

Trenco 818 Soundside Rd Edenton, NC 27932

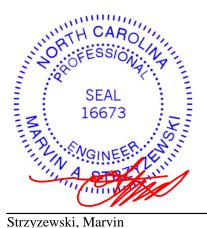
Re: Asheville 200691RT1

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carolina Structural Systems, LLC.

Pages or sheets covered by this seal: E14365734 thru E14365744

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



May 4,2020

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type Qty Ply 200691RT1		200691RT1		
Asheville	A01	Common Supported Gable	1	1	Job Reference (optional)	E14365734

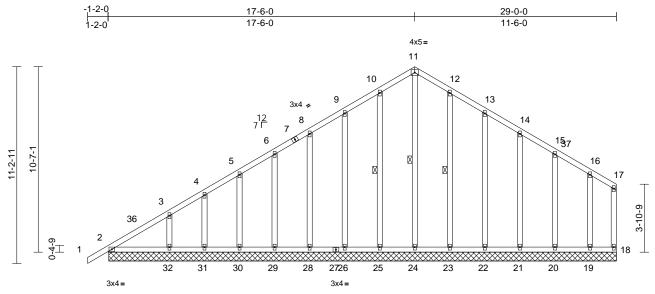
Scale - 1:65 7

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:04 ID:9LT43pMo?FBSpQ1Isi?WB7zKxaE-YzeXYrxSAez0lacXCnWXGJa0vM0puTozwWhk9kzJtxV

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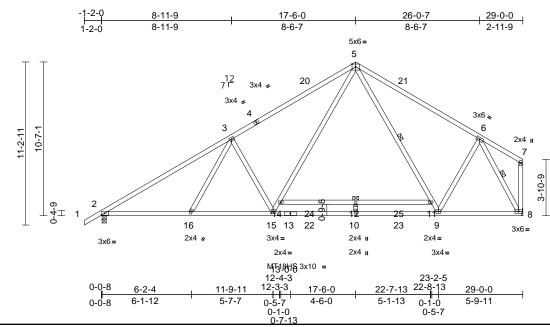
Scale = 1:65.7												
Loading (psf) TCLL (roof) 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Plate Grip DOL 1 Lumber DOL 1 * Rep Stress Incr Y	-0-0 .00 .15 /ES RC2018/TPI2014	CSI TC BC WB Matrix-AS	0.05	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 18	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 220 lb	GRIP 244/190 FT = 20%	
except end vertica BOT CHORD WEBS 1 Row at midpt REACTIONS (size) 2=28-1 19=28- 21=28- 23=28- 25=28- 30=28- 32=28- 30=28- 32=28- Max Horiz 2=249 Max Uplift 2=-29 (19=-36 21=-23 23=-9 (25=-96 28=-22 30=-24 32=-28 Max Grav 2=232 19=155 23=166 25=166 28=161 30=165 32=266		 this design. 2) Wind: ASC Vasd=99m B=45ft; L=2 MWFRS (d 1-10-0, Ext to 20-6-0, E cantilever la right expos for reaction DOL=1.60 3) Truss desi only. For s see Standa or consult c 	E 7-16; Vult=125mph ph; TCDL=6.0psf; BCI 29ft; eave=2ft; Cat. II; lirectional) and C-C Cc rerior(2N) 1-10-0 to 17 Exterior(2N) 20-6-0 to eft and right exposed is shown; Lumber DOI igned for wind loads in studs exposed to wind ard Industry Gable Enc qualified building desig	195/16 180/151 180/150	1, 5-6=-184/144 6, 8-9=-159/199 7/280, 57/245, 5/157, 105, 5, 30-31=-48/57, 7, 27-28=-48/57 7, 27-28=-48/57 7, 27-28=-48/57 7, 21-22=-48/57 7,	-0	on t 3-06 choo) Prov bea 18, at jc 24 II bu joint at jc lb u joint Inte R8C 0) This stru choo COAD C	he botto 5-00 tall rd and a vide med ring plat 29 lb up pint 25, 2 b uplift at joint 31, 2 plift at joi t 20, 36 i a Standa s truss d ctural we rational ctural we trad and 1 bottom c CASE(S)	m choi by 2-0 ny oth- chanica e capaa lift at jc 29 lb uy it joint : 8 lb up iint 22, lb uplif t Resic and refi- esign r ood sh /2" gyp chord) Star	een designed for rd in all areas wh 0-00 wide will fit er members. al connection (by, ible of withstandi bint 2, 31 lb uplift at joint 26, 22 29, 24 lb uplift at joint 19 and 2 ring condition. Fined in accordand lential Code sect erenced standare requires that a m eathing be applid ssum sheetrock for ndard	ere a rectang between the l rothers) of tr. ng 14 lb uplift at joint 24, 9 2 lb uplift at joint 30, 22 ll lb uplift at joint 30, 22 ll bu uplift at joint 12, 21 lb up 29 lb uplift at joint 21, 21 lb up 29 lb uplift at joint 20, 22 ll the with the 20 ions R502.11 d ANSI/TPI 1. inimum of 7/1 ed directly to f be applied directly to f	Ile bottom iss to a t joint Ib uplift int 28, 29 blift at joint 2. ed. 18 .1 and 6" the top
Tension	mpression/maximum		re 2x4 MT20 unless o s spaced at 2-0-0 oc.	otherwis	se indicated.					A ST	HZ 1000	, it

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	A02	Common	7	1	Job Reference (optional)	E14365735

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:06 ID:C8m2xhjYUyWzyHTZF4Y4YyzKxb3-Rlu2OD_zEtTSECwIRdaTQ8lbAz86qCbZr8fxIVzJtxR

Page: 1



Scale = 1:79.3 Plate Offsets (X, Y): [15:0-1-8,0-1-8]

