

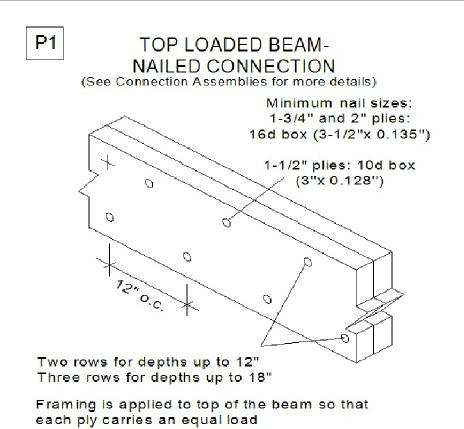
# U.S. LUMBER

Important Notes | WARNING: Failure to follow proper procedures for handling, storage and installation could result in unsatisfactory performance, unsafe

These instructions are offered as a guide to good practice in the handling, storage and installation of LP® SolidStart® I-Joists, LP SolidStart LVL & LP SolidStart LSL beams. They are, however, solely general recommendations and, in some instances, other or additional precautions may be desirable. In all cases, the procedures used should be as specified by the architect/engineer responsible for the entire building.

structures and possible collapse.

- This is not intended as a manual for selecting products and assumes that components and details have been specified correctly. Consult the LP SolidStart I-Joist, LP SolidStart LVL & LP SolidStart LSL brochures
- or contact your LP SolidStart products distributor for assistance. • All rim joists, blocking, connections and temporary bracing must be installed before
- erectors are allowed on the structure.
- No loads other than the weight of the erectors are to be imposed on the structure before it is permanently sheathed.
- After sheathing, do not overload joists with construction materials exceeding design loads.
- LP SolidStart I-Joists, LP SolidStart LVL & LP SolidStart LSL beams must be used under dry, covered and well ventilated interior conditions in which the equivalent moisture content in lumber will not exceed 16%.



**Customer Name:** 

## **BRAD CUMMINGS**

Job Name: THE LASERIA RESIDENCE

> Designer: **Tony Huneycutt**

> > Salesman:

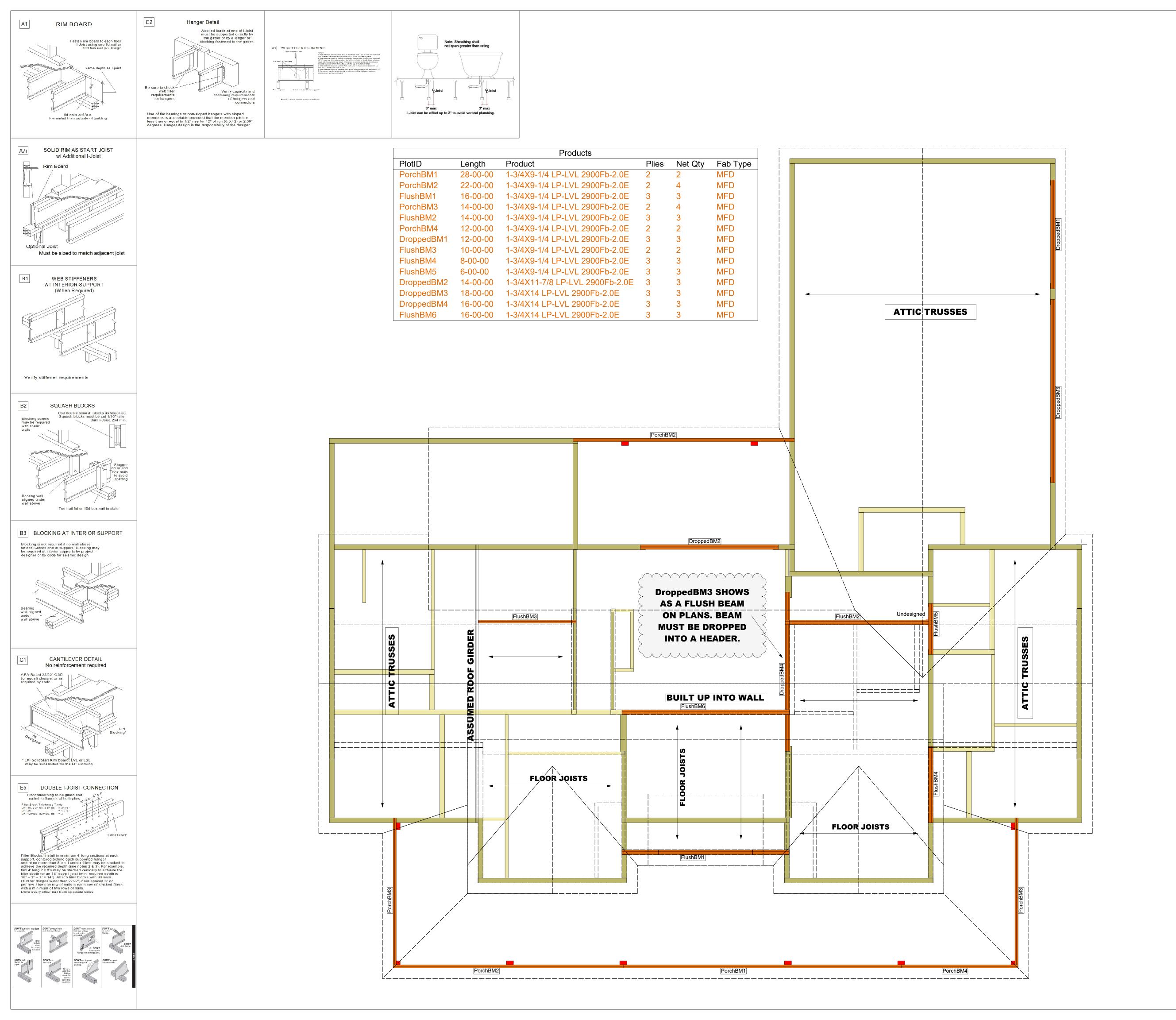
# **EDDIE BAUER**

Scale:1/4" = 1'

