

Trenco 818 Soundside Rd Edenton, NC 27932

Re: Yarbrough Lamco Custom Homes

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E12973927 thru E12973941

My license renewal date for the state of North Carolina is December 31, 2019.

North Carolina COA: C-0844



April 29,2019

Gilbert, Eric

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



0-0-8		45-8-0							
Plate Offsets (X,Y) [15:0-3-0	,Edge], [37:0-1-12,0-0-0], [38:0-0-0,0-1-	12], [45:0-2-8,0-3-0]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.09 BC 0.03 WB 0.13 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.00 -0.00 0.01 0.00	(loc) 46 46 28 46	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 399 lb	GRIP 244/190 FT = 20%

5x6 =

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.2	TOP CHORD	Structural wood sheathing	directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applie	ed or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3	WEBS	1 Row at midpt	14-42, 13-43, 12-44, 11-45, 16-41, 17-40,
				18-39. 19-37

REACTIONS. All bearings 45-7-0.

(lb) - Max Horz 2=-170(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 28, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30, 2

Max Grav All reactions 250 lb or less at joint(s) 28, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30, 2, 2

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 13-14=-237/269, 16-17=-237/269

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-8-6 to 2-3-10, Exterior(2) 2-3-10 to 22-10-0, Corner(3) 22-10-0 to 25-10-0, Exterior(2) 25-10-0 to 46-4-6 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30, 2.





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DODL	10.0					1
LUMBER-		BF	RACING-			
TOP CHORD	2x6 SP No.2	TC	OP CHORD	Structural wood sheat	hing directly appl	lied.
BOT CHORD	2x4 SP No.1 *Except*	BC	OT CHORD	Rigid ceiling directly a	pplied.	
	16-21: 2x4 SP 2400F 2.0E	W	EBS	1 Row at midpt	7-14, 5-22	
WEBS	2x4 SP No.3					

REACTIONS. (lb/size) 2=1270/0-3-8, 15=1376/0-3-8, 10=1834/0-3-8 Max Horz 2=170(LC 15) Max Grav 2=1440(LC 30), 15=1740(LC 30), 10=2073(LC 2)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-2416/100, 3-5=-2299/164, 5-6=-1556/201, 6-7=-1631/214, 7-9=-2347/191, 9-10=-2529/127

 BOT CHORD
 2-24=0/2172, 22-24=0/1607, 20-22=0/1034, 15-20=-1395/253, 14-15=0/1003, 12-14=0/1533, 10-12=-23/2101, 19-21=-124/2446, 17-19=-123/2446, 16-17=-124/2457

 WEBS
 6-16=-77/455, 14-16=-94/1118, 7-14=-1078/196, 7-12=-32/768, 9-12=-400/145,

21-22=-58/1106, 6-21=-14/665, 5-22=-714/165, 5-24=-34/740, 3-24=-416/146, 15-17=-742/0, 20-21=-2367/115, 15-16=-2353/119

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-6 to 2-3-10, Interior(1) 2-3-10 to 22-10-0, Exterior(2) 22-10-0 to 25-10-0, Interior(1) 25-10-0 to 46-4-6 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 486 lb down and 43 lb up at 27-6-4, and 486 lb down and 43 lb up at 45-8-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Lamco Custom Homes	
					E1297392	
Yarbrough	A2	COMMON	10	1		
					Job Reference (optional)	
Builders FirstSource, A	Ibemarle, NC 28001		8.2	220 s Nov	16 2018 MiTek Industries, Inc. Mon Apr 29 07:12:24 2019 Page 2	
		ID:jJA8O6Mv6xZ6?5VrYW_ggqykJNB-G8mUSvFgnOq73JGeVVtc37xMKxHeZFH_BNx5KVzLwvL				

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-6=-51, 6-37=-51, 10-11=-51, 25-28=-20, 16-21=-20

Concentrated Loads (lb)

Vert: 10=-430 35=-430 Trapezoidal Loads (plf)

Vert: 37=-51-to-10=-93

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April 29,2019



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0- <u>0-8</u> 0-0-8		45-8-0 45-7-8						
Plate Offsets (X,Y) [15:0-3-0	,Edge], [36:0-1-12,0-0-0], [37:0-0-0,0-1-	12], [44:0-2-8,0-3-0]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.03 WB 0.13 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT) Wind(LL)	in (loc) 0.00 45 0.00 45 0.01 28 0.00 45	l/defl >999 : >999 : n/a >999 :	L/d 360 240 n/a 240	PLATES MT20 Weight: 397 lb	GRIP 244/190 FT = 20%

