

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0420-1807 Lot 3 Stephenson Farm

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E14339381 thru E14339411

My license renewal date for the state of North Carolina is December 31, 2020.

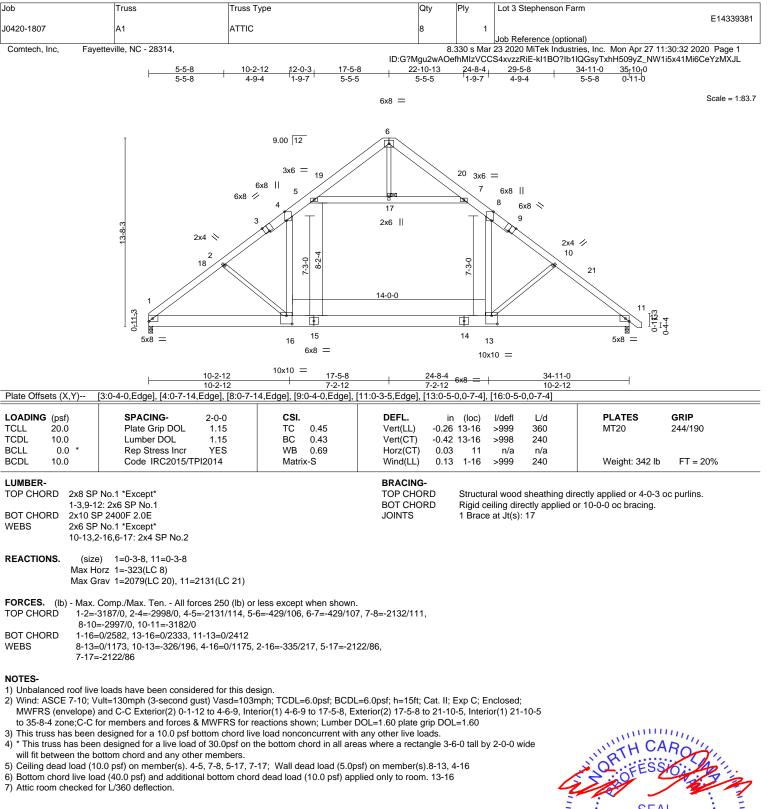
North Carolina COA: C-0844



April 27,2020

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Туре	Qty	Ply	Lot 3 Stephenson Farm	1	
J0420-1807	A1GE GABL		1	1	•		E14339382
Comtech, Inc, Fay	/etteville, NC - 28314,			8.330 s Mar	Job Reference (optional 23 2020 MiTek Industries		:30:33 2020 Page 1
	10-2-12	12-0-3 17-5·		3 24-8-4	xvzzRiE-CVbZbKIDo3Y7 34-11-0	35 _r 10 _r 0	oqT0BbMsIA_zMXJK
	10-2-12	1-9-7 5-5-5	5 5-5-5	1-9-7	10-2-12	0-11-0	
			6x8 =				Scale = 1:85.4
			12				
	6x8 2x4	9.00 $\overline{12}$ 9.00 $\overline{12}$ 11 10 9 $\overline{12}$ 6 7 $\overline{8}$ $\overline{11}$ $\overline{41}$		14 15 43 T B	16 17 6x8 17 18 2x4		
	2x4 5 2x4 4 2x4 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8-2-4	14-0-0	0; 	19 2x4 II 20	2x4 21 2x4 22 23 0 24 -	3-4-4
		5 34 33 32 6x8 =		31 6x8 =	29 28 27 26	25 5x8 =	٥
	10-2-12	17-5-8 7-2-12		-8-4 -12	34-11-0 10-2-12		
Plate Offsets (X,Y)	[6:0-4-0,Edge], [18:0-4-0,Edge], [23:0-3						
LOADING (psf) TCLL 20.0	SPACING-2-0-0Plate Grip DOL1.15	CSI. TC 0.11	DEFL. Vert(LL)	in (loc) 0.00 23	l/defl L/d n/r 120	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.18 WB 0.33	Vert(CT) Horz(CT)	0.00 23 0.01 23	n/r 120 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 391 lb	FT = 20%
BOT CHORD 2x10 S WEBS 2x6 SF 12-39, OTHERS 2x4 SF REACTIONS. All b. (lb) - Max H	-24: 2x6 SP No.1 SP 2400F 2.0E P No.1 *Except* 20-27,4-36: 2x4 SP No.2			Rigid cei	al wood sheathing directly applied or at Jt(s): 39, 42		
Max G	Grav All reactions 250 lb or less at join except 1=511(LC 21), 30=2891(LC						
TOP CHORD 1-2= 8-9= 13-1. 19-2 BOT CHORD 1-38 33-3 26-2	Comp./Max. Ten All forces 250 (lb) c -715/130, 2-3=-663/116, 3-4=-642/136, =-712/265, 9-10=-556/83, 10-11=-505/1 4=-505/119, 14-15=-556/83, 15-16=-71: 0=-578/140, 20-21=-591/90, 21-22=-61: =-37/489, 37-38=-37/489, 36-37=-37/48 4=-37/489, 30-33=-37/489, 29-30=-37/4 7=-37/489, 25-26=-37/489, 23-25=-37/4 0=-572/37, 8-33=-612/73	r less except when shown 4-5=-628/185, 5-7=-567/2 19, 11-12=-466/174, 12-13 2/265, 16-17=-502/219, 17 2/50, 22-23=-660/58 9, 35-36=-37/489, 34-35= 89, 28-29=-37/489, 27-28	44, 7-8=-518/270, 3=-466/174, '-19=-519/196, -37/489,			WITH CA	NRO(11), -
NOTES- 1) Unbalanced roof livv 2) Wind: ASCE 7-10; \ MWFRS (envelope) DOL=1.60 plate grip 3) Truss designed for ' Gable End Details a 4) All plates are 2x6 M 5) Gable requires cont 6) Gable studs spaced 7) This truss has been 8) * This truss has been will fit between the t	e loads have been considered for this d /ult=130mph (3-second gust) Vasd=103 gable end zone and C-C Exterior(2) zo DOL=1.60 wind loads in the plane of the truss only as applicable, or consult qualified buildir IT20 unless otherwise indicated. inuous bottom chord bearing. If at 2-0-0 oc. designed for a 10.0 psf bottom chord li en designed for a live load of 30.0psf on zottom chord and any other members, v 10.0 psf) on member(s). 8-9, 15-16, 9-4	mph; TCDL=6.0psf; BCDI ne;C-C for members and f For studs exposed to win g designer as per ANSI/Ti ve load nonconcurrent with the bottom chord in all are ith BCDL = 10.0psf.	forces & MWFRS fo nd (normal to the fac PI 1. n any other live load as where a rectang	r reactions sho ce), see Standa s. le 3-6-0 tall by	wn; Lumber ard Industry 2-0-0 wide	SEA 0363	EER ALI
Design valid for use only a truss system. Before u building design. Bracing is always required for st	esign parameters and READ NOTES ON THIS AN y with MITek® connectors. This design is based or use, the building designer must verify the applicabl g indicated is to prevent buckling of individual truss ability and to prevent collapse with possible persor ivery, erection and bracing of trusses and truss sys	ly upon parameters shown, and i ity of design parameters and pro web and/or chord members only al injury and property damage.	s for an individual buildin perly incorporate this des . Additional temporary a	g component, not ign into the overall nd permanent braci arding the	ng		A MiTek Affiliate

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

-	Job	Truss	Truss Type	Qty	Ply	Lot 3 Stephenson Farm		
	J0420-1807	A1GE	GABLE	1	1	E14339382		
						Job Reference (optional)		
	Comtech, Inc, Fayer		8.330 s Mar 23 2020 MiTek Industries, Inc. Mon Apr 27 11:30:33 2020 Page 2					
			ID:G?Mg	ID:G?Mgu2wAOefhMlzVCCS4xvzzRiE-CVbZbKIDo3Y7T627E?dFiA5FVwRpqT0BbMsIA_zMXJK				

NOTES-

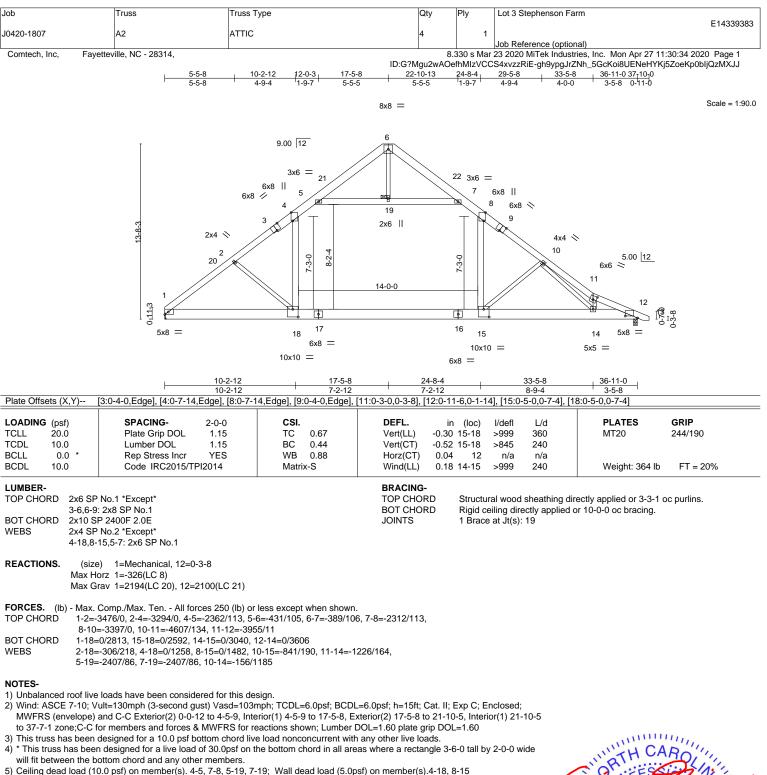
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 23, 35, 37, 28, 26 except (jt=lb) 34=2119, 38=145, 29=2119, 25=129, 27=109, 36=110.

11) This truss has large uplift reaction(s) from gravity load case(s). Proper connection is required to secure truss against upward movement at the bearings. Building designer must provide for uplift reactions indicated.

12) Attic room checked for L/360 deflection.

