

Foundation Plan

Span Table for Joist and Rafters.

-Floors shall be constructed in accordance with the provisions of Chapter 5 of the NC State Building Code, Sect. R502.2 and Sects R319 and R320. -Spans for floor joist shall be in accordance with Tables R502.3.1(1) and R502.3.1(2). For other grades and species and for other loading conditions, refer to the AF\$PA

-The allowable span of girders fabricated of dimension lumber shall not exceed the values set forth in Tables R502.5(1) and R502.5(2).

-Local soil conditions and/or local practice may necessitate a more stringent footing and foundation wall design. Consult with local building inspector. Soil design bearing pressure is assumed 2000 psf.

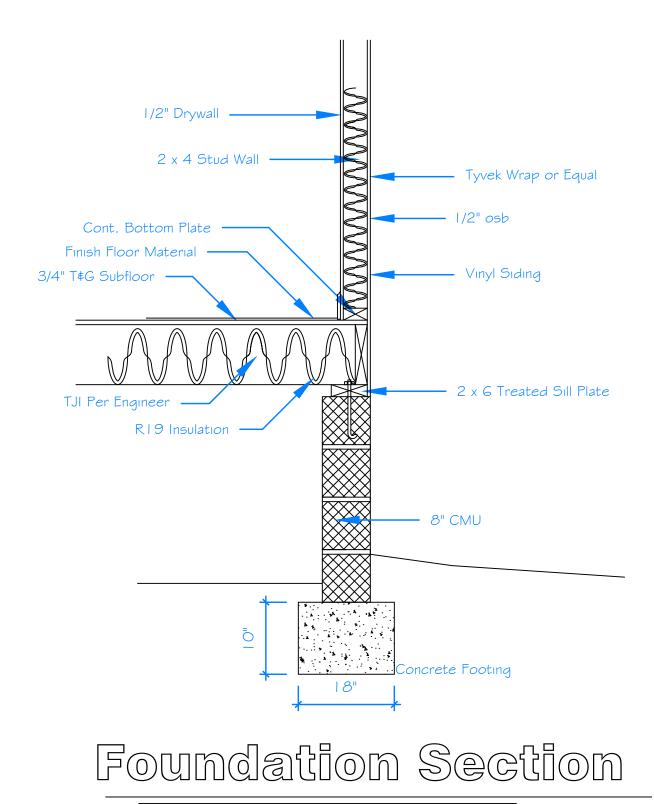
Carry all footings to firm undisturbed bearing: -24" x 10" footing for 8" foundation wall .

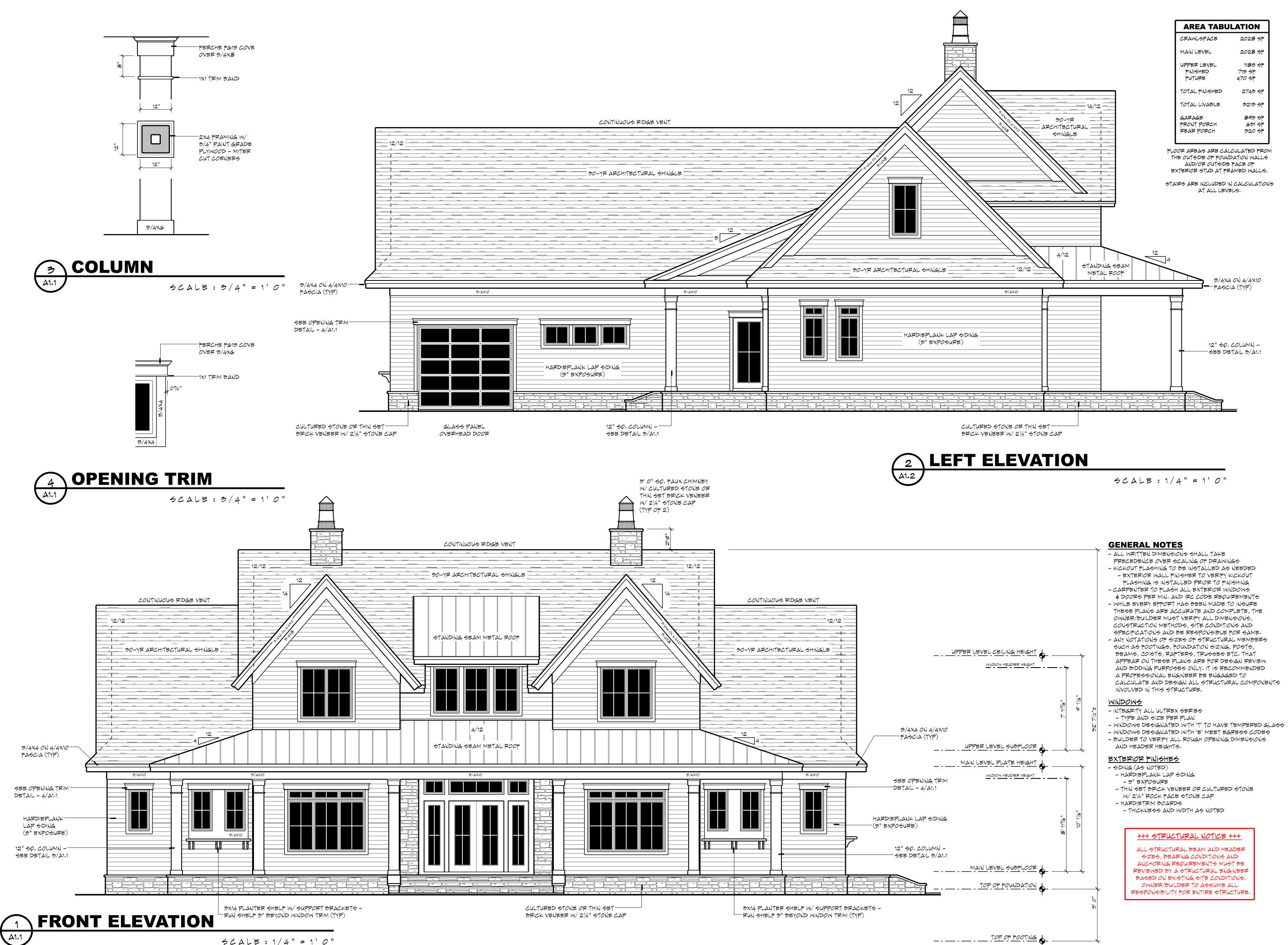
-24" x 10" footing for 12" foundation wall. Pier Footings (Typical Unless Otherwise Notes)

-Provide 1'-8" x 2'-4" x 1'-0" deep concrete footing under 8" x 16" masonry piers. -Provide 2'-0" square x 1'-0" deep concrete footing with

under 16" square masonry piers.

-Grout piers solid with 2500psi concrete (typ).





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- CALCULATE AND DESIGN ALL STRUCTURAL COMPONENTS

- WINDOWS DESIGNATED WITH 'E' MEET EGRESS CODES





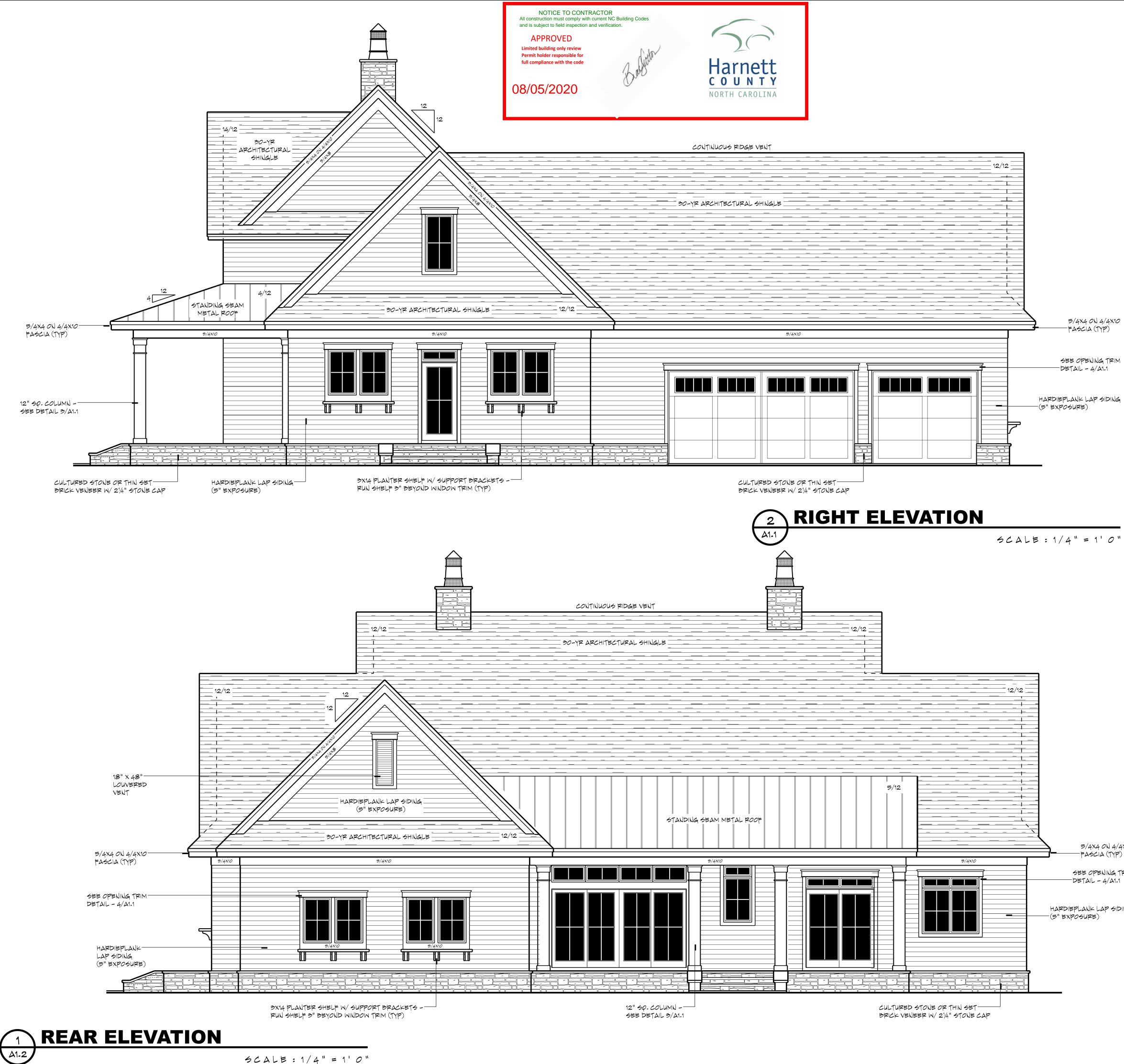
## ROYAL OAKS DESIGN, INC. ASSUMES NO RESPONSIBILITY FOR ANY STRUCTURAL OR DIMENSIONAL ERRORS OR OMISSIONS. THE CONTRACTOR AND/OR OWNER SHALL BE RESPONSIBLE FOR THE VERIFICATION AND CHECKING OF ALL NOTES, DETAILS, ELEVATIONS, SECTIONS AND OF ALL NOTES, DETAILS, ELEVATIONS, SECTIONS AND FLOOR PLANS AND NOTIFY ROYAL OAKS DESIGN, INC. OF ANY ERRORS PRIOR TO START OF CONSTRUCTION. ROYAL OAKS DESIGN, INC. ASSUMES NO RESPONSIBILITY FOR THE WORKMANSHIP OF THE SUB-CONTRACTORS.

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1 12/27/19 2X4 EXTERIOR -DML

EXTERIOR ELEVATIONS COLUMN DETAIL OPENING TRIM DETAIL





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5CALE: 1/4" = 1'0"

AREA TABU	LATION
CRAWLSPACE	2028 55
MAIN LEVEL	2028 55
UPPER LEVEL FINISHED FUTURE	1185 SF 715 SF 470 SF
TOTAL FINISHED	2743 5F
TOTAL LIVABLE	3213 SF
GAPAGE FRONT PORCH REAR PORCH	893 5F 631 5F 320 5F

FLOOP AREAS ARE CALCULATED FROM THE OUTSIDE OF FOUNDATION WALLS AND/OF OUTSIDE FACE OF EXTERIOR STUD AT FRAMED WALLS.

STAIRS ARE INCLUDED IN CALCULATIONS AT ALL LEVELS.

### **GENERAL NOTES**

- ALL WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALING OF DRAWINGS
- KICKOUT FLASHING TO BE INSTALLED AS NEEDED - EXTERIOR WALL FINISHER TO VERIFY KICKOUT FLASHING IS INSTALLED PRIOR TO FINISHING - CARPENTER TO FLASH ALL EXTERIOR WINDOWS
- & DOORS PER MIN. AND IRC CODE REQUIREMENTS - WHILE EVERY EFFORT HAS BEEN MADE TO INSURE THESE PLANS ARE ACCURATE AND COMPLETE, THE OWNER/BUILDER MUST VERIFY ALL DIMENSIONS, CONSTRUCTION METHODS, SITE CONDITIONS AND
- SPECIFICATIONS AND BE RESPONSIBLE FOR SAME. - ANY NOTATIONS OF SIZES OF STRUCTURAL MEMBERS SUCH AS FOOTINGS, FOUNDATION SIZING, POSTS, BEAMS, JOISTS, RAFTERS, TRUSSES ETC. THAT APPEAR ON THESE PLANS ARE FOR DESIGN REVIEW AND BIDDING PURPOSES ONLY. IT IS RECOMMENDED A PROFESSIONAL ENGINEER BE ENGAGED TO CALCULATE AND DESIGN ALL STRUCTURAL COMPONENTS INVOLVED IN THIS STRUCTURE.

#### MINDOWS

- INTEGRITY ALL ULTREX SERIES - TYPE AND SIZE PER PLAN
- WINDOWS DESIGNATED WITH 'T' TO HAVE TEMPERED GLASS - WINDOWS DESIGNATED WITH 'E' MEET EGRESS CODES - BUILDER TO VERIFY ALL ROUGH OPENING DIMENSIONS

#### EXTERIOR FINISHES

AND HEADER HEIGHTS.

- SIDING (AS NOTED)
- HARDIEPLANK LAP SIDING - 5" EXPOSURE
- THIN SET BRICK VENEER OF CULTURED STONE W/ 21/4" ROCK FACE STONE CAP
- HARDIETRIM BOARDS
- THICKNESS AND WIDTH AS NOTED

TRUGG SUPPLIER TO VERIFY ALL SPANG, PITCHES, HEEL HEIGHTS AND OTHER CONDITIONS CRITICAL TO PROPER TRUSS FABRICATION. ANY STRUCTURAL COMPONENTS THAT MAY NOTED ON THESE PLANS ARE INTENDED FOR DESIGN/BID PURPOSES ONLY. IT IS RECOMMENDED THAT ALL STRUCTURAL DESIGN ELEMENTS BE REVIEWED BY A LOCAL LICENSED PROFESSIONAL STRUCTURAL ENGINEER. FINAL ROOF AND FLOOP TRUSS DESIGN AND LAYOUT TO BE PROVIDED BY YOUR LOCAL TRUSS SUPPLIER.

+++ STRUCTURAL NOTICE +++

ALL STRUCTURAL BEAM AND HEADER SIZES, BEARING CONDITIONS AND ANCHORING REQUIREMENTS MUST BE REVIEWED BY A STRUCTURAL ENGINEER BASED ON EXISTING SITE CONDITIONS. OWNER/BUILDER TO ASSUME ALL RESPONSIBILITY FOR ENTIRE STRUCTURE

#### 5/4X4 ON 4/4X10 -FASCIA (TYP)

SEE OPENING TRIM -DETAIL - 4/A1.1

HAPDIEPLANK LAP SIDING

++ FOUNDATION ENGINEEPING ++ ALL BUILDING FOUNDATION, FOOTING SIZES AND REINFORCING, INCLUDING POST FOOTINGS, TO BE DESIGNED ON SITE BY LOCAL ENGINEER OR FOUNDATION CONTRACTOR BASED ON EXISTING SITE CONDITIONS.

+ FOOTING FROST DEPTH: ++ OWNER/CONTRACTOR TO ADJUST DEPTH OF ALL HOUSE, GARAGE, SLAB AND DECK POST FOOTINGS TO MEET LOCAL CODES.