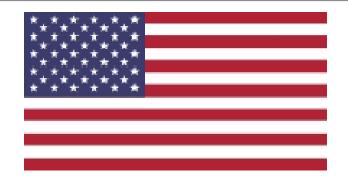
Date:07/22/20

1ST FLOOR









# U.S. LUMBER

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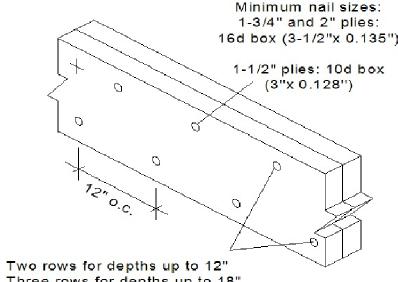
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- Consult the LP SolidStart I-Joist, LP SolidStart LVL & LP SolidStart LSL brochures or contact your LP SolidStart products distributor for assistance. • All rim joists, blocking, connections and temporary bracing must be installed before
- erectors are allowed on the structure.
- No loads other than the weight of the erectors are to be imposed on the structure
- before it is permanently sheathed.

content in lumber will not exceed 16%.

structures and possible collapse.

• After sheathing, do not overload joists with construction materials exceeding design loads. • LP SolidStart I-Joists, LP SolidStart LVL & LP SolidStart LSL beams must be used under dry, covered and well ventilated interior conditions in which the equivalent moisture

TOP LOADED BEAM-NAILED CONNECTION (See Connection Assemblies for more details) Minimum nail sizes: 1-3/4" and 2" plies: 16d box (3-1/2"x 0.135")



Three rows for depths up to 18"

Framing is applied to top of the beam so that each ply carries an equal load

## **Customer Name: BRAD CUMMINGS**

# Job Name:

# THE LASERIA RESIDENCE Designer:

**Tony Huneycutt** 

Salesman:

**EDDIE BAUER** 

Scale:1/4" = 1'

Date:07/22/20

2nd Floor



Client: Project: Address: Date: 7/22/2020 Input by: Tony Huneycutt

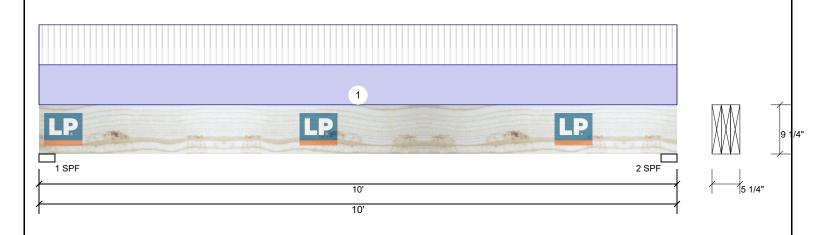
Job Name: Bartlett-Laseria

Project #:

9' Garage Dr. Header LP-LVL 2900Fb-2.0E 1.75

1.750" X 9.250"

3-Ply - PASSED Level: Level



Member Infor	rmation		Reactions UNPATTERNED lb (Uplift)						
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	3	Design Method:	ASD	1	1600	1669	0	0	0
Moisture Conditio	n: Dry	Building Code:	IBC/IRC 2015	2	1600	1669	0	0	0
Deflection LL:	480	Load Sharing:	Yes						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
	·			Bearing	<b>js</b>				
				Bearing	Length	Cap. Rea	ct D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	3.000"	49% 16	69 / 1600	3269 L	D+L
				2 - SPF	3.000"	49% 16	69 / 1600	3269 L	D+L

## **Analysis Results**

ĺ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	7572 ft-lb	5'	19369 ft-lb	0.391 (39%)	D+L	L
l	Shear	2643 lb	11 1/2"	9227 lb	0.286 (29%)	D+L	L
l	LL Defl inch	0.098 (L/1178)	5'	0.241 (L/480)	0.410 (41%)	L	L
I	TL Defl inch	0.200 (L/577)	5'	0.481 (L/240)	0.420 (42%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.102", Long Term = 0.153"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	320 PLF	320 PLF	0 PLF	0 PLF	0 PLF	Truss Reaction
	Self Weight				14 PLF					

## Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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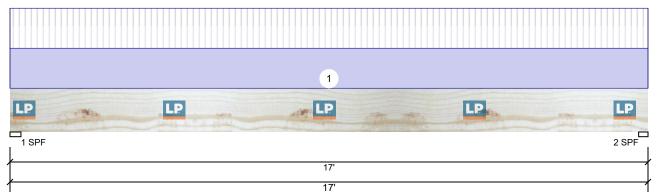
Page 1 of 1

Client: Project: Address: Date: 7/22/2020 Input by: Tony Huneycutt

Job Name: Bartlett-Laseria Project #:

16' Garage Dr. Header LP-LVL 2900Fb-2.0E 1.750" X 14.000" 3-Ply - PASSED

Level: Level



Member Infor	mation		Reactions UNPATTERNED lb (Uplift)							
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const	
Plies:	3	Design Method:	ASD	1	2727	2906	0	0	0	
Moisture Conditio	n: Dry	Building Code:	IBC/IRC 2015	2	2713	2892	0	0	0	
Deflection LL:	480	Load Sharing:	Yes							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal									
Temperature:	Temp <= 100°F									
				Bearing	gs					
				Bearing	g Length	Cap. Rea	ct D/L lb	Total Ld. C	Case Ld. Comb.	
				1 - SPF	3.500"	72% 290	06 / 2727	5633 L	D+L	
				2 - SPF	3.000"	84% 289	92 / 2713	5605 L	D+L	

## Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	22723 ft-lb	8'6 1/4"	42165 ft-lb	0.539 (54%)	D+L	L
Shear	4710 lb	15'7 3/4"	13965 lb	0.337 (34%)	D+L	L
LL Defl inch	0.244 (L/815)	8'6 5/16"	0.415 (L/480)	0.590 (59%)	L	L
TL Defl inch	0.504 (L/395)	8'6 5/16"	0.829 (L/240)	0.610 (61%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.260", Long Term = 0.390"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be continuously braced.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	320 PLF	320 PLF	0 PLF	0 PLF	0 PLF	Attic Truss Reactions
	Self Weight				21 PLF					

## Notes

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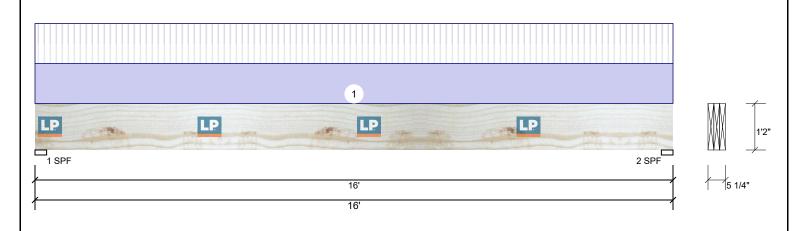
Date: 7/22/2020 Input by: Tony Huneycutt

Project #:

Beam @ Loft LP-LVL 2900Fb-2.0E 1.750" X 14.000" 3-Ply - PASSED

Level: Level

Job Name: Bartlett-Laseria



Member Infor	mation	Reactions UNPATTERNED Ib (Uplift)								
Туре:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Win	d	Const
Plies:	3	Design Method:	ASD	1	2560	2728	0		0	0
Moisture Conditio	n: Dry	Building Code:	IBC/IRC 2015	2	2560	2728	0		0	0
Deflection LL:	480	Load Sharing:	Yes							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal									
Temperature:	Temp <= 100°F									
				Bearing	S					
				Bearing	Length	Cap. Read	t D/L lb	Total Ld	. Case	Ld. Comb.
				1 - SPF	3.500"	68% 272	8 / 2560	5288 L		D+L
				2 - SPF	3.500"	68% 272	8 / 2560	5288 L		D+L

## Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	19958 ft-lb	8'	42165 ft-lb	0.473 (47%)	D+L	L
Shear	4366 lb	14'7 1/4"	13965 lb	0.313 (31%)	D+L	L
LL Defl inch	0.190 (L/981)	8' 1/16"	0.389 (L/480)	0.490 (49%)	L	L
TL Defl inch	0.393 (L/475)	8' 1/16"	0.777 (L/240)	0.510 (51%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.203", Long Term = 0.304"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 9'10 7/8" o.c.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	320 PLF	320 PLF	0 PLF	0 PLF	0 PLF	Roof Load
	Self Weight				21 PLF					

## Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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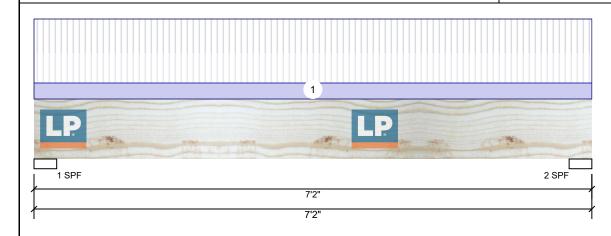
Date: 7/22/2020

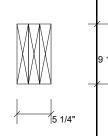
Input by: Tony Huneycutt Job Name: Bartlett-Laseria

Project #:

Beam left of Range LP-LVL 2900Fb-2.0E 1.750" X 9.250" 3-Ply - PASSED

Level: Level





Page 1 of 1

### **Member Information**

Type: Girder

Plies: 3

Moisture Condition: Dry

Deflection LL: 480

Deflection TL: 240

Importance: Normal

Temperature: Temp <= 100°F

Application: Floor
Design Method: ASD
Building Code: IBC/IRC 2015
Load Sharing: Yes
Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)													
Brg	Live	Dead	Snow	Wind	Const								
1	1003	301	0	0	0								
2	1003	301	0	0	0								

## Bearings

 Bearing
 Length
 Cap.
 React D/L lb
 Total
 Ld. Case
 Ld. Comb.

 1 - SPF
 3.500"
 17%
 301 / 1003
 1304
 L
 D+L

 2 - SPF
 3.500"
 17%
 301 / 1003
 1304
 L
 D+L

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2047 ft-lb	3'7"	19369 ft-lb	0.106 (11%)	D+L	L
Shear	940 lb	6'2"	9227 lb	0.102 (10%)	D+L	L
LL Defl inch	0.022 (L/3633)	3'7 1/16"	0.168 (L/480)	0.130 (13%)	L	L
TL Defl inch	0.029 (L/2795)	3'7 1/16"	0.335 (L/240)	0.090 (9%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.007", Long Term = 0.010"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	70 PLF	280 PLF	0 PLF	0 PLF	0 PLF	Floor Load	
	Self Weight				14 PLF						

## Notes

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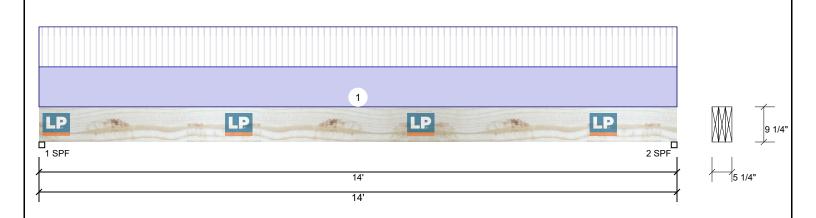
Date: 7/22/2020

Input by: Tony Huneycutt
Job Name: Bartlett-Laseria

Project #:

