

Trenco 818 Soundside Rd Edenton, NC 27932

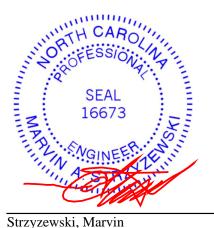
Re: J0420-1762 Weaver/Lot 1 Stephenson Farm/Harnett

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: E14331127 thru E14331154

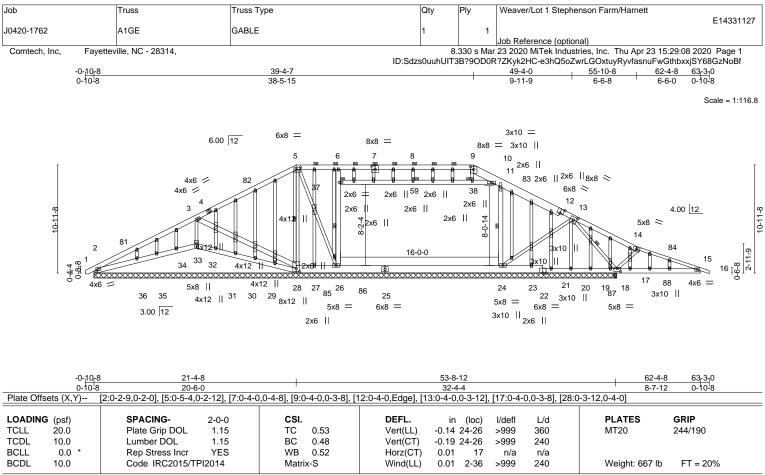
My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



April 24,2020

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.1 *Except*	TOP CHORD	Structural wood sheathing directly applied or 5-9-10 oc purlins,
	9-12: 2x10 SP No.1, 14-16: 2x4 SP No.1		except
BOT CHORD	2x6 SP No.1 *Except*		2-0-0 oc purlins (6-0-0 max.): 5-9.
	25-28: 2x10 SP No.1, 22-25: 2x10 SP 2400F 2.0E	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2x4 SP No.2 *Except*	WEBS	1 Row at midpt 26-37, 11-24, 5-28, 13-17
	3-28,6-26,11-24,10-37: 2x6 SP No.1	JOINTS	1 Brace at Jt(s): 37, 38, 59
OTHERS	2x4 SP No.2		

(lb) - Max Horz 2=215(LC 12)

All bearings 53-0-0.

- Max Uplift All uplift 100 lb or less at joint(s) 28, 26, 36, 35 except 2=-126(LC 13), 33=-451(LC 12), 24=-139(LC 13), 17=-994(LC 9), 27=-908(LC 18), 23=-903(LC 18), 18=-613(LC 3) Max Grav All reactions 250 lb or less at joint(s) 36, 35, 34, 32, 31, 30, 29, 20, 19
 - except 2=338(LC 24), 33=1127(LC 24), 28=312(LC 18), 26=1678(LC 18), 24=1532(LC 21), 17=2155(LC 25), 17=2148(LC 1), 21=412(LC 18), 18=332(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-413/233, 3-5=-740/454, 9-10=-851/629, 10-11=-718/545, 11-13=-609/378, 13-14=-1002/1157, 14-15=-963/911, 5-6=-586/519, 6-8=-684/563, 8-9=-688/562

 BOT CHORD
 2-36=-88/309, 35-36=-67/274, 34-35=-72/292, 33-34=-71/287, 32-33=-72/287, 31-32=-72/289, 30-31=-72/289, 29-30=-72/290, 28-29=-72/287, 27-28=-120/518, 26-27=-117/520, 24-26=-148/568, 23-24=-98/369, 12-123=-111/369, 20-21=-98/369, 19-20=-98/369, 18-19=-98/369, 17-18=-98/369, 15-17=-800/997

 WEBS
 3-28=-105/302, 26-37=-683/392, 6-37=-621/357, 13-24=-388/506, 3-33=-974/545, 11-24=-679/564, 13-17=-1676/1210, 14-17=-253/217

NOTES-

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-10 to 3-8-3, Exterior(2) 3-8-3 to 20-6-0, Corner(3) 20-6-0 to 24-9-4, Exterior(2) 24-9-4 to 38-5-15, Corner(3) 38-5-15 to 42-10-12, Exterior(2) 42-10-12 to 62-4-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

CONTINUED TO THE ADDRESS OF THE ADDRESS ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 1 Stephenson Farm/Harnett
					E14331127
J0420-1762	A1GE	GABLE	1	1	
					Job Reference (optional)
Comtech, Inc, Fayettev	ille, NC - 28314,		8	330 s Mar	23 2020 MiTek Industries, Inc. Thu Apr 23 15:29:09 2020 Page 2

8.330 s Mar 23 2020 MiTek Industries, Inc. Thu Apr 23 15:29:09 2020 Page 2 ID:Sdzs0uuhUIT3B?9OD0R?ZKyk2HC-6FFoI8ZZcfOFZ1T9?gRu63J3?Kc6Q2B4y6HfgizNoBe

NOTES-

5) Provide adequate drainage to prevent water ponding.

6) All plates are 2x4 MT20 unless otherwise indicated.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 26, 36, 35 except (jt=lb) 2=126, 33=451, 24=139, 17=994, 27=908, 23=903, 18=613.

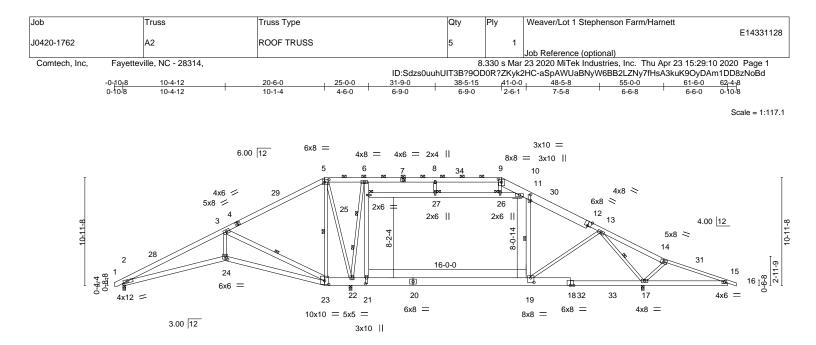
11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system, see **ANSVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





1	<u> </u>	23-1-12 25-0-0	<u>41-0-0</u> 16-0-0	52-10 11-10		
Plate Offsets (X,Y)	[2:0-4-9,0-2-0], [5:0-5-4,0-3-0], [9:0-4-0,					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.83 BC 0.74 WB 1.00 Matrix-S	Vert(LL) -0.39 Vert(CT) -0.68 Horz(CT) 0.28	(loc) l/defl L/d 19-21 >909 360 19-21 >526 240 17 n/a n/a 2-24 >999 240	PLATES MT20 Weight: 537 lb	GRIP 244/190 FT = 20%
9-12: 2 BOT CHORD 2x6 SF 20-23, WEBS 2x4 SF	P No.1 *Except* 2x10 SP No.1, 14-16: 2x4 SP No.1 P No.1 *Except* 18-20: 2x10 SP 2400F 2.0E P No.2 *Except* -21,11-19,10-25: 2x6 SP No.1, 6-22: 2x4	SP No.1	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Structural wood sheathing except 2-0-0 oc purlins (4-3-8 ma Rigid ceiling directly applie 1 Row at midpt 2 Rows at 1/3 pts 1 Brace at Jt(s): 25, 26, 22	x.): 5-9. ed or 6-0-0 oc bracing. 3-23, 11-19, 5-23, 13-1 6-22	•
Max H Max G FORCES. (lb) - Max. TOP CHORD 2-3=	e) 2=0-3-8, 22=0-3-8, 17=0-3-8 (req. lorz 2=-141(LC 10) Srav 2=1539(LC 2), 22=1834(LC 26), 17 Comp./Max. Ten All forces 250 (lb) or -4831/135, 3-5=-1891/71, 9-10=-2244/7	=3048(LC 27) less except when shown. 4, 10-11=-2284/25, 11-13=	-2598/0,	T Diace at 3((3), 23, 20, 21		
BOT CHORD 2-24 15-1 WEBS 3-23 3-24	4=-862/1146, 14-15=-853/902, 5-6=-183 =0/4346, 23-24=0/4337, 22-23=0/1619, 7=-792/858 =-3000/310, 6-22=-2612/0, 21-25=0/210 =0/2298, 11-19=-259/586, 9-26=0/349, 5 7=-3341/507	21-22=0/2177, 19-21=0/22 5, 6-25=0/2182, 13-19=-93	11, 17-19=0/1493, /1108,			
 Wind: ASCE 7-10; MWWFRS (envelope) 38-5-15, Exterior(2) & MWFRS for react WARNING: This lo handling and erection Trusses ("BCSI"), jog qualified registered permanent individual bracing. Provide adequate d This truss has been (a) This truss has been (b) This truss has been (c) This truss has been (c) Ceiling dead load (1) Bottom chord live load 	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=103 and C-C Exterior(2) -0-8-10 to 3-8-3, In 38-5-15 to 42-10-12, Interior(1) 42-10-1 ions shown; Lumber DOL=1.60 plate gri ng span truss requires extreme care and on guidance, see Guide to Good Practice inity produced by SBCA and TPI. The b design professional for the design and ir al truss member restraint/bracing. MiTel rainage to prevent water ponding. designed for a 10.0 psf bottom chord live no designed for a live load of 30.0psf on bottom chord and any other members, w 0.0 psf) on member(s). 10-11, 25-27, 26 wad (40.0 psf) and additional bottom chor al bearing size at joint(s) 17 greater thar	mph; TCDL=6.0psf; BCDL terior(1) 3-8-3 to 20-6-0, E 2 to 62-4-8 zone; cantileve o DOL=1.60 e pole and the second second second e for Handling, Installing & uilding owner or the owner isspection of the temporary a assumes no responsibility the load nonconcurrent with the bottom chord in all area ith BCDL = 10.0psf. -27, 10-26; Wall dead load d dead load (10.0 psf) app	Atterior (2) 20-6-0 to 24-7 r right exposed ;C-C for Bracing of Metal Plate (s authorized agent shal installation restraint/bra r for truss manufacture, any other live loads. Is where a rectangle 3-6 d (5.0psf) on member(s	6-0 tall by 2-0-0 wide).21-25, 11-19	SEA 166	73 EEER HUIL

- 7) Ceiling dead load (10.0 psf) on member(s). 10-11, 25-27, 26-27, 10-26; Wall dead load (5.0psf) on member(s).21-25, 11-19
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21
- 9) WARNING: Required bearing size at joint(s) 17 greater than input bearing size.

10) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface

Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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-	Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 1 Stephenson Farm/Harnett
	J0420-1762	A2	ROOF TRUSS	5	1	E14331128
ľ	JU42U-1762	A2	ROOF TRUSS	5	1	Job Reference (optional)
L	Comtech, Inc, Fayette	ville, NC - 28314,		8		23 2020 MiTek Industries, Inc. Thu Apr 23 15:29:10 2020 Page 2
		, ,	ID:Sdzs0uuhL	JIT3B?9OI	00R?ZKyk2	2HC-aSpAWUaBNyW6BB2LZNy7fHsA3kuK9OyDAm1DD8zNoBd

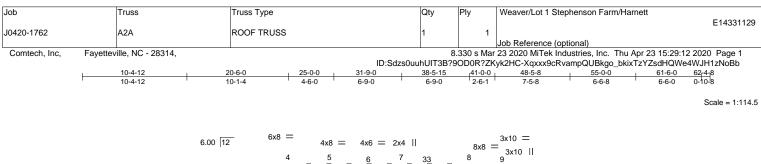
11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

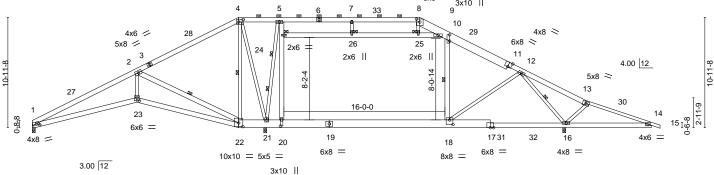
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

13) Attic room checked for L/360 deflection.

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⊢	<u>10-4-12</u> <u>20-6-0</u> 10-4-12 10-1-4	23-1-12 25-0-0	41-0-0	52-10-4		
Plate Offsets (X,Y)	[1:0-3-7,0-2-0], [4:0-5-4,0-3-0], [8:0-4-0,		11:0-4-0,Edge], [18:0-4-0	0,0-5-8], [20:0-7-4,0-1-8], [2	2:0-5-0,0-4-7], [23:0-3-0,0	-3-8]
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 1.00 BC 0.73 WB 1.00 Matrix-S	Vert(LL) -0.39		PLATES MT20 Weight: 535 lb	GRIP 244/190 FT = 20%
8-11: 2 BOT CHORD 2x6 SF 19-22, WEBS 2x4 SF	P No.1 *Except* 2x10 SP No.1, 13-15: 2x4 SP No.1 P No.1 *Except* 17-19: 2x10 SP 2400F 2.0E P No.2 *Except* -20,10-18,9-24: 2x6 SP No.1, 5-21: 2x4	SP No.1	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	Structural wood sheathing 2-0-0 oc purlins (4-3-8 ma Rigid ceiling directly appli 1 Row at midpt 2 Rows at 1/3 pts 1 Brace at Jt(s): 24, 25, 2	ax.): 4-8. ed or 6-0-0 oc bracing. 2-22, 10-18, 4-22, 12-1(5-21	6
Max H	e) 1=0-3-8, 21=0-3-8, 16=0-3-8 (req. lorz 1=-142(LC 10) Grav 1=1495(LC 2), 21=1835(LC 26), 16	,				
TOP CHORD 1-2= 12-1	Comp./Max. Ten All forces 250 (lb) or -4835/154, 2-4=-1891/71, 8-9=-2244/73, 3=-862/1146, 13-14=-853/902, 4-5=-183	9-10=-2283/23, 10-12=-2	2598/0,			
	=-9/4351, 22-23=-7/4342, 21-22=0/1618 8=0/1493, 14-16=-792/858	, 20-21=0/2177, 18-20=0/	2211,			
WEBS 2-22 2-23	8=0/1493, 14-16=-792/838 =-3007/337, 5-21=-2612/0, 20-24=0/210 =0/2299, 10-18=-259/586, 8-25=0/349, 4 6=-3341/507	, ,	,			
NOTES- 1) Unbalanced roof live	e loads have been considered for this de	sign.				

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 20-6-0, Exterior(2) 20-6-0 to 24-7-8, Interior(1) 24-7-8 to 38-5-15, Exterior(2) 38-5-15 to 42-10-12, Interior(1) 42-10-12 to 62-4-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 7) Ceiling dead load (10.0 psf) on member(s). 9-10, 24-26, 25-26, 9-25; Wall dead load (5.0psf) on member(s). 20-24, 10-18
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 18-20
- 9) WARNING: Required bearing size at joint(s) 16 greater than input bearing size.

10) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify Conticered by paperaging surface.

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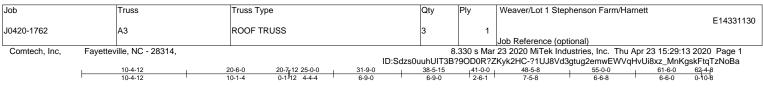


- [Job	Truss	Truss Type	Qty	Ply	Weaver/Lot 1 Stephenson Farm/Harnett
						E14331129
ŀ	J0420-1762	A2A	ROOF TRUSS	1	1	
						Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	330 s Mar	23 2020 MiTek Industries, Inc. Thu Apr 23 15:29:12 2020 Page 2
			ID:Sdzs0uu	hUIT3B?9	OD0R?ZK	yk2HC-Xqxxx9cRvampQUBkgo_bkixTzYZsdHQWe4WJH1zNoBb

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.12) Attic room checked for L/360 deflection.