Lake Elmo, MN 651-964-2726 www.royaloaksdesign.com



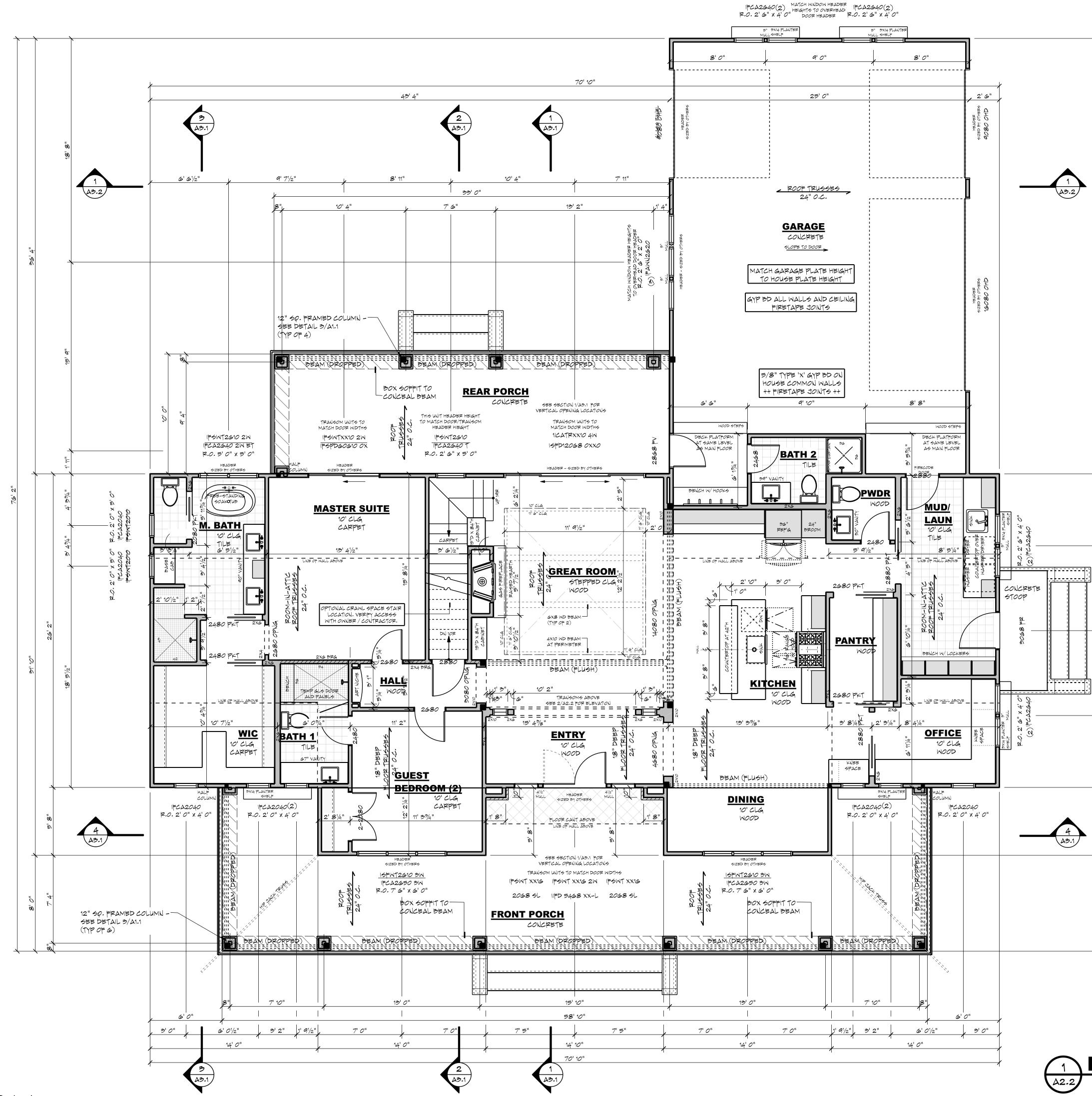
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1 12/27/19 2X4 EXTERIOR -DML

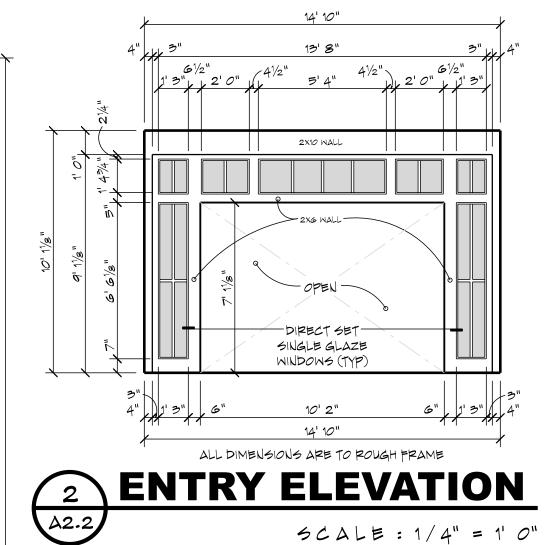
EXTERIOR ELEVATIONS





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## **GENERAL NOTES**

- ALL WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALING OF DRAWINGS
- DIMENSIONS ARE FROM EXTERIOR FACE OF EXTERIOR STUD WALLS AND CENTERLINE OF INTERIOR PARTITIONS - WHILE EVERY EFFORT HAS BEEN MADE TO INSURE THESE PLANS ARE ACCURATE AND COMPLETE, THE OWNER/BUILDER MUST VERIFY ALL DIMENSIONS, CONSTRUCTION METHODS, SITE CONDITIONS AND SPECIFICATIONS AND BE RESPONSIBLE FOR SAME.
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#### WINDOWS

- MARVIN INTEGRITY ALL ULTREX SERIES - STYLE AND SIZE AS NOTED
- WINDOWS DESIGNATED WITH 'T' TO HAVE TEMPERED GLASS - WINDOWS DESIGNATED WITH 'E' MEET EGRESS CODES
- WINDOW HEADER HEIGHTS SET TO 8' 11-3/8" (U.N.O.) - BUILDER TO VERIFY WINDOW AND DOOR ROUGH OPENINGS

## FLOOP SYSTEM

AND HEADER HEIGHTS

- ENGINEERED WOOD FLOOR TRUSSES
- DESIGNED TO MIN. L/480 DEFLECTION OF LESS - TRUSS MANUFACTURER TO PROVIDE CHASES FOR ALL
- SUPPLY AND RETURN DUCTWORK - TRUGG MANUFACTURER TO VERIFY FRAMING AT CANTILEVERG
- FOR POINT LOADS FROM ABOVE - TRUSS MANUFACTURER TO VERIFY LOCATIONS OF ANY
- CONCENTRATED LOADS, SUCH AS GRANITE COUNTERTOPS, AND PROVIDE PROPER FRAMING AS NEEDED

#### FRAMING

- 2X4 EXTERIOR WALL CONSTRUCTION
- 10' 1-1/8" PLATE HEIGHT (U.N.O.)
- 2XG WALLS AT ALL POCKET DOORS AND
- PLUMBING WALLS
- DOUBLE STUDS AT WINDOWS AND DOOP HEADERS - PROVIDE SOLID BLOCKING AT ALL POINT LOADS
- INDICATES BEARING POINT LOAD
  - PROVIDE CONTINUOUS SOLID BLOCKING TO FOUNDATION BELOW - VERIFY LOADS W/ LOCAL STRUCTURAL ENGINEER

DOOR SCHEDULE SYMBOL

2468 REPRESENTS A 2'-4" WIDE X 6'-8" HIGH DOOP

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++ FOUNDATION ENGINEEPING ++

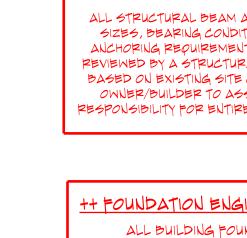
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BASED ON EXISTING SITE CONDITIONS.

1 12/27/19 2X4 EXTERIOR -DML

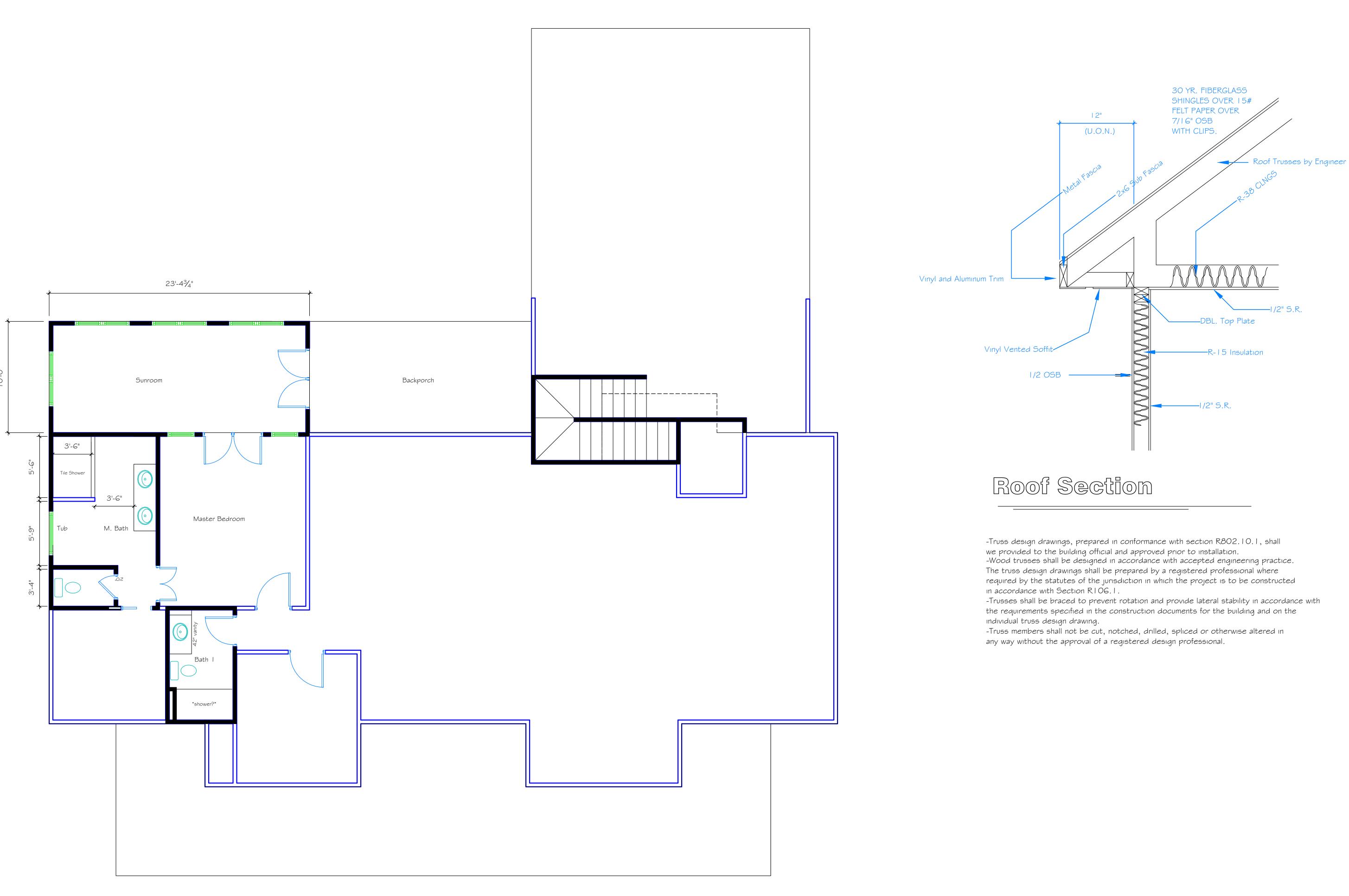
MAIN LEVEL FLOOR PLAN

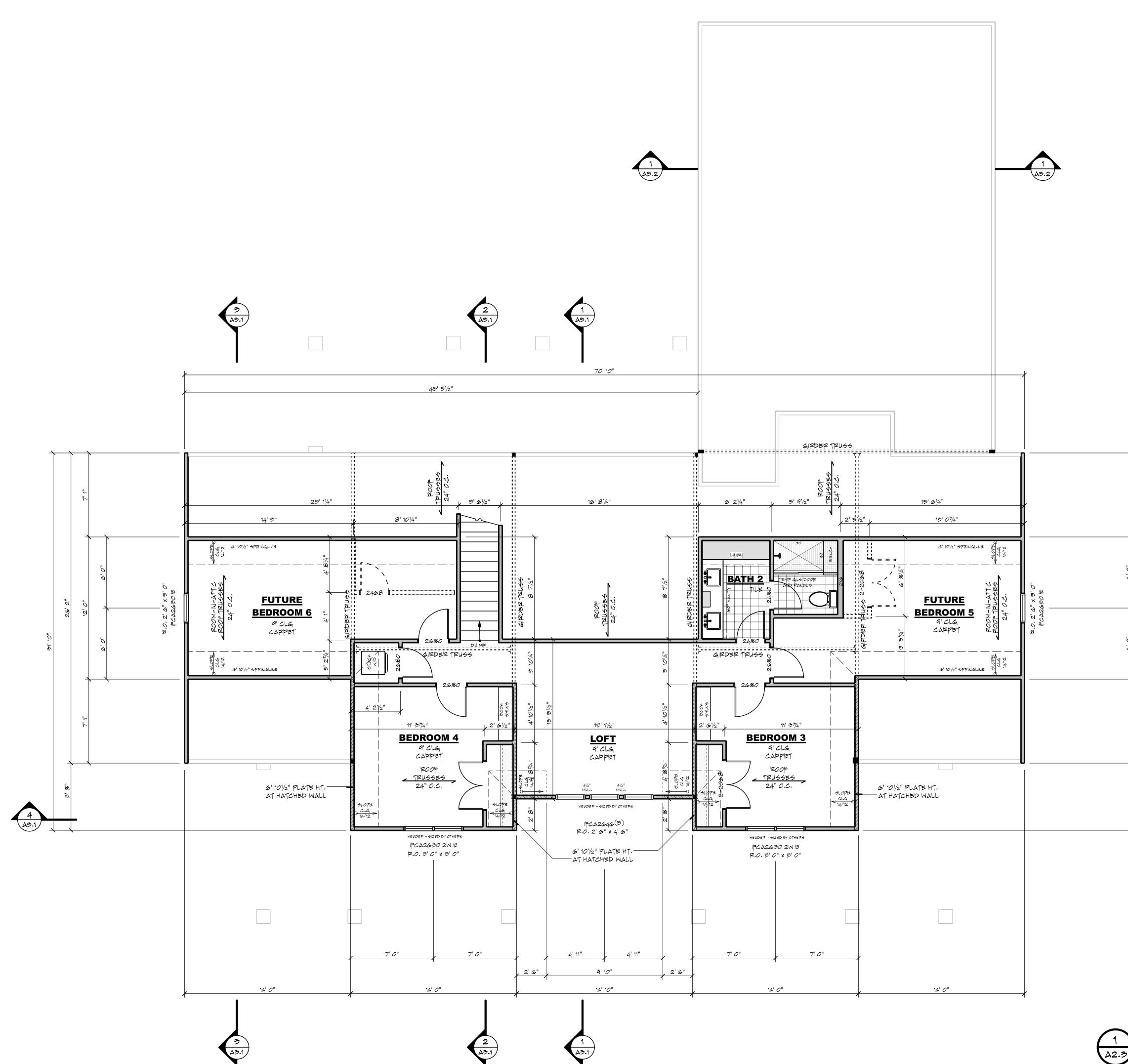




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# Main Level Changes





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## <u>FLOOP SYSTEM</u>

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- TRUSS MANUFACTURER TO PROVIDE CHASES FOR ALL SUPPLY AND RETURN DUCTWORK - TRUSS MANUFACTURER TO VERIFY FRAMING AT CANTILEVERS
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- FRAMING
- 2X4 EXTERIOR STUDS
- 9' 1-1/8" PLATE HEIGHT (U.N.O.)
- 2XG WALLS AT ALL POCKET DOORS AND PLUMBING WALLS.
- DOUBLE STUDS AT WINDOWS AND DOOP HEADERS - PROVIDE SOLID BLOCKING AT ALL POINT LOADS
- INDICATES BEARING POINT LOAD
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1 12/27/19 2X4 EXTERIOR -DML

UPPER LEVEL FLOOR PLAN





Kieran J. Liebl 24-106 ROYAL OAKS DESIGN, INC. ASSUMES NO RESPONSIBILITY FOR ANY STRUCTURAL OR DIMENSIONAL ERRORS OR OMISSIONS. THE CONTRACTOR AND/OR OWNER SHALL BE RESPONSIBLE FOR THE VERIFICATION AND CHECKING OF ALL NOTES, DETAILS, ELEVATIONS, SECTIONS AND FLOOR PLANS AND NOTIFY ROYAL OAKS DESIGN, INC. OF ANY ERRORS PRIOR TO START OF CONSTRUCTION. ROYAL OAKS DESIGN, INC. ASSUMES NO RESPONSIBILITY FOR THE WORKMANSHIP OF THE SUB-CONTRACTORS.

