

Trenco RE: 654049_130mph - H&H/Wilmington/ 818 Soundside Rd Site Information: Edenton, NC 27932 Project Customer: h and h Project Name: 654049 130mph Lot/Block: A Subdivision: All Model: Address: City: Fayetteville State: nc General Truss Engineering Criteria & Design Loads (Individual Truss Design **Drawings Show Special Loading Conditions):** Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.2 Wind Code: ASCE 7-10 Wind Speed: 130 mph Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-10 Roof Load: 40.0 psf Floor Load: N/A psf Mean Roof Height (feet): 25 Exposure Category: C No. Seal# Truss Name Date No. Seal# Truss Name Date C05 C06 C07 CP01 CP02 A01 A02 35 36 137223227 5/28/19 137223261 5/28/19 123456789111111111122222222222333333 137223228 137223262 28/19 5/28/19 37 38 13722 3229 A03 137223263 28/10 5/28/19 3230 A04 137223264 1372 5/28/19 28/19 **3**9 13722 3231 A04A 137223265 28. **4**0 D01 3232 A05 137223266 41 42 3233 3234 A05A 28/ 137223267 D02 5/28/19 A06 13722 D03 3268 28/1 43 3235 A07 137223269 G01 G02 28/ 5/28 36 A08 44 1372 28/19 A09 45 137223271 3237 G03 28/19 5/28/10 38 46 J01 A10 28/19 **3**9 4Ť A11 J02 /28/ 5/28 40 48 J03 A24 28/19 4ğ 13722 3241 A24A 137223275 J04 28/19 42 A25 50 276 Ĵ05 28/19 51 52 3243 A25A J06 44 278 Ĵ07 A26 28/19 53 54 55 56 45 A34 3279 **J**08 46 A34A ĴÕ9 J10 47 A35 281 48 A35A J11 57 58 249 A36 J12 13722 250 **B01** J13 59 251 B02 285 60 J15 61 J21 B05 62 55 B23 63 J23 J24 PB01 PB02 PB03 64 C01 65 291 Č02 C03 C04 3258 66 13722 3292 137223259 67 137223293 137223260 137223294 **PB04** 68

The truss drawing(s) referenced above have been prepared by

Incompared on the parameters Incompared on the parameters Incompared on the parameters Incompared by the design of the state of North Carolina is December 31, 2019 IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use the building designer should verify applicability of design parameters and incorporate these designs into the overall buildit.



Sevier, Scott

May 28,2019

1 of 2



RE: 654049_130mph - H&H/Wilmington/

Trenco 818 Soundside Rd Edenton, NC 27932

No.	Seal#	Job ID#	Truss	Name Date
69	137223295	654049_130	m128805	5/28/19
70	137223296	654049_130	m128806	5/28/19
71	137223297	654049_130	m128807	5/28/19



9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1462.8=1457.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Continued on page 2

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLODED MITER REFERENCE FACE INFERTOR FOR INFERTOR OF THE ADDRESS OF THE ADDRE fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
654049 130mph	A01	HIP GIRDER	5	_		137223227
			-	2	Job Reference (optional)	
Builders FirstSource. S	umter, SC - 29153.		8.2	40 s Mav	13 2019 MiTek Industries, Inc. Tue May 28 10:02:19 2019	Page 2

NOTES-

ID:X0jtQcFjQu8X?XjGN5R0bmzVOFf-IqHVQZNaDplf0U?UbJjeKyU5YZp4kgTK04seTqzC?c2

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 55 lb down and 84 lb up at 3-6-0, 20 lb down and 45 lb up at 5-6-0, 162 lb down and 199 lb up at 7-5-0, 167 lb down and 199 lb up at 9-5-0, 167 lb down and 199 lb up at 11-5-0, 167 lb down and 199 lb up at 13-5-0, 167 lb down and 199 lb up at 15-5-0, 167 lb down and 199 lb up at 15-5-0, 167 lb down and 199 lb up at 12-5-0, 167 lb down and 199 lb up at 13-5-0, 167 lb down and 199 lb up at 13-5-0, 167 lb down and 199 lb up at 23-5-0, and 20 lb down and 45 lb up at 25-5-0, and 55 lb down and 84 lb up at 27-5-0 on top chord, and 167 lb down and 101 lb up at 11-6-0, 95 lb down and 64 lb up at 3-6-0, 153 lb down and 161 lb up at 5-6-0, 54 lb down and 34 lb up at 7-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 12-5-0, 54 lb down and 34 lb up at 23-5-0, 54 lb down and 161 lb up at 25-5-0, and 95 lb down and 64 lb up at 27-5-0, 54 lb down and 34 lb up at 12-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 23-5-0, 54 lb down and 34 lb up at 23-5-0, 54 lb down and 34 lb up at 23-5-0, 54 lb down and 34 lb up at 23-5-0, 54 lb down and 34 lb up at 23-5-0, 54 lb down and 161 lb up at 23-5-0, 54 lb down and 64 lb up at 27-5-0, and 95 lb down and 101 lb up at 29-5-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-7=-60, 7-9=-60, 15-18=-20