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Scale = 1:114.4

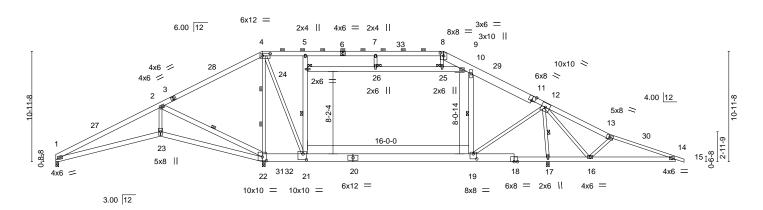


Plate Offsets (K) Quidage Plate Offsets (K) Quidage Plate Offsets (K) LOADING (pd) SPACING- 20-0 CSI. m (bac) (22-0-6)-0.4.7 Plate (Fig. DA). 15 CO 10-0 Vertil. 20-0	L	10-4-12	20-6-0	20-7t12 25-0-0	41-0-0		1	48-10-4	52-10-4	61-6-0	
Charling (gr) TCLL SPACING- 20:0 CSL TC DEFL TC in (loc) Value L/d PLATES GRIP MT20 24/190 Unimber Doll 1.15 BC 0.49 Vert(C1 0.32 19:21 >989 360 BCLL 0.0 Code RE2015TP2014 WB 0.33 Horz(C1) 0.13 17 r/d w/d BCLL 0.0 Code RE2015TP2014 WB 0.33 Horz(C1) 0.13 17 r/d w/d BCLL 0.0 Code RE2015TP2014 Watrix-S Wind(LL) 0.06 1-23 sept 240 Weight-528 Ib FT = 20% LUMBER TOP CHORD 2-60 co putints (5-0 co putints, except Structural wood sheathing directly applied or 6-50 co putints, except 20:0 CHORD 2-22.5 2:10.59 2:400 2:26, 14:18: 2x6 SP 2:400 F 2:0E WEIS X:40 midta Xinta Time availage 2:20 co putints (5-0 -51 conz): -4:30 2:20 co putints (5-0 +50 co putints, except 20:0 CHORD 2:22.5 2:10.59 2:40 1:20 1:7:10:10:10:10:10:10:10:10:10:10:10:10:10:	Plate Offsets (X,Y)					[22:0-5-0.	.0-4-71	7-10-4	4-0-0	8-7-12	I
TCLL 20.0 Plate Grp DOL 1.15 TC 0.71 Vert(CL) 0.32 2999 360 MT20 24/4180 BCLL 0.0 Code IRC2015/TPI2014 WB 0.83 H0r2(CT) 0.13 17 r/a n/a			, ,,,			·					
TCDL 10.0 Lumber DOL 1.15 BC 0.49 Veri(CT) 0.05 1.13 17 n/a n/a BCDL 10.0 Rep Stress Incr VB 0.43 Wind(LL) 0.06 1.23 >999 240 Weight: 528 lb FT = 20% LUMBER- TOP CHORD 2x6 SP No.1 Except TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purlins, except 2.07 CHORD 2x0 SP No.1 Structural wood sheathing directly applied or 4-5-0 oc purlins, except 2x2.1-82.00 × 200 × 20.5 (2.0 × 10.5 × 20.5										ES	
BCLL 0.0 * Rep Stress Incr YES WB 0.83 Horz(CT) 0.1 17 r/s m/a LUMBER- TOP CHORD 2x8 SP No.1 *Except* The Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.06 1.23 >999 240 Weight: 528 ib FT = 20% LUMBER- TOP CHORD 2x8 SP No.1 *Except* The Code IRC2015/TPI2014 The Code IRC2015/TPI2014 The Code IRC2015/TPI2014 Status Interview I									IVI 1 20		244/190
BCDL 10.0 Code IRC2015/TPI2014 Matrix-S Wind(LL) 0.06 1-23 >999 240 Weight: 528 lb FT = 20% LUMBER- TOP CHORD 2x6 SP No.1 * Except* BRACINO- TOP CHORD Structural wood sheathing directly applied or 4-5-0 oc purilins, except 20-22,18-20. 2x10 SP 2x0F 2.0E, 14-18: 2x6 SP 2400F 2.0E BRACINO- TOP CHORD Structural wood sheathing directly applied or 6-0-oc braing. PC -0-0 oc purilins (5-915 max): 4-8. BCD CHORD Structural wood sheathing directly applied or 6-0-oc braing. 20-22,18-20. 2x10 SP 2x0F 2.0E, 14-18: 2x6 SP 2400F 2.0E BOT CHORD Now at midpt 222,25-21,10-19,9-24: 2x6 SP No.1 BOT CHORD Year and picetly applied or 6-0-oc braing. 222,21-24, 10-19, 12-17 VEBS 140-38. (2ze-0-38, 17F-0-3-8 WEBS 140-38. 22e-0-38, 17F-0-3-8 BOT CHORD Year and picetly applied or 6-0-oc braing. 242,21-24,10-19,12-17 242,21-24,10-19,12-17 FORCES. (b) -Max Cora 1+875(LC 24), 22=-2498(12,0), 17F-015(LC 27) Year and picetly applied or 6-0-oc braing. 10 OT CHORD 12-a-2498(0,24-50693, 9-a-17)(19-10-1338(0, 10-12=-14160, 12-13-240194,22-23-419196,21-12-22-24064,21,22-22047,112-12-12-22425,12-12-10-12,112-12-12-22491,12-12-12-2247,12-14-12-12010,5-7-12220,7-8-12260 Year and Year and Year and Year 2-22-23-199246,12-22-23-199244,12-21-202425,12-12-14-24425,12-12-49-242242,12-12-199242,12-22-24-249,14-21-2422425,12-12-19-24-2424242,12-12-19-24-242425,12-12-19-24-24,12-12-124-24-24-24											
TOP CHORD 2x6 SP No.1 "Except" TOP CHORD Structural wood sheathing directly applied or 4-5-0 cc purlins, except BOT CHORD 2x6 SP No.1 Texcept" BOT CHORD Structural wood sheathing directly applied or 6-0-0 cc bracing. BOT CHORD 2x6 SP No.1 'Except" BOT CHORD Rigid celling directly applied or 6-0-0 cc bracing. WEBS 2x4 SP No.2 'Except" JOINTS 1 Brace at Jt(s): 24, 25, 26 REACTONS. (size) 1 = 0-34, 22=0-34, 17=0-38 JOINTS 1 Brace at Jt(s): 24, 25, 26 Max Grav 1 ==876 (LC 24), 22=2493 (LC 2), 17=3015 (LC 27) JOINTS 1 Brace at Jt(s): 24, 25, 26 FORCES. (b) - Max. Ten All forces 250 (b) or less except when shown. JOINTS 1 Brace at Jt(s): 24, 25, 26 FORCES. (b) - Max. Ten All forces 250 (b) or less except when shown. TOP CHORD 1-2a-24010, 24-506/83, 8-9=137011, 9-10=-13360, 10-12a-14160, 10-1									Weigh	t: 528 lb	FT = 20%
BOT CHORD 2x6 SP No.1 "Except" BOT CHORD Rigid ceiling directly applied or 60-00 oc braing. WEBS 2x218-20; ZV1 SD P2400 SP 20E, 14-18; 2x6 SP 2400F 2.0E WEBS 1 Row at midpt 222, 21-24, 10-19, 12-17 WEBS 2x22, 5-21, 10-19, 9-24; 2x6 SP No.1 JOINTS 1 Brace at Jt(s); 24, 25, 26 REACTIONS (size) 1-0-3-8, 22-0-3, 8, 17e-0-3-8 Max Horz: 1142(LC 10) Max Grav 1-875(LC 24), 22-2493(LC 2), 17=3015(LC 27) FORCES. (lb) Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-22-22490, 24-500930, 8-9-1370/11, 9-10=-13360, 10-12=-14160, 12-3-840/1215, 13-14-832976, 4-5e-1201/0, 57e-12220, 7-8e-1226/0 DC CHORD 1-23-2-201490, 24-2-500930, 8-9-1370/11, 9-10=-1426/1170, 1-67-1239/1166, 1-12-2-24160, 22-2-3-19/1466, 2-12-2-2640/44, 4-21-0/2425, 1-22-3-0/1165, 10-19-9-007518, 8-25-0350, 12-16=-441/446, 4-22-2340/44, 4-21-0/2425, 1-24-7-3102/791 NOTES- 1) Unbalanced roof live loads have been considered for this design. 1) Unbalanced roof live loads have been considered for this design. 1) Winch ASCE 7-10, Vult-1300000, 10-12 to 4-6-9, Interior(1) 4-0-10 -20-6-0, Exterior(2) 20-6-0 to 24-9-4, Interior(1) 42-10-2 to 42-9-4, Interior(1) 42-10-2 to 42-9, 42-100/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/2425, 1-24-10/24, 1-10/24, 1-10/24, 1-10/2	TOP CHORD 2x6 SP		1 SP No 1							or 4-5-0 o	c purlins, except
 2-22,5-21,10-19,9-24: 2x6 SP No.1 JOINTS 1 Brace at Jt(s): 24, 25, 26 REACTIONS. (size) 1-0-38, 22=0-38, 17=0-38. Max Horz 1=-142(LC 10) Max Grav 1=675(LC 24), 22=2493(LC 2), 17=3015(LC 27) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-28=22490, 2-48=506998, 9-9=137011, 9-10=133800, 10-12=-141600, 12-13=-8401/215, 13-14-832976, 4-58=-12200, 7-88=122200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-88=1200, 7-1200, 7-1210, 7-98=1400, 7-88=1200, 7-100, 7-1210, 7-98=1400, 7-1210, 7-98=1200, 7-100, 7-1210, 7-98=1200, 7-100, 7-1210, 7-98=1200, 7-100, 7-1210, 7-98=1200, 7-100, 7-1210, 7-98=1200, 7-100, 7-1210, 7-98=1200, 7-100, 7-1210, 7-98=1200, 7-100, 7	BOT CHORD 2x6 SP	No.1 *Except*		SP 2400F 2.0E		D Ri	igid ceiling d	irectly applie	ed or 6-0-0 oc bra		7
Max Horz 1=-142(LC 10) Max Grav 1=875(LC 24), 22=2493(LC 2), 17=3015(LC 27) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1=2=-22490, 2=4=506(33, 8=9=1370)(11, 9=10=13360, 10=12=-1416)0, 1=2=-22490, 2=4=506(33, 8=9=1370)(11, 9=10=13360, 10=12=-1416)0, 1=2=-22190, 2=22=-1992248, 2=12=4=-266(42, 19=21=01226)(1165, 10=19=907/518, 8=25=0/350, 12=16==441/446, 4=22==-230/1165, 10=19=907/518, 8=25=0/350, 12=16==441/446, 4=22==-2340/44, 4=21=0/2425, 12=17=-3102/791 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Winct: ASCE 7=10; Vull=130mph (3=second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15f; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C C Exterior(2) 10=12 0=4-69.0psf; BCDL=6.0psf; h=15f; Cat. II; Exp C; Enclosed; MWFRS for reactions shown; Lumber DOL=160 plate grip DOL=16.0 3) WARNING: This long span truss requires extreme care and experience for proper and safe handing and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restrain/bracing and the permanent Individual truss member restrain/bracing. MTek assumes no responsibility for truss manufacture, handling, erection, or bracing. 4) Provide adequate drainage to prevent water ponding. 5) This truss has been designed for a 10.0 psf bottom chord live load (10.0 psf) and methor drail and as (5.0 psf) on member(s).21-24, 10-19 8) Bottom chord live load (40.0 psf) and admitional bottom chord red ad laad (5.0 psf) on member(s).21-24, 10-19 8) Bottom chord live load (40.0 psf) and admitional bottom chord red ad laad (5.0 psf) on member(s).21-24, 10-19 8) Bottom chord live load (40.0 psf) and admitional bottom chord red ad laad (5.0 psf) on mem			No.1		JOINTS						
 TOP CHORD 1:22249(0, 24-=506/93, 8-9=1370/11, 9-10=-1386(0, 10-12=-1416(0, 12-3=-840/1215, 13-14=-832/976, 4-5=-1201/0, 5-7=-1222/0, 7-8=-1226/1470, 16-17=-1233/1048, 14-16=-861/840 BOT CHORD 1:2-3=-20(1994, 22:-23=-19/1996, 21-22=-26/642, 19-21=0/1221, 17-19=-1226/1170, 16-171233/1186, 14-16=-861/840 WEBS 2:-22=-1992/248, 21:-24=-944/229, 5:-24=-715/241, 12-19=-462/2641, 2-23=0/1165, 10-19=-907/518, 8-25=0/350, 12-16=-441/446, 4-22=-2340/44, 4-21=0/2425, 12-17=-3102/791 NOTES- Uholanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BcDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed: MWFRS (envelope) and C-C Exterior(2) 01-12 to 4-6-9, Interior(1) 4-6-9 to 20-6-0, Exterior(2) 20-6-0 to 24-9-4, Interior(1) 24-9-4 to 38-5-15, Exterior(2) 36-51 to 42-10-12, Interior(1) 42-10-12 to 62-4-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DDL=1.60 plate grip DDL=1.60 WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ('BGSI''), jointly produced by SBCA and TPI. The building owner or the owner's satuhorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. This truss has been designed for a 10.0 pst bottom chord level on onconcurrent with any other live loads. This truss has been designed for a vite load o3.0.0pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 1.0.0pst. 	Max Ho	orz 1=-142(LC 10)		3015(LC 27)							
 10-19=-907/518, 8-25=0/350, 12-16=-441/446, 4-22=-2340/44, 4-21=0/2425, 12-17=-3102/791 NOTES- Uhablanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 20-6-0, Exterior(2) 20-6-0 to 24-9-4, Interior(1) 24-9-4 to 38-5-15, Exterior(2) 38-5-15 to 42-10-12, Interior(1) 42-10-12 to 62-4-8 zone; cantilever right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. Yns truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord any other members, with BCDL = 10.0psf. Ceiling dead load (10.0 psf) and additional bottom chord dead load (5.0psf) on member(s).21-24, 10-19 Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 	TOP CHORD 1-2=-2 12-13 BOT CHORD 1-23=	2249/0, 2-4=-506/93, 8- 3=-840/1215, 13-14=-83 20/1994, 22-23=-19/19	9=-1370/11, 9-1 2/976, 4-5=-120 986, 21-22=-26/6	0=-1336/0, 10-12=-1416 1/0, 5-7=-1222/0, 7-8=-1	6/0, 226/0						
 Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 20-6-0, Exterior(2) 20-6-0 to 24-9-4, Interior(1) 24-9-4 to 38-5-15, Exterior(2) 38-5-15 to 42-10-12, Interior(1) 42-10-12 to 62-4-8 zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MITek assumes no responsibility for truss manufacture, handling, erection, or bracing. Provide adequate drainage to prevent water ponding. This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. Ceiling dead load (10.0 psf) on member(s). 9-10, 24-26, 25-26, 9-25; Wall dead load (5.0psf) on member(s).21-24, 10-19 Boatom chord live load 40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21 Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 	10-19	=-907/518, 8-25=0/350,									
To aphical partial reprocentation account application of the orientation of the partial along the top analytic top and/or bottom chord.	 Unbalanced roof live Wind: ASCE 7-10; V/ MWFRS (envelope) 3 38-5-15, Exterior(2) 3 & MWFRS for reacting 3) WARNING: This lon handling and erection Trusses ("BCSI"), joi qualified registered d permanent individual bracing. Provide adequate drr 5) This truss has been (6) * This truss has been will fit between the bo 7) Ceiling dead load (10 8) Boatom chord live loa 9) Bearing at joint(s) 1 of capacity of bearing s 	ult=130mph (3-second g and C-C Exterior(2) 0-1- 38-5-15 to 42-10-12, Intr ons shown; Lumber DOI g span truss requires ey n guidance, see Guide t intly produced by SBCA Jesign professional for th I truss member restraint ainage to prevent water designed for a 10.0 psf n designed for a live load ottom chord and any oth 0.0 psf) on member(s). S ad (40.0 psf) and additio considers parallel to gra surface.	gust) Vasd=103i -12 to 4-6-9, Inte erior(1) 42-10-1; L=1.60 plate grig ktreme care and o Good Practice and TPI. The bu- ne design and in /bracing. MiTek ponding. bottom chord liv d of 30.0psf on t er members, wi 9-10, 24-26, 25-; nal bottom chorn in value using A	mph; TCDL=6.0psf; BCC rrior(1) 4-6-9 to 20-6-0, f 2 to 62-4-8 zone; cantile o DOL=1.60 experience for proper a i for Handling, Installing uilding owner or the own spection of the tempora assumes no responsibi e load nonconcurrent wi he bottom chord in all ar th BCDL = 10.0psf. 26, 9-25; Wall dead load d dead load (10.0 psf) ar NSI/TPI 1 angle to grain	Exterior(2) 20-6-0 to ver right exposed ; Ind safe handling at & Bracing of Metal er's authorized age ry installation restra lity for truss manufa th any other live loa reas where a rectar d (5.0psf) on memb pplied only to room i formula. Building	24-9-4, İ C-C for me Id erectior Plate Con nt shall cc int/bracin; cture, har ds. gle 3-6-0 er(s).21-2 19-21 designer s	Interior(1) 24 embers and n. For genera- nected Woo portract with a g and the ndling, erecti tall by 2-0-0 tal, 10-19 should verify	-9-4 to forces al d a on, or wide	MARVIN	1667 NGINS	