Beam over Kitchen LP-LVL 2900Fb-2.0E 1.750" X 9.250" 3-Ply - PASSED

Level: Level



Member Info	rmation			Reaction	ons UNPAT	TERNED IL	(Uplift)			
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Win	d	Const
Plies:	3	Design Method:	ASD	1	1400	1497	0		0	0
Moisture Condition	on: Dry	Building Code:	IBC/IRC 2015	2	1400	1497	0		0	0
Deflection LL:	480	Load Sharing:	Yes							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal									
Temperature:	Temp <= 100°F									
				Bearing	gs					
				Bearin	g Length	Cap. Rea	ct D/L lb	Total Ld	. Case	Ld. Comb.
				1 - SPI	1.500"	87% 149	97 / 1400	2897 L		D+L
				2 - SPI	1.500"	87% 149	97 / 1400	2897 L		D+L

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9960 ft-lb	7'	19369 ft-lb	0.514 (51%)	D+L	L
Shear	2552 lb	10"	9227 lb	0.277 (28%)	D+L	L
LL Defl inch	0.252 (L/660)	7' 1/16"	0.347 (L/480)	0.730 (73%)	L	L
TL Defl inch	0.522 (L/319)	7' 1/16"	0.694 (L/240)	0.750 (75%)	D+L	L

## Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.270", Long Term = 0.405"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	200 PLF	200 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				14 PLF					

## Notes

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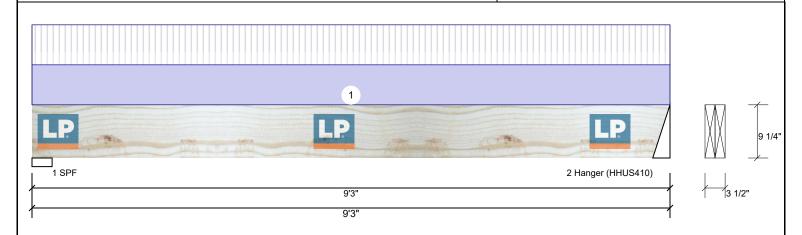
Date: 7/22/2020 Input by: Tony Huneycutt

Project #:

Beam over MBR LP-LVL 2900Fb-2.0E 1.750" X 9.250" 2-Ply - PASSED

Level: Level

Job Name: Bartlett-Laseria



Member Inforn	nation			Reaction	is UNPAT	TERNED Ib	(Uplift)		
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	1208	1251	0	0	0
Moisture Condition:	: Dry	Building Code:	IBC/IRC 2015	2	1197	1240	0	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearings	5				
				Bearing	Length	Cap. Read	t D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF	3.500"	47% 125	1 / 1208	2459 L	D+L
				2 -	3.000"	31% 124	0 / 1197	2437 L	D+L
Analysis Result	s			Hanger					
Analysis Act	ual Location	Allowed Capac	ity Comb. Cas	e					

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5162 ft-lb	4'7 3/4"	12416 ft-lb	0.416 (42%)	D+L	L
Shear	1930 lb	8'3 1/2"	6151 lb	0.314 (31%)	D+L	L
LL Defl inch	0.086 (L/1230)	4'7 3/4"	0.221 (L/480)	0.390 (39%)	L	L
TL Defl inch	0.175 (L/604)	4'7 3/4"	0.442 (L/240)	0.400 (40%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.089", Long Term = 0.134"
- 3 Fill all hanger nailing holes.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Multiple plies must be fastened together as per manufacturer's details.
- 6 Top loads must be supported equally by all plies.
- 7 Top braced at bearings.
- 8 Bottom braced at bearings.

	3										
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	260 PLF	260 PLF	0 PLF	0 PLF	0 PLF	Roof Load	
	Self Weight				9 PLF						

## Notes

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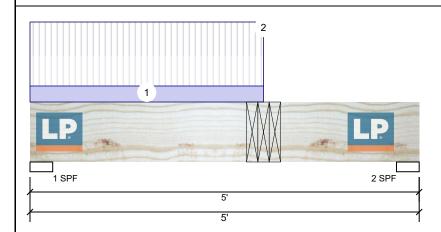


Date: 7/22/2020 Input by: Tony Huneycutt

Job Name: Bartlett-Laseria

Project #:

Beam Right of Range LP-LVL 2900Fb-2.0E 1.750" X 9.250" 3-Ply - PASSED Level: Level



Application:

Design Method:

**Building Code:** 

Load Sharing:

Deck:

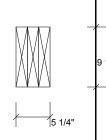
Floor

ASD

Yes

**IBC/IRC 2015** 

Not Checked



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## **Member Information**

Type: Girder

Plies: 3

Moisture Condition: Dry

Deflection LL: 480

Deflection TL: 240

Importance: Normal

Temperature: Temp <= 100°F

## Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	1149	769	0	0	0
2	1091	1007	0	0	0

## **Bearings**

Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF 3.500"	25% 769 / 1149	1919 L	D+L
2 - SPE 3500"	27% 1007 / 1091	2098 I	D+I

## **Analysis Results**

L							
ſ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	3688 ft-lb	3'	19369 ft-lb	0.190 (19%)	D+L	L
	Shear	2084 lb	4'	9227 lb	0.226 (23%)	D+L	L
	LL Defl inch	0.013 (L/4104)	2'9 3/16"	0.114 (L/480)	0.120 (12%)	L	L
	TL Defl inch	0.025 (L/2197)	2'10 1/16"	0.227 (L/240)	0.110 (11%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.012", Long Term = 0.017"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 3-0-0		Тор	70 PLF	280 PLF	0 PLF	0 PLF	0 PLF	Floor Load
2	Point	3-0-0		Near Face	1497 lb	1400 lb	0 lb	0 lb	0 lb	Beam over Kitchen Brg 2
	Self Weight				14 PLF					

## Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Manufacturer Info

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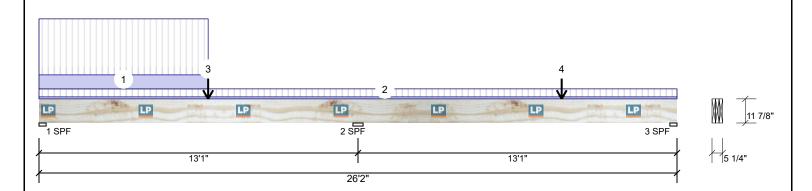