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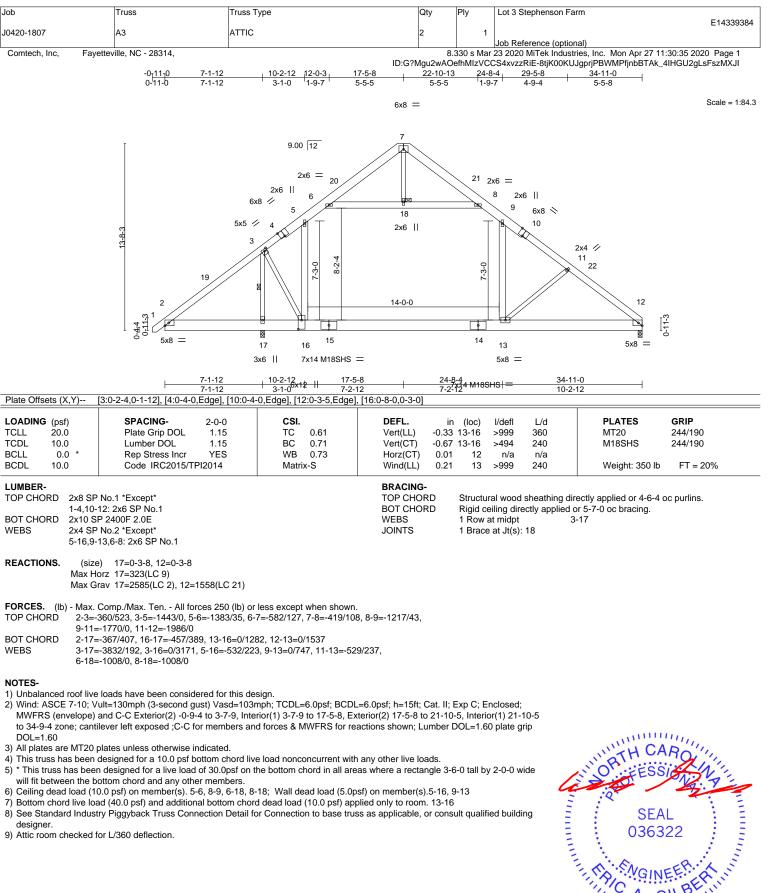


- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 15-18
- Refer to girder(s) for truss to truss connections.
- 8) Attic room checked for L/360 deflection.



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818 Soundside Road

Edenton, NC 27932

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Job	Truss	Truss Type	Qt	y Ply	Lot 3 Stephenson Farm	
J0420-1807	АЗА	ATTIC	1	2		E14339385
Comtech, Inc, Fay	etteville, NC - 28314,			8.330 s Mar 2		es, Inc. Mon Apr 27 11:30:36 2020 Page 1
	-0 <u>71170 7-1-1</u> 0-11-0 7-1-1		7-5-8 2	2-10-13 24-8-4	4 29-5-8	xiKZmiw7AyJojhX8GK1iNdHK4PnJzMXJH 34-11-0 5-5-8
	0-11-0 7-1-1	2 3-1-0 1-9-7 5	-5-5	5-5-5 '1-9-7	4-9-4	5-5-6 Scale = 1:84.3
			6x8 =			000ie - 1.04.0
	Ţ	9.00 $\overline{ 12 }$ 2x6 = 20	7	21 2xe	6 =	
		2x6 6x8 ≠ 6 5 ≠	gx 18			
			2x6 14-0-0	0.65		12
	で1 サビー サビー 5x8 =		23	14		
	0.00 —	17 16 ¹³ 3x6 8x16 M18SHS =			13 0x10 =	5x8 =
	7-1-1	2 10-2-12, the 11 17-5-	8	$\frac{24-8-4}{7-9-49}$ 6x8 =	34-11-0	
Plate Offsets (X,Y)	7-1-1 [3:0-2-0,0-1-12], [4:0-4-0,Edge	2 3-1-0 ^{0x12} 7-2-1 , [9:0-7-14,Edge], [10:0-4-0,Edge],		7-2-12	10-2-12 16:0-8-0,0-3-0]	2
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0 Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr N Code IRC2015/TPI201-	5 TC 0.44 5 BC 0.96 O WB 0.86	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.44 13-16 > -0.71 13-16 > 0.01 12	/defl L/d 746 360 468 240 n/a n/a 999 240	PLATES GRIP MT20 244/190 M18SHS 244/190 Weight: 700 lb FT = 20%
LUMBER-			BRACING-			
BOT CHORD 2x10 S WEBS 2x4 SF	2400F 2.0E *Except* 12: 2x6 SP 2400F 2.0E P 2400F 2.0E No.2 *Except* 13,6-8: 2x6 SP No.1		TOP CHOR BOT CHOR JOINTS		ng directly applied or	ctly applied or 6-0-0 oc purlins. 6-0-0 oc bracing.
Max H	e) 17=0-3-8, 12=0-3-8 orz 17=323(LC 9) rav 17=4603(LC 21), 12=2767	(LC 21)				
TOP CHORD 2-3=- 8-9=- BOT CHORD 2-17= WEBS 3-17=	360/515, 3-5=-3512/152, 5-6=- 2472/221, 9-11=-4020/206, 11 358/405, 16-17=-441/387, 13					
Top chords connect Bottom chords conn Webs connected as 2) All loads are conside ply connections hav 3) Unbalanced roof live 4) Wind: ASCE 7-10; V MWFRS (envelope) to 34-9-4 zone; cant DOL=1.60 5) All plates are MT20	ected as follows: 2x10 - 2 rows follows: 2x4 - 1 row at 0-9-0 oc ered equally applied to all plies, a been provided to distribute or loads have been considered f ult=130mph (3-second gust) V and C-C Exterior(2) -0-9-4 to 3 ilever left exposed ;C-C for mel plates unless otherwise indicat	gered at 0-9-0 oc, 2x8 - 2 rows sta staggered at 0-3-0 oc. , 2x6 - 2 rows staggered at 0-9-0 o except if noted as front (F) or back ly loads noted as (F) or (B), unless or this design. asd=103mph; TCDL=6.0psf; BCDL -7-9, Interior(1) 3-7-9 to 17-5-8, Ex nbers and forces & MWFRS for rea	c. (B) face in the L(otherwise indicat =6.0psf; h=15ft; C terior(2) 17-5-8 to actions shown; Lu	DAD CASE(S) see ed. at. II; Exp C; Enc 21-10-5, Interior(mber DOL=1.60 p	losed; 1) 21-10-5	SEAL
 7) * This truss has bee will fit between the b 8) Ceiling dead load (1 9) Bottom chord live lo 10) See Standard Indu designer. 11) Hanger(s) or other 	n designed for a live load of 30 ottom chord and any other mer 0.0 psf) on member(s). 5-6, 8-5 ad (40.0 psf) and additional bot stry Piggyback Truss Connecti connection device(s) shall be p	Opsf on the bottom chord in all are	as where a rectan sf) on member(s). lied only to room. Iss as applicable, ntrated load(s) 32	gle 3-6-0 tall by 2 5-16, 9-13 13-16 or consult qualifie 37 lb down and 4	ed building	036322
12) Attic room checked				,13.		April 27,2020
Design valid for use only a truss system. Before u building design. Bracing is always required for sta fabrication, storage, deli	sign parameters and READ NOTES O with MiTek® connectors. This design i se, the building designer must verify the indicated is to prevent buckling of indiv ibility and to prevent collapse with poss erv, erection and bracing of trusses an	N THIS AND INCLUDED MITEK REFERENCE based only upon parameters shown, and is applicability of design parameters and prop dual truss web and/or chord members only. ble personal injury and property damage. F truss systems, see ANS/TPI Q Lee Street, Suite 312, Alexandria, VA 2231.	for an individual build erly incorporate this de Additional temporary or general guidance re uality Criteria, DSB-8:	ng component, not sign into the overall and permanent bracing	g	AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

[Job	Truss	Truss Type	Qty	Ply	Lot 3 Stephenson Farm
						E14339385
	J0420-1807	A3A	ATTIC	1	2	
					2	Job Reference (optional)
-	Comtech, Inc, Fayettevi	lle, NC - 28314,		8.3	330 s Mar	23 2020 MiTek Industries, Inc. Mon Apr 27 11:30:36 2020 Page 2

8.330 s Mar 23 2020 MiTek Industries, Inc. Mon Apr 27 11:30:36 2020 Page 2 ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-c4HiDML64_xiKZmiw7AyJojhX8GK1iNdHK4PnJzMXJH

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-60, 5-6=

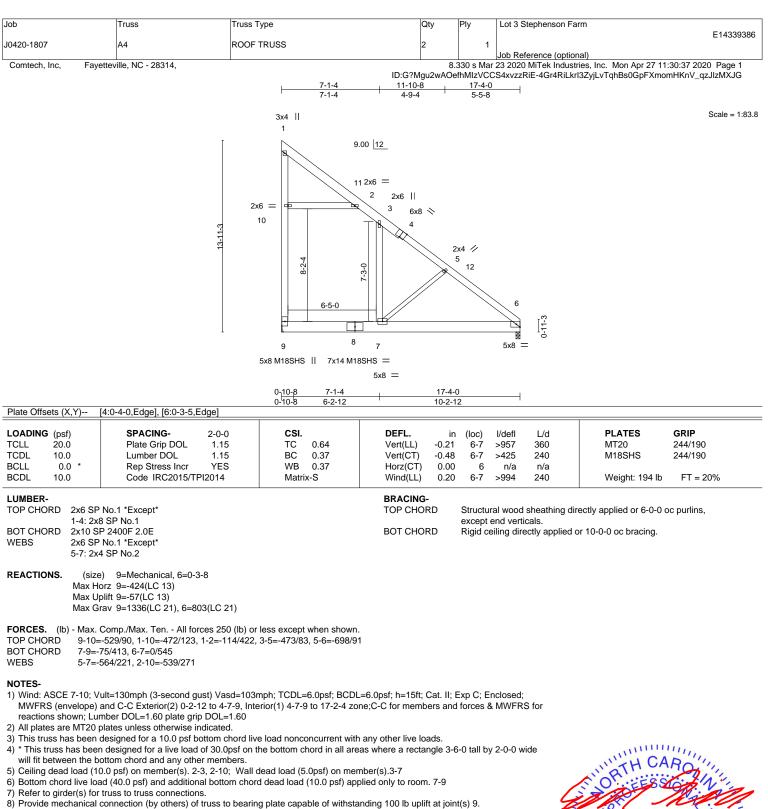
Vert: 1-5=-60, 5-6=-80, 6-7=-60, 7-8=-60, 8-9=-80, 9-12=-60, 2-16=-20, 13-16=-40, 12-13=-20, 6-8=-20 Drag: 5-16=-10, 9-13=-10