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

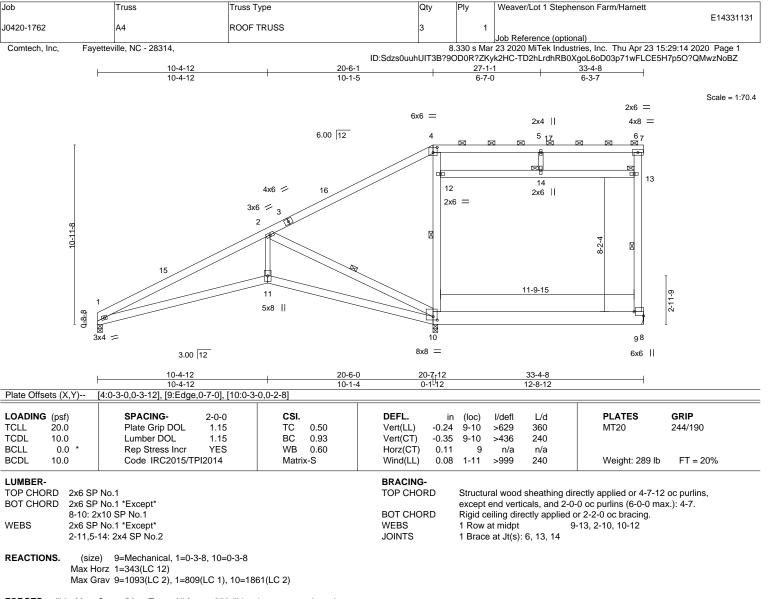


	ob	Truss	Truss Type	Qty	Ply	Weaver/Lot 1 Stephenson Farm/Harnett
						E14331130
,	0420-1762	A3	ROOF TRUSS	3	1	
						Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	330 s Mar	23 2020 MiTek Industries, Inc. Thu Apr 23 15:29:13 2020 Page 2
			ID:Sdzs00	uhUIT3B?	90D0R?Z	Kyk2HC-?1UJ8Vd3gtug2emwEWVqHvUi8xz_MnKgskFtqTzNoBa

11) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-1961/301, 2-4=-333/203, 9-13=-523/107, 6-13=-371/100

BOT CHORD 1-11=-935/1730, 10-11=-933/1720

WEBS 2-11=-328/1031, 2-10=-1810/826, 10-12=-836/436, 4-12=-589/425

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 20-6-1, Exterior(2) 20-6-1 to 26-8-11, Interior(1) 26-8-11

to 33-4-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 12-14, 13-14; Wall dead load (5.0psf) on member(s).10-12

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 9-10

7) Refer to girder(s) for truss to truss connections.

8) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

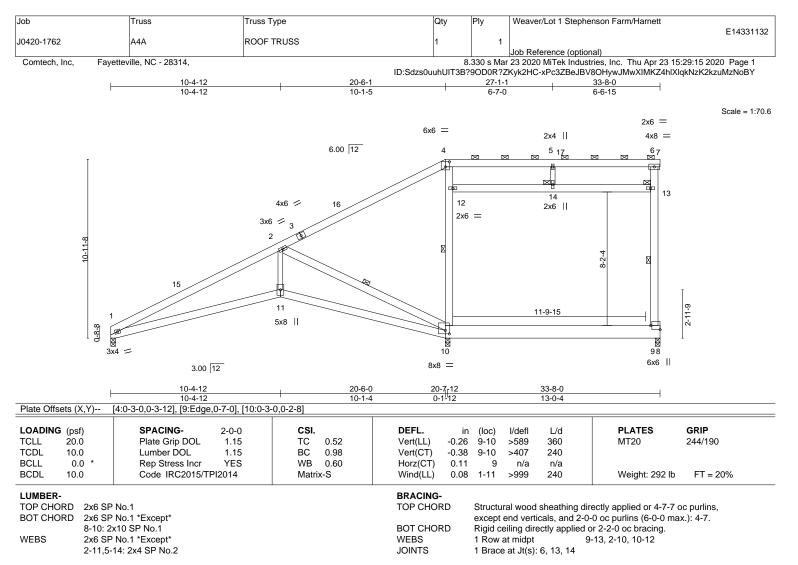
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) Attic room checked for L/360 deflection.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

A MiTek Af 818 Soundside Road Edenton, NC 27932



REACTIONS. (size) 9=0-3-8, 1=0-3-8, 10=0-3-8 Max Horz 1=343(LC 12) Max Grav 9=1120(LC 2), 1=811(LC 1), 10=1883(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-1965/303, 2-4=-332/200, 9-13=-536/110, 6-13=-382/106

- BOT CHORD 1-11=-936/1733, 10-11=-935/1723
- WEBS 2-11=-329/1032, 2-10=-1808/827, 10-12=-846/437, 4-12=-594/425

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 20-6-1, Exterior(2) 20-6-1 to 26-8-11, Interior(1) 26-8-11

to 33-8-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

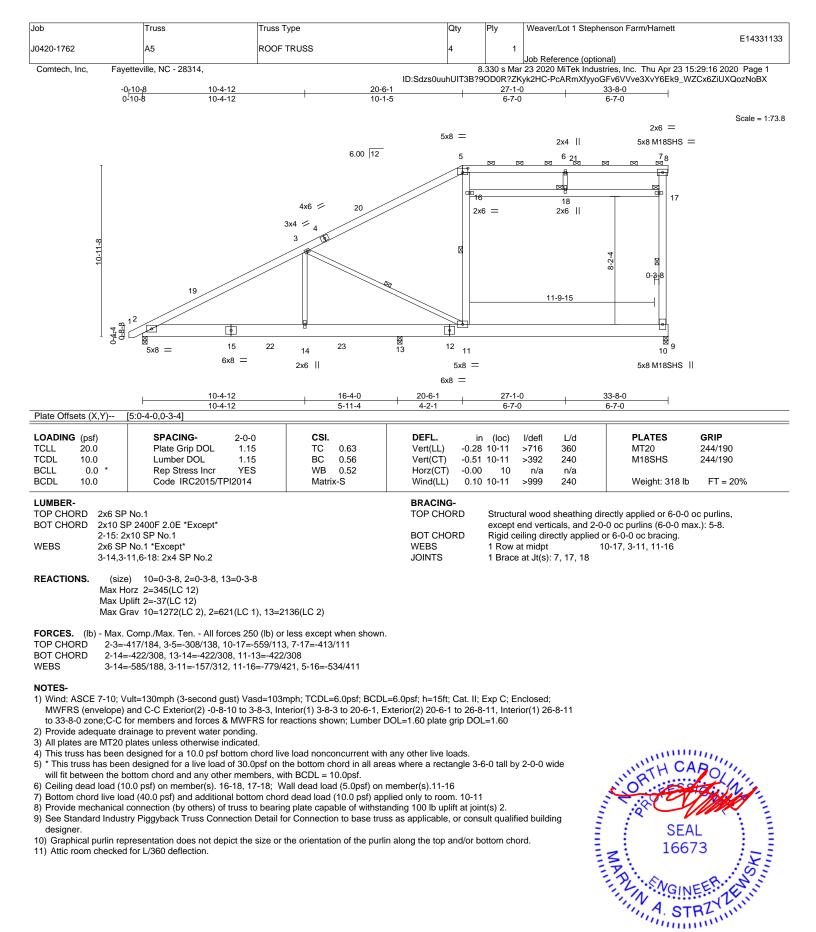
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Ceiling dead load (10.0 psf) on member(s). 12-14, 13-14; Wall dead load (5.0psf) on member(s).10-12
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 9-10
- 7) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 9) Attic room checked for L/360 deflection.



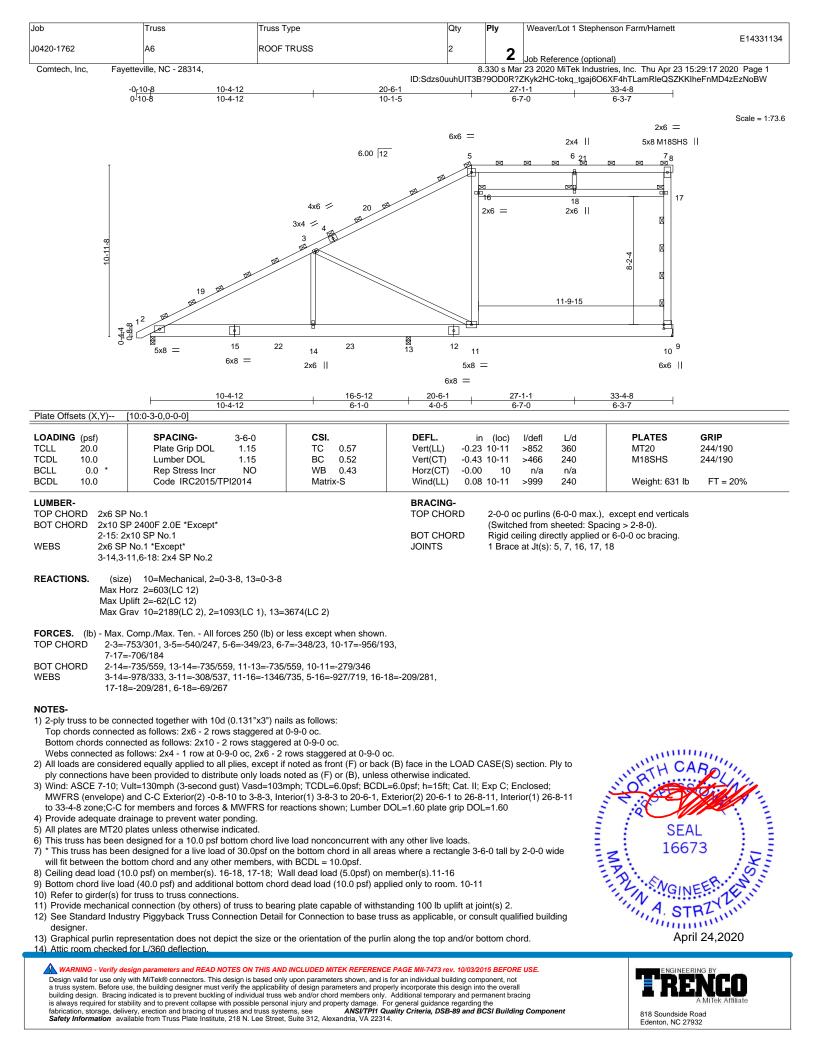
🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