-					1		1						
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.15 YES	18/TPI2014	CSI TC BC WB Matrix-AS	0.99	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.74 -1.17 0.05	(loc) 12 12-14 8	l/defl >467 >295 n/a	240	PLATES MT20 MT18HS Weight: 185 lb	GRIP 244/190 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.1 2x4 SP DSS *Excep 2x4 SP No.2 Structural wood she except end verticals Rigid ceiling directly 5-8-0 oc bracing: 11 1 Row at midpt (size) 2=0-3-8, 8 Max Horiz 2=249 (LC Max Uplift 2=-21 (LC	t* 14-11:2x4 SP No.1 athing directly applied, applied. Except: -14 6-8, 5-9 3= Mechanical C 10)	2	 Wind: ASCE Vasd=99mpl B=45ft; L=22 MWFRS (dir 1-10-0, Interi to 20-6-0, Intl left and right exposed;C-C reactions shu DOL=1.60 All plates are 0 * This truss F on the bottor 3-06-00 tall b 	7-16; Vult=125mp ; TCDL=6.0psf; E bft; eave=4ft; Cat. ectional) and C-C ior (1) 1-10-0 to 17 terior (1) 20-6-0 to exposed ; end vec c for members and own; Lumber DOL e MT20 plates unlen has been designed m chord in all area by 2-00-00 wide w by other members	CDL=6.0 II; Exp B Exterior(7-6-0, Ex 28-10-4 rtical left d forces & .=1.60 pl: ess other d for a liv is where ill fit betw	Dpsf; h=25ft; ; Enclosed; (2E) -1-2-0 tc terior(2R) 17 zone; cantilé and right & MWFRS fo ate grip wise indicaté e load of 20. a rectangle veen the bott	o 7-6-0 ever or ed. 0psf				weight: 100 lb	FT = 20%
FORCES	(lb) - Maximum Com Tension		, 5 6) Refer to gird	er(s) for truss to tr hanical connection	uss conr	nections.						
TOP CHORD	1-2=0/35, 2-3=-2409 4-20=-1967/64, 5-20	, ,		bearing plate 2.) This truss is	e capable of withst designed in accor	tanding 2 dance w	th the 2018	joint					
BOT CHORD	2-16=-67/2119, 15-1 13-15=0/1246, 13-2 10-23=0/1246, 9-23=	6=-69/2048, 2=0/1246, 10-22=0/12 =0/1246, 8-9=-38/862, =-134/0, 12-25=-134/0		International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.									Rollin
WEBS	5-11=-60/269, 9-11=	5=-12/1133, 5-14=0/13 157/87, 6-9=0/693, 5=0/150, 10-12=-159/0		OAD CASE(S)								× 0	M.
NOTES	ed roof live loads have	been considered for										SEA	L

1)

Unbalanced roof live loads have been considered for this design.



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Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	A03	Common	13	1	Job Reference (optional)	E14365736

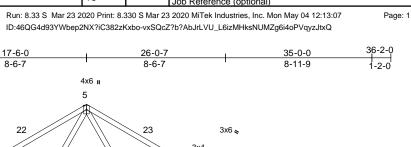
-1-2-0

8-11-9

8-11-9

12 7 Г

3x4 🍬



11-2-11 10-7-1		3x6 \$	4			3x4 = 6 7			
	2					/			8
	. 8	15 14	13 24		25 12	11	⊠ 10		<u> </u>
	3x6=	3x4 🥡 MT18	HS 3x10 =		3x4	-	3x4 🔥	:	3x6=
			3x4=			MT18HS 3x10	=		
	0-0-8 6-2-4	11-9-11		23-2-5		28-9-12	28-11-8	35-0-0	
le – 1:70 7	0-0-8 6-1-12	5-7-7	I	11-4-9	I	5-7-7	0-1-12	6-0-8	I

Scale = 1:70.7 Plate Offsets (X, Y): [8:0-2-9,Edge], [12:0-1-12,0-1-8], [13:0-1-12,0-1-8]

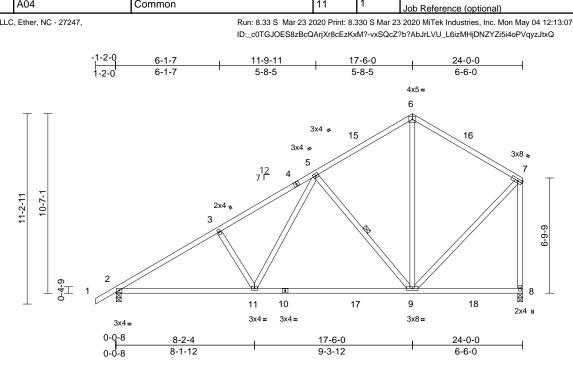
	(, .). [e.e = e,==9e];	[
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00		тс	0.70			12-13	>558		MT20	244/190
TCDL	10.0	Lumber DOL	1.15		BC	0.99	Vert(CT)	-0.97	12-13	>356		MT18HS	244/190
BCLL	0.0*	Rep Stress Incr	YES		WB	0.38	Horz(CT)	0.04	10	n/a	n/a		
BCDL	10.0	Code		8/TPI2014	Matrix-AS	0.00		0.0.				Weight: 185 lb	FT = 20%
	10.0	0000	11(0201	0/11/12/011	Matrix / to							Wolgin. Too lo	11-2070
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD WEBS REACTIONS	 2x4 SP No.1 2x4 SP No.2 Structural wood she Rigid ceiling directly 1 Row at midpt (size) 2=0-3-8, ' Max Horiz 2=-202 (L Max Uplift 2=-63 (LC) 	applied. 5-12, 7-10 10=0-3-8 C 9) C 11), 10=-96 (LC 11)	5) ed. 6)) 7)	on the bottor 3-06-00 tall h chord and ar Provide mec bearing plate 2 and 96 lb u This truss is International R802.10.2 a This truss de structural wo	has been designer in chord in all area by 2-00-00 wide w by other members hanical connectio capable of withs uplift at joint 10. designed in accor Residential Code and referenced sta sign requires that od sheathing be a	as where vill fit betw s, with BC on (by othe standing 6 rdance wi e sections undard AN t a minimu applied di	a rectangle reen the botto DL = 10.0psf. ars) of truss to 3 lb uplift at jo th the 2018 R502.11.1 ar SI/TPI 1. um of 7/16" rectly to the to	o Do Doint Ind					
FORCES	(lb) - Maximum Com	Iax Grav 2=1319 (LC 16), 10=1944 (LC 18) chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. (lb) - Maximum Compression/Maximum LOAD CASE(S) Standard											
TOP CHORD		2=-1475/160, 3=-830/105, 6-7=-95		JAD CASE(S)	Standard								
BOT CHORD	,	=0/849, 12-25=0/849),										
WEBS	5-13=-44/1077, 5-12 7-10=-1984/324, 3-1												in the second se
NOTES												"ATH UP	HO MA
	ced roof live loads have	been considered for									5	O	in Main
, this desig	jn.									1	64		TRAN
 Wind: AS Vasd=99r B=45ft; L= MWFRS 2-4-0, Into 21-0-0, Into and right exposed; 	CE 7-16; Vult=125mph mph; TCDL=6.0psf; BC =35ft; eave=5ft; Cat. II; (directional) and C-C E erior (1) 2-4-0 to 17-6-0 tterior (1) 21-0-0 to 36-2 exposed ; end vertical C-C for members and f	DL=6.0psf; h=25ft; Exp B; Enclosed; xterior(2E) -1-2-0 to 0, Exterior(2R) 17-6-0 2-0 zone; cantilever I left and right orces & MWFRS for	eft								MAT	SEA 1667	• •
DOL=1.60	shown; Lumber DOL= 0 are MT20 plates unles		d.								11	A ST	RZY1111