Concentrated Loads (lb)

Vert: 23=-1837(F)

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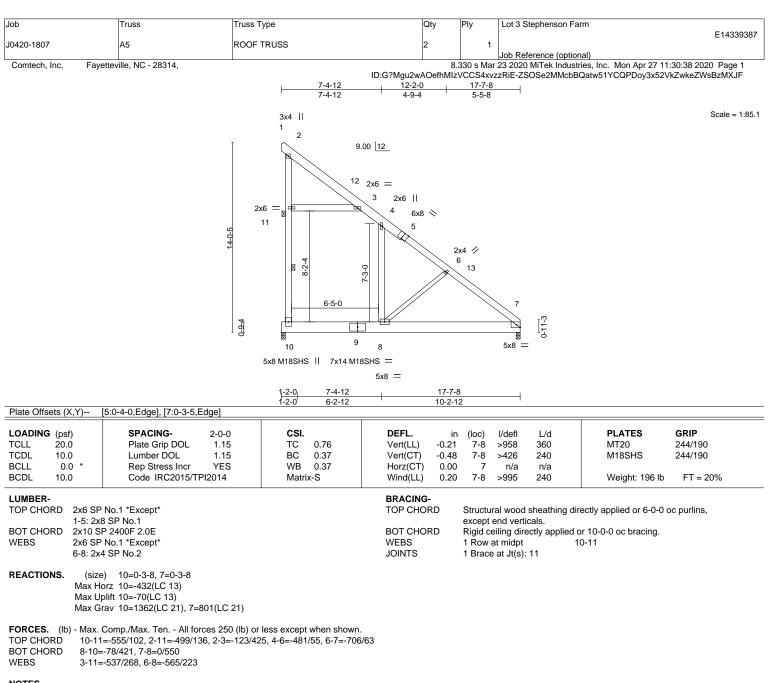




9) Attic room checked for L/360 deflection.



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NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-0 to 4-5-13, Interior(1) 4-5-13 to 17-5-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) All plates are MT20 plates unless otherwise indicated.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 3-11; Wall dead load (5.0psf) on member(s).4-8

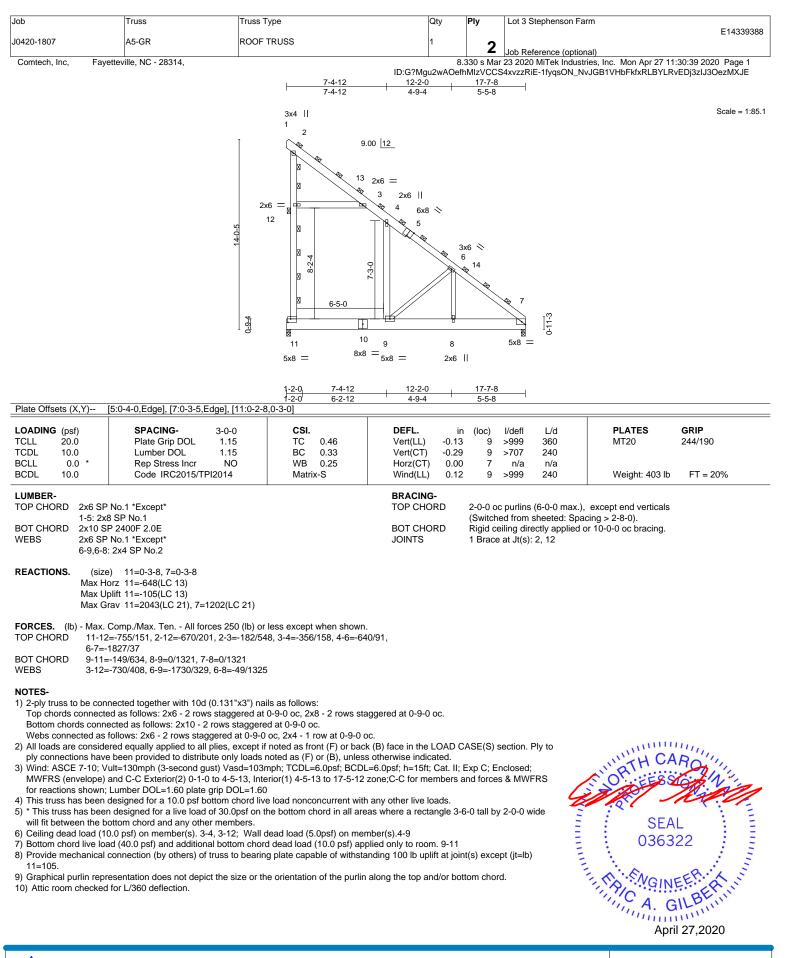
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 8-10
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10.

8) Attic room checked for L/360 deflection.

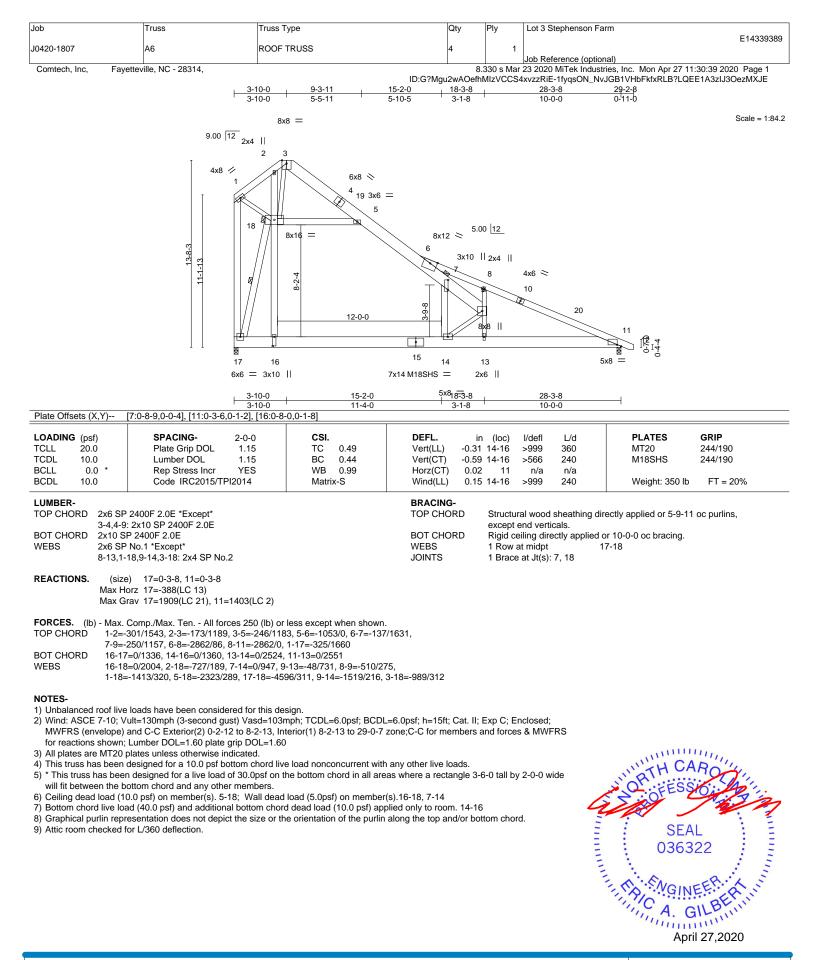


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Edenton, NC 27932

Job	Truss	russ Type	Qty	Ply	Lot 3 Stephenson Fa	arm	
J0420-1807		OOF TRUSS	1		Lot 3 Stephenson 1		E14339390
	etteville, NC - 28314,		'	2	Job Reference (optio	nal) ries, Inc. Mon Apr 27 11	30:42 2020 Page 1
	<u> 3-10-0</u> → 3-10-0					2UEsGOHMZ3zjAYT7R 29-2-β 0-11-0	
	9.00 $\overline{12}_{2x4}$ 2 4x8 $_{1}$ $_{2}$ 4x8 $_{1}$ $_{1}$ 4x8 $_{1}$ $_{1}$ 4x8 $_{1}$ $_{1}$ 6x6 = 3 $_{3}$ -10-0 $_{3}$ -10-0 $_{3}$ -10-0	15-2-0	8x12 = 5 6 3x6 6 3x6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	00 12 1 2x4 8 8 8 8 8 8 8 8 8 8 8 8 8	4x6 = 10 28-3-8 10-0	5x8 =	
Plate Offsets (X,Y)	[7:0-0-3,Edge], [11:0-3-6,Edge], [1	6:0-7-8,0-1-8], [18:0-4-8,0-1-3]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-3-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.37 BC 0.36 WB 0.37 Matrix-S	Vert(LL) -0.23 Vert(CT) -0.44 Horz(CT) 0.02	n (loc) 3 14-16 4 14-16 2 11 14-16	l/defl L/d >999 360 >754 240 n/a n/a >999 240	PLATES MT20 Weight: 700 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x10 SF WEBS 2x6 SP 8-13,1- REACTIONS. (size Max Ho	2400F 2.0E *Except* : 2x10 SP 2400F 2.0E P 2400F 2.0E No.1 *Except* 18,9-14,3-18: 2x4 SP No.2 :) 17=0-3-8, 11=0-3-8 orz 17=-582(LC 13) row 17=264(LC 21) 11=2104(LC	2)	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	(Switch Rigid ce 1 Row a	ed from sheeted: Spa	or 10-0-0 oc bracing.	
FORCES. (lb) - Max. TOP CHORD 1-2= 7-9= BOT CHORD 16-17 WEBS 16-18 1-18= NOTES- 1) 2-ply truss to be coni	rav 17=2864(LC 21), 11=2104(LC Comp./Max. Ten All forces 250 (452/2315, 2-3=-260/1784, 3-5=-36 375/1735, 6-8=-4293/129, 8-11=-4 =0/2004, 14-16=0/2039, 13-14=0/ =0/3007, 2-18=-1090/284, 7-14=0, -2120/480, 5-18=-3485/434, 17-18 nected together with 10d (0.131"x3 d as follows: 2x6 - 2 rows stagger	 ib) or less except when shown. 8/1774, 5-6=-1579/0, 6-7=-205/ 293/0, 1-17=-487/2490 3786, 11-13=0/3827 1421, 9-13=-72/1096, 8-9=-765 =-6895/466, 9-14=-2278/324, 3 ") nails as follows: 	5/413, 3-18=-1483/467				
Bottom chords conne Webs connected as 1 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-10; Vi MWFRS (envelope) for reactions shown; 5) This truss has been 6) * This truss has been will fit between the bo 7) Ceiling dead load (10 8) Bottom chord live load	ected as follows: 2x10 - 2 rows sta follows: 2x6 - 2 rows staggered at red equally applied to all plies, exc e been provided to distribute only lk loads have been considered for th ult=130mph (3-second gust) Vasd- and C-C Exterior(2) 0-2-12 to 8-2- Lumber DOL=1.60 plate grip DOL designed for a 10.0 psf bottom chc n designed for a 10.0 psf bottom chc n designed for a live load of 30.0ps bottom chord and any other membe 0.0 psf) on member(s). 5-18; Wall dd (40.0 psf) and additional bottom esentation does not depict the size	ggered at 0-9-0 oc. 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc pept if noted as front (F) or back bads noted as (F) or (B), unless is design. =103mph; TCDL=6.0psf; BCDL: 13, Interior(1) 8-2-13 to 29-0-7 z =1.60 rd live load nonconcurrent with f on the bottom chord in all area rs. dead load (5.0psf) on member(chord dead load (10.0 psf) app	c. (B) face in the LOAD (otherwise indicated. =6.0psf; h=15ft; Cat. II; cone;C-C for members any other live loads. as where a rectangle 3- s).16-18, 7-14 lied only to room. 14-1	Exp C; E and forces 6-0 tall by	nclosed; s & MWFRS	SEA 0363	22 EFR. A.

- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) Attic room checked for L/360 deflection.



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Job	Truss Truss	Туре	Qty Ply	Lot 3 Stephenson Fa	rm E14339391
J0420-1807	A6GE GABL	E	1 1	Job Reference (option	nal)
Comtech, Inc, Fay	etteville, NC - 28314, _ 3-10-0	9-3-11			ies, Inc. Mon Apr 27 11:30:41 2020 Page 1 _RKfgigm70sQWg96kixVMQcoATWzMXJC 29-2-8
	3-10-0	5-5-11	5-10-5 3-1-8	10-0-0	0- ¹ 11-0
	8 9.00 12 2x4	x8 =			Scale = 1:84.2
		3			
		6x8 5 6 3x6	=		
	27 2×4 II 2×4 II	33 34 8x16 =	8x12 ≈ ^{5.00} 12		
		4	3x10 2x4 ²		
		8-2-4		2x4 13 14 2x4	
		12-0-0		15	
	26 25 6x6 = 3x10	II	²⁴ 23 22 21 2 7x14 M18SHS = 3x10	20 19 18 5	x8 = 0
	<u> 3-10-0</u> 3-10-0	15-2-0	5x8 ₁₈₌₃₋₈ 3-1-8	<u>28-3-8</u> 10-0-0	
	[5:0-4-0,Edge], [9:0-8-9,0-0-4], [16:0-11				
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.48	DEFL. in (loc) Vert(LL) -0.31 23-25	l/defl L/d >999 360	PLATES GRIP MT20 244/190 M48SUS 244/400
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.43 WB 1.00 Matrix-S	Vert(CT) -0.58 23-25 Horz(CT) 0.02 16 Wind(LL) 0.21 23-25	>576 240 n/a n/a >999 240	M18SHS 244/190 Weight: 381 lb FT = 20%
BOT CHORD 2x10 S WEBS 2x6 SP	1: 2x10 SP 2400F 2.0E P 2400F 2.0E No.1 *Except* I-27,11-23,3-27: 2x4 SP No.2		except BOT CHORD Rigid co WEBS 1 Row a	end verticals. eiling directly applied o	6-27
Max H Max U	e) 26=0-3-8, 16=0-3-8 orz 26=-566(LC 13) plift 26=-102(LC 13), 16=-124(LC 13) rav 26=1884(LC 21), 16=1403(LC 2)				
TOP CHORD 1-2=- 7-8=- 12-14	Comp./Max. Ten All forces 250 (lb) o 495/1545, 2-3=-349/1195, 3-4=-391/12 1053/8, 8-9=-340/1584, 9-11=-479/115 8-2768/233, 14-15=-2780/195, 15-16=- 3=0/1361, 23-25=0/1384, 22-23=-41/25	19, 4-6=-425/1150, 6-7=-4 9, 8-10=-2839/359, 10-12 2816/127, 1-26=-548/170	141/904, =-2759/250, 17		
19-20 WEBS 25-27 1-27=		/2550 1-21=-302/826, 10-11=-2 315/538, 7-34=-2349/539	284/237,		
NOTES-					
2) Wind: ASCE 7-10; V	Ioads have been considered for this de ult=130mph (3-second gust) Vasd=103 gable end zone and C-C Exterior(2) zo	mph; TCDL=6.0psf; BCD			HINTH CAR
DOL=1.60 plate grip 3) Truss designed for w	DOL=1.60 vind loads in the plane of the truss only.	For studs exposed to wi	nd (normal to the face), see Stanc	dard Industry	A Fran
4) All plates are MT20	s applicable, or consult qualified buildin plates unless otherwise indicated. T20 unless otherwise indicated.	g designer as per ANSI/T	PI 1.		SEAL
6) Gable studs spaced7) This truss has been	at 2-0-0 oc. designed for a 10.0 psf bottom chord liv			y 2-0-0 wide	036322
will fit between the b	n designed for a live load of 30.0psf on ottom chord and any other members. 0.0 psf) on member(s). 27-33, 33-34, 7-		с ,	y 2-0-0 wide	NGINEER ON
10) Bottom chord live lo	and (40.0 psf) and additional bottom ch l connection (by others) of truss to beau	ord dead load (10.0 psf) a	pplied only to room. 23-25	cept (jt=lb)	A. GILBERT
26=102, 16=124. 12) Graphical purlin rep 13) Attic room checked	presentation does not depict the size or	the orientation of the purl	in along the top and/or bottom cho	ord.	April 27,2020
WARNING - Verify de	sign parameters and READ NOTES ON THIS AN				ENGINEERING BY
a truss system. Before us building design. Bracing	with MiTek® connectors. This design is based on se, the building designer must verify the applicabil indicated is to prevent buckling of individual truss with provide prevent buckling of a second	ty of design parameters and pro web and/or chord members only	perly incorporate this design into the overal v. Additional temporary and permanent bra		
fabrication, storage, deliv	bility and to prevent collapse with possible person very, erection and bracing of trusses and truss sys pilable from Truss Plate Institute. 218 N. Lee Stree	tems, see ANSI/TPI1 G	Quality Criteria, DSB-89 and BCSI Buildir	ng Component	818 Soundside Road

fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DS** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	Lot 3 Stephenson Fa	arm]
J0420-1807	B1	ATTIC	3	1			E14339392
					Job Reference (optio		1:20:42 2020 Dage 1
Comtech, Inc, Fay	etteville, NC - 28314,		ID:G?Mgu2wAOefh	MIzVCCS	34xvzzRiE-vQCLhlQVF	ries, Inc. Mon Apr 27 1 R8pigeo2q5ob6HWnLyjl	
			<u>10-9-8 13-11-4 </u> 3-1-12 3-1-12	<u>16-10-4</u> 2-11-0	<u>21-7-0</u> <u>4-8-12</u>		
			5x5 =				Scale = 1:76.9
			ene -				
		12.00 12	6				
	2-11-13	5x8 / 2x4 = $2x4 = $ $2x4 17$ 3 $16 =$	15 2x6	2x4 = 7 18			
	·	8,2,4 8,2,4	11-8-0	5-8-12	4x6 X 9	5.5.5	
		⊠ 14 13	12		11 10	-	
		3x6 8x8 =	8x8 =		8x8 = 3x6	11	
		4-8-12 10-9-8 4-8-12 6-0-12		-4	21-7-0		
Plate Offsets (X,Y)	[2:0-0-8,0-2-0], [9:0-1-8,0-2-0],	[11:0-4-0,0-4-12], [13:0-4-0,0-4-12]				T	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0 Plate Grip DOL 1. Lumber DOL 1. Rep Stress Incr YE Code IRC2015/TPI201	5 TC 0.79 5 BC 0.75 S WB 0.20	Vert(LL) -0.23 Vert(CT) -0.38 Horz(CT) 0.01		l/defl L/d >999 360 >659 240 n/a n/a >999 240	PLATES MT20 Weight: 226 lb	GRIP 244/190 FT = 20%
			BRACING- TOP CHORD BOT CHORD JOINTS	except e Rigid ce	al wood sheathing di end verticals. eiling directly applied at Jt(s): 15	rectly applied or 4-2-1	5 oc purlins,
Max H	e) 14=0-3-8, 10=Mechanical orz 14=329(LC 9) rav 14=1486(LC 21), 10=1445	(LC 20)					
TOP CHORD 2-3= BOT CHORD 13-14	.1617 [/] 0, 3-4=-981/145, 7-8=-98 4=-312/478, 11-13=0/995	50 (lb) or less except when shown. 4/149, 8-9=-1597/0, 2-14=-1643/8, 9- 30/189, 7-15=-1030/189, 2-13=0/854					
 Wind: ASCE 7-10; \ MWFRS (envelope) 21-4-4 zone; end ve plate grip DOL=1.60 This truss has been 4) * This truss has been will fit between the b 	and C-C Exterior(2) -0-9-6 to 3 rtical left and right exposed;C-0 designed for a 10.0 psf bottom n designed for a live load of 30 tottom chord and any other me	asd=103mph; TCDL=6.0psf; BCDL=6 -7-7, Interior(1) 3-7-7 to 10-9-8, Exter for members and forces & MWFRS chord live load nonconcurrent with an 0psf on the bottom chord in all areas nbers.	ior(2) 10-9-8 to 15-2-5 for reactions shown; L ny other live loads. where a rectangle 3-6	5, Interior Lumber D 6-0 tall by	(1) 15-2-5 to OL=1.60		
6) Bottom chord live lo	ad (40.0 psf) and additional bo truss to truss connections.	, 4-15, 7-15; Wall dead load (5.0psf) tom chord dead load (10.0 psf) applie			4	TH CA	ROIN