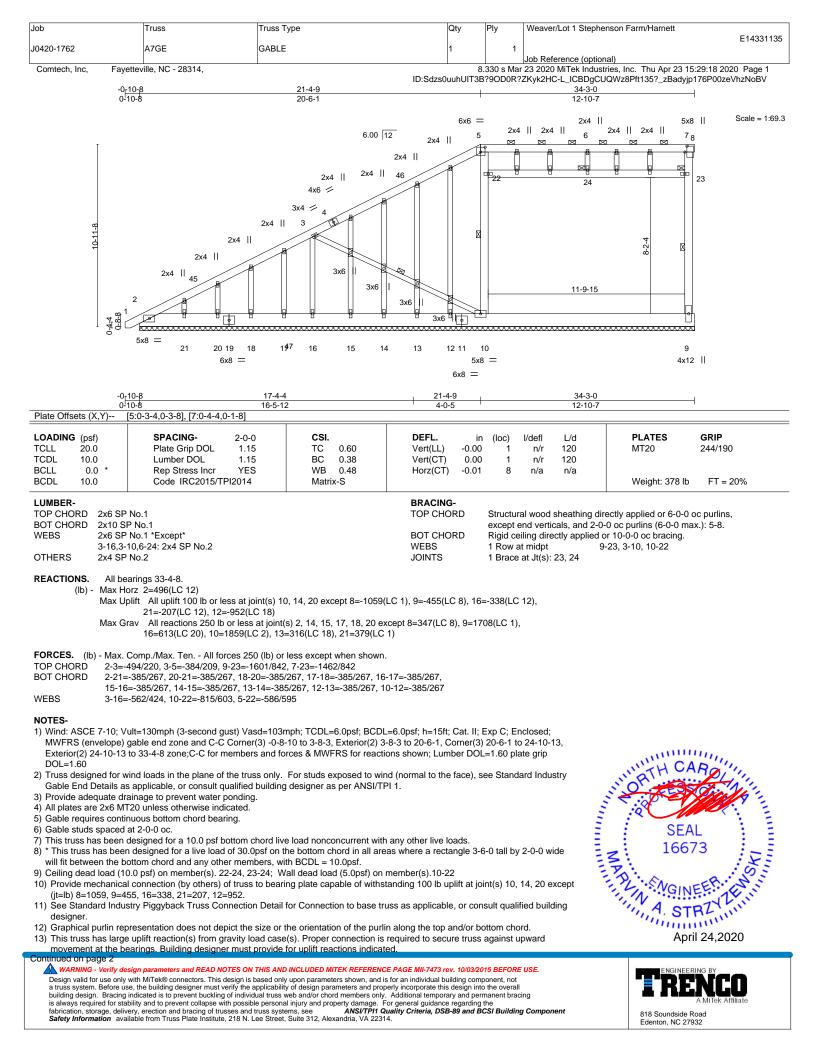
818 Soundside Road Edenton, NC 27932



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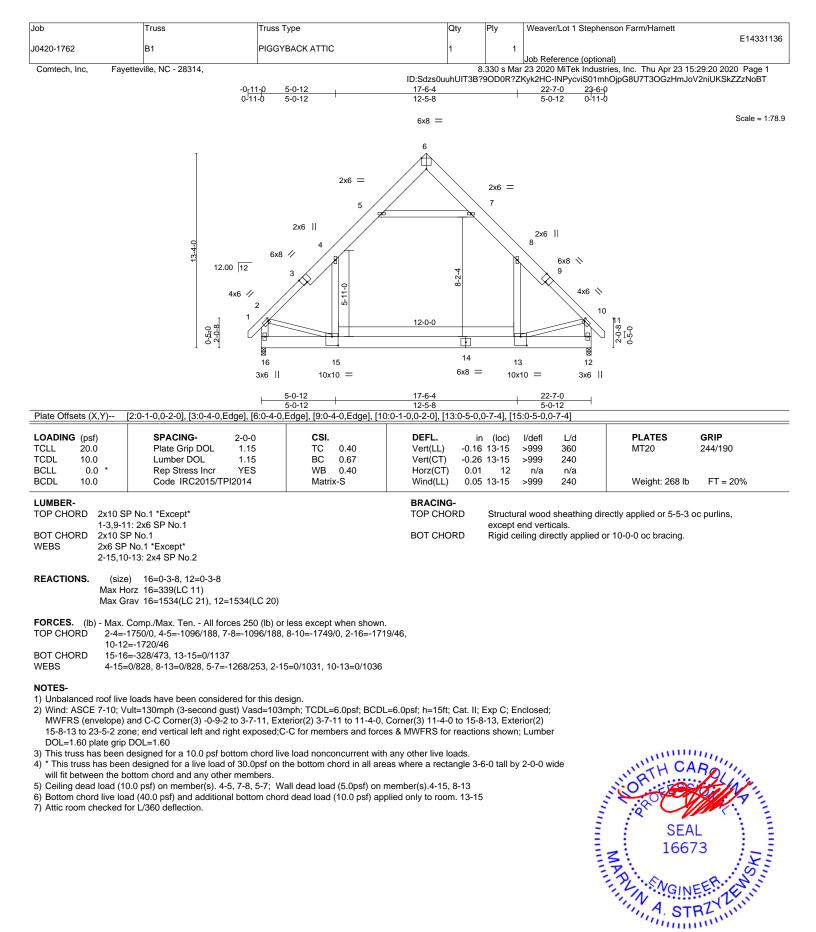


Ī	ob	Truss	Truss Type	Qty	Ply	Weaver/Lot 1 Stephenson Farm/Harnett
						E14331135
	0420-1762	A7GE	GABLE	1	1	
						Job Reference (optional)
	Comtech, Inc, Fayettev	ille, NC - 28314,		8	330 s Mar	23 2020 MiTek Industries, Inc. Thu Apr 23 15:29:19 2020 Page 2
			ID:Sdzs0uu	hUIT3B?9	ODOR?ZK	yk2HC-qAsaPZhqFjeqmZE4bmcEWAkIMM22maLYFgiB17zNoBU

14) Attic room checked for L/360 deflection.

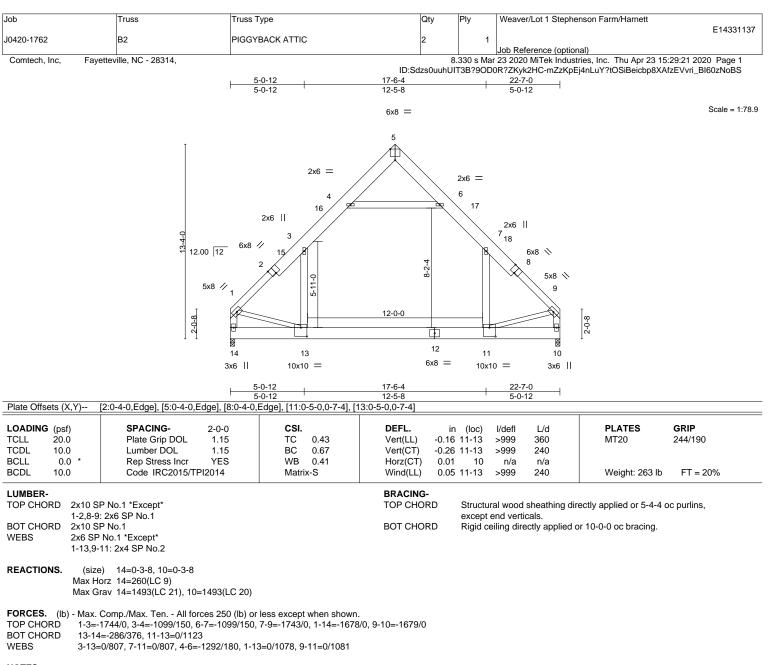
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property dmage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-4 to 4-8-1, Interior(1) 4-8-1 to 11-4-0, Exterior(2) 11-4-0 to 15-8-13, Interior(1) 15-8-13 to 22-4-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s).3-13, 7-11

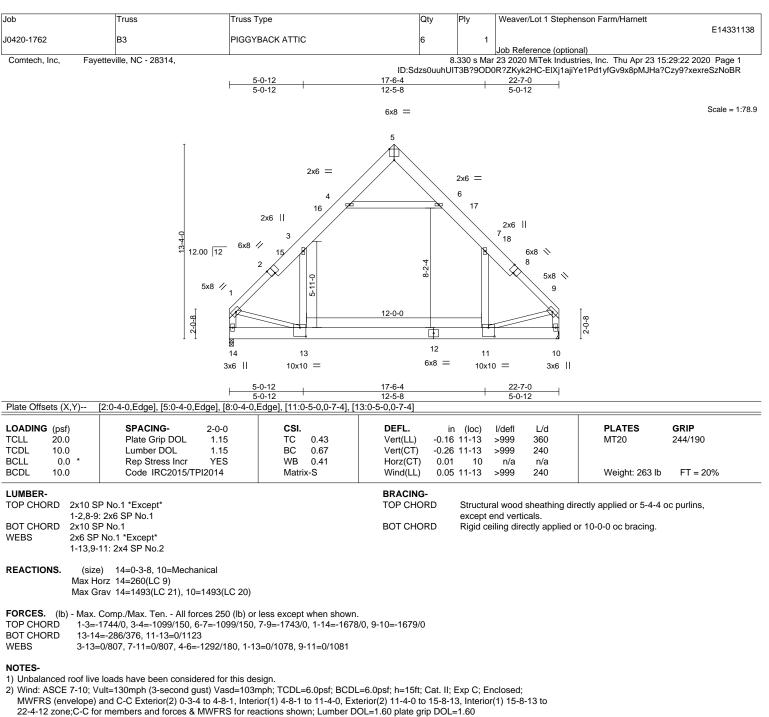
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

7) Attic room checked for L/360 deflection.