Date: 7/22/2020 Input by: Tony Huneycutt

Job Name: Bartlett-Laseria

Project #

3-Ply - PASSED Level: Level Beam under Kit./Great Room Wall LP-LVL 2900Fb-2.0E 1.750" X 11.875"



### **Member Information** Reactions UNPATTERNED Ib (Uplift) Application: Wind Const Type: Floor Brg Live Dead Snow Plies: 3 Design Method: ASD 3095 1494 0 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 6310 4032 0 0 0 Deflection LL: 480 Load Sharing: Yes 3 1628 1238 0 0 0 Deflection TL: 240 Deck: Not Checked Importance: Normal Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1479 / 3363 D+L 1 - SPF 3.500" 4842 L 2 - SPF 5.000" 93% 4062 / 6357 10419 LL D+I 3 - SPF 3.500" 43% 1222 / 2169 3391 D+L

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-18067 ft-lb	13'1"	31048 ft-lb	0.582 (58%)	D+L	LL
Pos Moment	21720 ft-lb	6'11 1/4"	31048 ft-lb	0.700 (70%)	D+L	L_
Shear	6359 lb	12'1 1/8"	11845 lb	0.537 (54%)	D+L	LL
LL Defl inch	0.259 (L/595)	6'6 5/8"	0.321 (L/480)	0.810 (81%)	L	L_
TL Defl inch	0.382 (L/403)	6'6 7/16"	0.643 (L/240)	0.590 (59%)	D+L	L_

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.123", Long Term = 0.184"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 6'11 5/8" o.c.
- 7 Bottom must be laterally braced at a maximum of 9'2 1/2" o.c.

7 Dottom mus	t be laterally braced at									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 6-11-4		Тор	70 PLF	280 PLF	0 PLF	0 PLF	0 PLF	Floor Load
2	Uniform		1-7-3	Тор	10 PSF	40 PSF	0 PSF	0 PSF	0 PSF	Std. Floor Loading
3	Point	6-11-4		Тор	3013 lb	4286 lb	0 lb	0 lb	0 lb	Header from Above
	Bearing Length	0-3-8								
4	Point	21-5-4		Тор	2380 lb	3131 lb	0 lb	0 lb	0 lb	Header From Above
	Bearing Length	0-3-8								
	Self Weight				18 PLF					

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LADBS: RR-25783, Florida: FL15228

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United States



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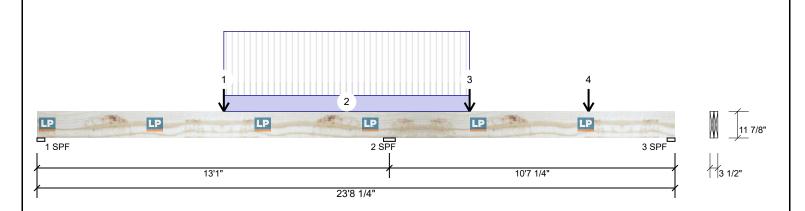
Date: 7/22/2020 Input by: Tony Huneycutt Job Name: Bartlett-Laseria

Project #

Beam under Kit/Pantry Wall LP-LVL 2900Fb-2.0E

1.750" X 11.875" 2-Ply - PASSED

Level: Level



### **Member Information** Reactions UNPATTERNED Ib (Uplift) Application: Wind Const Type: Floor Brg Live Dead Snow Plies: 2 Design Method: ASD 0 0 0 511 147 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 4531 2040 0 0 0 Deflection LL: 480 Load Sharing: No 3 757 812 0 0 0 Deflection TL: 240 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 15% 141 / 654 795 (-15) L D+L(D+L) 2 - SPF 5.250" 85% 2054 / 4561 6615 LL D+L 3 - SPF 3.500" 35% 804 / 1034 1838 D+L

## Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-7436 ft-lb	13'1"	19902 ft-lb	0.374 (37%)	D+L	LL
Pos Moment	5415 ft-lb	20'5 3/4"	19902 ft-lb	0.272 (27%)	D+L	_L
Shear	3284 lb	14' 7/8"	7897 lb	0.416 (42%)	D+L	LL
LL Defl inch	0.123 (L/1257)	6'11 1/4"	0.321 (L/480)	0.380 (38%)	L	L_
TL Defl inch	0.116 (L/1069)	18'6 3/8"	0.519 (L/240)	0.220 (22%)	D+L	_L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = -0.006", Long Term = -0.009"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Tie-down connection required at bearing 1 for uplift 15 lb (Combination D+L, Load Case \_L).
- 7 Top braced at bearings.
- 8 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Point	6-11-4		Тор	301 lb	1003 lb	0 lb	0 lb	0 lb	Beam Above
	Bearing Length	0-3-8								
2	Part. Uniform	6-11-4 to 16-0-12		Тор	70 PLF	280 PLF	0 PLF	0 PLF	0 PLF	Floor Load
3	Point	16-0-12		Тор	770 lb	1150 lb	0 lb	0 lb	0 lb	Beam Above
	Bearing Length	0-3-8								

Continued on page 2...