LUMBER-		BRACING-		
TOP CHORD	2x6 SP No.2	TOP CHORD	Structural wood sheathing	directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applie	d or 10-0-0 oc bracing.
OTHERS	2x4 SP No.3	WEBS	1 Row at midpt	14-41, 13-42, 12-43, 11-44, 16-40, 17-39,
				18-38, 19-36

REACTIONS. All bearings 45-7-8.

(lb) -Max Horz 2=170(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 39, 38, 36, 35, 34, 33. 32. 31. 30. 29. 28 Max Grav

All reactions 250 lb or less at joint(s) 2, 2, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 40, 39, 38, 36, 35, 34, 33, 32, 31, 30, 29, 28

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 13-14=-235/264, 16-17=-235/264

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner(3) -0-8-6 to 2-3-10, Exterior(2) 2-3-10 to 22-10-0, Corner(3) 22-10-0 to 25-10-0, Exterior(2) 25-10-0 to 45-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 39, 38, 36, 35, 34, 33, 32, 31, 30, 29, 28.



818 Soundside Road Edenton, NC 27932

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April 29,2019



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0-Q-8	9-11-10	11-9-4	19-9-14	1	29-0-4	29-9-8	
0-0-8	9-11-2	1-9-10	8-0-10		9-2-6	0-9-4	
Plate Offsets (X,Y) [7:0-6-0,	0-0-6], [13:0-2-0,0-0-4], [15:0-4-8,0-2-8	3], [18:0-2-0,0-0-4]					
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.57 BC 0.41 WB 0.38 Matrix-AS	DEFL. Vert(LL) -C Vert(CT) -C Horz(CT) C Wind(LL) C	in (loc) 0.07 18-33 0.17 18-33 0.01 13 0.04 18-33	l/defl L/d >999 360 >844 240 n/a n/a >999 240	PLATES MT20 Weight: 228 lb	GRIP 244/190 FT = 20%
		-	PACING				

LUMBER-		BRACING-		
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood shea	thing directly applied.
BOT CHORD	2x6 SP No.2	BOT CHORD	Rigid ceiling directly a	applied.
WEBS	2x4 SP No.2	WEBS	1 Row at midpt	4-13
OTHERS	2x4 SP No 3			

REACTIONS. All bearings 17-3-0 except (jt=length) 1=0-3-8.

(lb) - Max Horz 1=-113(LC 14)

- Max Uplift All uplift 100 lb or less at joint(s) 13, 16, 17, 9 except 10=-131(LC 35) Max Grav All reactions 250 lb or less at joint(s) 16, 17, 17, 14, 12, 11, 10 except 1=669(LC 34), 13=1133(LC 2), 9=443(LC 35)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 1-2=-895/80, 2-4=-673/115, 4-6=0/409
- BOT CHORD 1-18=-8/714
- WEBS 4-13=-895/69, 6-13=-416/150, 4-18=-1/554, 2-18=-439/148

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0 to 3-0-0, Interior(1) 3-0-0 to 14-10-12, Exterior(2) 14-10-12 to 17-10-12, Interior(1) 17-10-12 to 30-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 16, 17, 9 except (jt=lb) 10=131.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

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- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 13, 9, 8.



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5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 13, 10, 8.



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8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 6.



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REACTIONS. All bearings 14-8-9.

(Ib) - Max Horz 1=-49(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 8, 6 Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=276(LC 2), 8=324(LC 33), 6=324(LC 34)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) and C-C Exterior(2) 0-7-0 to 3-4-12, Interior(1) 3-4-12 to 7-4-12, Exterior(2) 7-4-12 to 10-4-12, Interior(1) 10-4-12 to 14-2-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.



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TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10



- MWFRS (envelope) and C-C Exterior(2) 0-7-0 to 3-7-0, Interior(1) 3-7-0 to 5-4-12, Exterior(2) 5-4-12 to 8-4-12, Interior(1) 8-4-12 to 10-2-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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Max Horz 1=-20(LC 12)

Max Grav 1=225(LC 2), 3=225(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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