

RE: J0320-1197 Lot 15 Blackberry Manor Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0320-1197 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.1 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 19 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	E13912794	a1	4/7/2020
2	E13912795	a1ge	4/7/2020
3	E13912796	a2	4/7/2020
4	E13912797	a3	4/7/2020
5	E13912798	a3ge	4/7/2020
6	E13912799	b1	4/7/2020
7	E13912800	b1sg	4/7/2020
8	E13912801	b2	4/7/2020
9	E13912802	b3	4/7/2020
10	E13912803	b4	4/7/2020
11	E13912804	b5	4/7/2020
12	E13912805	b6	4/7/2020
13	E13912806	c1ge	4/7/2020
14	E13912807	m1	4/7/2020
15	E13912808	m1ge	4/7/2020
16	E13912809	m2	4/7/2020
17	E13912810	m2ge	4/7/2020
18	E13912811	pb	4/7/2020
19	E13912812	pbge	4/7/2020

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2020

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



	8-2-12	14-8-4	21-5-8	28-6-4	29-1	I-12 35-	7-4	43-10-0	
Plate Offsets (X,Y)	[5:0-4-0,0-3-12], [6:0-5-4,0-	2-12], [8:0-5-4,0	0-9-4 0-2-12], [9:0-4-0,0-3-12]	, [15:0-4-0,0-3-4],	[22:0-2-	-0,0-3-0], [2	23:0-2-0,0-3-0]	0-2-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPl2	2-0-0 1.15 1.15 YES 2014	CSI. TC 0.37 BC 0.84 WB 0.65 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.31 2 -0.56 2 0.05 0.25	(loc) l/de 0-21 >81 0-21 >45 12 n 21 >99	efl L/d 18 360 54 240 /a n/a 39 240	PLATES MT20 Weight: 462 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x10 S 15-20: WEBS 2x4 SP 5-20,9-	No.1 P No.1 *Except* 2x8 SP No.1 No.3 *Except* 15: 2x6 SP No.1, 22-23: 2x	8 SP No.1	I	BRACING- TOP CHORI BOT CHORI JOINTS	D S 2 D F 8 8 1	Structural w 2-0-0 oc pur Rigid ceiling 3-1-14 oc br 3-6-8 oc bra Brace at J	ood sheathing dii rlins (6-0-0 max.): i directly applied o racing: 2-21 icing: 20-21. it(s): 24, 25, 26	rectly applied or 4-2-0 c : 6-8. or 10-0-0 oc bracing, I	oc purlins, except Except:
REACTIONS. All be (lb) - Max H Max U Max G	earings 0-3-8. orz 2=272(LC 11) plift All uplift 100 lb or less rav All reactions 250 lb or 16=678(LC 25)	s at joint(s) 2 exc less at joint(s) e	ept 16=-356(LC 8) except 2=2229(LC 20),	12=2093(LC 20),	18=103	8(LC 18),			
FORCES. (lb) - Max. TOP CHORD 2-4=- 8-9=- BOT CHORD 2-21=	Comp./Max. Ten All force 2880/257, 4-5=-2819/467, 3 1520/356, 9-10=-2854/494, 69/2413, 20-21=-72/2417,	es 250 (lb) or les 5-6=-1834/368, 6 10-12=-3028/29 18-20=-85/2463	s except when shown. 6-7=-1283/215, 7-8=-12 93 3, 16-18=-85/2463, 15-	283/215, 16=-85/2463,					
14-15 WEBS 20-22 24-25 8-24=	5=-72/2397, 12-14=-68/239 2=-91/889, 5-22=0/419, 15- 5=-794/234, 24-26=-1174/2 567/355, 6-25=-170/952, 4	5 23=-157/926, 9-2 47, 23-26=-1227 3-26=-150/809, 4	23=-221/482, 22-25=-8 7/252, 6-24=-956/181, 7 4-21=-414/295, 10-14=	58/241, 7-24=-324/196, -388/291					
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) 33-10-11 to 44-7-1 z 3) Provide adequate dr 4) This truss has been between the bottom 6) Ceiling dead load (11 7) Bottom chord live loa 8) Provide mechanical 16=356. 9) See Standard Indust designer. 10) Graphical purlin rer	e loads have been consider 'ult=130mph (3-second gus and C-C Exterior(2) -0-9-1 ione;C-C for members and designed for a 10.0 psf bot n designed for a 10.0 psf bot n designed for a live load of chord and any other memb 0.0 psf) on member(s). 22-2 ad (40.0 psf) and additional connection (by others) of tr try Piggyback Truss Conne- presentation does not depic	ed for this design t) Vasd=103mpf to 3-7-12, Interic forces & MWFR: nding. tom chord live lo 30.0psf on the l ers, with BCDL = 25, 24-25, 24-26 bottom chord de uss to bearing pl ction Detail for C t the size or the	n. n; TCDL=6.0psf; BCDL= r(1) 3-7-12 to 16-2-0, E S for reactions shown; ad nonconcurrent with bottom chord in all area = 10.0psf. , 23-26; Wall dead loa ad load (10.0 psf) app late capable of withstar connection to base trus orientation of the purlir	=6.0psf; h=15ft; C Exterior(2) 16-2-0 Lumber DOL=1.6(any other live load as with a clearance d (5.0psf) on mem lied only to room. nding 100 lb uplift s as applicable, or a along the top and	at. II; Ex to 33-10) plate g ds. e greate ber(s).2 18-20, 1 at joint(s consult d/or bott	p C; enclos I-11, Interio Irip DOL=1. r than 6-0-(0-22, 15-2: 6-18, 15-11 s) 2 except c qualified b om chord.	sed; r(1) 60 3 6 (jt=lb) uilding	UN OF OF OF	CARO SSIC 14 SEAL 36322

11) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

January 2,2020

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TRENGINE BY A MITEK Atfiliate 818 Soundside Road

Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Lot 15 Blackberry Manor	
						E13912795
J0320-1197	A1GE	GABLE	1	1		
					Job Reference (optional)	
Comtech, Inc., Fayette	/ille, NC 28309		8	.130 s Ma	11 2018 MiTek Industries, Inc. Thu Jan 2 07:31:39 2020	0 Page 2
		ID:Khil	M10Q 9dy	0362zxkF	fqAyJzV8- ?cKXsAtY?AsihSsXOvH5FCcywqpHx4?qR8v	OPzzT?2

NOTES-

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Ceiling dead load (10.0 psf) on member(s). 50-54, 53-54, 52-53, 52-57, 57-58, 51-58; Wall dead load (5.0psf) on member(s).42-50, 37-51
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 40-42, 38-40, 37-38
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 251 lb uplift at joint 28 and 470 lb uplift at joint 38.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

14) Attic room checked for L/360 deflection.

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BRACING-

TOP CHORD

BOT CHORD

WEBS

TOP CHORD	2-3=-748/0
BOT CHORD	2-9=-225/618, 8-9=-225/618

2x6 SP No.1

2x6 SP No.1

2x4 SP No.3

(lb/size) 2=668/0-3-8, 7=592/0-3-8

Max Grav 2=668(LC 1), 7=639(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Max Horz 2=347(LC 12) Max Uplift 7=-178(LC 12)

WEBS 3-9=0/397, 3-8=-787/285

NOTES-

LUMBER-

WEBS

TOP CHORD

BOT CHORD

REACTIONS.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 15-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 7.

5) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

5-8, 3-8

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

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LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.26	Vert(LL) -0.02 2-9 >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.20	Vert(CT) -0.05 2-9 >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.30	Horz(CT) 0.01 8 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.02 2-9 >999 240 Weight: 117 lb FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD2x6 SP No.1BOT CHORD2x6 SP No.1WEBS2x4 SP No.3

REACTIONS. (Ib/size) 8=605/Mechanical, 2=651/0-3-8 Max Horz 2=347(LC 12) Max Uplift 8=-185(LC 12) Max Grav 8=654(LC 19), 2=651(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-708/0

TOP CHORD2-3=-708/0BOT CHORD2-9=-210/581, 8-9=-210/581

WEBS 3-9=0/360, 3-8=-742/268

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 15-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 8.

6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

SEAL 036322 January 2,2020

Structural wood sheathing directly applied or 6-0-0 oc purlins,

5-8, 3-8

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

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<u>16-2-12</u> 15-3-12

SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl	
Plate Grip DOL 1.15	TC 0.05	Vert(LL) -0.00 1 n/r	120 MT20 244/190
Lumber DOL 1.15	BC 0.02	Vert(CT) -0.00 1 n/r	120
Rep Stress Incr YES	WB 0.17	Horz(CT) -0.01 13 n/a	n/a
Code IRC2015/TPI2014	Matrix-S		Weight: 145 lb FT = 20%
	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	Lumber DOL 1.15 BC 0.02 Rep Stress Incr YES WB 0.17 Code IRC2015/TPI2014 Matrix-S	Index Onp DoL 1.15 IC 0.00 Vert(CT) 0.00 1 I/r Lumber DOL 1.15 BC 0.02 Vert(CT) -0.00 1 n/r Rep Stress Incr YES WB 0.17 Horz(CT) -0.01 13 n/a Code IRC2015/TPI2014 Matrix-S

BOT CHORD

T-Brace:

WEBS

3x4 ||

except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 11-13.

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.