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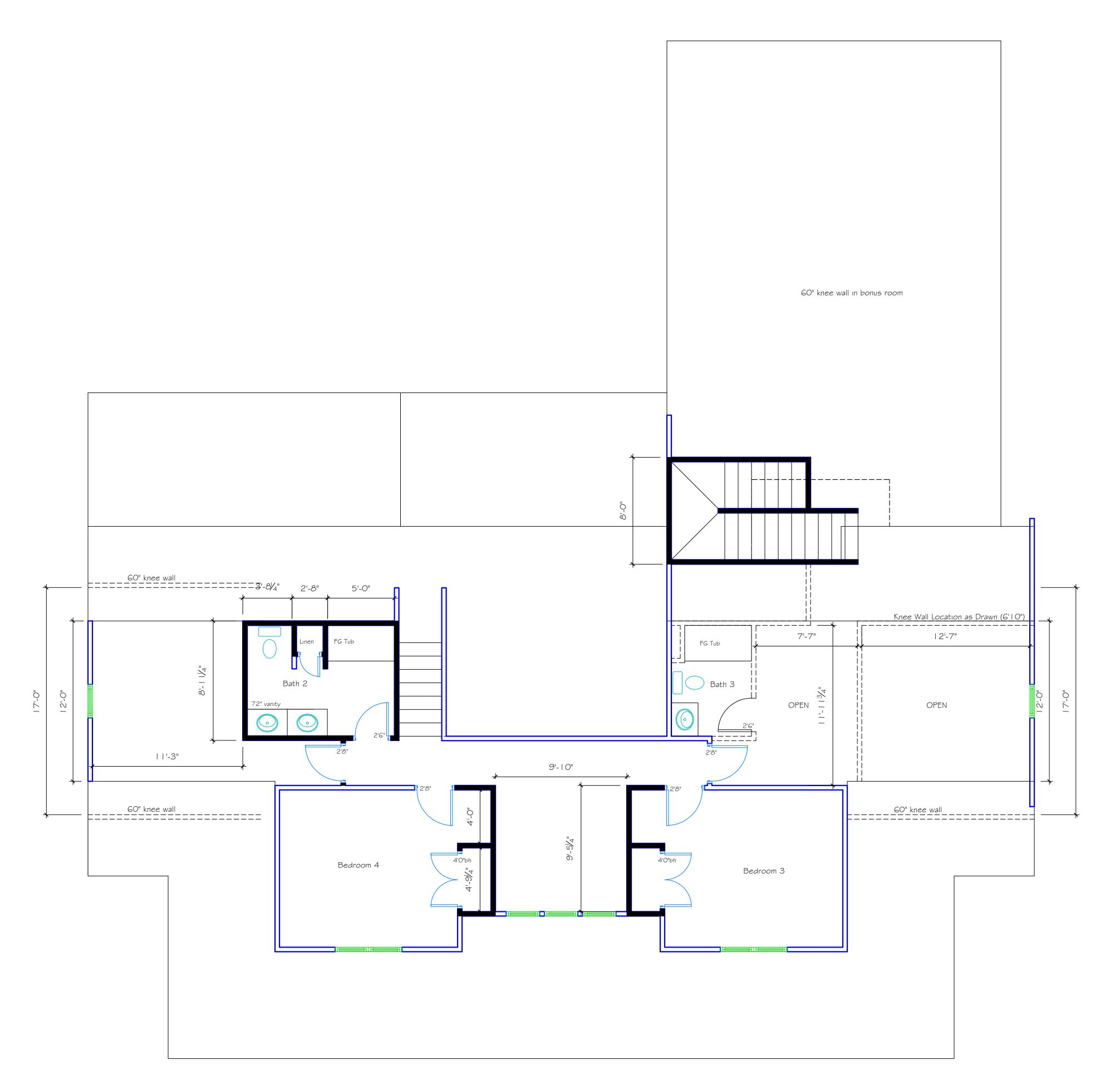
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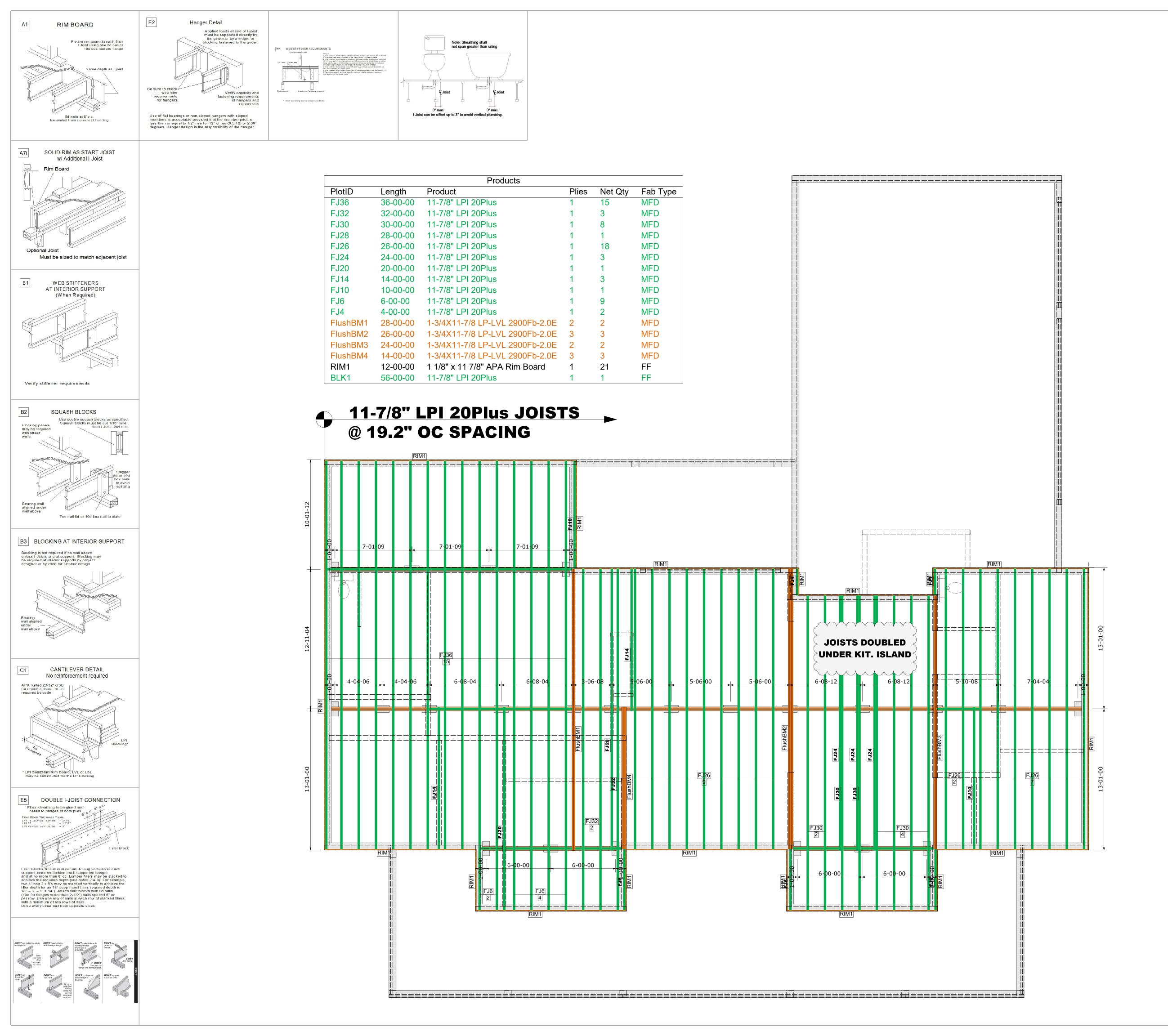