## Notes

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Page 1 of 2

This design is valid until 10/31/2021

Version 20.40.087 Powered by iStruct™

Client: Project: Address: Date: 7/22/2020
Input by: Tony Huneycutt
Job Name: Bartlett-Laseria

Project #:

Beam under Kit/Pantry Wall LP-LVL 2900Fb-2.0E 1.750" X 11.875" 2-Ply - PASSED

Level: Level

1 SPF 2 SPF 3 SPF 3 SPF 3 1/2" 23'8 1/4"

...Continued from page 1

Self Weight

Trib Width Dead 0.9 Wind 1.6 Const. 1.25 Comments ID Load Type Location Side Live 1 Snow 1.15 4 Point 20-5-12 Тор 1007 lb 1091 lb 0 lb 0 lb 0 lb Beam Above Bearing Length 0-3-8

12 PLF

## Notes

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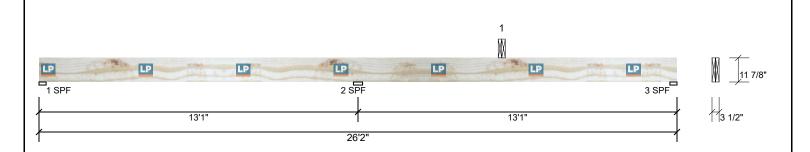
Client: Project: Address: Date: 7/22/2020 Input by: Tony Huneycutt

Job Name: Bartlett-Laseria

Project #:

Beam under Stair Wall LP-LVL 2900Fb-2.0E 1.750" X 11.875" 2-Ply - PASSED

Level: Level



### Member Information Reactions UNPATTERNED Ib (Uplift) Application: Wind Type: Floor Brg Live Dead Snow Const Plies: 2 Design Method: ASD 0 0 0 0 (-112) (-56)1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 879 1100 0 0 0 Deflection LL: 480 Load Sharing: No 3 442 518 0 0 0 Deflection TL: 240 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" D+L(D+L) 0% -60 / -115 -175 (-175)2 - SPF 4.500" 30% 1108 / 885 D+L 1993 \_L Analysis Results 3 - SPF 3.500" 18% 514 / 438 953 D+I L Comb. Analysis Actual Location Allowed Capacity Case Neg Moment -3270 ft-lb 13'1" 19902 ft-lb 0.164 (16%) D+L

\_L

\_L

LL

LL

## **Design Notes**

Shear

Pos Moment 6322 ft-lb

LL Defl inch 0.077 (L/1997)

TL Defl inch 0.161 (L/958)

1650 lb

1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.

18'11 3/4" 19902 ft-lb

19'3 1/4" 0.321 (L/480) 0.240 (24%) L

19'3 9/16" 0.643 (L/240) 0.250 (25%) D+L

14' 7/8" 7897 lb

0.318 (32%) D+L

0.209 (21%) D+L

- 2 Dead Load Deflection: Instant = 0.084", Long Term = 0.126"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Tie-down connection required at bearing 1 for uplift 175 lb (Combination D+L, Load Case \_L).
- 7 Top braced at bearings.
- 8 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Point	18-11-12		Тор	1251 lb	1208 lb	0 lb	0 lb	0 lb	Beam over MBR Brg 1	
	Bearing Length	0-3-8									
	Self Weight				12 PLF						

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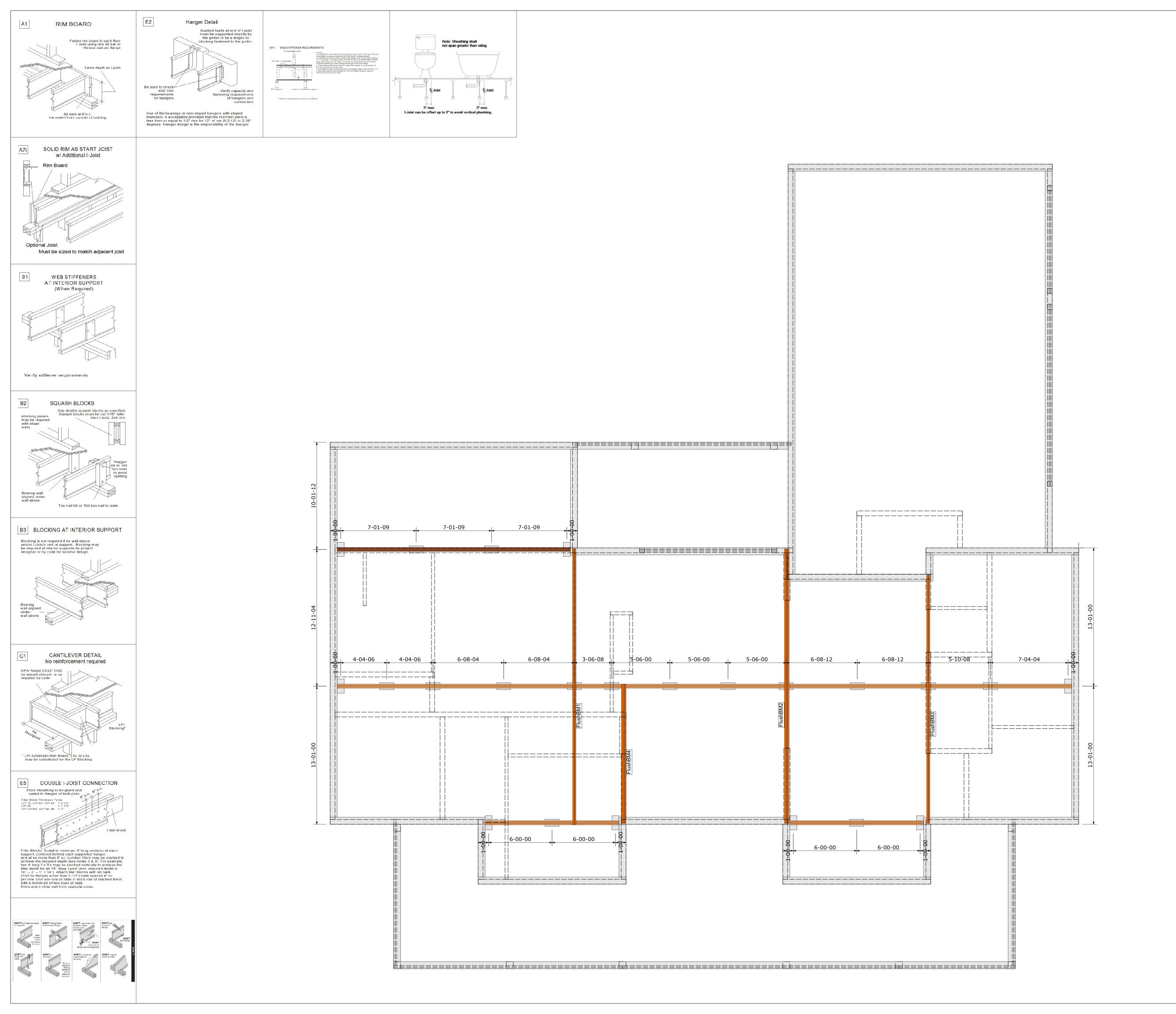
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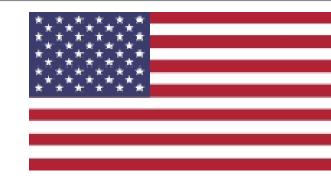
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# U.S. LUMBER