May 4,2020

818 Soundside Road Edenton, NC 27932



Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	A04	Common	11	1	Job Reference (optional)	E14365737



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00	TC	0.74	Vert(LL)	-0.24	9-11	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.65	Vert(CT)	-0.37	9-11	>765	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.26	Horz(CT)	0.03	8	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-AS							Weight: 147 lb	FT = 20%

NOTES

this design.

DOL=1.60

1)

2)

Scale = 1:67.9

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.2
BRACING	
TOP CHORD	Structural wood sheathing directly applied,
	except end verticals.
BOT CHORD	Rigid ceiling directly applied.
WEBS	1 Row at midpt 5-9
REACTIONS	(size) 2=0-3-8, 8=0-3-8
	Max Horiz 2=289 (LC 10)
	Max Uplift 2=-61 (LC 11), 8=-36 (LC 11)
	Max Grav 2=1155 (LC 16), 8=1120 (LC 16)
FORCES	(lb) - Maximum Compression/Maximum
	Tension
TOP CHORD	1-2=0/35, 2-3=-1680/97, 3-4=-1549/113,
	4-5=-1418/130, 5-15=-706/139,
	6-15=-603/157, 6-16=-613/146,
	7-16=-697/128, 7-8=-1010/104
BOT CHORD	2-11=-235/1533, 10-11=-177/1026,
	10-17=-177/1026, 9-17=-177/1026,
	9-18=-71/89, 8-18=-71/89
WEBS	3-11=-325/117, 5-11=0/738, 5-9=-699/122,
	6-9=-39/349, 7-9=-62/794

Unbalanced roof live loads have been considered for

Wind: ASCE 7-16; Vult=125mph (3-second gust)

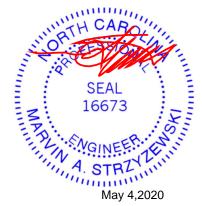
Vasd=99mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed;

MWFRS (directional) and C-C Exterior(2E) -1-2-0 to

1-10-0, Interior (1) 1-10-0 to 17-6-0, Exterior(2R) 17-6-0

to 20-6-0, Interior (1) 20-6-0 to 23-10-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to
- bearing plate capable of withstanding 61 lb uplift at joint 2 and 36 lb uplift at joint 8.
- 5) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- LOAD CASE(S) Standard



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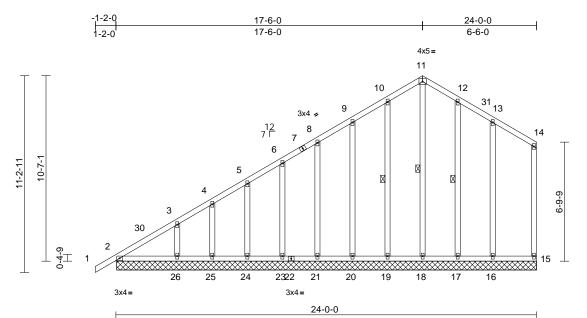
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Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	A05	Common Supported Gable	1	1	Job Reference (optional)	E14365738

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:08 ID:LNOe0T3MGM0FK1Sc?IrHu2zKxL7-N7?opu?DmUjATV3hY2dxVZq08n2KIBurlS82NOzJtxP