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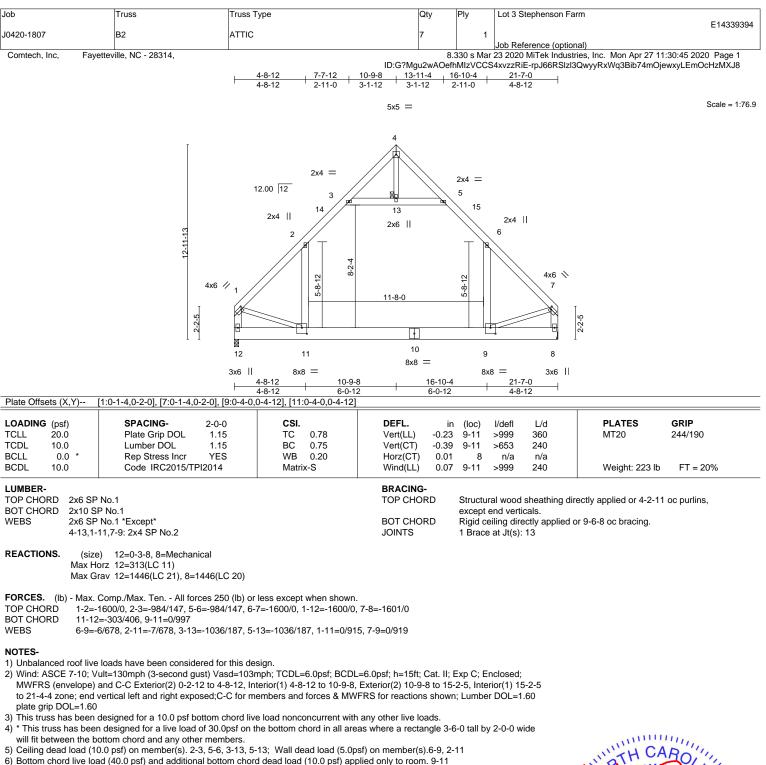


Job	Truss	Truss Type	Qty	Ply	Lot 3 Stephenson Farm	
10420-1807	B1GE	GABLE	1	1		E1433939
	etteville, NC - 28314,		'		Job Reference (optional) s, Inc. Mon Apr 27 11:30:44 2020 Page 1
Connech, Inc, Fay	elleville, NC - 20314,	-0 ₁ 11-0 4-8-12 7-7-12 7	ID:G?Mgu2wAOefh	MIzVCCS	4xvzzRiE-Ncmjv5R7CRxZ	IoNFOoJqeU2_SM21vMco6a0q3rzMXJ9
				- <u>10-4</u> 11-0	<u>21-7-0</u> <u>22-6-0</u> 4-8-12 0-11-0	
			5x5 =			Scale = 1:81
		12.00 12				
				8 🔪		
	0- <u>4</u> .8 	4 2 2 2 2 2 2 2 2 5 4 4 7 2 2 6 2 5 4 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		2-8-12	12 13 4x6 \ 28 29 14 15 17 17 17 17 17 17 17 17 17 17	6.4. 6
		8 22 21 20 4x6 8x8 =	19 8x8 =	18 8x8	⊠ 17 17 16 = 4x6	
Plate Offsets (X,Y)	[2:0-1-0,0-2-0], [14:0-1-0,0-2-(2x6 <u>4-8-12 10-9-8</u> <u>4-8-12 6-0-12</u>], [18:0-4-0,0-5-8], [20:0-4-0,0-5-8]			2x6 21-7-0 4-8-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-C Plate Grip DOL 1. Lumber DOL 1. Rep Stress Incr Y1 Code IRC2015/TPI201	15 TC 0.70 15 BC 0.72 S WB 0.72	Vert(LL) -0.21 Vert(CT) -0.35 Horz(CT) 0.01	(loc) 18-20 18-20 16 18-20	l/defi L/d >999 360 >726 240 n/a n/a >999 240	PLATES GRIP MT20 244/190 Weight: 244 lb FT = 20%
	P No.1 9 No.1 *Except* 20,14-18: 2x4 SP No.2		BRACING- TOP CHORD BOT CHORD JOINTS	except e Rigid ce	al wood sheathing direc end verticals. illing directly applied or - at Jt(s): 23, 25, 28	tly applied or 4-9-4 oc purlins, 10-0-0 oc bracing.
Max H	e) 22=0-3-8, 16=0-3-8 orz 22=422(LC 11) rav 22=1480(LC 21), 16=148()(LC 20)				
FORCES. (lb) - Max. TOP CHORD 2-3=- 12-12 BOT CHORD 21-22 WEBS 12-18 11-27 18-28	Comp./Max. Ten All forces 2 1601/0, 3-4=-1592/42, 4-5=-9 3=-1591/42, 13-14=-1600/0, 2- 2=-379/571, 20-21=-379/571, 1 3=0/790, 4-20=0/790, 5-24=-1 7=-1075/235, 8-23=-438/0, 2-2	50 (lb) or less except when shown. 15/179, 7-8=-26/326, 8-9=-26/326, 1	=-83/286 070/236, //874,			
 Wind: ASCE 7-10; V MWFRS (envelope) MWFRS for reaction Truss designed for v Gable End Details a All plates are 2x4 Mi Gable studs spaced This truss has been This truss has been This truss has been This truss has been Ceiling dead load (1 4-20 	gable end zone and C-Č Exte is shown; Lumber DOL=1.60 p vind loads in the plane of the ti s applicable, or consult qualifie 720 unless otherwise indicated at 2-0-0 oc. designed for a 10.0 psf bottom n designed for a live load of 30 ottom chord and any other me 0.0 psf) on member(s). 4-5, 11	asd=103mph; TCDL=6.0psf; BCDL= ior(2) zone; end vertical left and righ late grip DOL=1.60 uss only. For studs exposed to wind d building designer as per ANSI/TP chord live load nonconcurrent with .0psf on the bottom chord in all area mbers. -12, 5-24, 23-24, 23-27, 11-27; Wal	It exposed;C-C for men (normal to the face), s 1. any other live loads. s where a rectangle 3-f I dead load (5.0psf) on	bers and see Stand 6-0 tall by member(I forces & ard Industry	SEAL 036322
 Bottom chord live lo. Attic room checked 		tom chord dead load (10.0 psf) app	iea only to room. 18-20	,		April 27,2020

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April 27,2020



7) Refer to girder(s) for truss to truss connections.

8) Attic room checked for L/360 deflection.

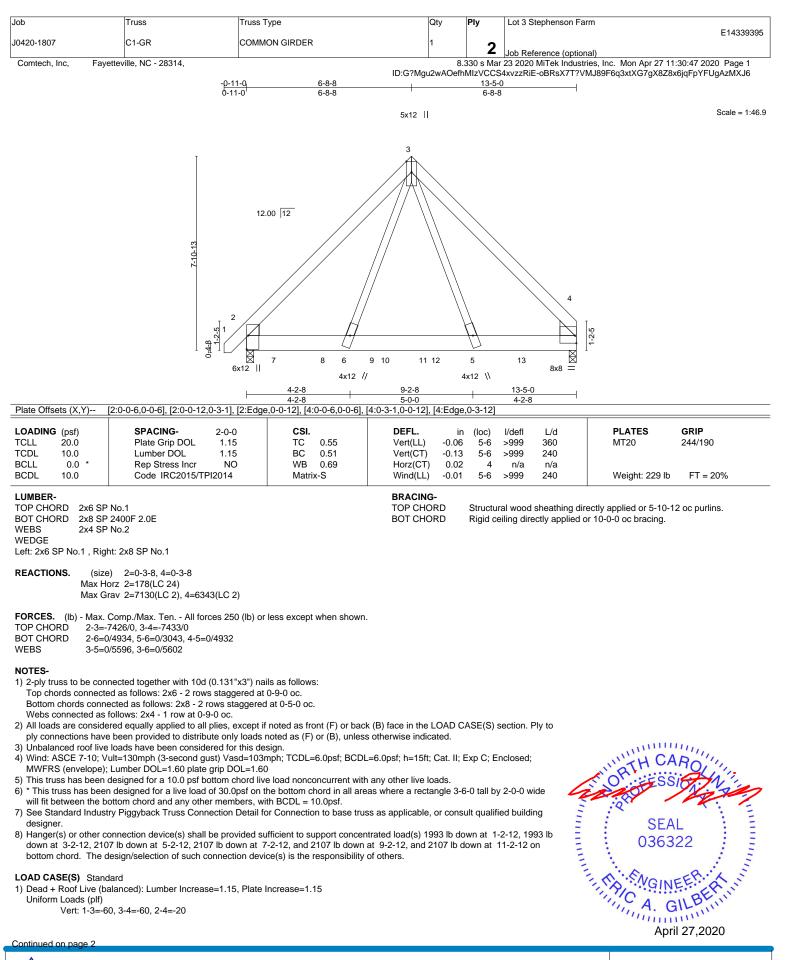


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Edenton, NC 27932



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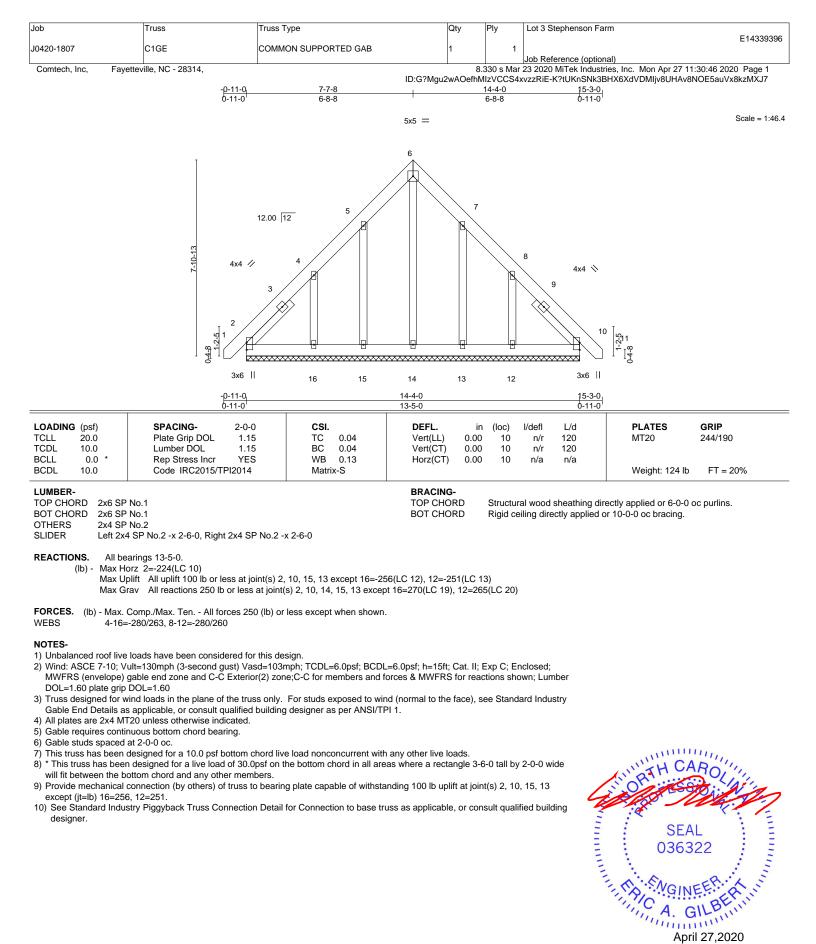
Job		Truss	Truss Type	Qty	Ply	Lot 3 Stephenson Farm		
							E14339395	
J0420-1807		C1-GR	COMMON GIRDER	1	2			
					∠	Job Reference (optional)		
Comtech, Inc, Fayetteville, NC - 28314, 8.330 s Mar 23 20					23 2020 MiTek Industries,	Inc. Mon Apr 27 11:30:47 2020 Page 2		
	-			ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-oBRsX7T?VMJ89F6q3xtXG7gX8Z8x6jqFpYFUgAzMXJ6				