Concentrated Loads (lb)

Vert: 21=-15(F) 23=-68(F) 24=-68(F) 25=-68(F) 26=-68(F) 27=-68(F) 28=-68(F) 29=-68(F) 30=-68(F) 31=-68(F) 33=-15(F) 34=-167(F) 35=-95(F) 36=-148(F) 37=-42(F) 38=-42(F) 38=-42(F) 40=-42(F) 41=-42(F) 42=-42(F) 43=-42(F) 45=-42(F) 46=-148(F) 47=-95(F) 48=-167(F)

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	L	9-6-5	1	21-4-11	1	30-11-0		
		9-6-5		11-10-5		9-6-5	5	
Plate Offsets (2	X,Y)	[2:0-8-0,0-0-12], [3:0-3-0,0-2-12], [5:0	-3-0,0-2-12], [6:0-8-0,0-0-12	2]				
LOADING (ps TCLL 20. TCDL 10. BCLL 0. BCDL 10.	af) .0 .0 .0 * .0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.49 BC 0.62 WB 0.26 Matrix-AS	DEFL. ir Vert(LL) -0.14 Vert(CT) -0.23 Horz(CT) 0.04 Wind(LL) 0.14	i (loc) l/defl 8-10 >999 8-10 >999 6 n/a 10-13 >999	L/d PL 360 MT 240 n/a 240 We	ATES GRIP F20 244/190 eight: 198 lb FT = 20'	%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SF 3-5: 2x 2x6 SF 2x4 SF	P No.2 *Except* 4 SP No.2 P No.2 P No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood si 2-0-0 oc purlins (4 Rigid ceiling direc 1 Row at midpt	heathing directly appl 4-9-10 max.): 3-5. tly applied. 4-10, 4-8	lied, except	
REACTIONS.	(lb/size Max H	e) 2=1283/0-5-8, 6=1283/0-4-8 lorz 2=256(LC 11)						

Max Horz 2=256(LC 11) Max Uplift 2=-275(LC 12), 6=-275(LC 13) Max Grav 2=1331(LC 2), 6=1331(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1758/555, 3-4=-1317/581, 4-5=-1317/581, 5-6=-1758/555
- BOT CHORD 2-10=-285/1305, 8-10=-314/1429, 6-8=-207/1305
- WEBS 3-10=-61/619, 4-10=-319/288, 4-8=-319/288, 5-8=-61/619

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 6) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=275, 6=275.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



TRENCO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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	10-3-14		20-7-2		30-11-0		_
	10-3-14		10-3-4	1	10-3-14		1
Plate Offsets (X,Y)	[2:0-6-4,0-0-12], [3:0-3-0,0-3-4], [4:0-5-3	,Edge], [5:0-3-3,Edge], [6:	0-3-0,0-3-4], [7:0-0-0,0-0	0-4]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.58 BC 0.49 WB 0.32 Matrix-AS	DEFL. in Vert(LL) -0.16 Vert(CT) -0.23 Horz(CT) 0.03 Wind(LL) 0.06	(loc) l/defl 9-11 >999 9-11 >999 7 n/a 11-14 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 195 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x6 SI WEBS 2x4 SI 4-9: 2: WEDGE Left: 2x4 SP N3	P No.2 P No.2 P No.3 *Except* 44 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood s 2-0-0 oc purlins (Rigid ceiling direc 1 Row at midpt	sheathing directly a 4-8-11 max.): 4-5. ctly applied. 4-9	applied, except	
REACTIONS. (Ib/siz Max H Max U	e) 2=1289/0-5-8, 7=1289/0-5-8 Horz 2=324(LC 11) Jplift 2=-305(LC 12), 7=-305(LC 13)						

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1699/611, 3-4=-1465/625, 4-5=-1097/567, 5-6=-1465/625, 6-7=-1699/611

BOT CHORD 2-11=-376/1403, 9-11=-172/1034, 7-9=-333/1281

WEBS 3-11=-431/359, 4-11=-153/628, 5-9=-118/553, 6-9=-430/359

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=305, 7=305.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



ENGINEERING BY AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid for dise only with with every connectors. This design is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