Page: 1



Scale = 1:65.7

Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(lo	oc) l/de	fl L	/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.00		тс	0.22	Vert(LL)	n/a		- n/	a 9	99	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15		BC	0.11	Vert(CT)	n/a		- n/	a 9	99			
BCLL	0.0*	Rep Stress Incr	YES		WB	0.16	Horz(CT)	0.00		15 n/	'a r	n/a			
BCDL	10.0	Code	IRC2018/T	PI2014	Matrix-AS								Weight: 184 lb	FT = 20%	6
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.1 2x4 SP No.2 2x4 SP No.2 Structural wood sh except end vertical Rigid ceiling directl 1 Row at midpt (size) 2=23-11 16=23-1 18=23-1 20=23-1 23=23-1	eathing directly applie	TOP d.	CHORD CHORD S	1-2=0/35, 2-30=-2 3-4=-220/162, 4-5 6-7=-182/107, 7-8 9-10=-159/196, 11 11-12=-173/234, 13-31=-150/186, 14-15=-127/126 2-26=-84/101, 25 24-25=-84/101, 25 24-25=-84/101, 12 20-21=-84/101, 12 18-19=-84/101, 11 16-17=-84/101, 11 11-18=-175/81, 11 9-20=-119/69, 8-2 5-24=-125/65, 4-2	5=-206/14 3=-170/11 0-11=-17; 12-31=-14 13-14=-1; -26=-84/1 3-24=-84/ 1-22=-84/ 9-20=-84/ 7-18=-84/ 0-19=-12; 2-1=-120/6 25=-98/55	6, 5-6=-194/1 7, 8-9=-170/1 3/234, 40/194, 38/163, 01, 101, 101, 101, 101, 101, 101, 101	/63,	8) [9) - 10) - 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	bearing p 15, 11 lb at joint 19 24 lb upli at joint 29 lb uplift a Non Star This truss Internatic R802.10. This truss structural	late c uplift : 0, 28 ll ft at jo 5, 30 ll dard l s is de nal R 2 and 2 and 2 and 3 desig wood 1/2" m cho	apal at jo b up int 2 b up 16 a sigr esid refe gn re gyp rd.	al connection (by ble of withstandi jint 2, 36 lb uplift blift at joint 20, 2; 23, 24 lb uplift at joint 26, 7 and 11 lb uplift at ring condition. F red in accordance lential Code sect prenced standare equires that a m eathing be applied ssum sheetrock to	v others) of ng 48 lb up at joint 18, 3 lb uplift at joint 24, 22 lb uplift at ju t joint 2. Review requ ce with the 2 tions R502. d ANSI/TPI inimum of 7 ed directly t	truss to lift at joint 11 lb uplift joint 21, 2 lb uplift oint 17, 39 iired. 2018 11.1 and 1. 7/16" o the top
FORCES	27=23-1 Max Horiz 2=289 (I Max Uplift 2=-11 (L 16=-39 (20=-28 (23=-24 (25=-22 (27=-11 (Max Grav 2=258 (I 16=259 18=209 20=187 23=186 25=139 27=258	1-8 .C 10), 27=289 (LC 1(C 7), 15=-48 (LC 11), LC 11), 17=-7 (LC 11) LC 10), 19=-11 (LC 1 LC 11), 21=-23 (LC 1 LC 11), 24=-24 (LC 1 LC 11), 26=-30 (LC 1 C 7) .C 17), 15=144 (LC 17) (LC 16), 19=197 (LC 2 (LC 16), 21=190 (LC 2 (LC 16), 24=201 (LC 2) (LC 16), 26=319 (LC 2) .C 16), 26=	1) L , 11 , 2) V ,), 2) V ,), 1 ,), 1 ,), 1 ,), 6), 6), 6), 6), 3) 6), 3) 6), 3) 6), 3) 6), 3) 6), 3) 6), 5) 6), 5) 6), 5) 6), 5) 6), 5) 6), 5) 6), 5) 6), 5) 6) 5) 6) 5) 6) 5) 6) 5) 6) 5) <th>ES Jnbalanced nis design. Vind: ASCE asd=99mp =45ft; L=22 MWFRS (dir -10-0, Exte o 20-6-0, E: antilever le ght expose or reactions DOL=1.60 Truss design nly. For stu ee Standar r consult qu</th> <th>12-17=-121/56, 1: roof live loads ha 7-16; Vult=125m h; TCDL=6.0psf; I 4ft; eave=2ft; Cat. rectional) and C-C rior(2N) 1-10-0 to xterior(2N) 20-6-0 ft and right expose d; C-C for membe a shown; Lumber I uned for wind load: uds exposed to wi d Industry Gable I ualified building de e 2x4 MT20 unles</th> <th>ve been o ph (3-sec BCDL=6.0 II; Exp B; Corner(3 17-6-0, C to 23-10- ed; end v rs and for DOL=1.60 s in the pl ind (norm: End Detai esigner as</th> <th>considered fo cond gust) pps; h=25ft; Enclosed; BE) -1-2-0 to Corner(3R) 17 4 zone; rertical left an ces & MWFR 0 plate grip ane of the tru al to the face] is as applical s per ANSI/TF</th> <th>7-6-0 d 2S Jss),</th> <th></th> <th></th> <th>and and a second s</th> <th>AN ANT</th> <th>SEA 166</th> <th>72</th> <th>NSKI INSKI</th>	ES Jnbalanced nis design. Vind: ASCE asd=99mp =45ft; L=22 MWFRS (dir -10-0, Exte o 20-6-0, E: antilever le ght expose or reactions DOL=1.60 Truss design nly. For stu ee Standar r consult qu	12-17=-121/56, 1: roof live loads ha 7-16; Vult=125m h; TCDL=6.0psf; I 4ft; eave=2ft; Cat. rectional) and C-C rior(2N) 1-10-0 to xterior(2N) 20-6-0 ft and right expose d; C-C for membe a shown; Lumber I uned for wind load: uds exposed to wi d Industry Gable I ualified building de e 2x4 MT20 unles	ve been o ph (3-sec BCDL=6.0 II; Exp B; Corner(3 17-6-0, C to 23-10- ed; end v rs and for DOL=1.60 s in the pl ind (norm: End Detai esigner as	considered fo cond gust) pps; h=25ft; Enclosed; BE) -1-2-0 to Corner(3R) 17 4 zone; rertical left an ces & MWFR 0 plate grip ane of the tru al to the face] is as applical s per ANSI/TF	7-6-0 d 2S Jss),			and and a second s	AN ANT	SEA 166	72	NSKI INSKI
			5) G 6) * 0	Bable studs This truss I In the botto	spaced at 2-0-0 c has been designe m chord in all area by 2-00-00 wide w	oc. d for a live as where	e load of 20.0 a rectangle				and the	Pull	A ST	EER.	Allin

chord and any other members, with BCDL = 10.0psf.