LOAD CASE(S) Standard Concentrated Loads (Ib)

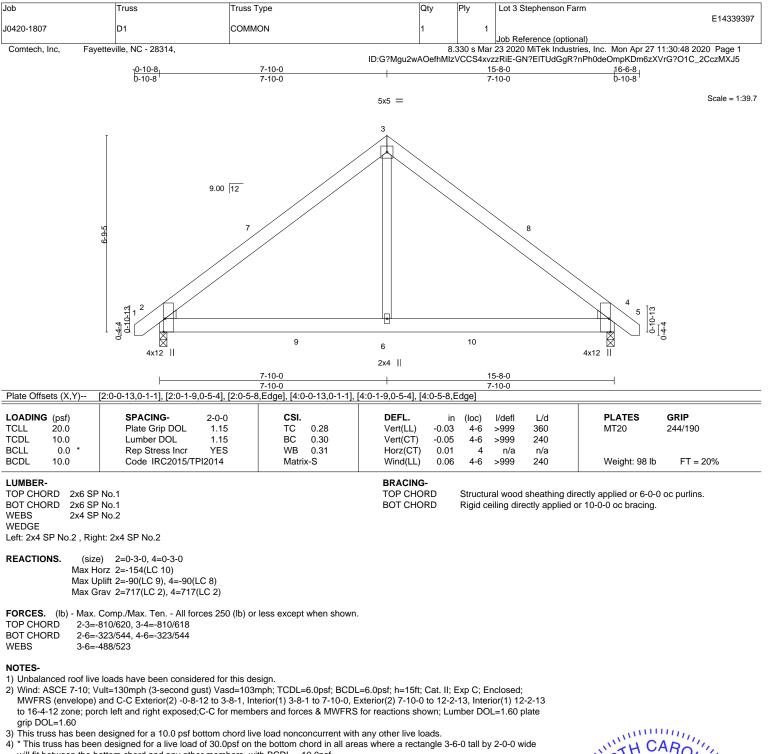
Vert: 5=-1835(B) 7=-1732(B) 8=-1732(B) 9=-1835(B) 11=-1835(B) 13=-1835(B)

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will fit between the bottom chord and any other members, with BCDL = 10.0psf.

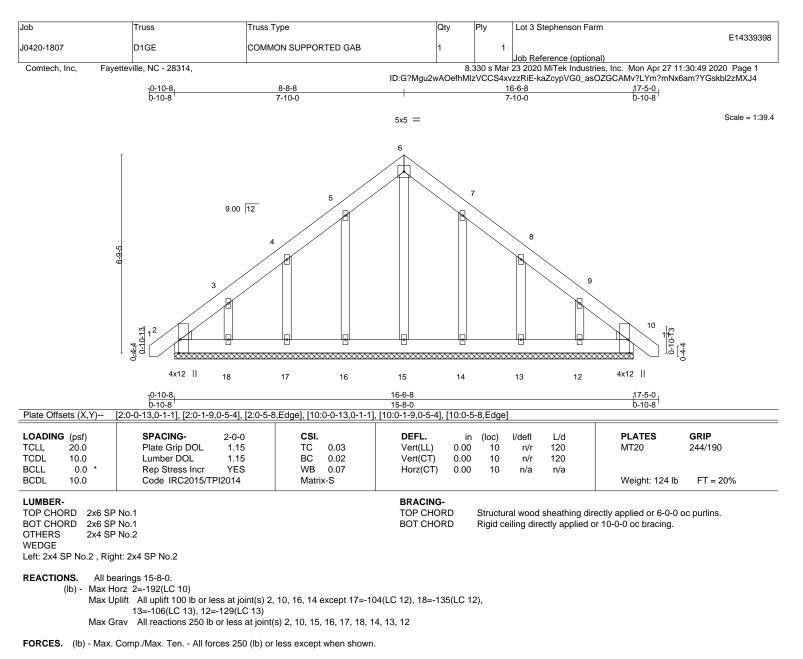
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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NOTES-

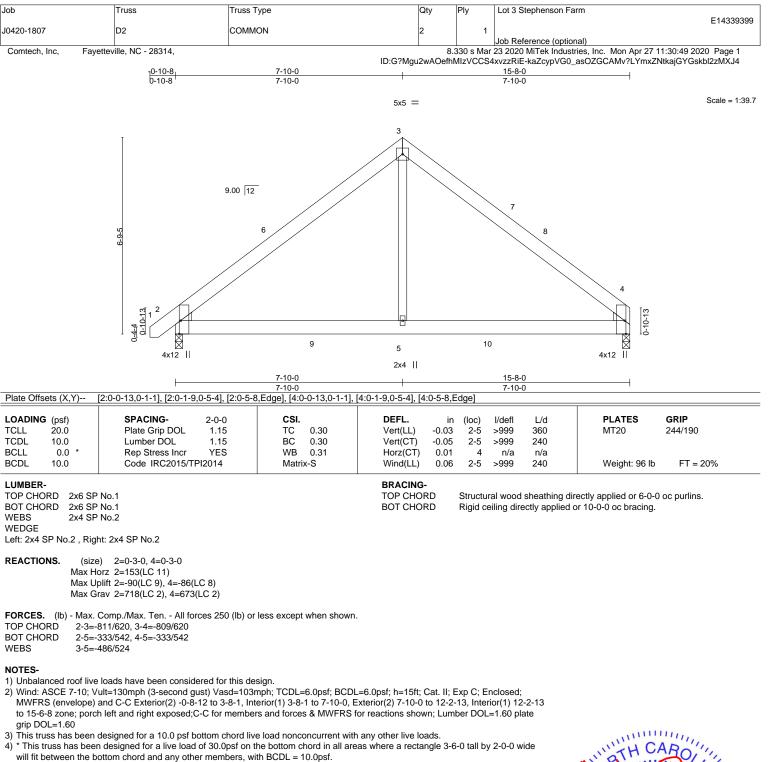
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 16, 14 except (jt=lb) 17=104, 18=135, 13=106, 12=129.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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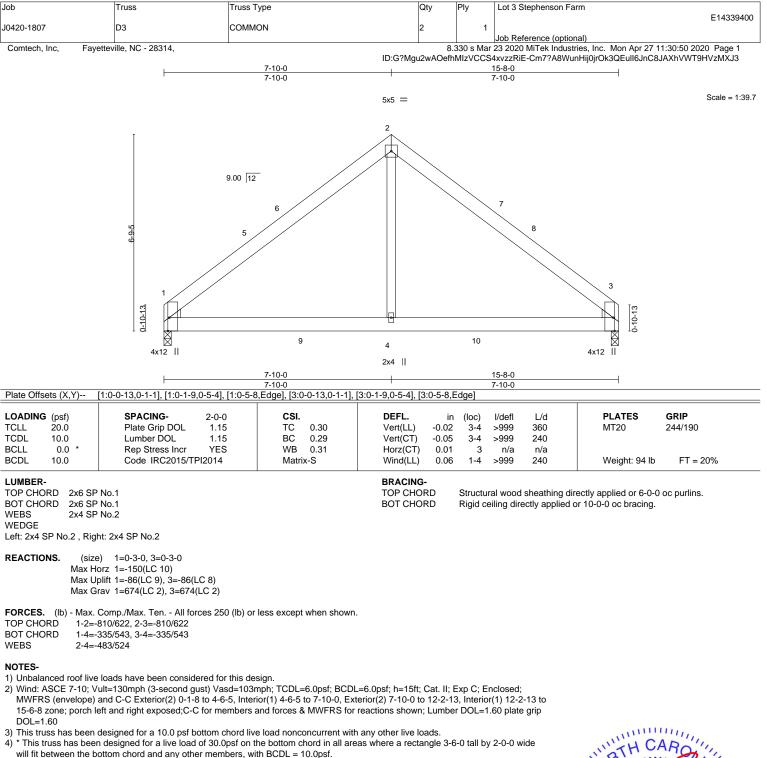


- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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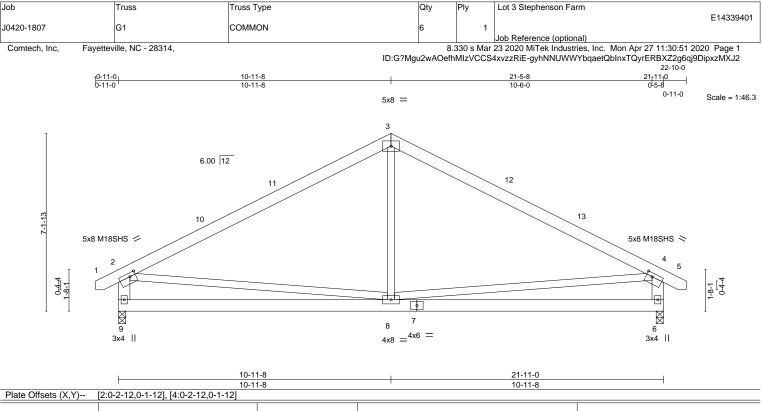