Important Notes WARNING: Failure to follow proper procedures for handling, storage and installation could result in unsatisfactory performance, unsafe

These instructions are offered as a guide to good practice in the handling, storage and installation of LP® SolidStart® I-Joists, LP SolidStart LVL & LP SolidStart LSL beams. They are, however, solely general recommendations and, in some instances, other or additional precautions may be desirable. In all cases, the procedures used should be as specified by the architect/engineer responsible for the entire building.

- This is not intended as a manual for selecting products and assumes that components and details have been specified correctly.
  Consult the LP SolidStart I-Joist, LP SolidStart LVL & LP SolidStart LSL brochures
- or contact your LP SolidStart products distributor for assistance.

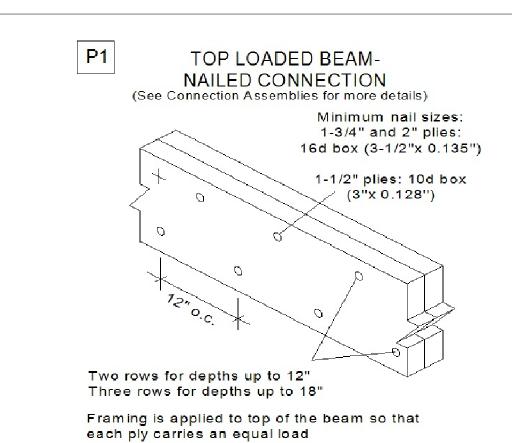
   All rim joists, blocking, connections and temporary bracing must be installed before
- erectors are allowed on the structure.

  No loads other than the weight of the erectors are to be imposed on the structure
- before it is permanently sheathed.

   After sheathing, do not overload joists with construction materials exceeding design loads.

structures and possible collapse.

 LP SolidStart I-Joists, LP SolidStart LVL & LP SolidStart LSL beams must be used under dry, covered and well ventilated interior conditions in which the equivalent moisture content in lumber will not exceed 16%.



**Customer Name:** 

## **BRAD CUMMINGS**

## Job Name: THE LASERIA RESIDENCE

Designer:
Tony Huneycutt

Salesman:

**EDDIE BAUER** 

Scale:1/4" = 1'

Date:07/22/20

1ST FLOOR



## FIRST FLOOR FRAMING

1634	LF	12IJ	11-7/8" LPI	20Plus JOISTS	15/36' 3/32'	8/30' 1/28'	3	4902
					18/26' 3/24'	1/20' 3/14'		
					1/10' 9/6' 2/4	1' + 56' BLKG		
21	PCS	12RIM12	1-1/8" x 1	1-7/8" x 12' RIN	/I BOARD		43.2	907.2
224	LF	12LVL	1-3/4" x 1	1-7/8" LVL	2/28' 3/26' 2	/24' 3/14'	5.4	1209.6

7018.8

## **SECOND FLOOR EWP**

412	LF	9LVL	1-3/4" x 9-1/4" LVL	2/28' 4/22' 3/16' 7/14' 5/12'	4.2	1730.4
				2/10' 3/8' 3/6'		
42	LF	12LVL	1-3/4" x 11-7/8" LVL	3/14'	5.4	226.8
150	LF	14LVL	1-3/4" x 14" LVL	3/18' 6/16'	6.4	960

2917.2

BRAD CUMMINGS
THE LASERIA RESIDENCE

Client: Project: Address: Date: 7/22/2020 Input by: Tony Huneycutt

Project #:

Bearing Length

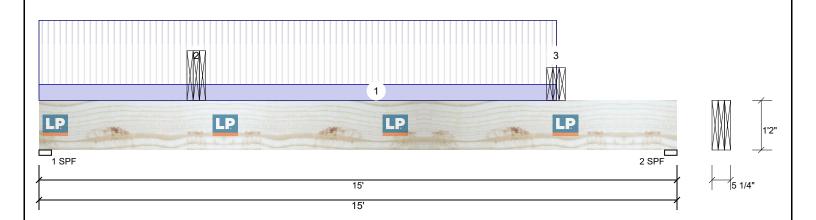
1-SPF 3.500"

2 - SPF 3.500"

Header @ Kit./Great Room LP-LVL 2900Fb-2.0E 1.750" X 14.000" 3-Ply - PASSED

Level: Level

Job Name: Bartlett-Laseria



### **Member Information** Reactions UNPATTERNED Ib (Uplift) Application: Brg Dead Snow Wind Const Type: Floor Live Plies: 3 Design Method: ASD 4236 3013 0 0 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 0 2 3131 2380 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 240 Deck: Not Checked Importance: Normal Temperature: Temp <= 100°F **Bearings**

**Analysis Results** 

I	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	23084 ft-lb	5'3 7/16"	42165 ft-lb	0.547 (55%)	D+L	L
l	Shear	6731 lb	1'4 3/4"	13965 lb	0.482 (48%)	D+L	L
l	LL Defl inch	0.241 (L/724)	7'2 13/16"	0.364 (L/480)	0.660 (66%)	L	L
l	TL Defl inch	0.410 (L/425)	7'2 3/8"	0.727 (L/240)	0.560 (56%)	D+L	L

## Design Notes

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.169", Long Term = 0.254"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 8'1 1/2" o.c.
- 7 Bottom braced at bearings.