A. STRZ May 4,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	B01	Common Supported Gable	1	1	Job Reference (optional)	E14365739

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:08 ID:?EA1gCdDRUpjr5zSiyg3llzKxKP-N7?opu?DmUjATV3hY2dxVZq2Xn3LICVrlS82NOzJtxP

Page: 1

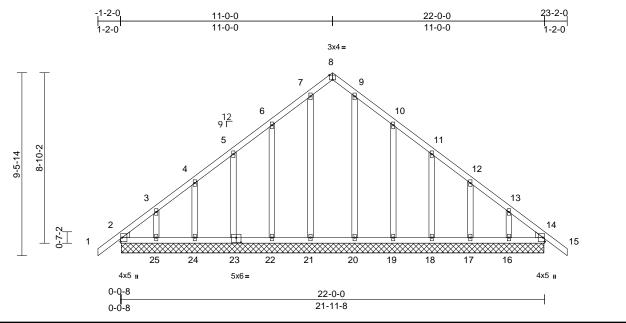


Plate Offsets (X, Y): [8:0-2-0,Edge], [23:0-3-0,0-3-0]

Scale = 1:59.7

													-
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(lo	oc) I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00		TC	0.07	Vert(LL)	n/a	(- n/a	999	MT20	244/190
TCDL	10.0		1.15		BC	0.04	Vert(CT)	n/a		- n/a	999		
BCLL	0.0*	Rep Stress Incr	YES		WB		Horz(CT)	0.01		14 n/a	n/a		
BCDL	10.0	Code	IRC2018	3/TPI2014	Matrix-AS							Weight: 146 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS WEDGE BRACING TOP CHORD BOT CHORD REACTIONS FORCES	2x4 SP No.1 2x4 SP No.2 Left: 2x4 SP No.2 Right: 2x4 SP No.2 Structural wood sh Rigid ceiling directl (size) 2=21-11 16=21-1 20=21-1 22=21-1 24=21-1 24=21-1 24=21-1 24=21-1 24=21-1 (Max Horiz 2=172 (L 18=-33 (24=-32 (Max Grav 2=200 (L 16=185 18=193 20=196 22=184 24=198 26=200 (L) - Maximum Con Tension	eathing directly applied y applied. -0, 14=21-11-0, 1-0, 17=21-11-0, 1-0, 19=21-11-0, 1-0, 23=21-11-0, 1-0, 25=21-11-0, 1-0, 29=21-11-0, C 10), 26=172 (LC 10) LC 11), 17=-34 (LC 11) LC 11), 19=-52 (LC 11) LC 11), 19=-52 (LC 11) LC 11), 19=-52 (LC 11) LC 11), 19=-52 (LC 11) LC 11), 17=-34 (LC 16) (LC 17), 17=195 (LC 17) (LC 17), 19=193 (LC 17) (LC 17), 19=193 (LC 16) (LC 16), 25=188 (LC 16) (LC 16), 25=188 (LC 16) (LC 17), 29=179 (LC 16) (LC 17), 29=179 (LC 16)	NC 1) 2) , , , , , , , , , , , , ,	EBS Unbalanced this design. Wind: ASCE Vasd=99mpt B=45ft; L=24 MWFRS (dir 1-10-4, Exter to 14-112, E cantilever lef right exposer for reactions DOL=1.60 Truss design only. For stu see Standard or consult qu All plates are Gable studs * This truss f on the bottor	2-25=-93/221, 24-2 23-24=-90/221, 22- 23-24=-90/221, 22- 21-22=-93/224, 18- 17-18=-93/224, 18- 17-18=-93/224, 16- 14-16=-93/224 7-21=-118/1, 9-20= 5-23=-120/78, 4-24 10-19=-128/104, 11 12-17=-125/83, 13- roof live loads have 7-16; Vult=125mph n; TCDL=6.0psf; BC fit; eave=2ft; Cat. II ectional) and C-C C rior(2N) 1-10-4 to 1 x:terior(2N) 14-1-12 t and right exposed d;C-C for members shown; Lumber DC ned for wind loads i uds exposed to wind d Industry Gable Er ialified building des a 2x4 MT20 unless spaced at 2-0-0 oc nas been designed n chord in all areas	23=-93/ 21=-93/ 19=-93/ 17=-93/ -109/0, =-125/8 -18=-11 16=-11/ 6 been of n (3-sec CDL=6.0; (3-sec CDL=6.0; (2 to 23- 1; end V and for DL=1.60 in the pid of (norm nd Deta igner as otherwi for a liv where	$^{\prime}$ 224, $^{\prime}$ 224, $^{\prime}$ 224, $^{\prime}$ 224, $^{\prime}$ 224, $^{\prime}$ 224, $^{\prime}$ 224, $^{\prime}$ 224, $^{\prime}$ 22, $^{\prime}$ 2, $^{\prime}$ 2, $^{$	-7, -7, s e, 1. sf	/ I I 10) ⁻ s t	Internationa R802.10.2 : This truss of structural w chord and ² the bottom D CASE(S	al Resir and ref lesign n ood sh chord.) Star	erenced standard requires that a m leathing be applie osum sheetrock b ndard	ANSI/TPI 1. ANSI/TPI 1. Ad directly to the top be applied directly to
TOP CHORD 1-2=0/42, 2-3=-189/109, 3-4=-128/89, 4-5=-112/70, 5-6=-95/54, 6-7=-85/90, 7-8=-70/90, 8-9=-70/90, 9-10=-70/90, 10-11=-71/22, 11-12=-84/41, 12-13=-131/68, 13-14=-192/106, 14-15=0/42				chord and ar Provide mec bearing plate 23, 53 lb upli uplift at joint 18, 34 lb upli	by 2-00-00 wide will by other members, i hanical connection a capable of withsta ft at joint 22, 32 lb i 25, 52 lb uplift at jo ift at joint 17 and 46 d bearing condition	with BC (by oth Inding 3 uplift at int 19, 3 b lb uplif	DL = 10.0psf. ers) of truss to 2 lb uplift at joi joint 24, 46 lb 33 lb uplift at jo t at joint 16.	nt			MATIN	A ST	EER