5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.46 BC 0.34 WB 0.09 Matrix-S	Vert(CT) -(Horz(CT) ().06	(loc) l/def 8-9 >999 8-9 >999 6 n/3 8 >999	9 360 9 240 a n/a	PLATES MT20 M18SHS Weight: 158 lb	GRIP 244/190 244/190 FT = 20%
			BRACING- TOP CHORD BOT CHORD	ex	xcept end v	erticals.	rectly applied or 6-0-0 o	oc purlins,
Max U	e) 9=0-3-8, 6=0-3-8 prz 9=122(LC 11) plift 9=-63(LC 12), 6=-63(LC 13) rav 9=918(LC 1), 6=918(LC 1)							

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1013/264, 3-4=-1013/264, 2-9=-821/343, 4-6=-821/343
- BOT CHORD 8-9=-270/547, 6-8=-227/538
- WEBS 2-8=0/378, 4-8=0/378, 3-8=0/409

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-2 to 3-7-11, Interior(1) 3-7-11 to 10-11-8, Exterior(2) 10-11-8 to 15-4-5, Interior(1) 15-4-5 to 22-8-2 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

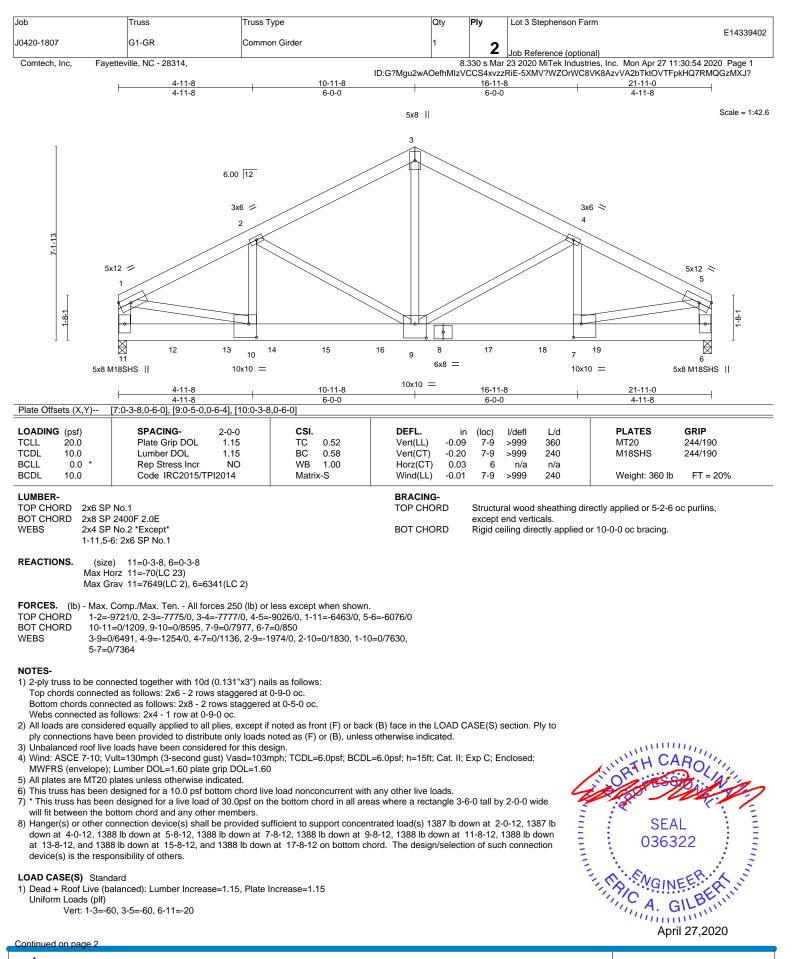
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 6.



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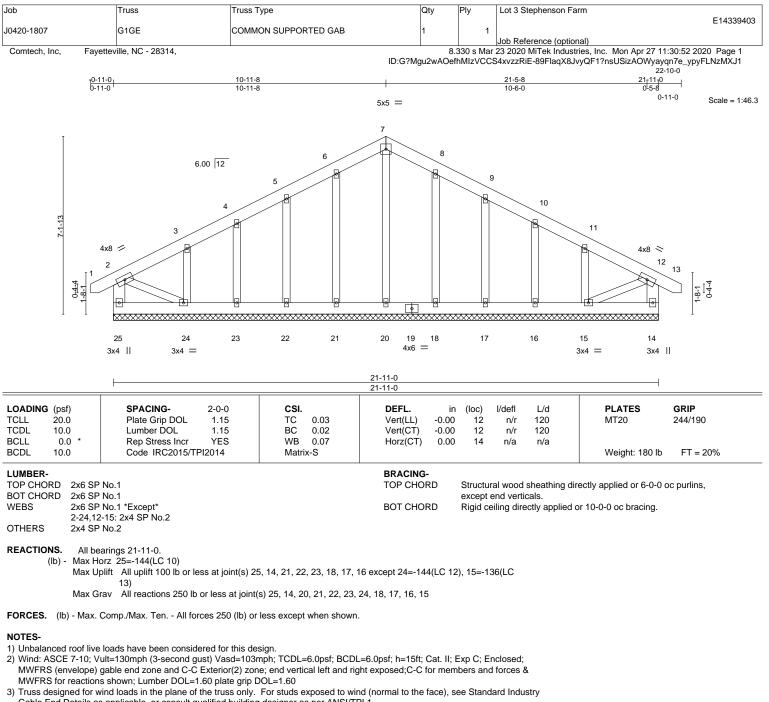
[Job	Truss	Truss Type	Qty	Ply	Lot 3 Stephenson Farm	
						E143394	102
	J0420-1807	G1-GR	Common Girder	1	2		
					~	Job Reference (optional)	
	Comtech, Inc, Fayettev	ille, NC - 28314,	8.330 s Mar 23 2020 MiTek Industries, Inc. Mon Apr 27 11:30:54 2020 Page 2				
			ID:G?Mgu2wAOefhMIzVCCS4xvzzRiE-5XMV?WZOrWC8VK8AzvVA2bTkt0VTFpkHQ7RMQGzMXJ?				

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 8=-1130(B) 12=-1128(B) 13=-1128(B) 14=-1130(B) 15=-1130(B) 16=-1130(B) 17=-1130(B) 18=-1130(B) 19=-1130(B)

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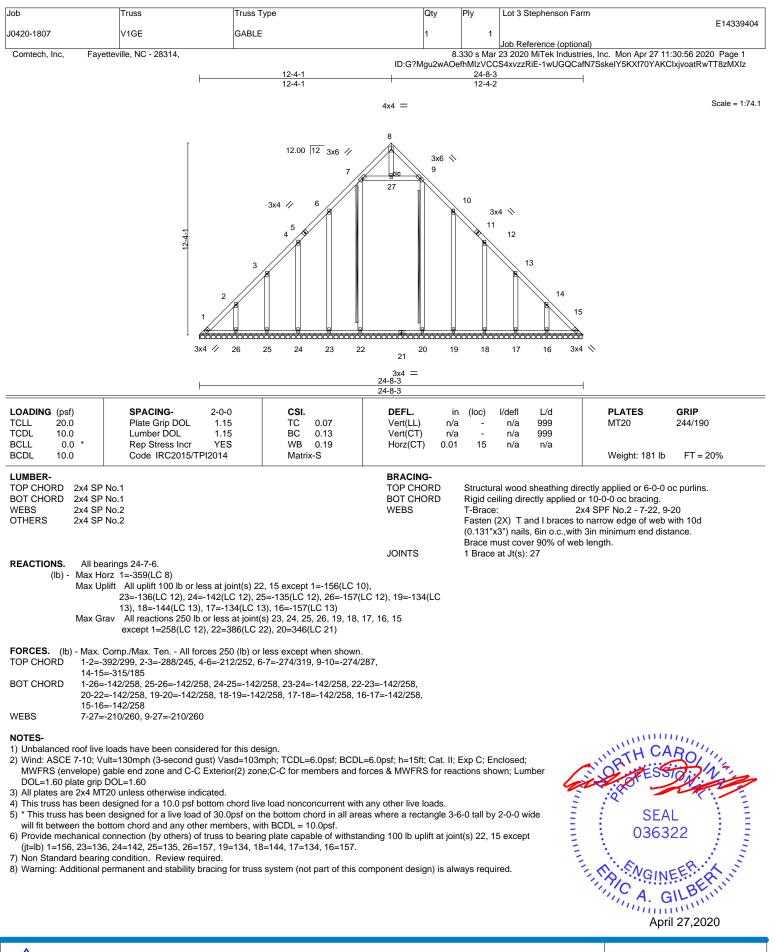




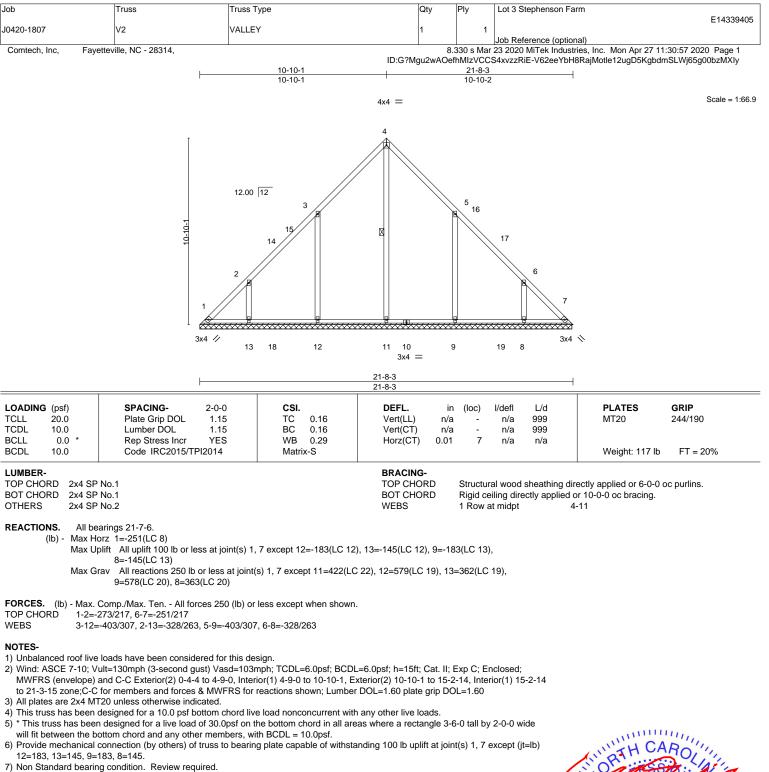
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 14, 21, 22, 23, 18, 17, 16 except (jt=lb) 24=144, 15=136.