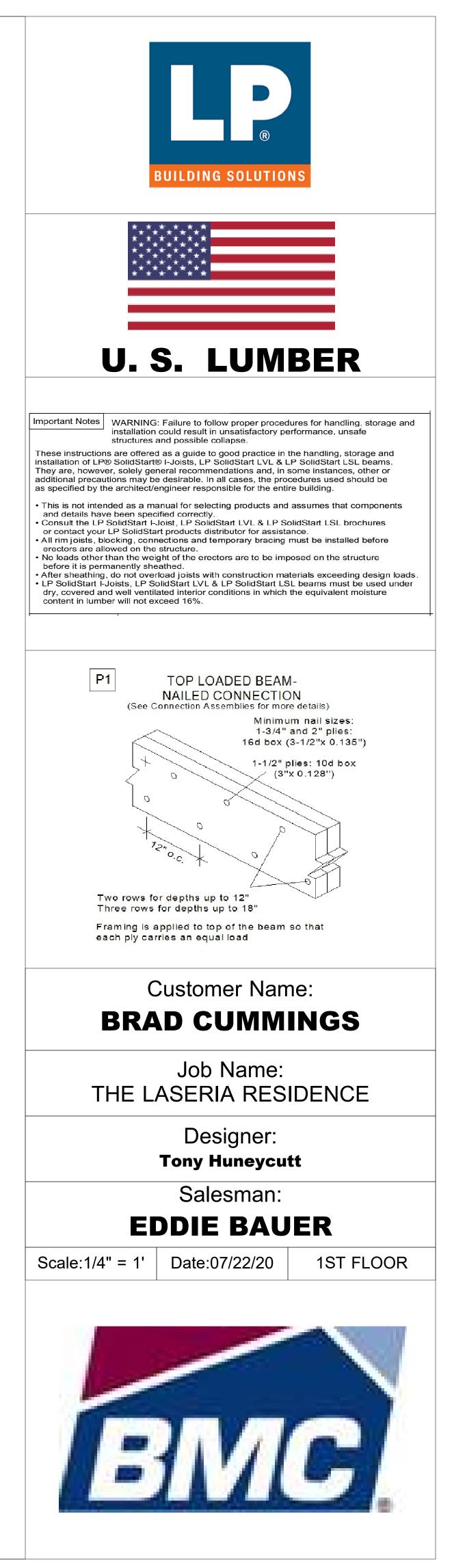


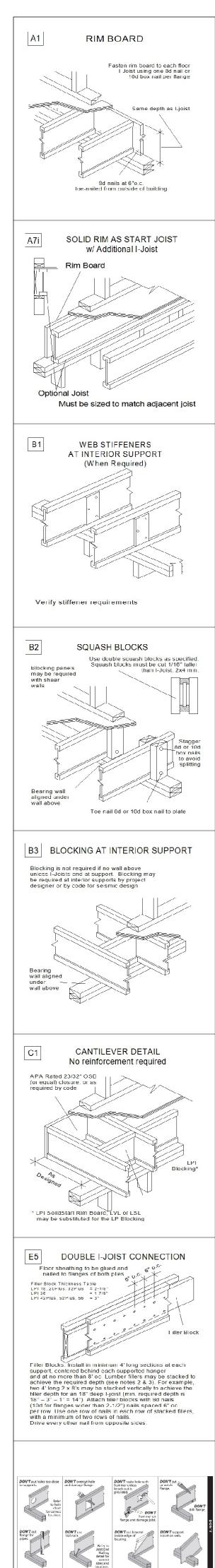
## Upper Level Changes

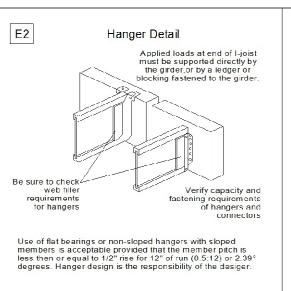


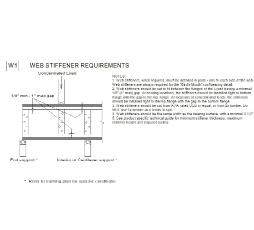


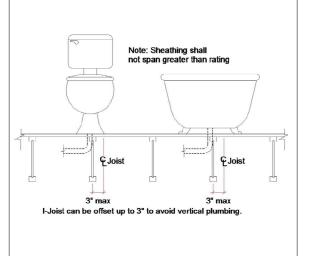




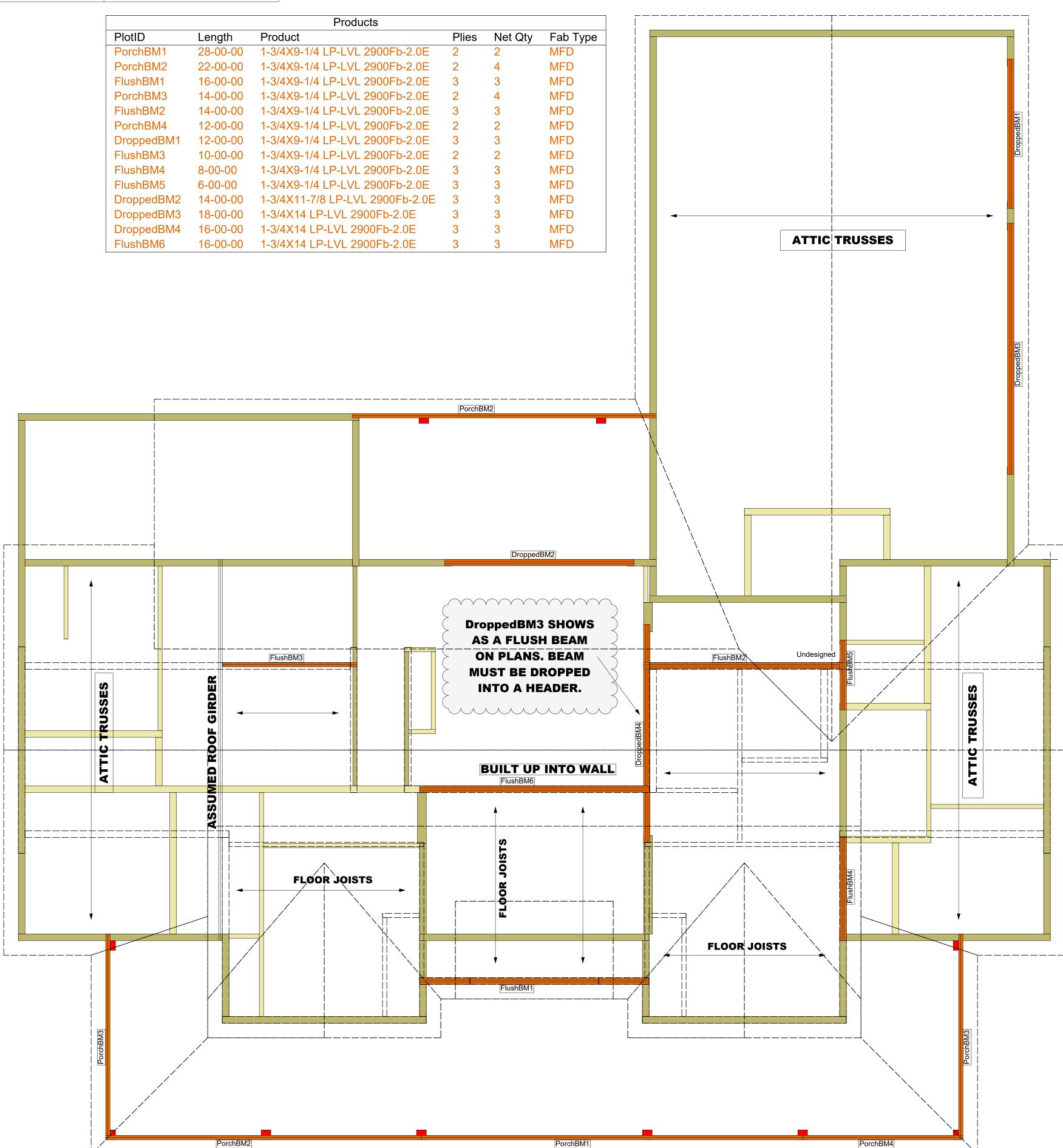




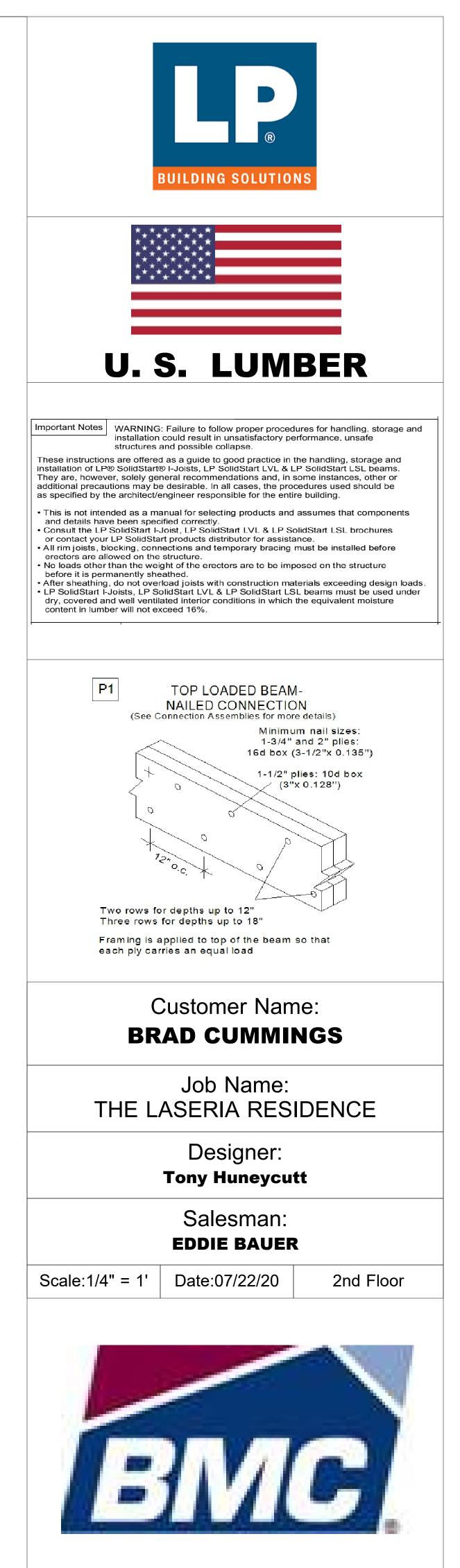




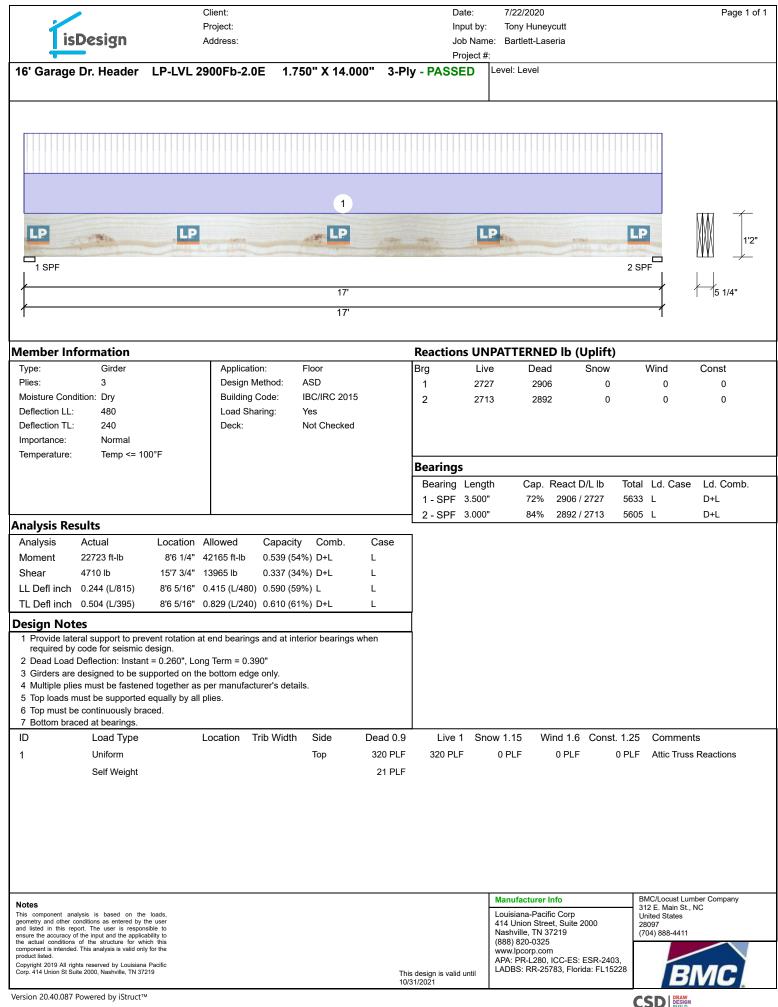
PlotID	Length	Produc
PorchBM1	28-00-00	1-3/4X
PorchBM2	22-00-00	1-3/4X
FlushBM1	16-00-00	1-3/4X
PorchBM3	14-00-00	1-3/4X
FlushBM2	14-00-00	1-3/4X
PorchBM4	12-00-00	1-3/4X
DroppedBM1	12-00-00	1-3/4X
FlushBM3	10-00-00	1-3/4X
FlushBM4	8-00-00	1-3/4X9
FlushBM5	6-00-00	1-3/4X
DroppedBM2	14-00-00	1-3/4X <sup>-</sup>
DroppedBM3	18-00-00	1-3/4X <sup>-</sup>
DroppedBM4	16-00-00	1-3/4X <sup>-</sup>
FlushBM6	16-00-00	1-3/4X



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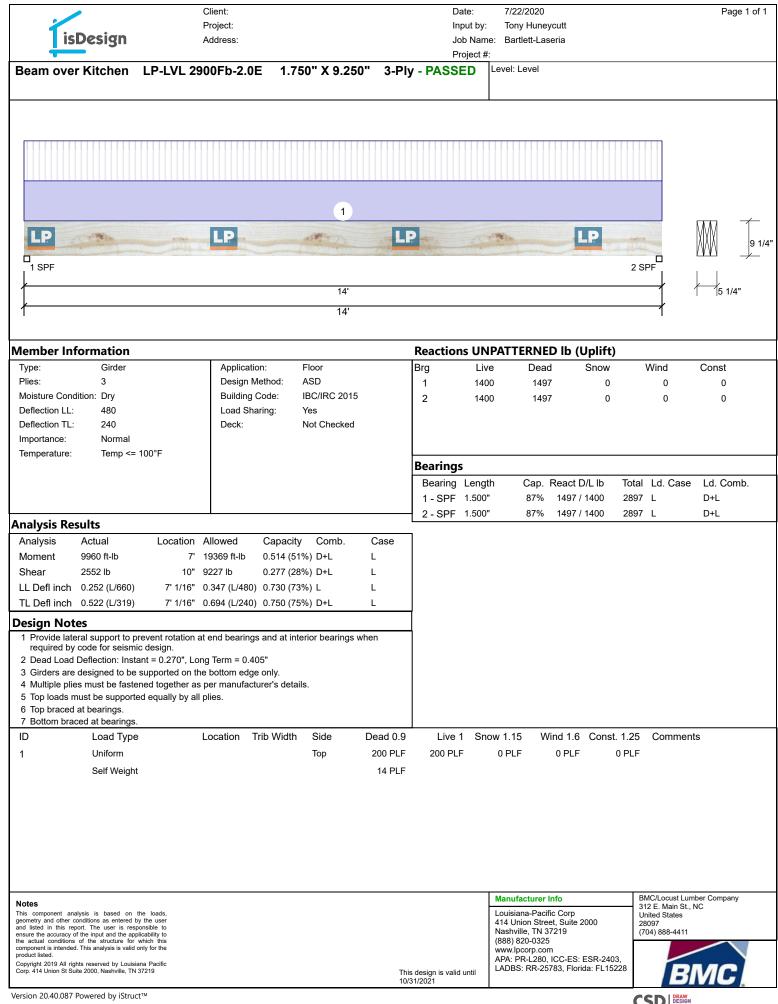


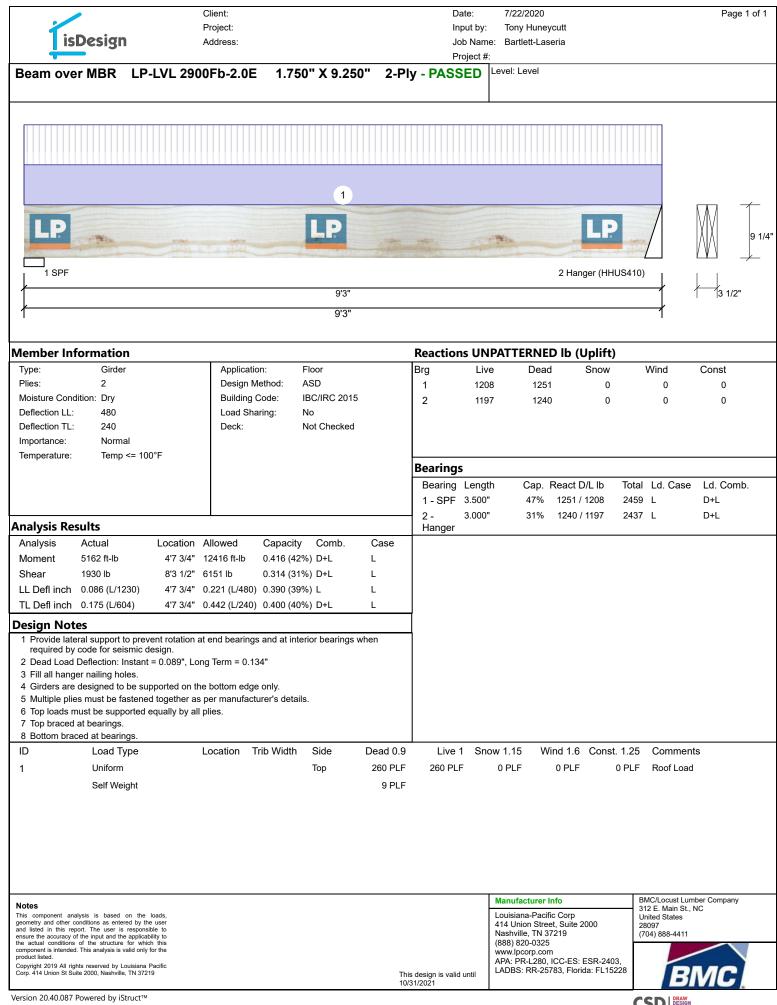
	ient: roject:	Date: 7/22/2020 Page 1 of Input by: Tony Huneycutt
Ac Ac	ddress:	Job Name: Bartlett-Laseria Project #:
' Garage Dr. Header LP-LVL 29	00Fb-2.0E 1.750" X 9.250"	3-Ply - PASSED Level: Level
	1	
LP	LP	
	10'	2 SPF 5 1/4"
<u> </u>	10'	
ember Information		Reactions UNPATTERNED lb (Uplift)
Type:     Girder       Plies:     3       Moisture Condition:     Dry       Deflection LL:     480       Deflection TL:     240	Application:     Floor       Design Method:     ASD       Building Code:     IBC/IRC 2015       Load Sharing:     Yes       Deck:     Not Checked	Brg         Live         Dead         Snow         Wind         Const           1         1600         1669         0         0         0           2         1600         1669         0         0         0
mportance: Normal Femperature: Temp <= 100°F		
		Bearings           Bearing Length         Cap. React D/L lb         Total         Ld. Case         Ld. Comb.           1 - SPF         3.000"         49%         1669 / 1600         3269         L         D+L
nalysis Results		2 - SPF 3.000" 49% 1669 / 1600 3269 L D+L
Shear         2643 lb         11 1/2"         92           LL Defl inch         0.098 (L/1178)         5'         0.2	9369 ft-lb 0.391 (39%) D+L L	
esign Notes		
<ol> <li>Provide lateral support to prevent rotation at e required by code for seismic design.</li> <li>Dead Load Deflection: Instant = 0.102", Long</li> <li>Girders are designed to be supported on the b</li> <li>Multiple plies must be fastened together as per</li> <li>Top loads must be supported equally by all plie</li> <li>Top braced at bearings.</li> <li>Bottom braced at bearings.</li> </ol>	Term = 0.153" pottom edge only. er manufacturer's details.	
ID Load Type Lo 1 Uniform	ocation Trib Width Side Dead ( Top 320 F	PLF 320 PLF 0 PLF 0 PLF 0 PLF Truss Reaction
0 - 16 \ \ \ - 1 - 1 - 1	14 F	'LF
Self Weight		
Self Weight		
Self Weight		
Self Weight  Self weight  Interpretation of the second sec		Manufacturer Info       BMC/Locust Lumber Company         Louisiana-Pacific Corp       312 E. Main St., NC         414 Union Street, Suite 2000       United States         Nashville, TN 37219       28097         (888) 820-0325       (704) 888-4411         www.lpcorp.com       0