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This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s).3-13, 7-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

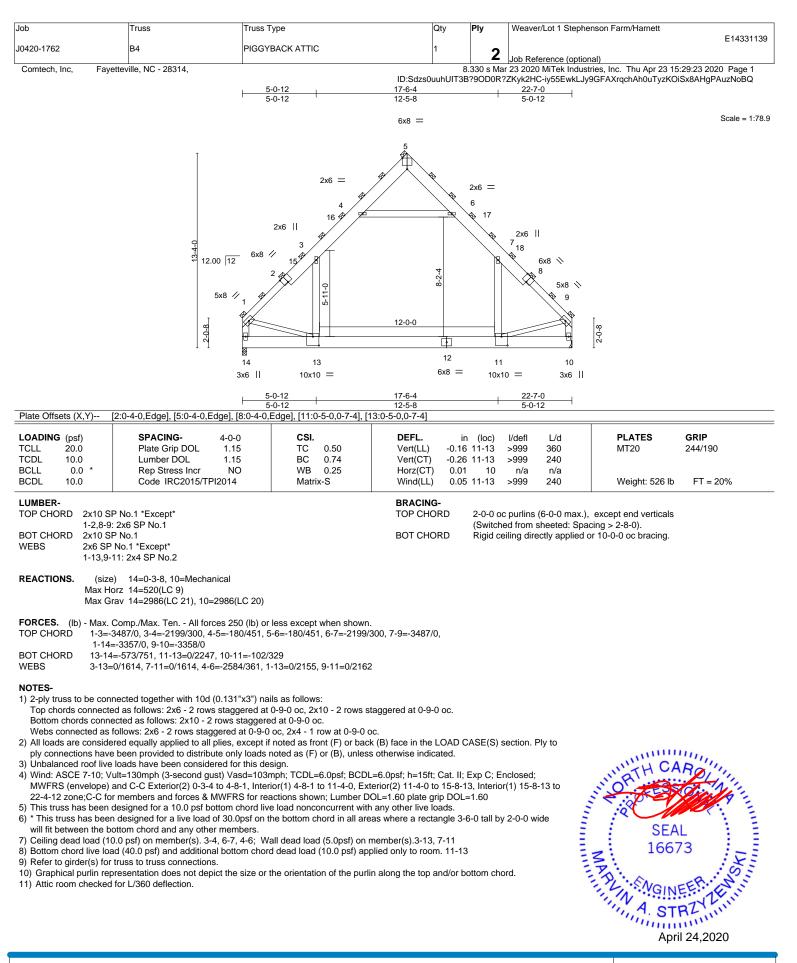
7) Refer to girder(s) for truss to truss connections.

8) Attic room checked for L/360 deflection.



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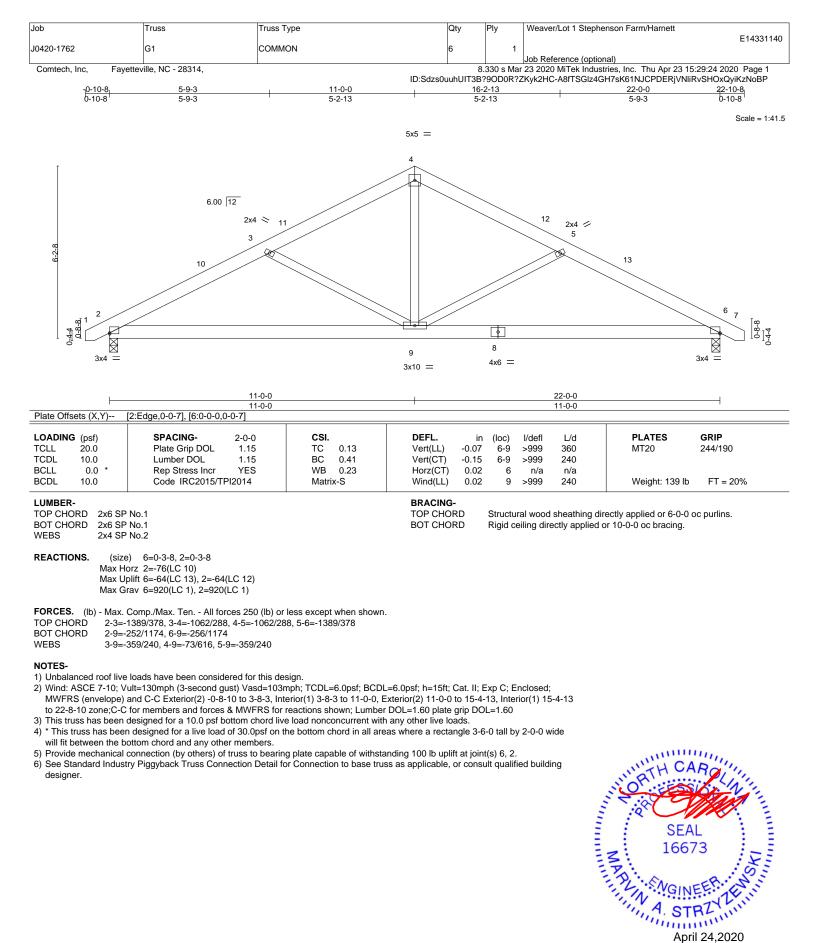




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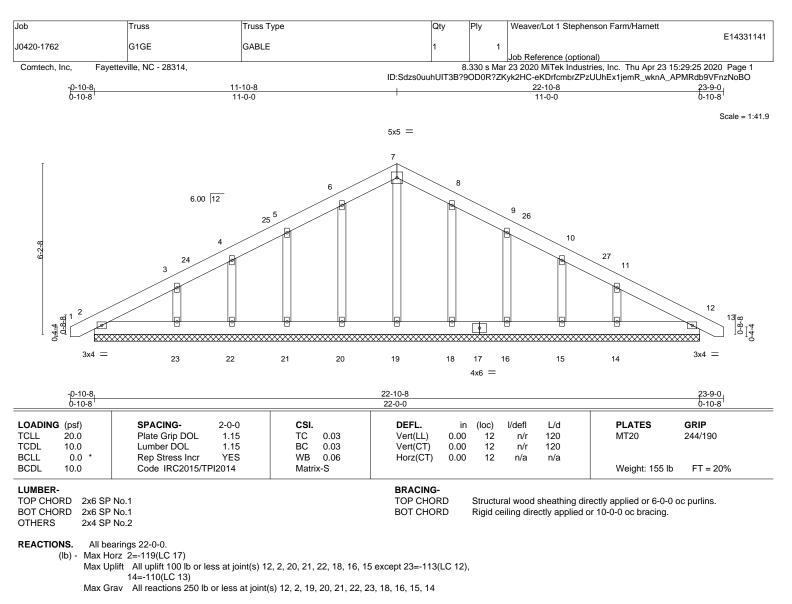
RENC

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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

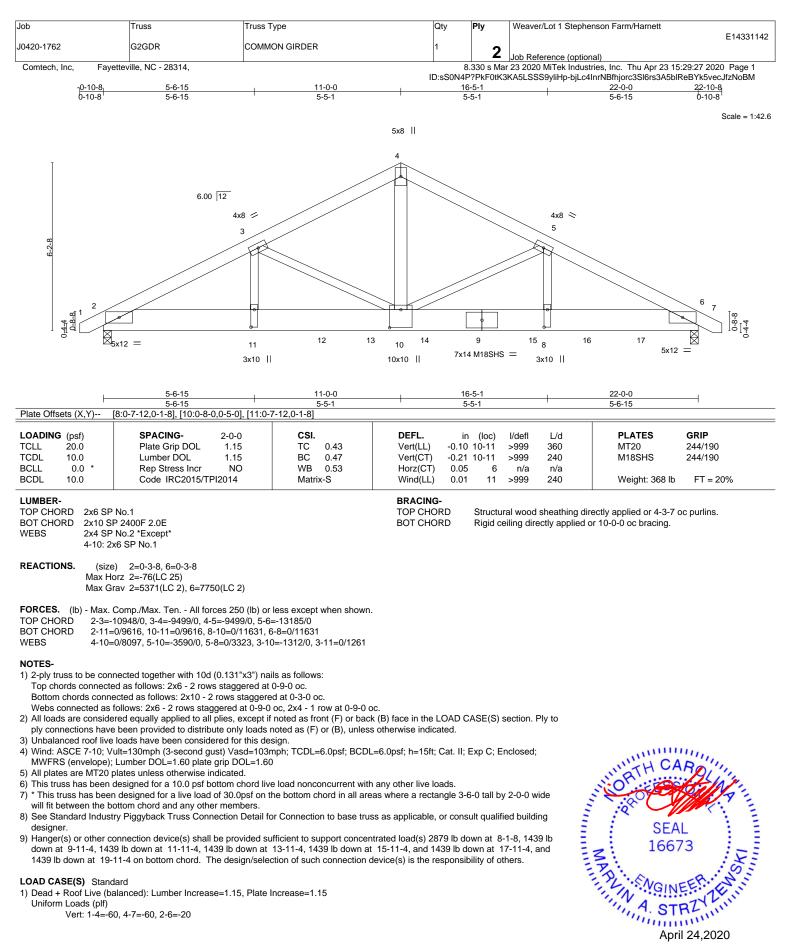
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-8-10 to 3-8-3, Exterior(2) 3-8-3 to 11-0-0, Corner(3) 11-0-0 to 15-4-13, Exterior(2) 15-4-13 to 22-8-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 2, 20, 21, 22, 18, 16, 15 except (jt=lb) 23=113, 14=110.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Continued on page 2

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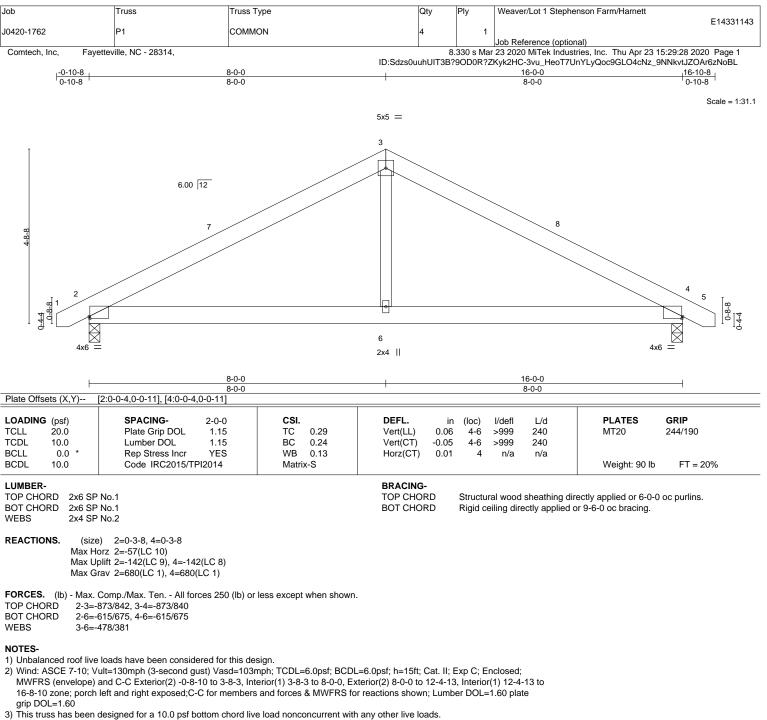
	ob	Truss	Truss Type	Qty	Ply	Weaver/Lot 1 Stephenson Farm/Harnett	
						E	14331142
ŀ	0420-1762	G2GDR	COMMON GIRDER	1	2		
					~	Job Reference (optional)	
-	Comtech, Inc, Fayettev	ille, NC - 28314,		8.	330 s Mar	23 2020 MiTek Industries, Inc. Thu Apr 23 15:29:27 2020 F	Page 2
				ID:sS0N4P	PkF0tK3?	KA5LSSS9yliHp-bjLc4InrNBfhjorc3Sl6rs3A5blReBYk5vecJfz	NoBM