2x4 SPF No.2 - 12-14, 10-15

Rigid ceiling directly applied or 10-0-0 oc bracing.

Brace must cover 90% of web length.

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.3
OTHERS	2x4 SP No 3

REACTIONS. All bearings 15-3-12. (lb) - Max Horz 2=466(LC 1

Max Horz 2=466(LC 12) Max Uplift All uplift 100 lb or less at joint(s) 13, 14, 2, 15, 16, 17, 18, 19, 20 except 21=-181(LC 12)

-0-11-0

0-11-0

Max Grav All reactions 250 lb or less at joint(s) 13, 14, 17, 18, 19, 20, 21 except 2=360(LC 12), 15=252(LC 19), 16=277(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-583/474, 3-4=-457/369, 4-5=-377/304, 5-6=-303/246

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Bearing at joint(s) 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 14, 2, 15, 16, 17, 18, 19, 20 except (jt=lb) 21=181.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-9-1 to 3-7-12, Interior(1) 3-7-12 to 9-3-8, Exterior(2) 9-3-8 to 13-8-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



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818 Soundside Road Edenton, NC 27932

January 2,2020



1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-12 to 4-6-9, Interior(1) 4-6-9 to 9-3-8, Exterior(2) 9-3-8 to 13-8-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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			10-8-11							18-3-8		_
	I		10-8-11			1				7-6-13		
Plate Off	sets (X,Y)	[2:0-1-12,0-2-0], [3:0-3-0	,0-3-7]									
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.50	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
FCDL	10.0	Lumber DOL	1.15	BC	0.28	Vert(CT)	-0.12	7-8	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.01	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	k-S	Wind(LL)	0.02	4-7	>999	240	Weight: 126 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x6 SP No.1		except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 1-3.
WEBS	2x6 SP No.1 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
	2-7: 2x4 SP No.2, 3-7: 2x4 SP No.3		

REACTIONS. (lb/size) 8=739/Mechanical, 4=767/0-3-8 Max Horz 8=-176(LC 13) Max Uplift 8=-87(LC 8), 4=-28(LC 13) Max Grav 8=739(LC 1), 4=767(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-8=-630/280, 2-3=-665/208, 3-4=-929/153

- NOTES-
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 10-8-11, Exterior(2) 10-8-11 to 15-1-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 4.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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BOT CHORD 4-7=0/665 WEBS 2-7=-199/627



∟		12-8-11										
				12-8-11					1		5-6-13	
Plate Offs	sets (X,Y)	[9:0-1-8,0-2-0]										
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.31	Vert(LL)	-0.13	7-9	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.39	Vert(CT)	-0.26	7-9	>836	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.01	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S	Wind(LL)	0.01	5-7	>999	240	Weight: 125 lb	FT = 20%
	L					RRACING						

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x6 SP No.1		except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 1-4.
WEBS	2x4 SP No.3 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
	2-9: 2x6 SP No.1		

REACTIONS. (lb/size) 9=739/Mechanical, 5=767/0-3-8 Max Horz 9=-133(LC 13) Max Uplift 9=-86(LC 8), 5=-26(LC 8) Max Grav 9=739(LC 1), 5=767(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 3-4=-758/200, 4-5=-1038/167

BOT CHORD 7-9=-87/754, 5-7=-20/765 WEBS 3-9=-782/328, 4-7=0/364

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 12-8-11, Exterior(2) 12-8-11 to 17-1-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 5.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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		7-7-13		14-8-11				18-3-8	
		7-7-13	,	7-0-13				3-6-13	
LOADIN TCLL TCDL BCLL BCDL	G (psf) 20.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.19 BC 0.17 WB 0.51 Matrix-S	DEFL0.0 Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0 Wind(LL) 0.0	n (loc) 4 9 8 7-9 1 5 3 9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 123 lb	GRIP 244/190 FT = 20%
LUMBE TOP CH BOT CH WEBS	R- ORD 2x6 SP ORD 2x6 SP 2x4 SP 2-10: 2	No.1 No.1 No.3 *Except* x6 SP No.1		BRACING- TOP CHORD BOT CHORD	Structi except Rigid o	ural wood t end verti ceiling dire	sheathing di cals, and 2-0 ectly applied	rectly applied or 6-0-0 c -0 oc purlins (6-0-0 ma or 10-0-0 oc bracing.	oc purlins, x.): 1-4.
REACTI	ONS. (Ib/size Max H Max U Max G	e) 10=739/Mechanical, 5=767/0-3-8 orz 10=-89(LC 13) plift 10=-86(LC 8), 5=-46(LC 8) rav 10=739(LC 1), 5=767(LC 24)							
TOP CH BOT CH WEBS	ORD 2-10= ORD 7-9=- 2-9=-	Comp./Max. Ten All forces 250 (ib) or 641/231, 2-3=-1308/325, 3-4=-1310/32 117/824, 5-7=-114/832 310/1232, 3-9=-462/237, 4-9=-99/554, 4	less except when shown. 27, 4-5=-1089/243 1-7=0/250						
NOTES- 1) Unba 2) Wind: MWF memb 3) Provid 4) This t 5) * This between	anced roof live ASCE 7-10; V RS (envelope) bers and forces de adequate dr russ has been truss has been truss has been the bottom	loads have been considered for this de ult=130mph (3-second gust) Vasd=103 and C-C Exterior(2) 0-0-0 to 4-4-13, Inte & MWFRS for reactions shown; Lumbe ainage to prevent water ponding. designed for a 10.0 psf bottom chord liv n designed for a live load of 30.0psf on to chord and any other members.	sign. mph; TCDL=6.0psf; BCDL erior(1) 4-4-13 to 14-8-11, r DOL=1.60 plate grip DC e load nonconcurrent with he bottom chord in all are	L=6.0psf; h=15ft; Cat. II Exterior(2) 14-8-11 to DL=1.60 n any other live loads. as with a clearance gre	; Exp C; 6 19-0-9 zc	enclosed; one;C-C fo o 6-0-0	ır		Muun,