7 Dottom b	oracoa at boarings.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Part. Uniform	0-0-0 to 12-2-0		Тор	70 PLF	280 PLF	0 PLF	0 PLF	0 PLF	Floor Load
2	Point	3-8-6		Тор	2728 lb	2560 lb	0 lb	0 lb	0 lb	Beam @ Loft Brg 1
	Bearing Length	0-3-8								
3	Point	12-1-14		Тор	1497 lb	1400 lb	0 lb	0 lb	0 lb	Beam over Kitchen Brg 2
	Bearing Length	0-3-8								
	Self Weight				21 PLF					

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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Cap. React D/L lb

71%

3013 / 4236

2380 / 3131

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0

0

Ld. Comb.

D+L

D+I

Total Ld. Case

7249 L

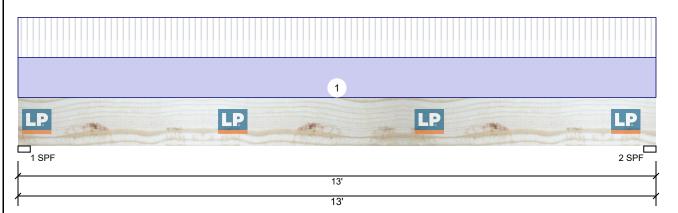
5510 L



Date: 7/22/2020
Input by: Tony Huneycutt

Job Name: Bartlett-Laseria Project #:

Header over Door in Great Room LP-LVL 2900Fb-2.0E 1.750" X 11.875" 3-Ply - PASSED Level: Level





Page 1 of 1

**Member Information** 

 Type:
 Girder

 Plies:
 3

 Moisture Condition:
 Dry

 Deflection LL:
 480

 Deflection TL:
 240

 Importance:
 Normal

 Temperature:
 Temp <= 100°F</td>

Application: Floor
Design Method: ASD
Building Code: IBC/IRC 2015
Load Sharing: Yes
Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift) Brg Snow Wind Const Live Dead 2275 2391 0 0 0 1 0 2 2275 2391 0 0

## Bearings

 Bearing
 Length
 Cap.
 React D/L lb
 Total
 Ld. Case
 Ld. Comb.

 1 - SPF
 3.000"
 70%
 2391 / 2275
 4666
 L
 D+L

 2 - SPF
 3.000"
 70%
 2391 / 2275
 4666
 L
 D+L

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	14302 ft-lb	6'6"	31048 ft-lb	0.461 (46%)	D+L	L
Shear	3821 lb	1'2 1/8"	11845 lb	0.323 (32%)	D+L	L
LL Defl inch	0.149 (L/1014)	6'6"	0.316 (L/480)	0.470 (47%)	L	L
TL Defl inch	0.306 (L/494)	6'6"	0.631 (L/240)	0.490 (49%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.157", Long Term = 0.236"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be continuously braced.
- 7 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	350 PLF	350 PLF	0 PLF	0 PLF	0 PLF	Roof Load
	Self Weight				18 PI F					

## Notes

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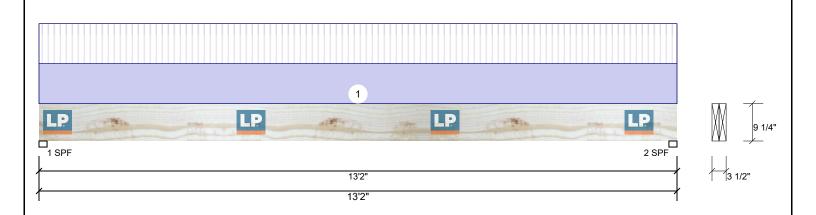
Date: 7/22/2020 Input by: Tony Huneycutt

Job Name: Bartlett-Laseria

Project #:

Rear Porch Header LP-LVL 2900Fb-2.0E 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Infor	mation			Reaction	ns UNPAT	TERNED	lb (Uplift)			
Type:	Girder	Application:	Floor	Brg	Live	Dead	Snow	W	/ind	Const
Plies:	2	Design Method:	ASD	1	658	719	0		0	0
Moisture Conditio	n: Dry	Building Code:	IBC/IRC 2015	2	658	719	0		0	0
Deflection LL:	480	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal									
Temperature:	Temp <= 100°F									
				Bearings	5					
				Bearing	Length	Cap. Re	eact D/L lb	Total I	Ld. Case	Ld. Comb.
				1 - SPF	2.000"	46%	719 / 658	1378 L	L	D+L
				2 - SPF	2.000"	46%	719 / 658	1378 L	L	D+L

## Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4392 ft-lb	6'7"	12416 ft-lb	0.354 (35%)	D+L	L
Shear	1195 lb	10 1/2"	6151 lb	0.194 (19%)	D+L	L
LL Defl inch	0.145 (L/1073)	6'7"	0.324 (L/480)	0.450 (45%)	L	L
TL Defl inch	0.303 (L/513)	6'7"	0.648 (L/240)	0.470 (47%)	D+L	L

## **Design Notes**

- 1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.
- 2 Dead Load Deflection: Instant = 0.158", Long Term = 0.237"
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Multiple plies must be fastened together as per manufacturer's details.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	100 PLF	100 PLF	0 PLF	0 PLF	0 PLF	Roof Load
	Self Weight				9 PLF					

This component analysis is based on the loads, geometry and other conditions as entered by the user and listed in this report. The user is responsible to ensure the accuracy of the input and the applicability to the actual conditions of the structure for which this component is intended. This analysis is valid only for the product listed.

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