Project #:           Beam @ Loft LP-LVL 2900Fb-2.0E         1.750" X 14.000"         3-Ply - PASSED         Level: Level		Client: Project:	Date: Input by		Page 1 c
Image: Section 1         Appendix         Root 1         Control         Section 1         Section 2         <	isDesign	Address:			
Image: Second	Beam @ Loft LP-LVL 2900	Fb-2.0E 1.750" X 14.000" 3-Ply	- PASSED	Level: Level	
Image: Second					
Image: Second					
19F       10*         10*       10*         Reactions UNPATTERNED Ib (Uplift)         Temperators:       Sinder         View       Data Manda         Bailing Capitalian:       Floor         Detection 11:       400         Detection 11:       400         Detection 11:       400         Detection 11:       400         Detection 11:       500         Namesta Condition:       Normal         Bailing Capitalian::       Ves         Detection 11:       400         Detection 11:       500         Namesta Condition:       Normal         Bailing Capitalian::       Ves         Detection 11:       500         Analysis       Activation Allowed         Analysis       Capacity Conth       Capacity Conth         Analysis       Activation Allowed       Capacity Conth       Case         Li Definich 0.030 (LMF3)       e1/16* 0.237 (LM20) 0.510(15%) Did       L         To Definich 0.032 (LMF3)       e1/16* 0.237 (Lm20) 0.510(15%) Did       L         1       Option to benefinity and the detection of the olden mage and a interlor bearings when       2.500 PLF       OPLF       OPLF       Definich 0.328 (LMF3) <td< td=""><td></td><td>1</td><td></td><td></td><td></td></td<>		1			
19F     2 SPF       10°     10°       The state of the st	LP	LP LP		LP.	1'2
Ite       Reactions UNPATTERNED Ib (Uplift)       Type::::::::::::::::::::::::::::::::::::				2	
Importance     Reactions UNPATTERNED bit (Uplift)       Type:     Girder     Application:     Floor       Detection L1:     400       Detection T1::     240       Importance:     Temps < 100°F	·				5 1/4"
Type:     Girder     Application:     Floor     Basing Method:     Application:     Floor       Measure Condition Dry     Design Method:     ASD     Dublication Condition     ASD       Deflection L1:     480     Deflection T1:     240     0     0       Importance:     Normal     Deck:     Not Checked     1     2.2660     2728     0     0     0       Bearing Length     Cap. React D/L Ib     Total Ld. Case     Ld. Comb.     1     Special     Deck:       Importance:     Normal     Deck:     Nor Checked     Deck     Deck:     Deck     Deck       Importance:     Normal     Location Allowed     Capacity Comb.     Case     Deck     Deck     Deck       Moment     19958 1-bb     4 2165 fb 0.473 (47%) D+L     L     L     Deck     Deck     Deck     Deck       1 Deflection 0.1091/1981 /bb     8 1/16" 0.389 (L4473)     8 1/16" 0.389 (L4473)     Deck     Deck     Deck       1 Deflection 0.388 (L4473)     8 1/16" 0.379 (C7%) D+L     L     Deck     Deck     Deck       2 Deck Ld Deflection: Instart = 0.237', Long Term = 0.304"     3 dicters are designed to be supported on the totom dege ony.     4 Multiple is Decker are manumed 9'10 /8" o.c.     Top 320 PLF     0 PLF     0 PLF     0 PLF	1	16'			1
Piles:       3       Design Method: ASD         Mosture Condition:       Dy       Design Method: Code:       11       2560       2728       0       0       0         Deflection LL:       480       Deck:       Not Checked       1       2560       2728       0       0       0       0         Importance:       Nordification:       Temperature:       Temperature:       Nordification:       1       2560       2728       0       0       0       0       0         Importance:       Nordification:       Nordification:       Nordification:       Nordification:       1       2560       2728       0	lember Information		Reactions UN	NPATTERNED lb (Uplift)	
Moistare Condition: Dry Deflection: Li: 480 Deflection: Li: 480 Deflection: Normal Importance: Normal Temperature: Temp <= 100°F	71				
Deflection LL:       480 importance:       Normal Temperature:       Load Sharing:       Yes Deck:         Bearings       Bearings         Bearing Length Cap.       Cap.       React DI. Lb       Total       Ld. Case       Ld. Case         Analysis       Actual       Location       Allowed       Capacity       Comb.       Case         Sheer       4966       b       17 114'       1306 b       313 (313's)       D-1       L         Sheer       4966 b       117 114'       1306 b       333 (147's)       D-1       L         LLD defineh       0.190 (Lu81)       8' 11/16'       0.338 (U480)       D-40 (57's)       L       L         LLD defineh       0.393 (U47's)       8' 11/16'       0.338 (U480)       D-40 (14's)       L       L         1       Deck       may into the started capather at the temperature into the attribute on dago only.       L </td <td></td> <td>0</td> <td></td> <td></td> <td></td>		0			
Importance:         Normal Temperature:         Temp == 100°F           Bearings         Bearings           Indysis Actual Analysis Actual Shear 4386 lb         Location Allowed Capacity Comb. 848, 2728 / 2500         5288 L         D+L           Nalysis Actual Shear 4386 lb         147 1/4" 13965 lb         0.473 (47%) D+L         L           LDefl inch         0.190 (L981)         8" 1/16" 0.377 (L7240)         0.510 (51%) D+L         L           LDefl inch         0.393 (L475)         8" 1/16" 0.777 (L7240)         0.510 (51%) D+L         L           Physics Actual Whitep Else mathematic design.         2         Dead coal Deflection: Instant - 0.20% Loss 10 (51%) D+L         L           2 Obdit coal Deflection: Instant - 0.20% Loss 10 (stars)         8" 1/16" 0.777 (L7240) 0.510 (51%) D+L         L           2 Obdit coal Deflection: Instant - 0.20% Loss 10 (stars)         0.510 (51%) D+L         L           2 Obdit coal Deflection: Instant - 0.20% Loss 10 (stars)         1.016 (stars)         1.016 (stars)           3 Girdes rave designed to be supported on the bottom edge only.         4.0016 (stars)         1.016 (stars)         1.02 (stars)           1 Dead coal Deflection: Instant - 0.20% Loss 10 (uniform)         Location Trib Width Side         Dead 0.9         Live 1 Snow 1.15         Wind 1.6         Const. 1.25         Comments 1.016 (stars)           1 Londorm	Deflection LL: 480	Load Sharing: Yes			
Temperature:       Temp < 100°F		Deck: Not Checked			
Bearings       Bearings       1. S. PF 3.500°     Cap. React D/L Ib     Total     Ld. Case     Ld. Comb.       2. SPF 3.500°     68%     2728 / 2560     5288     D+L       Analysis     Actual     Location     Allowed     Capacity Comb.     Case       Shear     4366 lb     18' 176'     0.338 (14%) D+L     L       LL Defl inch     0.393 (1475)     8' 116'     0.338 (14%) D+L     L       LL Defl inch     0.393 (1475)     8' 116'     0.398 (1480)     0.490 (49%) L     L       Reside by code for seismic design.     2     2037 (15%) D+L     L     E       2 Ded Load Defloction: Instant = 0.203'. Long Term = 0.304'     3     3     3       3 Griders are designed to be supported or the bottom edge only.     4     Multipe jelies manufacturer's details.     5       5 Top loads must be fastened togethera a per manufacturer's details.     5     5     5     2032 LC and Topic Deflore age manufacturer's details.     5       1     Uniform     Top as 20 PLF     320 PLF     0 PLF     0 PLF     0 PLF       1     Uniform     Top as 20 PLF     320 PLF     0 PLF     0 PLF     Roof Load       1     Uniform     Top as 20 PLF     320 PLF     0 PLF     0 PLF     Roof Load <td>•</td> <td></td> <td></td> <td></td> <td></td>	•				
Bearing Length     Cap. React D/L Ib     Total     L4. Case     L4. Comb.       nalysis     Actual     Location Allowed     Capacity Comb.     Case       Voment     19958 Hzb     8' 42165 ftb     0.473 (47%) D+L     L       2. SPF     3.500°     68%     2728 / 2560     5288 L     D+L       2. SPF     3.500°     68%     2728 / 2560     5288 L     D+L       2. SPF     3.500°     68%     2728 / 2560     5288 L     D+L       2. SPF     3.500°     68%     2728 / 2560     5288 L     D+L       2. SPF     3.500°     68%     2728 / 2560     5288 L     D+L       2. SPF     3.500°     68%     2728 / 2560     5288 L     D+L       1. LD efilion     0.190 (Li981)     8' 116°     0.383 (19%) D+L     L       1. Definit     0.398 (L428)     0.490 (49%) L     L       2. Cad Code Defection: Instant = 0.203°; Long Term = 0.304°     3     3     3       3 Circles are developed on the backings     500 load markings the backened together as per manufacturer's details.     5     5     For must be laterally brand at maximum of 9'10 7/8' o.c.       1     Bottom braced at bearings.     1     Uniform     Top     320 PLF     320 PLF     0 PLF     0 PLF     0 PLF	Temperature: Temp <= 100°F		Boarings		
1 - SPF 3.500*         68%         2728 / 2560         5288         L         D+L           Analysis         Actual         Location         Allowed         Capacity         Comb.         Case         2. SPF 3.500*         68%         2728 / 2560         5288         L         D+L           Analysis         Actual         Location         Allowed         Capacity         Comb.         Case           Moment         19958 ft-b         8' 42165 ft-b         0.473 (47%) D+L         L </td <td></td> <td></td> <td></td> <td></td> <td>tal Id Casa Id Camb</td>					tal Id Casa Id Camb
analysis       2. SPF 3.50°       68%       2728 / 2580       5288 L       D+L         Analysis       Actual       Location       Allowed       Capacity Comb.       Case         Moment       19958 ft-lb       8' 42165 ft-lb       0.473 (47%) D+L       L         Libe Definicion (1901 (U881)       8' 114°       13965 lb       0.313 (31%) D+L       L         Libe Definicion (1901 (U881)       8' 1116°       0.777 (U240)       0.510 (51%) D+L       L         esign Notes					
nalysis Results         Analysis       Actual       Location       Allowed       Capacity       Comb.       Case         Moment       19958 ft-lb       8' 42165 ft-lb       0.473 (47%) D+L       L         Shear       4366 lb       1477 114"       13965 lb       0.313 (31%) D+L       L         LL Defl inch       0.190 (L981)       8' 1116"       0.399 (L475)       B' 1116"       0.399 (L475)         The Defl inch       0.193 (L475)       8' 1116"       0.777 (L/240)       0.510 (51%) D+L       L         Esign Notes       1       Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for selsmic design.       2       Dead Load Deflection: Instant = 0.203", Long Term = 0.304"       3         2 Dead Load Deflection: instant = 0.203", Long Term = 0.304"       3       Giders are designed to be supported on the bottom edge only.       4         4 Multiple pleas are designed.       10 bead must be supported organity at a maximum of 9'10 7/8" o.c.       7       Foldom braced at bearings.       10       Live 1       Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform       Top       320 PLF       320 PLF       0 PLF       0 PLF       Roof Load         1       Uniform       Self Weight					
Moment       19958 ft-lb       8' 42165 ft-lb       0.473 (47%) D+L       L         Shear       4366 lb       14'7 1/4'       13965 lb       0.313 (31%) D+L       L         LL Defl inch       0.190 (L/981)       8' 1/16'       0.389 (L/480)       0.490 (49%) L       L         TD Defl inch       0.333 (L/475)       8' 1/16'       0.389 (L/480)       0.490 (49%) L       L         esign Note:       1       1       0.333 (L/475)       8' 1/16'       0.373 (4'7%) D+L       L         esign Note:       1       0.333 (L/475)       0.510 (51%) D+L       L       L         esign Note:       1       0.333 (L/475)       0.377 (L/240)       0.510 (51%) D+L       L         esign Note:       1       0.330 (L/75)       0.511 (51%) D+L       L       L         esign Note:       1       0.304       1.051 (51%) D+L       L       L         esign Note:       1       0.045 (51%) D+L       L       L       L       L       L         1       0.045 (198) D+1       1       D       Load Type       Location Trib Width       Side       Dead 0.9       Live 1       Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform	nalysis Results		2-011 0.000		
Shear       4366 lb       147 1/4' 13965 lb       0.313 (31%) D+L       L         LL Definch       0.190 (U981)       8' 1/16''       0.389 (L/480)       0.490 (49%) L       L         TL Definch       0.393 (U475)       8' 1/16''       0.3777 (L/240)       0.510 (51%) D+L       L         esign Notes	Analysis Actual Location	n Allowed Capacity Comb. Case			
LL Defl inch       0.190 (L/981)       8' 1/16''       0.389 (L/475)       8' 1/16''       0.393 (L/475)       8' 1/16''       0.393 (L/475)       8' 1/16''       0.393 (L/475)       8' 1/16''       0.2777 (L/240)       0.510 (51%) D+L       L         tesign Notes         1       Provide lateral support to prevent rotation at end bearings and at interior bearings when required by orde for seismic design.       2       2       2       2       2       2       2       2       2       3       2       3       2       3       2       3       3       1       1       0.303 (L/475)       8' 1/16'' 0.348 (L/480) (L	Moment 19958 ft-lb 8	3' 42165 ft-lb 0.473 (47%) D+L L			
TL Defl inch 0.393 (L475)       8' 1/16" 0.777 (L240) 0.510 (51%) D+L       L         esign Notes       1       Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for sasimic design.       2         2 Dead Load Deflection: Instant = 0.203", Long Term = 0.304"       3       Girders are designed to be supported on the bottom edge only.         4 Multiple places must be fastened together as per manufacturer's details.       5       To post be laterally braced at a maximum of 9'107/8" o.c.         7 Do tads must be supported equally by all ples.       6       Dead Type       Location Trib Width Side       Dead 0.9       Live 1 Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform       Top x320 PLF       320 PLF       0 PLF       0 PLF       0 PLF       Roof Load         2 Self Weight       21 PLF       21 PLF       Dualsane-Pacific Cop       Nature 1.00       <		( )			
esign Notes         1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.         2 Dead Load Deflection: Instant = 0.203°, Long Term = 0.304°         3 Giders are designed to be supported on the bottom edge only.         4 Multiple plies must be fastened together as per manufacturer's details.         5 Top loads must be supported equally by all plies.         6 Top must be laterally braced at a maximum of 9'10 7/8" o.c.         7 Bottom braced at bearings.         D       Load Type         1 Uniform       Top 320 PLF         3 Self Weight       21 PLF					
1 Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design. 2 Dead Load Deflection: Instant = 0.203', Long Term = 0.304* 3 Girders are designed to be supported on the bottom edge only. 4 Multiple plices must be fastened together as per manufacturer's details. 5 Top loads must be supported equally by all plies. 6 Top must be laterally braced at a maximum of 9'10 7/8" o.c. 7 Bottom braced at bearings. D Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 320 PLF 320 PLF 0 PLF 0 PLF 0 PLF Roof Load Self Weight 21 PLF	TL Defl inch 0.393 (L/475) 8' 1/16	" 0.777 (L/240) 0.510 (51%) D+L L	4		
required by code for seismic design. 2 Dead Load Deflection: Instant = 0.203", Long Term = 0.304" 3 Girders are designed to be supported on the bottom edge only. 4 Multiple plies must be supported equally by all plies. 5 Top loads must be supported equally by all plies. 6 Top must be laterally braced at a maximum of 9°10 7/8" o.c. 7 Bottom braced at bearings. D Load Type Location Trib Width Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 320 PLF 320 PLF 0 PLF 0 PLF 0 PLF Roof Load Self Weight 21 PLF					
2 Dead Load Deflection: Instant = 0.203*, Long Term = 0.304*         3 Giders are designed to be supported on the bottom edge only.         4 Multiple pities must be fastened together as per manufacturer's details.         5 Top loads must be supported equally by all piles.         6 Top must be laterally braced at a maximum of 9'10 7/8' o.c.         7 Bottom braced at bearings.         ID       Load Type       Location         1       Uniform         5 Self Weight       21 PLF         Self Weight       21 PLF		n at end bearings and at interior bearings when			
4 Multiple plies must be fastened together as per manufacturer's details.       5 Top loads must be supported equally by all plies.         5 Top must be laterally braced at a maximum of 9'10 7/8" o.c.         7 Bottom braced at bearings.         ID       Load Type       Location       Trib Width       Side       Dead 0.9       Live 1       Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform       Top       320 PLF       320 PLF       0 PLF       0 PLF       0 PLF       Roof Load         3 Self Weight       21 PLF       21 PLF       Description of the pust of the specific top of the specific to		Long Term = 0.304"			
5 Top loads must be supported equally by all plies.         6 Top must be laterally braced at a maximum of 9'10 7/8" o.c.         7 Bottom braced at bearings.         ID       Load Type       Location       Trib Width       Side       Dead 0.9       Live 1       Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform       Top       320 PLF       320 PLF       0 PLF       0 PLF       0 PLF       Roof Load         Self Weight       21 PLF       21 PLF       BMCLocust Lumber Company         Notes       Self Weight       21 PLF       Louisiana-Pacific Corp       Manufacturer Info       BMCLocust Lumber Company         In the report. The user is responsible to more reading to the base, or the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the standys is valid only for the reading of the storade to the loads, the storade to the storade to the loads, the storade t					
6 Top must be laterally braced at a maximum of 9'10 7/8" o.c.         7 Bottom braced at bearings.         ID       Load Type       Location       Trib Width       Side       Dead 0.9       Live 1       Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform       Top       320 PLF       0 PLF       0 PLF       0 PLF       Roof Load         Self Weight       21 PLF       21 PLF       BMC/Locust Lumber Company       Manufacturer Info       BMC/Locust Lumber Company         Notes       Name the accase, of the input and the input able is based on the loads, emerged by the user and is the report. The user is responsible to may the input able is instructed. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurrent is indended. This analysis is valid only for the incurent is indended. This analysis is valid only for the incurrent is					
ID       Load Type       Location       Trib Wildth       Side       Dead 0.9       Live 1       Snow 1.15       Wind 1.6       Const. 1.25       Comments         1       Uniform       Top       320 PLF       320 PLF       0 PLF       0 PLF       0 PLF       0 PLF       Roof Load         Self Weight       21 PLF       21 PLF        Manufacturer Info       BMC/Locust Lumber Company, 312 E. Main St., NC       Noted States       Noted	6 Top must be laterally braced at a maximu				
Image: Note of the second of the standard of					
Self Weight     21 PLF       Integer     21 PLF         Integer     Manufacturer Info       Integer     Louisiana-Pacific Corp 414 Union Street, Suite 2000, Nashville, TN 37219       Opright 2019 Alt rights reserved by Louisiana Pacific 0, 214 Union Stute 2000, United States 2007       Opright 2019 Alt rights reserved by Louisiana Pacific 0, 214 Union Stute 2000, United States 2007       This design is valid until	51				
Manufacturer Info     BMC/Locust Lumber Company 312 E. Main St, NC       Inised in the report. The user is responsible to have the accuracy of the input and the applicability to secure to accuracy of the input and the applicability to the acturacy of the input and the applicability to secure to accuracy of the input and the applicability to securacy of the input and the applicability to secure to ac	1 Uniform			0 PLF 0 PLF 0 P	LF Roof Load
Jotes       312 E. Main St., NC         bis component analysis is based on the loads,       E. Douisiana-Pacific Corp         sometry and other conditions as entered by the user       141 Union Street, Suite 2000         nd listed in this report. The user is responsible to       Nashville, TN 37219         nsure the accuracy of the input and the applicability to       (888) 820-0325         momponent is interded. This analysis is valid only for the       www.lpcorp.com         opyright 2019 All rights reserved by Louisiana Pacific       APA: PR-L280, ICC-ES: ESR-2403,         LADBS: RR-25783, Florida: FL15228       Image: State 2000	Self Weight	21 PL	=		
Notes       312 E. Main St., NC         his component analysis is based on the loads,       Envisiona-Pacific Corp         eometry and other conditions as entered by the user       141 Union Street, Suite 2000         noise the accuracy of the input and the applicability to       Nashville, TN 37219         readul conditions of the structure for which this       (888) 820-0325         omponent initiated.       Nashville, TN 37219         (B88) 820-0325       Wow.lpcorp.com         APA: PR-L280, ICC-ES: ESR-2403,       LADBS: RR-25783, Florida: FL15228					
Jotes       312 E. Main St., NC         bis component analysis is based on the loads,       E. Douisiana-Pacific Corp         sometry and other conditions as entered by the user       141 Union Street, Suite 2000         nd listed in this report. The user is responsible to       Nashville, TN 37219         nsure the accuracy of the input and the applicability to       (888) 820-0325         momponent is interded. This analysis is valid only for the       www.lpcorp.com         opyright 2019 All rights reserved by Louisiana Pacific       APA: PR-L280, ICC-ES: ESR-2403,         LADBS: RR-25783, Florida: FL15228       Image: State 2000					
Jotes       312 E. Main St., NC         bis component analysis is based on the loads,       E. Douisiana-Pacific Corp         sometry and other conditions as entered by the user       141 Union Street, Suite 2000         nd listed in this report. The user is responsible to       Nashville, TN 37219         nsure the accuracy of the input and the applicability to       (888) 820-0325         momponent is interded. This analysis is valid only for the       www.lpcorp.com         opyright 2019 All rights reserved by Louisiana Pacific       APA: PR-L280, ICC-ES: ESR-2403,         LADBS: RR-25783, Florida: FL15228       Image: State 2000					
Otes       312 E. Main St., NC         us component analysis is based on the loads,       Louisiana-Pacific Corp         sometry and other conditions as entered by the user       1414 Union Street, Suite 2000         Nashville, TN 37219       (888) 820-0325         www.lpcorp.com       APA: PR-L280, ICC-ES: ESR-2403,         pyright 2019 All rights reserved by Louisiana Pacific org, 414 Union St Suite 2000, Nashville, TN 37219       This design is valid untit					
Otes       312 E. Main St., NC         us component analysis is based on the loads,       Louisiana-Pacific Corp         sometry and other conditions as entered by the user       1414 Union Street, Suite 2000         Nashville, TN 37219       (888) 820-0325         www.lpcorp.com       APA: PR-L280, ICC-ES: ESR-2403,         pyright 2019 All rights reserved by Louisiana Pacific org, 414 Union St Suite 2000, Nashville, TN 37219       This design is valid untit					
Otes       312 E. Main St., NC         us component analysis is based on the loads,       Louisiana-Pacific Corp         sometry and other conditions as entered by the user       1414 Union Street, Suite 2000         Nashville, TN 37219       (888) 820-0325         www.lpcorp.com       APA: PR-L280, ICC-ES: ESR-2403,         pyright 2019 All rights reserved by Louisiana Pacific org, 414 Union St Suite 2000, Nashville, TN 37219       This design is valid untit					
Jotes       312 E. Main St., NC         bis component analysis is based on the loads,       E. Douisiana-Pacific Corp         sometry and other conditions as entered by the user       141 Union Street, Suite 2000         nd listed in this report. The user is responsible to       Nashville, TN 37219         nsure the accuracy of the input and the applicability to       (888) 820-0325         momponent is interded. This analysis is valid only for the       www.lpcorp.com         opyright 2019 All rights reserved by Louisiana Pacific       APA: PR-L280, ICC-ES: ESR-2403,         LADBS: RR-25783, Florida: FL15228       Image: State 2000					
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Ind listed in this report. The user is responsible to market accuracy of the input and the applicability to the actual conditions of the structure for which this omponent is intended. This analysis is valid only for the roduct listed.       Nashville, TN 37219 (888-4411         Nashville, TN 37219 (888) 820-0325       Work Jacorp.com         ApA: PR-L280, ICC-ES: ESR-2403, LADBS: RR-25783, Florida: FL15228	eometry and other conditions as entered by the user				United States
omponent is intended. This analysis is valid only for the roduct listed. roduct listed. opyright 2019 All rights reserved by Louisiana Pacific orp. 414 Union St Suite 2000, Nashville, TN 37219 This design is valid until This design is valid until	nsure the accuracy of the input and the applicability to			Nashville, TN 37219	
Copyright 2019 All rights reserved by Louisiana Pacific Corp. 414 Union St Suite 2000, Nashville, TN 37219 This design is valid until	omponent is intended. This analysis is valid only for the			www.lpcorp.com	
	Copyright 2019 All rights reserved by Louisiana Pacific	-		APA: PR-L280, ICC-ES: ESR-2403, LADBS: RR-25783, Florida: FL15228	DMO
					BIVIC
rsion 20.40.087 Powered by iStruct™ CSD BRAW DESIGN BUILD ESIGN B					