May 4,2020

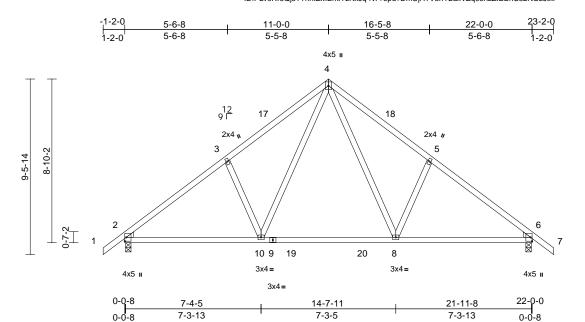
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	B02	Common	3	1	Job Reference (optional)	E14365740

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:08 ID:PLv0Rr3QjsTTnnIZMErklYzKxJq-N7?opu?DmUjATV3hY2dxVZq08nzZIBErlS82NOzJtxP

08 Page: 1



Scale = 1:62.2												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00	тс	0.22	Vert(LL)	-0.10	8-10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.16	8-10	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.02	6	n/a	n/a		
BCDL	10.0	Code	IRC2018/TPI2014	Matrix-AS							Weight: 120 lb	FT = 20%

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LUMBER	
TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.2
WEDGE	Left: 2x4 SP No.2
	Right: 2x4 SP No.2
BRACING	
TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
REACTIONS	(size) 2=0-3-8, 6=0-3-8
	Max Horiz 2=172 (LC 10)
	Max Uplift 2=-62 (LC 11), 6=-62 (LC 11)
	Max Grav 2=1047 (LC 16), 6=1047 (LC 17)
FORCES	(lb) - Maximum Compression/Maximum
	Tension
TOP CHORD	1-2=0/42, 2-3=-1244/93, 3-17=-1162/142,
	4-17=-1074/164, 4-18=-1074/164,
	5-18=-1162/142, 5-6=-1244/93, 6-7=0/42
BOT CHORD	2-10=-20/1053, 9-10=0/699, 9-19=0/699,
	19-20=0/699, 8-20=0/699, 6-8=0/957
WEBS	4-8=-51/608, 5-8=-291/144, 4-10=-51/608,
	3-10=-291/144

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=125mph (3-second gust) Vasd=99mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-2-0 to 1-10-0, Interior (1) 1-10-0 to 11-0-0, Exterior(2R) 11-0-0 to 14-0-0, Interior (1) 14-0-0 to 23-2-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

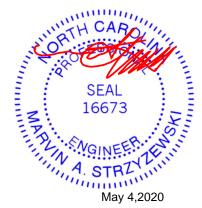
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 2 and 62 lb uplift at joint 6.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

This truss design requires that a minimum of 7/16"

structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



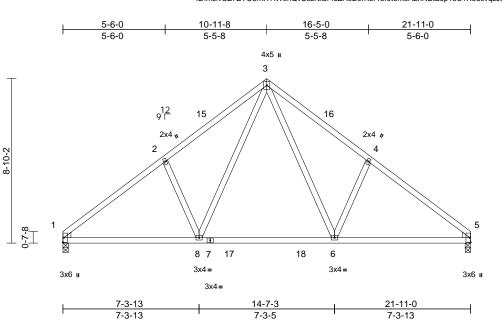
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	B03	Common	7	1	Job Reference (optional)	E14365741

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:09 ID:mJivUZ7ZYO6mtYAW9nQvSczKxJI-rJZA0E0rXor15fet6m8A2nNBtBJp1eU?X6ucvqzJtxO

Page: 1



Scale =	1:61.9
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00010 - 1.01.0												
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.00 1.15 YES IRC2018/TPI2014	CSI TC BC WB Matrix-AS	0.22 0.41 0.14	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.10 -0.16 0.02	(loc) 6-8 6-8 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 115 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS WEDGE BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.1 2x4 SP No.1 2x4 SP No.2 Left: 2x4 SP No.2 Right: 2x4 SP No.2 Structural wood she Rigid ceiling directly	athing directly applie applied. 5=0-3-8 C 10) C 11), 5=-29 (LC 11)	 4) Provide m bearing pl 1 and 29 l 5) This truss Internation R802.10.2 6) This truss 6) This truss 6) This trust 6) Structural chord and the bottom LOAD CASE(echanical connecti ate capable of with o uplift at joint 5. is designed in accu al Residential Coo and referenced st design requires tha wood sheathing be 1/2" gypsum shee chord.	standing 2 ordance w le sections andard AN at a minim	9 lb uplift at th the 2018 R502.11.1 ISI/TPI 1. um of 7/16" irectly to the	joint and top					
FORCES	(lb) - Maximum Com											
TOP CHORD	Tension 1-2=-1242/98, 2-15= 3-15=-1072/170, 3-1 4-16=-1160/148, 4-5	6=-1072/170,										
BOT CHORD	1-8=-48/1042, 7-8=0 17-18=0/689, 6-18=0)/689, 7-17=0/689,										
WEBS	· · · · ·	90/145, 3-8=-52/607	,									
NOTES												
	ed roof live loads have	been considered for										in the second se
this design											TH UA	ROUL
Vasd=99m B=45ft; L= MWFRS (d 3-0-0, Inte to 13-11-8 cantilever right expos	CE 7-16; Vult=125mph nph; TCDL=6.0psf; BC :24ft; eave=4ft; Cat. II; directional) and C-C E: rior (1) 3-0-0 to 10-11- , Interior (1) 13-11-8 to left and right exposed sed;C-C for members and ns shown; Lumber DO	DL=6.0psf; h=25ft; Exp B; Enclosed; xterior(2E) 0-0-0 to .8, Exterior(2R) 10-1 o 21-11-0 zone; ; end vertical left and and forces & MWFR:	t							MAS	SEA 1667	• -
 This trust on the bott 	s has been designed f tom chord in all areas	where a rectangle									A ST	E.E. LE