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 5.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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ł	8-1-13			I	<u> </u>				18-3-8	<u>18-3-8</u> 2-6-13	
LOADIN TCLL TCDL BCLL BCDL	G (psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC 0.27 BC 0.21 WB 0.63 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.07 -0.14 0.01 0.06	(loc) 9 7-9 5 9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 120 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 WEBS 2x4 SP No.3 *Except* 2-10: 2x6 SP No.1			BRACING- TOP CHOR BOT CHOR	:D :D	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (5-6-1 max.): 1-4. Rigid ceiling directly applied or 10-0-0 oc bracing.						
REACTIONS. (lb/size) 10=739/Mechanical, 5=767/0-3-8 Max Horz 10=-67(LC 13) Max Uplift 10=-86(LC 8), 5=-56(LC 8)											
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-10=-620/227, 2-3=-1757/417, 3-4=-1757/417, 4-5=-1200/255 BOT CHORD 9-10=-28/286, 7-9=-154/931, 5-7=-149/945											

WEBS 2-9=-364/1512, 3-9=-454/242, 4-9=-180/850, 4-7=0/293

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0 to 4-4-13, Interior(1) 4-4-13 to 15-8-11, Exterior(2) 15-8-11 to 19-0-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0

between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 5.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0
- between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 11, 10 except (jt=lb) 14=101.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 8.



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			6-0-0				Į.	
Plate Offsets (X,Y)	[2:0-0-6,Edge]	1						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL) -0	.01 2-4	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.12	Vert(CT) -0	.03 2-4	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0	.00	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0	.03 2-4	>999	240	Weight: 36 lb	FT = 20%
			BRACING.					

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x6 SP No.1 WEBS

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=277/0-3-0, 4=222/0-1-8 Max Horz 2=86(LC 12)

Max Uplift 2=-72(LC 8), 4=-74(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-1 to 3-8-12, Interior(1) 3-8-12 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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LUMBER-	

TOP CHORD 2x6 SP No.1

BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

- REACTIONS. All bearings 6-0-0.
 - (lb) Max Horz 2=129(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 6, 7, 2, 8, 9

Max Grav All reactions 250 lb or less at joint(s) 6, 7, 2, 8, 9

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 7, 2, 8, 9.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/defl	L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.37	Vert(LL) -0.05	2-6 >999	360	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.24	Vert(CT) -0.09	2-6 >999	240	
CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00	n/a	n/a	
SCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.10	2-6 >923	240	Weight: 46 lb FT = 20

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.3 WEBS

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 6=313/Mechanical, 2=354/0-3-0 Max Horz 2=116(LC 12)

Max Uplift 6=-99(LC 8), 2=-89(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) -0-8-1 to 3-8-12, Interior(1) 3-8-12 to 8-0-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.
- 6) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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LUMBER-

TOP CHORD	2x6 SP No.1
BOT CHORD	2x6 SP No.1
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 8-9.

REACTIONS. All bearings 6-3-8.

(lb) - Max Horz 2=167(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 11, 12 except 10=-118(LC 12) Max Grav All reactions 250 lb or less at joint(s) 2, 11, 12 except 10=294(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0
- between the bottom chord and any other members. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 12 except
- (jt=lb) 10=118. 7) Non Standard bearing condition. Review required.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-2 to 4-7-15, Interior(1) 4-7-15 to 5-9-0, Exterior(2) 5-9-0 to 10-1-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=137, 8=137.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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