7-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 5=0-7-0, 4=Mechanical Max Horz 5=103(LC 12) Max Uplift 3=-66(LC 12), 4=-19(LC 12) Max Grav 3=49(LC 19), 5=269(LC 1), 4=41(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

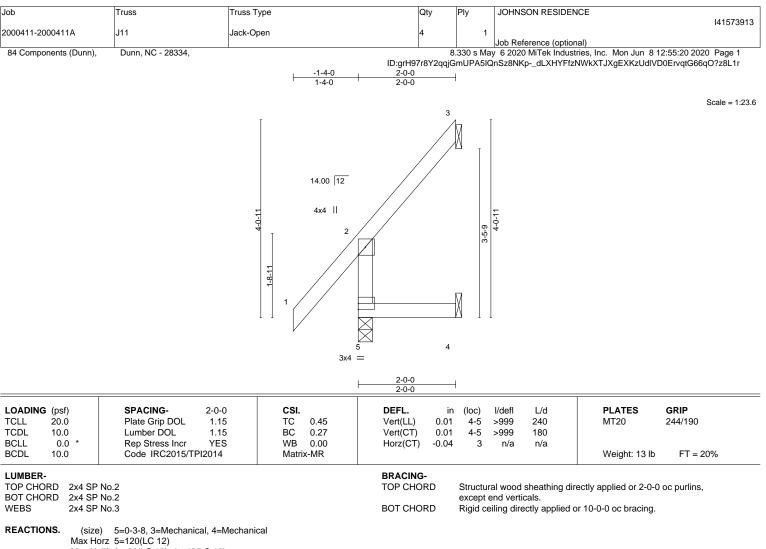
6) Refer to girder(s) for truss to truss connections.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 3 and 19 lb uplift at joint 4.



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Max Uplift 3=-88(LC 12), 4=-42(LC 12)

Max Grav 5=198(LC 1), 3=57(LC 19), 4=47(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 3 and 42 lb uplift at joint 4.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