DOL=1.60 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom 3) chord and any other members, with BCDL = 10.0psf.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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May 4,2020

Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	C01	Common Supported Gable	1	1	Job Reference (optional)	E14365742

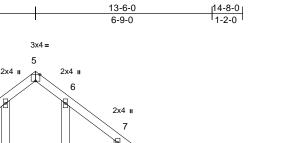
|-1-2-0 | 1-2-0

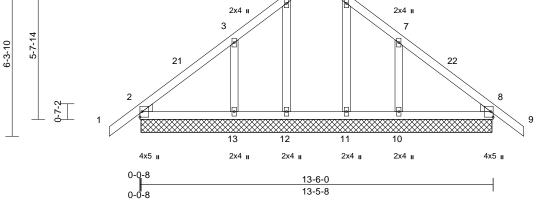
Carolina Structural Systems, LLC, Ether, NC - 27247,

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:09 ID:M?ZCQLILFhtnZiFCzjgC0ZzKxJX-rJZA0E0rXor15fet6m8A2nND9BO81g5?X6ucvqzJtxO

Page: 1

6-9-0 13-6-0 6-9-0 6-9-0 3x4 = 5 2x4 II 2x4 II 9¹² 4





Scale = 1:44

Plate Offsets (X, Y): [5:0-2-0,Edge]

Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00		TC	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15		BC	0.07	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES		WB	0.03	Horz(CT)	0.00	8	n/a	n/a		
BCDL	10.0	Code	IRC201	8/TPI2014	Matrix-AS							Weight: 72 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS WEDGE BRACING TOP CHORD BOT CHORD REACTIONS	 2x4 SP No.1 2x4 SP No.1 2x4 SP No.2 Left: 2x4 SP No.2 Right: 2x4 SP No.2 Structural wood she Rigid ceiling directly (size) 2=13-5-0, 11=13-5-(14=13-5-(14=13-5-(Max Horiz 2=112 (LC Max Uplift 2=-2 (LC (LC 11), 1 11), 18=-2 Max Grav 2=248 (LC 10=332 (L 10=332 (L 10=332 (L 14=248 (LC 10=332 (L 14=248 (LC 10=332 (L 14=248 (LC 10=332 (L 12=137 (L 14=248 (LC 10=332 (L 12=137 (L 12=37 (L 14=248 (LC 10=32 (L 12=137 (L 14=248 (L 10=32 (L) 10) (L) 10) (L 10=32 (L) 10) (L)	athing directly applied. applied. 8=13-5-0, 10=13-5-0, 0, 12=13-5-0, 13=13-5-0, 110, 14=13-5-0 2 100, 14=112 (LC 10) 110, 8=-2 (LC 11), 10=- 3=-71 (LC 11), 14=-2 (2 2 (LC 11) 2 1), 8=248 (LC 1), 1, 8=248 (LC 1), 2 1, 8=248 (LC 1), 1,	2) 0, 3) 71 4) LC 5) 6)), 7) 8) 9)	Wind: ASCE Vasd=99mpl B=45ft; L=24 MWFRS (dir 1-10-0, Exter 9-10-12, Extr left and right exposed;C-C reactions sho DOL=1.60 Truss desigg only. For stu see Standard or consult qu All plates arc Gable studs * This truss h on the bottor 3-06-00 tall b chord and ar Provide mec bearing plate 2, 2 lb uplift a at joint 10, 2 Non Standar This truss is International R802.10.2 at	7-16; Vult=125mpl 7-16; Vult=125mpl 7-16; Vult=125mpl 7-16; Call Electronic State 7-16; Vult=125mpl 7-16; Vult=125mpl 7-16; Vult=125mpl 7-16; Vult=125mpl 7-16; Vult=125mpl 8-16; Vult=125mpl 8-16; Vult=125mpl 8-16; Vult=125mpl 9-16; Vult=125mpl 9-1	CDL=6.(; Exp B; Corner(3; -9-0, Cc o 14-8-C tical left forces & =1.60 pla in the pl d (norm nd Detai igner as otherwis for a liv s where I fit betw with BC (by oth- anding 2 lb to ance wis sections dance wis sections dance A	ppsf; h=25ft; Enclosed; En	ever l, ole, l, lpsf om o nt ift			and the second s	Weight 72 ID	
WEBS	4-12=-66/9, 6-11=-6 7-10=-205/185	6/9, 3-13=-204/185,			od sheathing be ap 2" gypsum sheetro hord.					CHILLING STREET		SEA 166	72 : =
	ed roof live loads have n.	been considered for	L	DAD CASE(S)							MATIN	NGIN A ST	EEP. CALIN





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	C02	Common	4	1	Job Reference (optional)	E14365743

4x5 = 2

6-8-8

6-8-8

Carolina Structural Systems, LLC, Ether, NC - 27247.

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:09 ID:EmojGjLrJwNC1JY_CZI8BPzKxJT-rJZA0E0rXor15fet6m8A2nN95BLw1gr?X6ucvqzJtxO

13-5-0

6-8-8



GRIP

244/190

FT = 20%

Page: 1

12 9 Г 11 12 5-7-14 3 -7-0 Ø 4 2x4 🛛 3x6 II 3x6 II 6-8-8 13-5-0 6-8-8 6-8-8 (psf) Spacing 2-0-0 CSI DEFL in l/defl L/d PLATES (loc) 20.0 Plate Grip DOL 1.00 тс 0.34 Vert(LL) -0.05 4-7 >999 240 MT20 Lumber DOL BC 10.0 1 15 0.28 Vert(CT) >999 180 -0.09 4-7 0.0* Rep Stress Incr YES WB 0.05 Horz(CT) 0.02 n/a 1 n/a IRC2018/TPI2014 10.0 Code Matrix-AS Weight: 54 lb 5) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. This truss design requires that a minimum of 7/16" 6) structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. LOAD CASE(S) Standard 1=0-3-8, 3=0-3-8 THE MADE