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 9=-1176(F) 12=-2353(F) 13=-1176(F) 14=-1176(F) 15=-1176(F) 16=-1176(F) 17=-1176(F)

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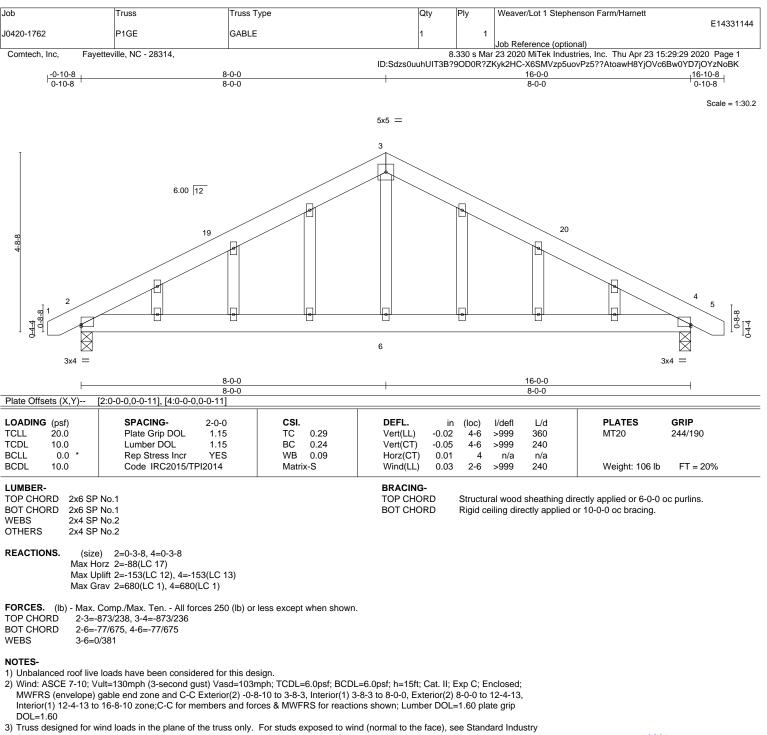
4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=142, 4=142.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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(3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see Standar Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 (4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

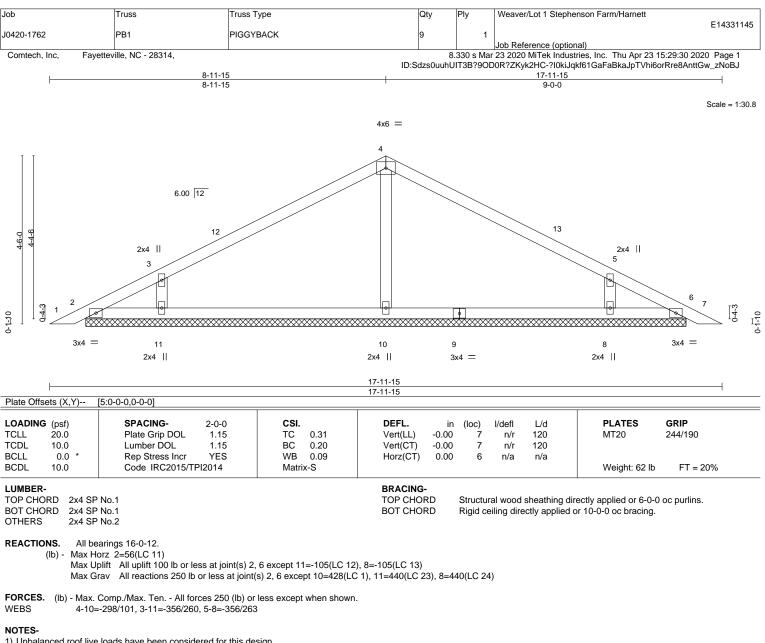
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=153, 4=153.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-15 to 4-8-11, Interior(1) 4-8-11 to 8-11-15, Exterior(2) 8-11-15 to 13-4-12, Interior(1) 13-4-12 to 17-8-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 11=105, 8=105.

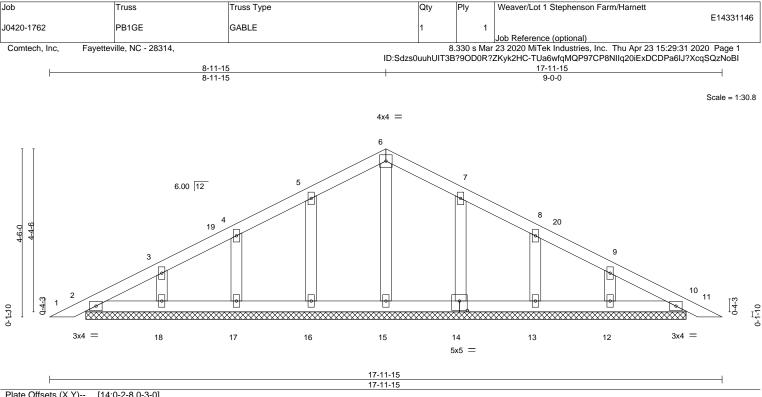
6) Non Standard bearing condition. Review required.

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REPERIENCE PAGE mit-14/3 at 900, 1002/015 BEPORE 052. Design valid for use only with MITeR works connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





CDL 0.0 * Rep Stress Incr YES WB 0.03 Horz(CT) 0.00 10 n/a n/a 3CDL 10.0 Code IRC2015/TPI2014 Matrix-S Matrix-S Weight: 76 lb FT
--

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. All bearings 16-0-13.

(lb) - Max Horz 2=88(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 16, 17, 10, 18, 14, 13, 12

Max Grav All reactions 250 lb or less at joint(s) 2, 15, 16, 17, 10, 18, 14, 13, 12

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-3-15 to 4-11-15, Interior(1) 4-11-15 to 8-11-15, Exterior(2) 8-11-15 to 13-4-12, Interior(1) 13-4-12 to 17-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate ario DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 17, 10, 18,

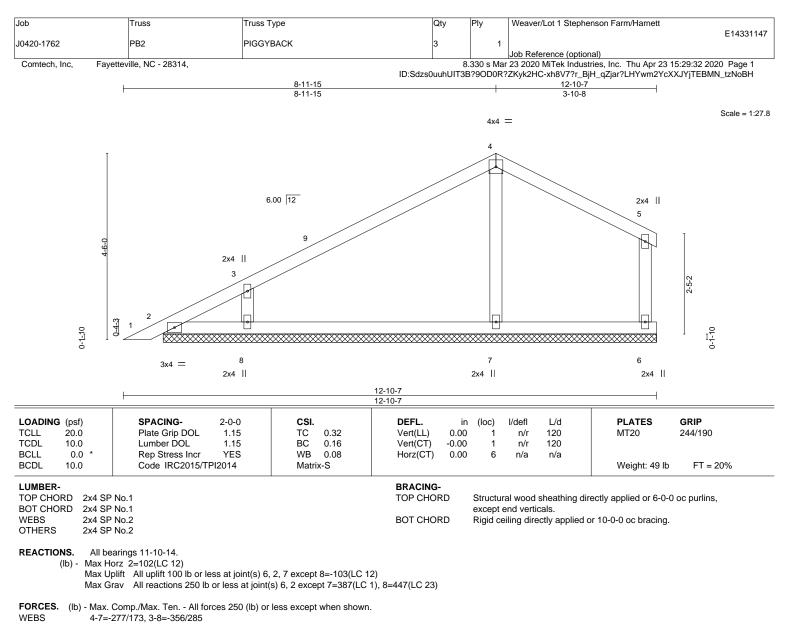
14, 13, 12.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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TREERING BY REALCO A MITEK Affiliate 818 Soundside Road



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-15 to 4-8-11, Interior(1) 4-8-11 to 8-11-15, Exterior(2) 8-11-15 to 12-7-3 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

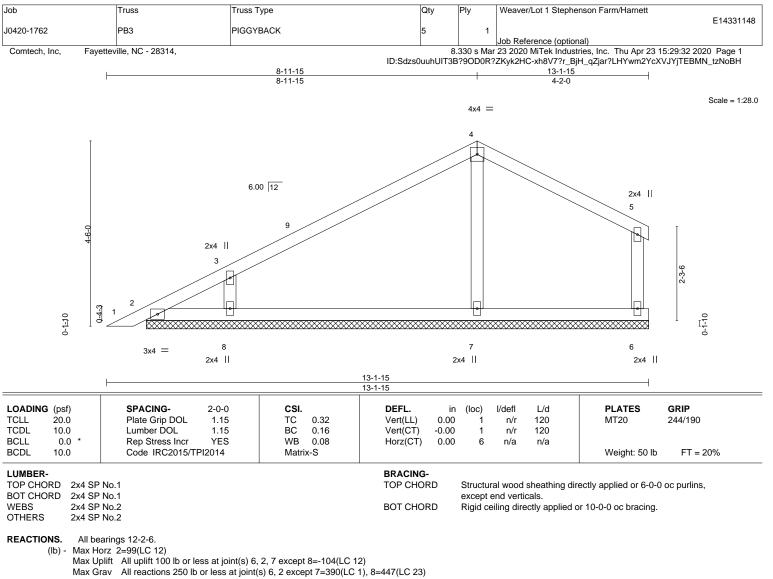
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7 except (jt=lb) 8=103.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing tabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





- ----
- FORCES.
 (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

 WEBS
 4-7=-279/166, 3-8=-356/282

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-15 to 4-8-11, Interior(1) 4-8-11 to 8-11-15, Exterior(2) 8-11-15 to 12-10-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