Address: Beam left of Range LP-LVL 2900Fb-2.0E 1.750" X 9.250" 3-P	Input by: Tony Huneycutt Job Name: Bartlett-Laseria
	Project #: IY - PASSED Level: Level
i	
LP LP	
1 SPF 7'2" 7'2"	2 SPF 5 1/4"
ember Information	Reactions UNPATTERNED Ib (Uplift)
Type:GirderApplication:FloorPlies:3Design Method:ASDMoisture Condition:DryBuilding Code:IBC/IRC 2015Deflection LL:480Load Sharing:YesDeflection TL:240Deck:Not CheckedImportance:NormalNormalNormal	Brg         Live         Dead         Snow         Wind         Const           1         1003         301         0         0         0           2         1003         301         0         0         0
Temperature: Temp <= 100°F	Bearings
	Bearing         Length         Cap.         React D/L lb         Total         Ld. Case         Ld. Comb.           1 - SPF         3.500"         17%         301 / 1003         1304         L         D+L
nalysis Results	2 - SPF 3.500" 17% 301 / 1003 1304 L D+L
Analysis         Actual         Location         Allowed         Capacity         Comb.         Case           Moment         2047 ft-lb         3'7"         19369 ft-lb         0.106 (11%)         D+L         L           Shear         940 lb         6'2"         9227 lb         0.102 (10%)         D+L         L           LL Defl inch         0.022 (L/3633)         3'7 1/16"         0.168 (L/480)         0.130 (13%) L         L           TL Defl inch         0.029 (L/2795)         3'7 1/16"         0.335 (L/240)         0.090 (9%)         D+L         L	
Pesign Notes	-
<ol> <li>Provide lateral support to prevent rotation at end bearings and at interior bearings when required by code for seismic design.</li> <li>Dead Load Deflection: Instant = 0.007", Long Term = 0.010"</li> <li>Girders are designed to be supported on the bottom edge only.</li> <li>Multiple plies must be fastened together as per manufacturer's details.</li> <li>Top loads must be supported equally by all plies.</li> <li>Top braced at bearings.</li> <li>Bottom braced at bearings.</li> </ol>	
ID Load Type Location Trib Width Side Dead 0.5 1 Uniform Top 70 PLF	
1 Uniform Top 70 PLF	

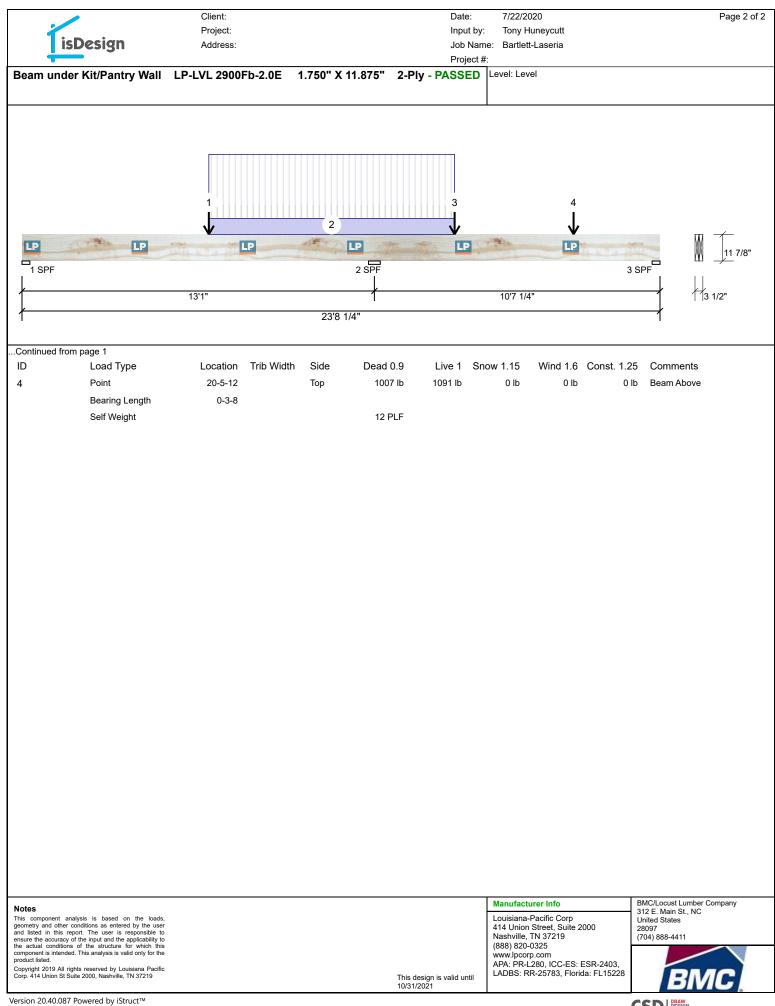




	Client:				/22/2020			Page 1 of 1
	Project: .ddress:		Jo	b Name: B	ony Huneycutt artlett-Laseria			
Geam Right of Range LP-LVL 2	900Fb-2.0F 1.750" X 9 '	250" 3-P	Pr Ply - PAS	oject #:	el: Level			
	.5001 D-2.0E 1.750 X 5.	200 5-6	iy-rA0					
	2							
1								
LP		P						
1 SPF		2 SPF						
<u>د</u> ــــــــــــــــــــــــــــــــــــ	5'		,					5 1/4"
1	5'	1						
Nember Information			Reaction	s UNPAT	TERNED Ib	(Uplift)		
Type: Girder Plies: 3	Application: Floor Design Method: ASD		Brg	Live	Dead 769	Snow	Wind	Const
Moisture Condition: Dry	Building Code: IBC/IRC 2015		1 2	1149 1091	769 1007	0 0	0 0	0 0
Deflection LL: 480 Deflection TL: 240 Importance: Normal	Load Sharing: Yes Deck: Not Checked							
Temperature: Temp <= 100°F		-	Bearings					
		-	Bearing 1 - SPF	Length 3.500"		) / 1149 1	otal Ld. Case	D+L
nalysis Results	1	[	2 - SPF	3.500"	27% 1007	/ 1091 2	2098 L	D+L
Analysis Actual Location A	1 2	Case						
	9369 ft-lb 0.190 (19%) D+L 227 lb 0.226 (23%) D+L	L						
	.114 (L/480) 0.120 (12%) L	L						
TL Defl inch 0.025 (L/2197) 2'10 1/16" 0.	.227 (L/240) 0.110 (11%) D+L	L						
Design Notes								
<ol> <li>Provide lateral support to prevent rotation at a required by code for seismic design.</li> <li>Dead Load Deflection: Instant = 0.012", Long</li> <li>Girders are designed to be supported on the</li> </ol>	; Term = 0.017"	when						
<ul> <li>4 Multiple plies must be fastened together as p</li> <li>5 Top loads must be supported equally by all pl</li> <li>6 Top braced at bearings.</li> <li>7 Bottom braced at bearings.</li> </ul>	er manufacturer's details.							
ID Load Type Lo	ocation Trib Width Side	Dead 0.9		Snow 1.		6 Const. 1		
	to 3-0-0 Top	70 PLF	280 PLF				PLF Floor Loa	-
	3-0-0 Near Face		1400 lb	) (	ווס מונ	D (	UID Beam ove	er Kitchen Brg 2
1 Part. Uniform 0-0-0 2 Point Self Weight	to 3-0-0 Top 3-0-0 Near Face	70 PLF 1497 lb 14 PLF	280 PLF 1400 lb		₽LF 0 PLI			d er Kitchen Brg 2
				Man	ufacturer Info		BMC/Locust Lur 312 E. Main St.,	mber Company
Notes				Loui	isiana-Pacific Corp		United States	NO
This component analysis is based on the loads, geometry and other conditions as entered by the user				414	Union Street, Suite	e 2000	28097	
This component analysis is based on the loads,				414 Nasi (888		e 2000		

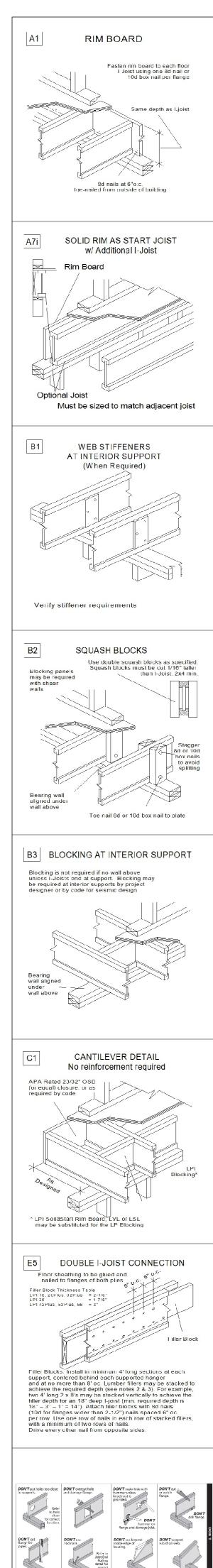
		Client:					ate:	7/22/2020				Page 1 c
<b>T</b> isD	esign	Project: Address:					put by:	Tony Hune Bartlett-La	-			
	631311	Address.					roject #:	Dartiett-Ed.	50110			
Beam under K	it./Great Room Wall	LP-LVL 29	00Fb-2.0I	E 1.750" X	11.875"	3-Ply - PA	SSEDL	evel: Level				
	1	3						2	L			
					2				/			m <del>/</del>
LP				LP		LP		LP	- illin	LP	-	11 7/8
1 SPF				2 SPF						3 5	SPF	
r	13'1	"					1	3'1"				5 1/4"
,				26'2"								1 10 17 1
1				202							I	
lember Info	mation					Reaction	ns UNP	ATTERNE	D lb (Upl	ift)		
Туре:	Girder	Applica		Floor		Brg	Live	Dea			Wind	Const
Plies: Moisture Conditic	3 n: Dn/	, e	Method: g Code:	ASD IBC/IRC 2015		1	3095	149		0	0	0
Deflection LL:	480		haring:	Yes		2	6310 1628	403 123		0 0	0 0	0 0
Deflection TL:	240	Deck:	0	Not Checked		5	1020	123	0	0	U	U
mportance:	Normal											
Temperature:	Temp <= 100°F					Boaring						
						Bearing: Bearing		Con	React D/L	h Tota	Ld. Case	e Ld. Comb.
						1 - SPF	-	62%	1479 / 336		LO. Case L_	D+L
						2 - SPF		93%	4062 / 635		_	D+L
nalysis Resu						3 - SPF		43%	1222 / 216			D+L
,		n Allowed	Capacit		Case							
Neg Moment -1		" 31048 ft-lb		8%) D+L	LL							
Pos Moment 2 <sup>-</sup> Shear 6		" 31048 ft-lb		0%) D+L 4%) D+l	L_ LL							
Shear 63 LL Defl inch 0.		8" 11845 lb 8" 0.321 (L/48	•	4%) D+L 1%) I								
LL Definch 0. TL Definch 0.		6" 0.321 (L/48 6" 0.643 (L/24	, ,		L_ L							
esign Notes	. ,		, 5.000 (0	,		1						
	support to prevent rotatio	n at end bearir	ngs and at in	terior bearings v	when	1						
required by co	de for seismic design.		0	5								
	flection: Instant = 0.123", signed to be supported or	•										
4 Multiple plies r	nust be fastened together	as per manufa	• •	ails.								
-	t be supported equally by terally braced at a maxim		0.C.									
	e laterally braced at a mai											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live	1 Snow	v 1.15 W	/ind 1.6 Co	onst. 1.25	Comme	nts
1	Part. Uniform 0-	0-0 to 6-11-4		Тор	70 PLF	280 PL	F	0 PLF	0 PLF	0 PLF	Floor Loa	ad
2	Uniform		1-7-3	Тор	10 PSF	40 PS	F	0 PSF	0 PSF	0 PSF	Std. Floo	r Loading
3	Point	6-11-4		Тор	3013 lb	4286 I	b	0 lb	0 lb	0 lb	Header fi	rom Above
	Bearing Length	0-3-8										
1	Point	21-5-4		Тор	2380 lb	3131	b	0 lb	0 lb	0 lb	Header F	rom Above
	Bearing Length	0-3-8										
	Self Weight				18 PLF							
lotes							1	Manufacturer	Info	E	BMC/Locust Lu	Imber Company
	is based on the loads,						ı	Louisiana-Paci	fic Corp		312 E. Main St. Jnited States	., NC
nd listed in this report.	ions as entered by the user The user is responsible to input and the applicability to						1	414 Union Stre Nashville, TN 3	37219		28097 704) 888-4411	
omponent is intended. Thi	input and the applicability to the structure for which this s analysis is valid only for the						(	(888) 820-032 www.lpcorp.co	5			
roduct listed. opyright 2019 All rights r	eserved by Louisiana Pacific						1	APA: PR-L280 LADBS: RR-25	, ICC-ES: ESR			
	000, Nashville, TN 37219				Thi	s design is valid	Lundil L					