SEAL

16673

MAN DALL

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

LUMBER

Scale = 1:41.9 Loading

TCLL (roof)

TCDI

BCLL

BCDL

TOP CHORD 2x4 SP No.1 2x4 SP No.1 BOT CHORD 2x4 SP No.2 WEBS WEDGE Left: 2x4 SP No.2 Right: 2x4 SP No.2 BRACING TOP CHORD Structural wood sheathing directly applied. BOT CHORD Rigid ceiling directly applied. REACTIONS (size) Max Horiz 1=95 (LC 10) Max Uplift 1=-18 (LC 11), 3=-18 (LC 11) Max Grav 1=537 (LC 1), 3=537 (LC 1) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-11=-602/93, 2-11=-484/117, 2-12=-484/117, 3-12=-602/93 BOT CHORD 1-4=-67/398, 3-4=0/398 WEBS 2-4=0/192

NOTES

Unbalanced roof live loads have been considered for 1) this desian.

- Wind: ASCE 7-16; Vult=125mph (3-second gust) Vasd=99mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; 2) B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 6-8-8, Exterior(2R) 6-8-8 to 9-8-8, Interior (1) 9-8-8 to 13-5-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- * This truss has been designed for a live load of 20.0psf 3) on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to 4) bearing plate capable of withstanding 18 lb uplift at joint 1 and 18 lb uplift at joint 3.

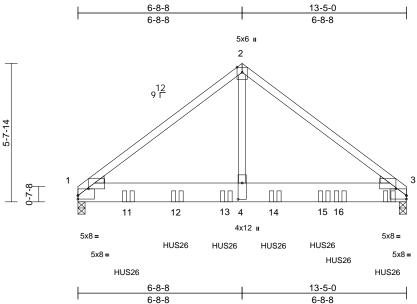


Job	Truss	Truss Type	Qty	Ply	200691RT1	
Asheville	C03	Common Girder	1	2	Job Reference (optional)	E14365744

Run: 8.33 S Mar 23 2020 Print: 8.330 S Mar 23 2020 MiTek Industries, Inc. Mon May 04 12:13:10 ID:iBu6RAyffhr7hABVHgHJn3zKxRj-rJZA0E0rXor15fet6m8A2nN4jBGI1TB?X6ucvqzJtxO



13-5-0 6-8-8



Scale = 1:47.1

Plate Offsets (X, Y): [1:Edge,0-1-12], [1:0-5-2,0-3-4], [3:Edge,0-1-12], [3:0-5-2,0-3-4], [4:0-8-0,0-2-0]

			5 /-			· · · · · ·							
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.00		TC	0.68	Vert(LL)	-0.07	4-10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15		BC	0.61	Vert(CT)	-0.13	4-10	>999	180		
BCLL	0.0*	Rep Stress Incr	NO		WB	0.86	Horz(CT)	0.01	3	n/a	n/a		
BCDL	10.0	Code	IRC201	8/TPI2014	Matrix-MS							Weight: 172 lb	FT = 20%
(0.131"x3 Top chorc oc. Bottom ch staggerec Web conr 2) All loads a except if r CASE(S) provided t unless oth	2x10 SP DSS 2x4 SP No.2 Left: 2x4 SP No.3 Right: 2x4 SP No.3 Structural wood shea 4-8-7 oc purlins. Rigid ceiling directly bracing. (size) 1=0-3-8, 3 Max Horiz 1=-95 (LC Max Grav 1=5086 (L (lb) - Maximum Com Tension 1-2=-6008/0, 2-3=-51 1-11=0/4740, 1-12= 4-13=0/4740, 3-16= 2-4=0/6774 s to be connected togel ") nails as follows: b connected as follows: at 0-4-0 oc. nected as follows: 2x4 - are considered equally noted as front (F) or bac section. Ply to ply com to distribute only loads nerwise indicated.	applied or 10-0-0 oc 3=0-3-8 : 24) .C 2), 3=6655 (LC 2) pression/Maximum 983/0 =0/4740, 12-13=0/47 0/4740, 12-13=0/47 4=0/4740 ther with 10d s: 2x4 - 1 row at 0-9-1 ows: 2x10 - 2 rows 1 row at 0-9-0 oc. applied to all plies, ck (B) face in the LO nections have been noted as (F) or (B),	6, 7, 40, L 0, 1, 0 AD	Vasd=99mpł B=45ft; L=24 MWFRS (dir end vertical I plate grip DC * This truss h on the bottor 3-06-00 tall b chord and ar This truss is International R802.10.2 ar Use Simpson Truss) or equ 2-0-12 from 1 to back face Fill all nail ho OAD CASE(S) Dead + Roo Plate Increa Uniform Loa Vert: 1-2: Concentrate Vert: 10=	as been designe n chord in all area by 2-00-00 wide w y other members designed in acco Residential Code draferenced sta a Strong-Tie HUS uivalent spaced a he left end to 12- of bottom chord. les where hange Standard of Live (balanced) ads (lb/ft) =-60, 2-3=-60, 5-1 ed Loads (lb) -1265 (B), 11=-1. 2 (B), 14=-1262 (I	CDL=6.(II; Exp B; er left annysed; Lum d for a liv as where vill fit betw s. rdance wi s sections ndard AN 26 (14-10 t 2-0-0 oc 8-10 to ca r is in com y: Lumber B=-20 262 (B), 1	Dipsf; h=25ft; Enclosed; d right exposibler DOL=1.6 e load of 20.1 a rectangle veen the bott th the 2018 R502.11.1 a ISI/TPI 1. Dd Girder, 4- max. startin ponnect truss(tact with lum Increase=1.	50 Opsf om 10d g at es) ber. 15,			38	A ST	13 HSS

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May 4,2020