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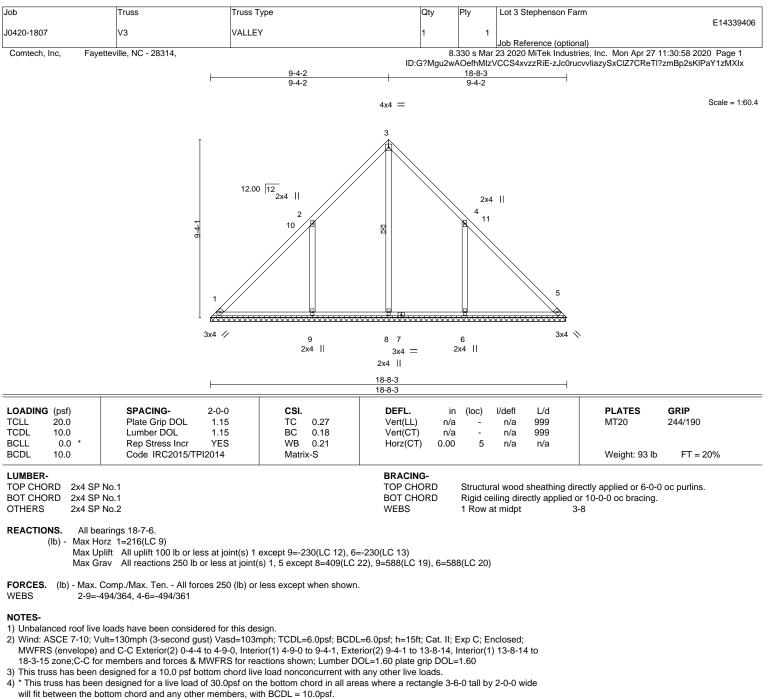
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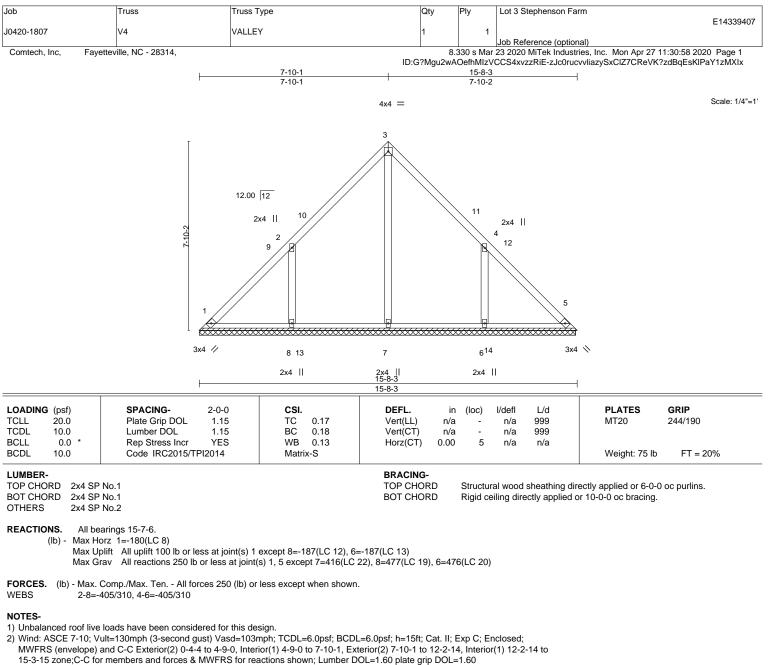


5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=230, 6=230.

6) Non Standard bearing condition. Review required.



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This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

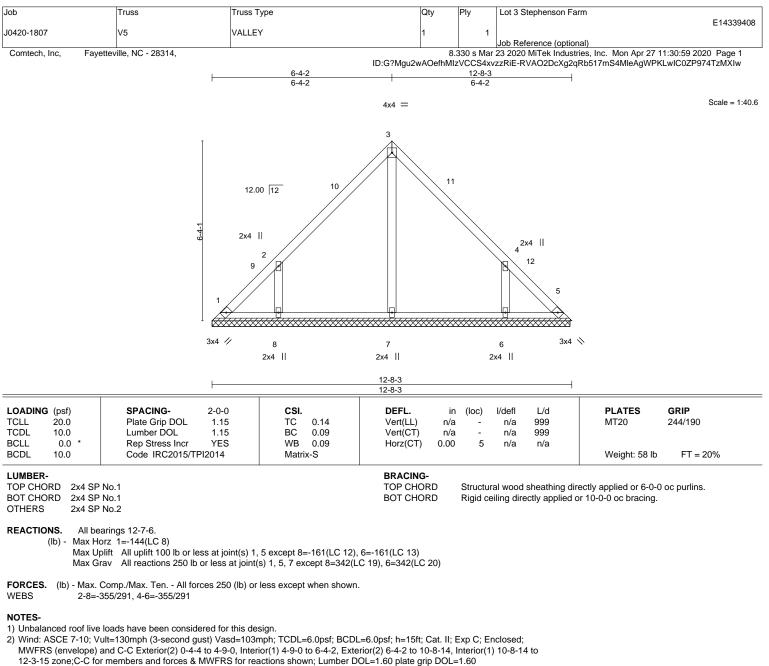
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=187, 6=187.

6) Non Standard bearing condition. Review required.



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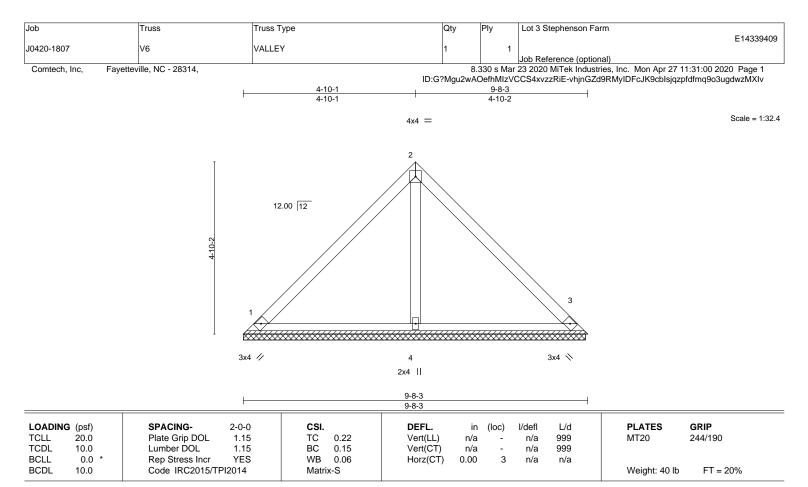
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161, 6=161.

6) Non Standard bearing condition. Review required.



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LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size)

CTIONS. (size) 1=9-7-6, 3=9-7-6, 4=9-7-6 Max Horz 1=-108(LC 8) Max Uplift 1=-27(LC 13), 3=-27(LC 13) Max Grav 1=204(LC 1), 3=204(LC 1), 4=311(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

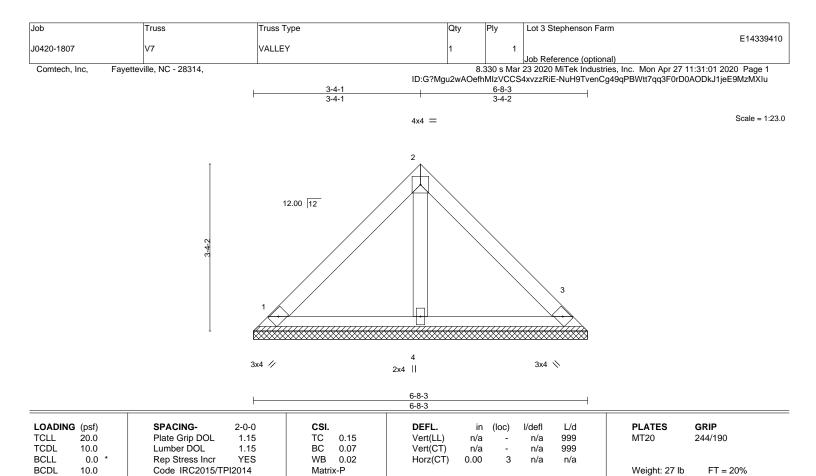
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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LUM	BE	R-

BCDL

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 OTHERS

10.0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 27 lb

FT = 20%

REACTIONS. (size) 1=6-8-3, 3=6-8-3, 4=6-8-3 Max Horz 1=-72(LC 8)

Max Uplift 1=-26(LC 13), 3=-26(LC 13) Max Grav 1=146(LC 1), 3=146(LC 1), 4=187(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arip DOL=1.60

Matrix-P

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

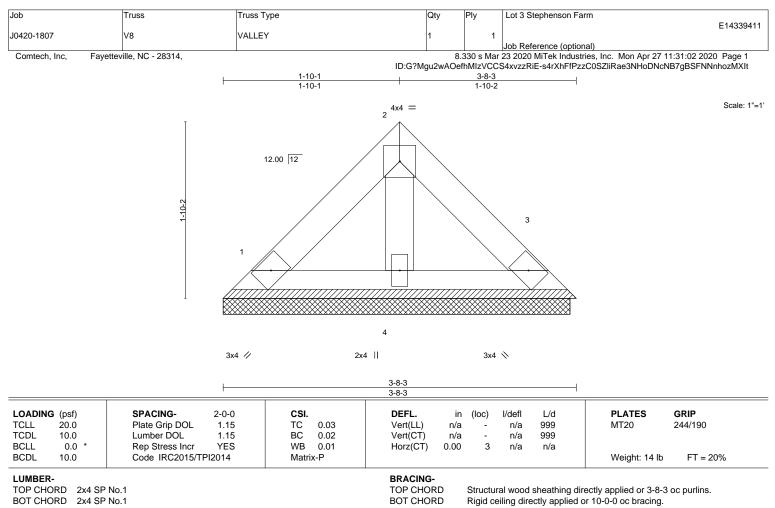
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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OTHERS 2x4 SP No.2

REACTIONS. (size) 1=3-7-6, 3=3-7-6, 4=3-7-6 Max Horz 1=-36(LC 8) Max Uplift 1=-13(LC 13), 3=-13(LC 13) Max Grav 1=73(LC 1), 3=73(LC 1), 4=93(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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