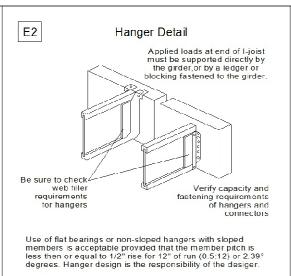
<b>IsDe</b> Beam under Kit		Client:				Da	ate:	7/22/2020				Page 1 o
		Project:				In	put by:	Tony Hune	ycutt			0
Geam under Kit	sign	Address:				Jo	b Name:	: Bartlett-Las	seria			
Beam under Kit							roject #:					
	t/Pantry Wall	LP-LVL 2900F	<sup>-</sup> b-2.0E	1.750" X 11	.875" 2-	Ply - PAS	SED L	evel: Level.				
				2		3	,					
LP	LP.		LP		P	Y	LP	Win .				11 7/8"
1 SPF					2 SPF					3 SI	⊃F	ш <i></i>
,		13'1"						4017 4/4"			_	3 1/2"
		13 1			1			10'7 1/4"				3 1/2
1				23'8 1/4	"						1	
ember Inform	nation					Reaction	ns UNP	ATTERNE	D lb (Uplif	t)		
уре:	Girder	Applica		Floor		Brg	Live	Dea	d Snow	1	Wind	Const
Plies:	2	°	n Method:	ASD		1	511	14	7 C	)	0	0
Noisture Condition:			ng Code:	IBC/IRC 2015		2	4531	204	D C	)	0	0
Deflection LL: Deflection TL:	480 240	Load S Deck:	Sharing:	No Not Checked		3	757	81	2 0	)	0	0
mportance:	240 Normal	Deck.		NUL CHECKEU								
emperature:	Temp <= 100°F											
						Bearings	5					
						Bearing		Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
						1 - SPF	-	15%	141 / 654			D+L(D+L)
						2 - SPF		85%	2054 / 4561	6615	—	D+L
nalysis Results						3 - SPF	3.500"	35%	804 / 1034	1838	_L	D+L
Analysis Actu		cation Allowed	Capacity	/ Comb.	Case							
Neg Moment -743		13'1" 19902 ft-lb		-	LL							
Pos Moment 541		0'5 3/4" 19902 ft-lb	-	-	_L							
Shear 3284		14' 7/8" 7897 lb	0.416 (42	-	LL							
L Defl inch 0.12	. ,	11 1/4" 0.321 (L/48			L_							
L Defl inch 0.11	6 (L/1069) 18	3'6 3/8" 0.519 (L/24	40) 0.220 (22	2%) D+L	_L	ļ						
esign Notes						l						
	upport to prevent re for seismic design	otation at end beari	ngs and at int	erior bearings v	when							
	•	.006", Long Term = ·	-0.009"									
3 Girders are desiç		ted on the bottom ed										
	-		acturer's detai	llS.								
		earing 1 for uplift 15	ί lb (Combina	tion D+L, Load	Case _L).							
5 Top loads must b	arings.											
5 Top loads must b 6 Tie-down connec 7 Top braced at be	the entry			Sida	Dood 0.0	1	1 0	N 1 1E \A	lind 1 6 Carr	ot 1.05	Commercia	to
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at	-	Looster	Trib Width	Side	Dead 0.9 301 lb	Live '			ind 1.6 Con		Commen	
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at D L	oad Type			Tor	3117 ID	1003 II	U	0 lb	0 lb	0 lb	Beam Abo	ve
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at D L P	Load Type Point	6-11-4		Тор	00110							
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at D L P B	Load Type Point Bearing Length	6-11-4 0-3-8		·			_			0.51 -	<b>F</b> L :	
5 Top loads must b 5 Tie-down connect 7 Top braced at be 8 Bottom braced at D L P B P	Load Type Point Bearing Length Part. Uniform	6-11-4 0-3-8 6-11-4 to 16-0-12		Тор	70 PLF	280 PLF		0 PLF	0 PLF	0 PLF	Floor Load	
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at D L B B 2 P	Load Type Point Bearing Length	6-11-4 0-3-8 6-11-4 to 16-0-12 16-0-12		·				0 PLF 0 lb	0 PLF 0 lb	0 PLF 0 lb	Floor Load Beam Abo	
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at D L P B 2 P 8 P 8 B	oad Type Point Bearing Length Part. Uniform Point Bearing Length	6-11-4 0-3-8 6-11-4 to 16-0-12		Тор	70 PLF	280 PLF						
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at D L P B 2 P 3 P B	oad Type Point Bearing Length Part. Uniform Point Bearing Length	6-11-4 0-3-8 6-11-4 to 16-0-12 16-0-12		Тор	70 PLF	280 PLF						
5 Top loads must b 6 Tie-down connec 7 Top braced at be 8 Bottom braced at ID L 1 P 2 P 3 P sontinued on page 2	Load Type Point Bearing Length Part. Uniform Point Bearing Length	6-11-4 0-3-8 6-11-4 to 16-0-12 16-0-12		Тор	70 PLF	280 PLF	b	0 lb Manufacturer	0 lb Info	0 lb	Beam Abo MC/Locust Lun 12 E. Main St.,	ve Iber Company
5 Top loads must b 6 Tie-down connect 7 Top braced at be 8 Bottom braced at D L 1 P 2 P 3 P 3 P s ntinued on page 2 otes	s based on the loads, se entered by the user	6-11-4 0-3-8 6-11-4 to 16-0-12 16-0-12		Тор	70 PLF	280 PLF	b	0 lb Manufacturer Louisiana-Paci 414 Union Stre	0 lb Info fic Corp et, Suite 2000	0 lb	Beam Abo MC/Locust Lun 12 E. Main St., nited States 3097	ve Iber Company
5 Top loads must b 6 Tie-down connect 7 Top braced at be 8 Bottom braced at 10 L 1 P 2 P 3 P 4 5 Top braced at be 8 Bottom braced at 10 L 1 P 10 P 1	Load Type Point Bearing Length Part. Uniform Point Bearing Length  s based on the loads, is as entered by the user ie user is responsible to put and the applicability to is structure for which this	6-11-4 0-3-8 6-11-4 to 16-0-12 16-0-12 0-3-8		Тор	70 PLF	280 PLF	b	0 lb Manufacturer Louisiana-Paci 414 Union Stre Nashville, TN 3	0 lb Info fic Corp et, Suite 2000 7219	0 lb	Beam Abo MC/Locust Lun 12 E. Main St., nited States	ve Iber Company
7 Top braced at be 8 Bottom braced at ID L 1 P 8 2 P 3 P	Load Type Point Bearing Length Part. Uniform Point Bearing Length  is based on the loads, is as entered by the user ie user is responsible to put and the applicability to is structure for which this inalysis is valid only for the	6-11-4 0-3-8 6-11-4 to 16-0-12 16-0-12 0-3-8		Тор	70 PLF	280 PLF		0 lb Manufacturer Louisiana-Paci 414 Union Stre Nashville, TN 3 (888) 820-0325 (888) 820-0325	0 lb Info fic Corp et, Suite 2000 7219	0 lb	Beam Abo MC/Locust Lun 12 E. Main St., nited States 3097	ve Iber Company

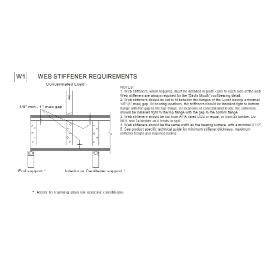


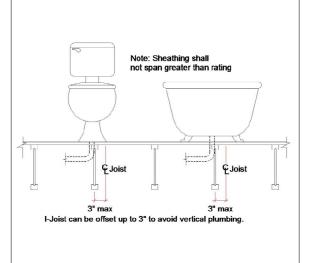
isl	Design		Project: Address:					nput by: ob Name:	Tony Hune Bartlett-La	-			
	er Stair Wall	LP-LVL 2	900Fb-2.0	E 1.7	50" X 11.87	75" 2-P	P Iy - PASS	roject #:	evel: Level				
								1 					
LP	LP.		LP		LP	NT.	LP	M	LP	- Mine	LP		11 7/8"
1 SPF					2 SPF						3 SF		,
<i>د</i>		13'1"						1	3'1"			$\rightarrow$	3 1/2"
<i>,</i>		101			0.010#			I	15 1				5 1/2
					26'2"							1	
ember Info			1							D lb (Uplift			
ype: lies:	Girder 2		Applicatio Design Me		Floor ASD		Brg	Live	Dea			Wind	Const
loisture Condi			Building C		IBC/IRC 2015		1	0 (-112) 879	(-56 110	-		0 0	0
eflection LL:	480		Load Sha		No		3	442	51			0	0
eflection TL:	240		Deck:		Not Checked				01	0 0		0	0
nportance:	Normal												
emperature:	Temp <= 10	0°F					D						
							Bearing						
							Bearing 1 - SPF	-	Cap. 0%	React D/L lb -60 / -115	Total -175 (-175)	Ld. Case _L	E Ld. Comb. D+L(D+L)
alysis Res	aults		1				2 - SPF	4.500"	30%	1108 / 885	1993	_L	D+L
	Actual	Location A	Allowed	Capacity	Comb.	Case	3 - SPF	3.500"	18%	514 / 438	953	_L	D+L
leg Moment			9902 ft-lb	0.164 (16		_L							
Pos Moment		18'11 3/4" 1	9902 ft-lb	0.318 (32	%) D+L	 _L							
Shear	1650 lb	14' 7/8" 7	'897 lb	0.209 (219	%) D+L	_L							
L Defl inch	0.077 (L/1997)	19'3 1/4" 0	.321 (L/480)	0.240 (24	%) L	LL							
L Defl inch	0.161 (L/958)	19'3 9/16" (	.643 (L/240)	0.250 (25	%) D+L	LL							
esign Note							1						
	ral support to prev code for seismic o		end bearings	and at inte	rior bearings v	vhen							
	Deflection: Instant												
	designed to be su s must be fastene				9								
	ust be supported	•		i oi o dotaii									
	nnection required	at bearing 1 f	or uplift 175 lb	(Combina	tion D+L, Load	Case							
_L). ' Top braced a	at bearings.												
Bottom brac	ed at bearings.												
D	Load Type	L	ocation Tr	ib Width	Side	Dead 0.9	Live	1 Snov	v 1.15 V	Vind 1.6 Cons	t. 1.25	Commer	nts
	Point		18-11-12		Тор	1251 lb	1208	b	0 lb	0 lb	0 lb	Beam ove	er MBR Brg 1
	Bearing Lengt	n	0-3-8										
	Self Weight					12 PLF							
ites									Manufacturer	Info			mber Company
s component anal	lysis is based on the inditions as entered by th	loads,							Louisiana-Paci	ific Corp	Ur	2 E. Main St., nited States	, NC
d listed in this repo	ort. The user is respons	ble to						1	Nashville, TN 3			8097 04) 888-4411	
	the input and the applical of the structure for which	h this						(	(888) 820-032	5			
mponent is intended.	This analysis is valid only	for the						1	www.lpcorp.co	m			
nponent is intended. duct listed. pyright 2019 All right	This analysis is valid only its reserved by Louisiana te 2000, Nashville, TN 372	for the Pacific					is design is valio	1	www.lpcorp.co APA: PR-L280 LADBS: RR-24	m , ICC-ES: ESR-240 5783, Florida: FL15	03,		

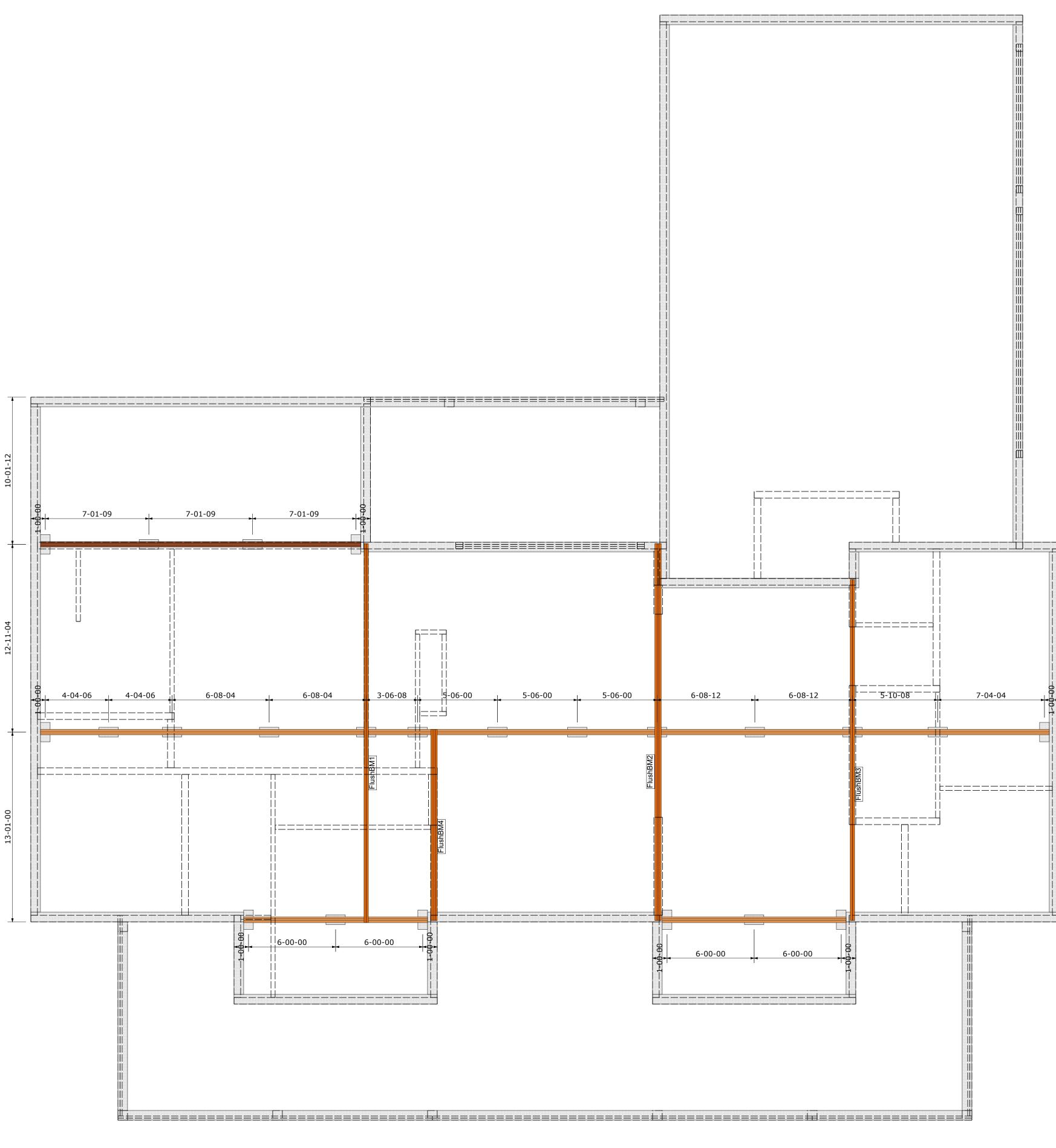
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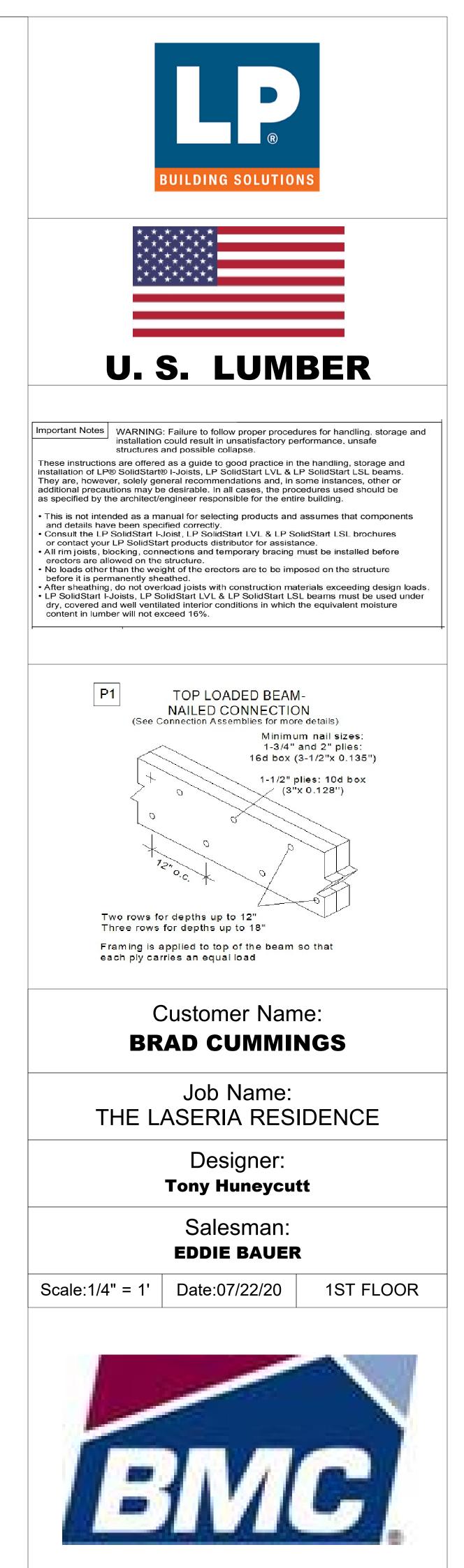