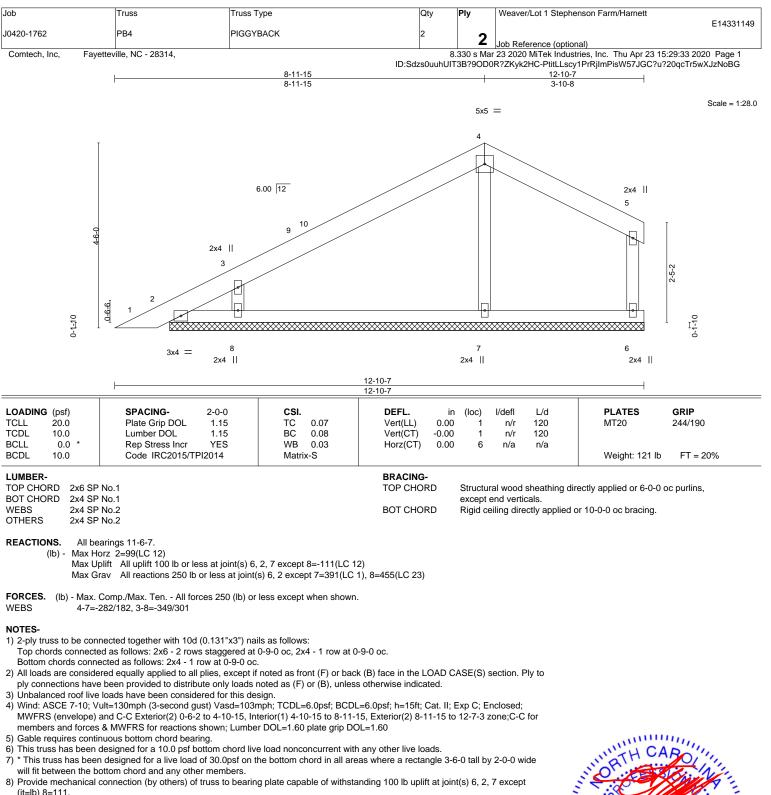
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7 except (it=b) 8=104.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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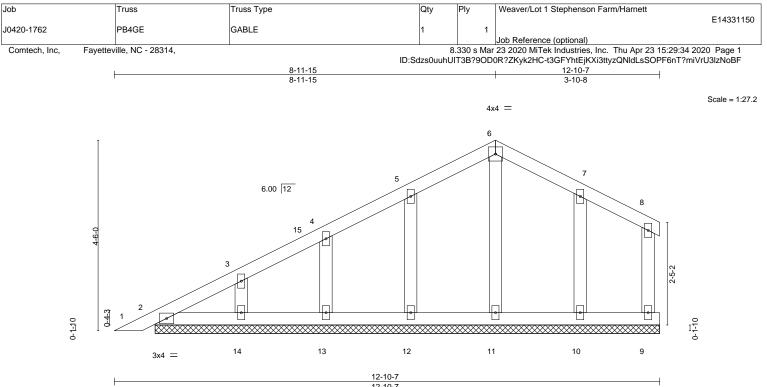


9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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LOADING (psf) ICLL 20.0 ICDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.04 BC 0.02 WB 0.03	Vert(LL) 0.0 Vert(CT) 0.0	0 1	l/defl n/r n/r	L/d 120 120	PLATES MT20	GRIP 244/190
3CLL 0.0 * 3CDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.03 Matrix-S	Horz(CT) 0.0	0 9	n/a	n/a	Weight: 60 lb	FT = 20%
LUMBER-			BRACING- TOP CHORD		ural wood	0	rectly applied or 6-0-0) oc purlins,

BOT CHORD

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.1
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.2
OTHERS	2x4 SP No.2

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-10-14.

(lb) - Max Horz 2=153(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 9, 2, 12, 13, 14, 10

Max Grav All reactions 250 lb or less at joint(s) 9, 2, 11, 12, 13, 14, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

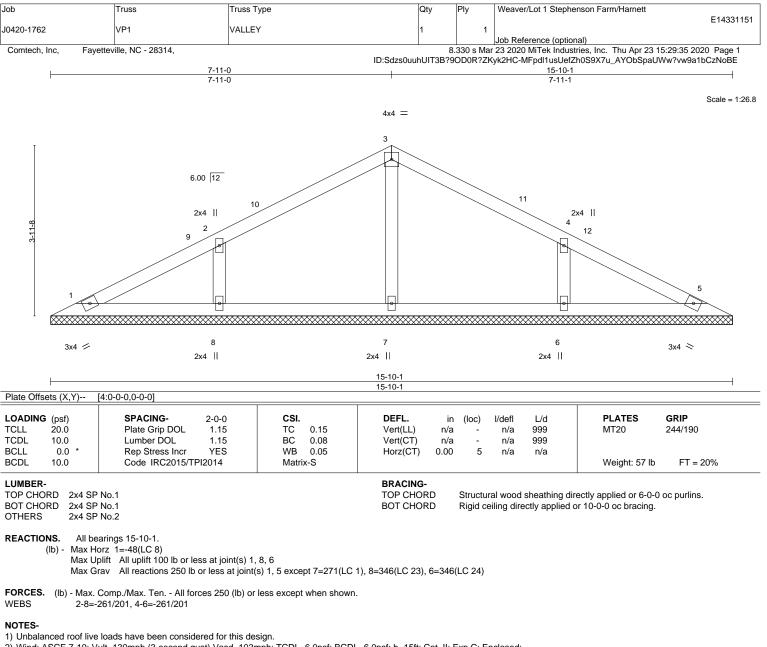
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-3-15 to 4-11-15, Interior(1) 4-11-15 to 8-11-15, Exterior(2) 8-11-15 to 12-7-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 2, 12, 13, 14, 10.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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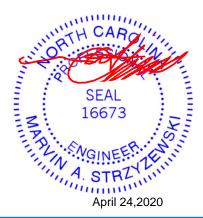


- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-7 to 5-0-3, Interior(1) 5-0-3 to 7-11-0, Exterior(2) 7-11-0 to 12-3-13, Interior(1) 12-3-13 to
- 15-2-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

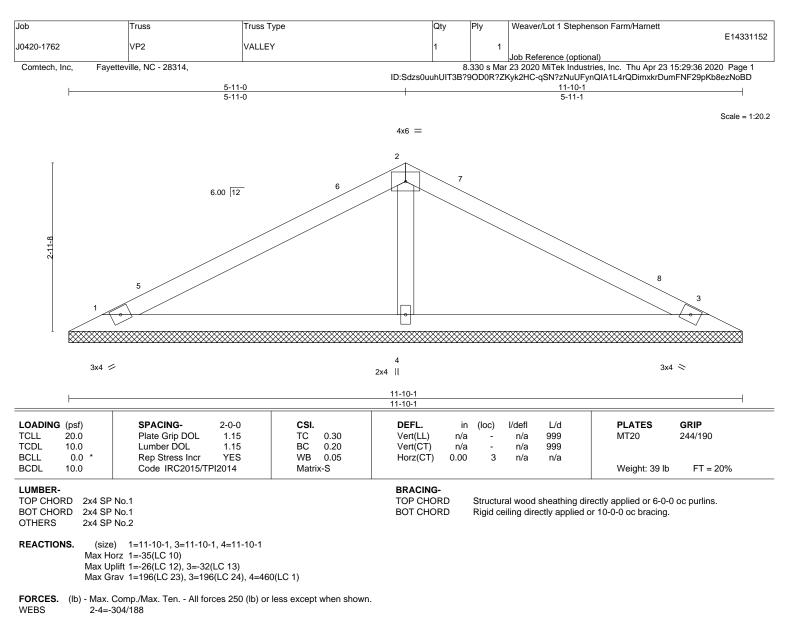
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.

6) Non Standard bearing condition. Review required.



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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-7-7 to 5-0-3, Interior(1) 5-0-3 to 5-11-0, Exterior(2) 5-11-0 to 10-3-13, Interior(1) 10-3-13 to 11-2-10 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

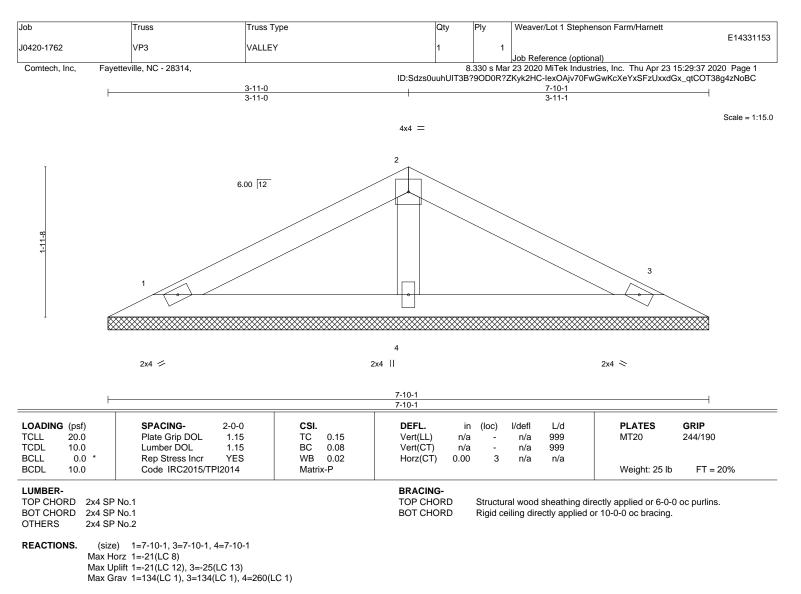
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

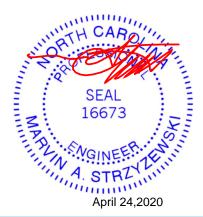
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

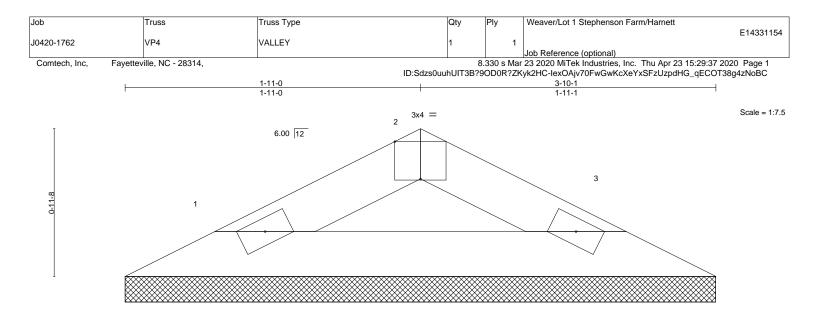
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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2x4 💋

2x4 🛸

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y) [2:0-2-0,Edge]					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL) n/a - n/a 999	MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.06	Vert(CT) n/a - n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 10 lb FT = 20%	

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

REACTIONS. (size) 1=3-10-1, 3=3-10-1 Max Horz 1=-8(LC 8) Max Uplift 1=-6(LC 12), 3=-6(LC 13) Max Grav 1=104(LC 1), 3=104(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

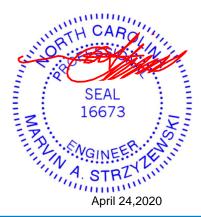
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

6) Non Standard bearing condition. Review required.



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