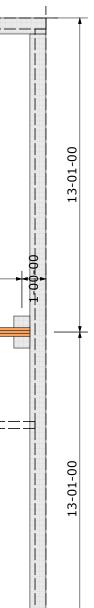












### **FIRST FLOOR FRAMING**

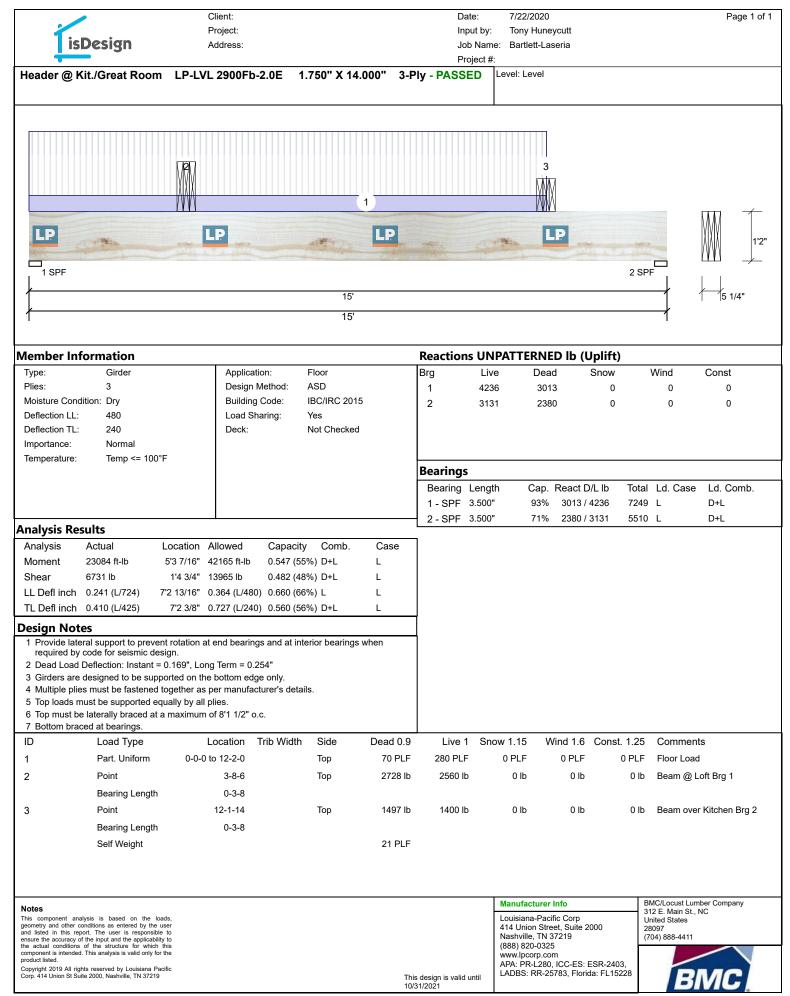
1634	LF	12IJ	11-7/8" LPI 20Plus JOISTS		15/36' 3/32' 3	8/30' 1/28'	3	4902	
						18/26' 3/24' :	1/20' 3/14'		
						1/10' 9/6' 2/4	4' + 56' BLKG		
21	PCS	12RIM12	1-1/8" x	11-7/8"	x 12' RIM	1 BOARD		43.2	907.2
224	LF	12LVL	1-3/4" x	11-7/8"	LVL	2/28' 3/26' 2	/24' 3/14'	5.4	1209.6

7018.8

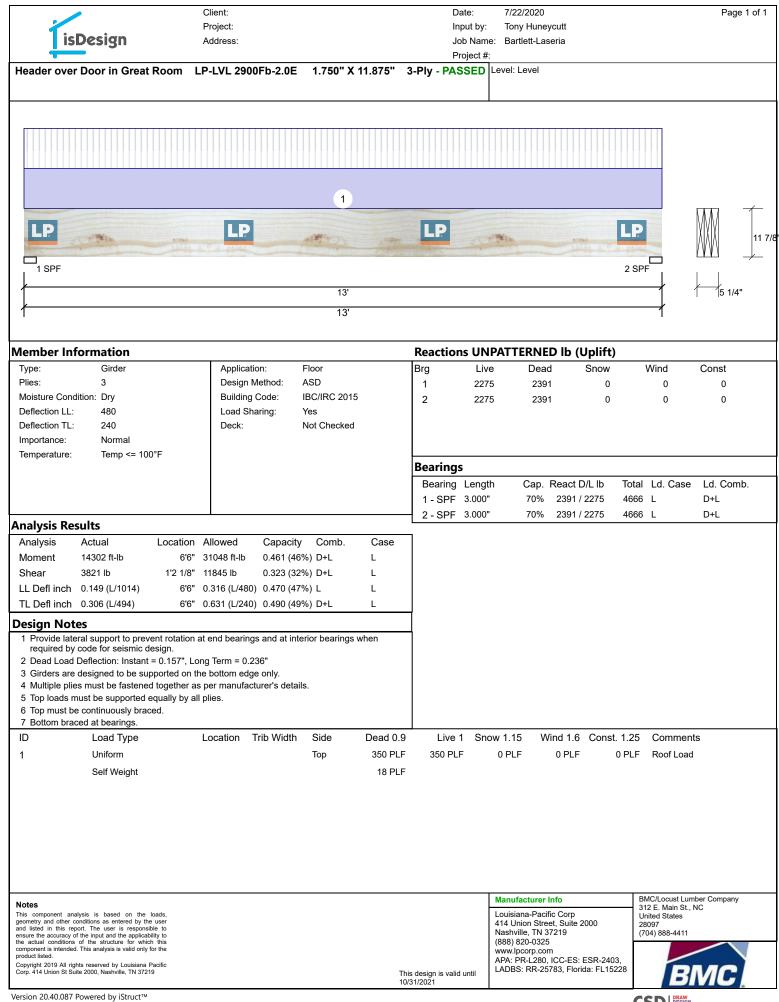
## **SECOND FLOOR EWP**

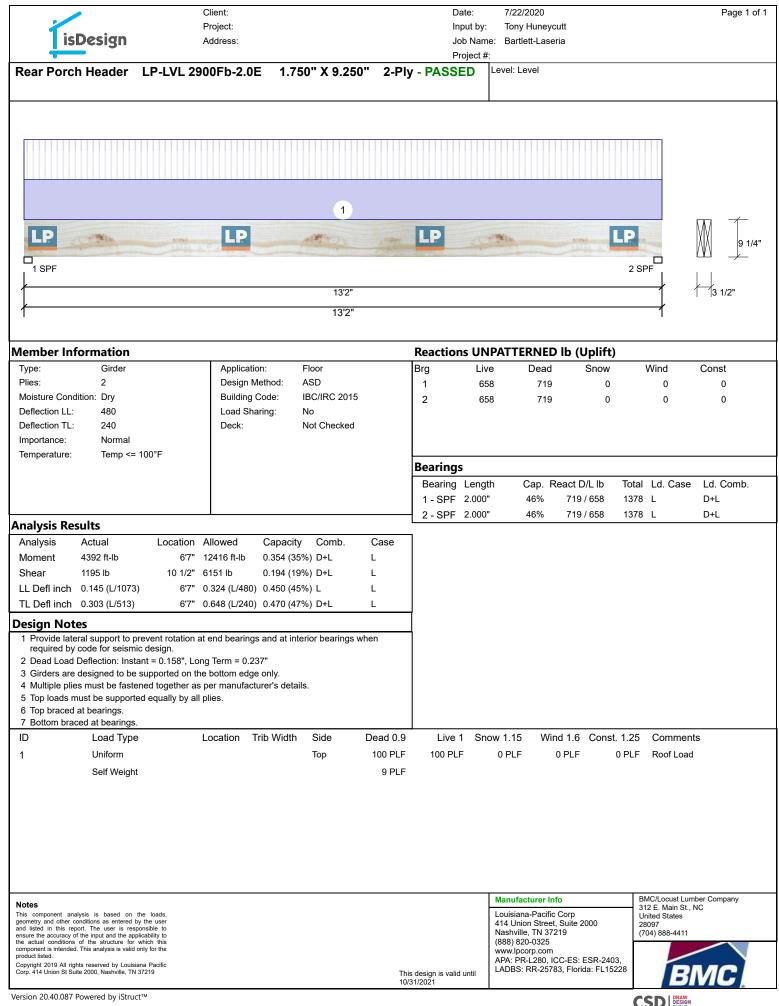
412	LF	9LVL	1-3/4" x 9-1/4" LVL		2/28' 4/22' 3/16' 7/14	' 5/12'	4.2	1730.4
					2/10' 3/8' 3/6'			
42	LF	12LVL	1-3/4" x 11-7/8"	LVL	3/14'		5.4	226.8
150	LF	14LVL	1-3/4" x 14" LVL		3/18' 6/16'		6.4	960

2917.2

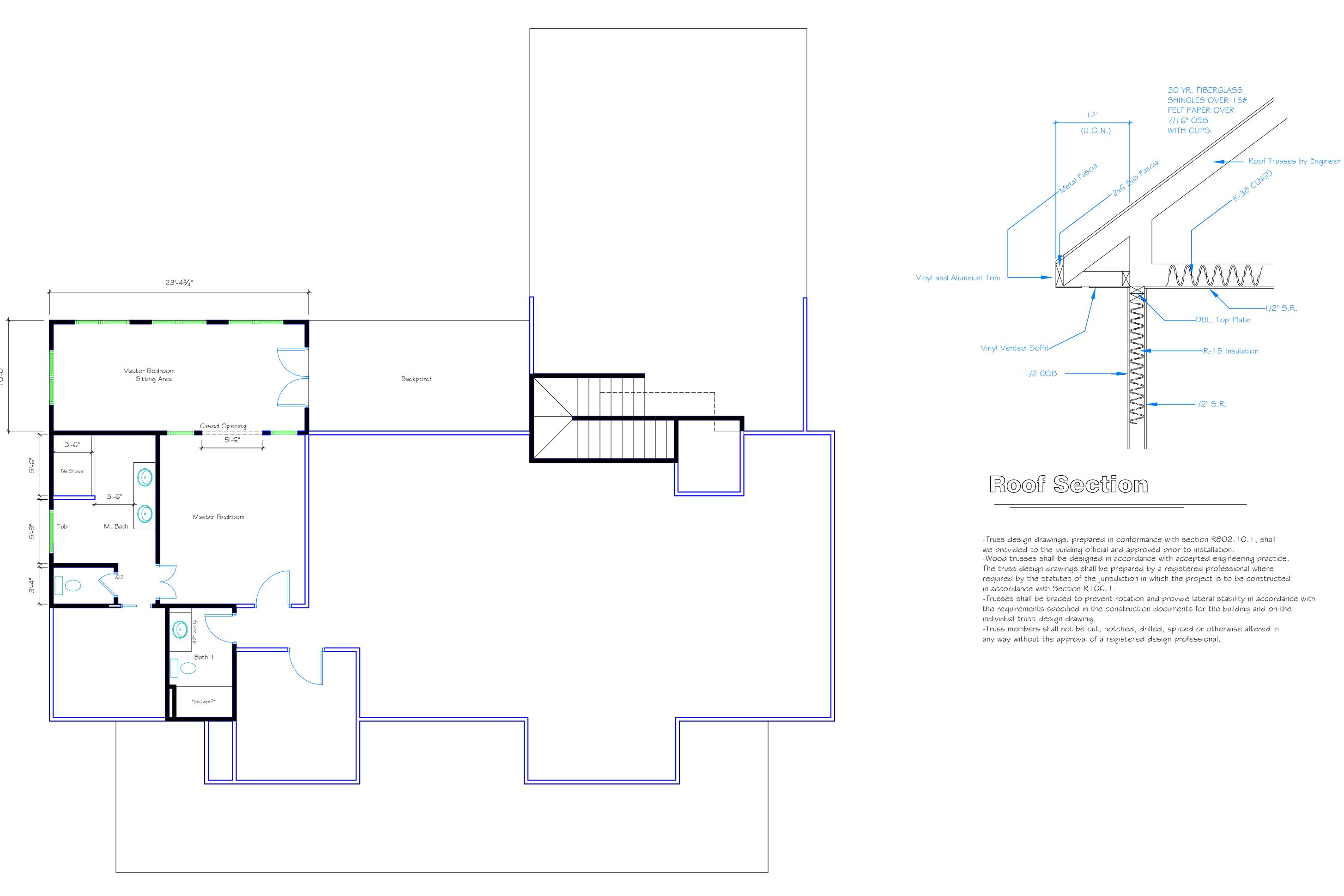


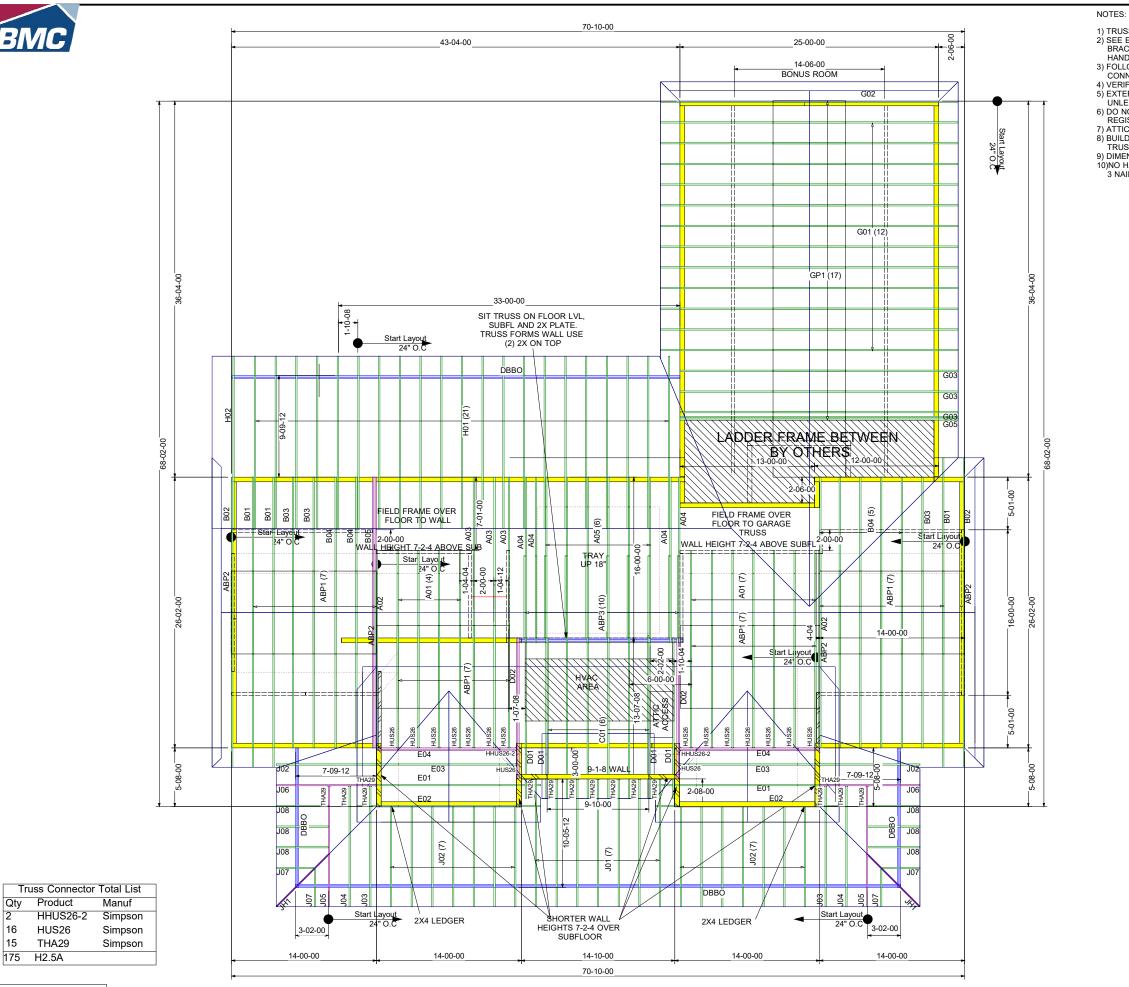
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## Main Level Changes





Sq Ft Roof Area =5920.56

2

CONNECTIONS.

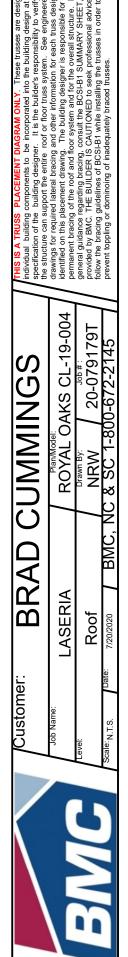
1) TRUSS SPACING 24" o/c UNLESS NOTED OTHERWISE. 2) SEE ENGINEERED TRUSS DRAWINGS FOR NOTES AND REQUIRED BRACING OF TRUSS WEBS IN ADDITION TO BCSI-B1 SUMMARY SHEET FOR HANDLING, INSTALLING AND BRACING. 3) FOLLOW SIMPSONS INSTALLATION RECOMMENDATIONS FOR HANGER

4) VERIFY ALL BUILDING DIMENSIONS PRIOR TO TRUSS ERECTION.
 5) EXTERIOR DIMENSIONS ARE FROM OUT TO OUT OF STUD

a) LATENDA DIMILEIGNMENT AND AND ADDIMINATION OF TO OUT OF STOD UNLESS NOTED OTHERWISE.
b) DO NOT CUT, DRILL OR ALTER TRUSS WITH OUT CONSULTING A REGISTERED PROFESSIONAL ENGINEER.
c) ATTIC ACCESS MUST BE PLACED BETWEEN TRUSSES.

8) BUILDER IS RESPONSIBLE FOR PROVIDING ADEQUATE BEARING TO SUPPORT TRUSS REACTIONS.

10USS REACTIONS. 9) DIMENSIONS ARE IN FEET-INCHES-SIXTEENTHS. 10)NO HANGERS ARE REQUIRED FOR SMALL, OPEN-ENDED TRUSSES. INSTEAD, USE 3 NAILS IN BOTH THE TOP AND BOTTOM CHORDS.



g design at the liity to verify th e engineered i truss design onsible for structure. For XY SHEET,

For