	10-9-12	15-5-8	1 20-1-6	30-11-0	
	10-9-12	4-7-12	4-7-14	10-9-10	I
Plate Offsets (X,Y)-	- [2:0-0-0,0-0-4], [5:0-3-12,0-1-12], [6:0-3-	12,0-1-12]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-12Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCodeIRC2015/TPI2014	CSI. TC 0.68 BC 0.66 WB 0.42 Matrix-MS	DEFL. in Vert(LL) -0.12 Vert(CT) -0.21 Horz(CT) 0.03 Wind(LL) 0.07	(loc) I/defl L/d 13 >999 360 11-23 >999 240 9 n/a n/a 11-23 >999 240	PLATES GRIP MT20 244/190 Weight: 211 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 *Except* 4-5,6-7: 2x4 SP No.1 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 *Except* 15-16: 2x4 SP No.2			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing dire except 2-0-0 oc purlins (5-5-5 max.): a Rigid ceiling directly applied o 1 Row at midpt 15	ectly applied or 3-11-5 oc purlins, 5-6. r 10-0-0 oc bracing. 5-16, 6-14
REACTIONS. (Ib)	/size) 2=1329/0-5-8, 9=1329/0-4-8				

REACTIONS. (lb/size) 2=1329/0-5-8, 9=1329/0-4-8 Max Horz 2=-364(LC 10) Max Uplift 2=-322(LC 12), 9=-322(LC 13) Max Grav 2=1336(LC 19), 9=1347(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1734/622, 3-5=-1521/661, 5-6=-1114/578, 6-8=-1537/661, 8-9=-1742/622

BOT CHORD 2-14=-400/1512, 13-14=-44/1013, 11-13=-44/1013, 9-11=-325/1324

WEBS 3-14=-508/410, 5-14=-183/657, 6-16=-216/743, 11-16=-217/705, 8-11=-509/410

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=322, 9=322.

B Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-62, 5-6=-62, 6-10=-62, 18-21=-21



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1707/605, 3-5=-1508/642, 5-6=-1083/563, 6-8=-1520/644, 8-9=-1718/606
- BOT CHORD 2-13=-403/1483, 12-13=-74/997, 10-12=-74/997, 9-10=-345/1308
- WEBS 3-13=-493/397, 5-13=-176/663, 6-15=-210/733, 10-15=-211/694, 8-10=-494/398

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;

- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) Refer to girder(s) for truss to truss connections.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=312, 9=283.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/		
						137223233	
654049_130mph	A05A	PIGGYBACK BASE	35	1			
					Job Reference (optional)		
Builders FirstSource,	Sumter, SC - 29153,		8.	240 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:02:28 2019	Page 2	
		ID:XOjtQcFjQu8X?XjGN5R0bmzVOFf-XZKuJeUD5aQNbsBDdiNmBrMdUBqVLiQe4zXdHpzC?bv					

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-5=-60, 5-6=-60, 6-9=-60, 17-20=-20

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		8-11-6	8-4-2	11-2-0	2-5-8	I.	
Plate Offsets (X,Y)	[2:0-3-8,Edge], [2:0-0-8,0-3-12], [2:0-0-4,0-0-5	5], [5:0-3-0,0-2-2], [6:0-4-4,0-1-	8], [7:0-3-0,0-3-4], [10:0-5-7,0-0-0], [5:0-4-12,0-3	-8], [16:Edge,0-1-8],	
	[17:0-3-8 0-3-	01					

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.93 BC 0.83 WB 0.78 Matrix-AS	DEFL. in (loc) l/defl L/d Vert(LL) -0.27 14-15 >999 360 Vert(CT) -0.64 14-15 >582 240 Horz(CT) 0.22 11 n/a n/a Wind(LL) 0.25 14-15 >999 240	PLATES GRIP MT20 244/190 Weight: 214 lb FT = 20%						
LUMBER- TOP CHORD 2x4 SP 5-7,7-8 BOT CHORD 2x4 SP 10-15: 2 WEBS 2x4 SP WEDGE Left: 2x4 SP NJ.3	No.2 *Except* 2x6 SP No.2, 8-12: 2x6 SP No.1 No.2 *Except* 2x6 SP No.2 No.3		BRACING- TOP CHORD Structural wood sheathing dir 2-0-0 oc purlins (6-0-0 max.): BOT CHORD Rigid ceiling directly applied. WEBS 1 Row at midpt 5-	ectly applied, except 5-7. -15						
REACTIONS. (Ib/size Max Ho Max Up	EACTIONS. (lb/size) 11=1281/0-5-8, 2=1289/0-5-8 Max Horz 2=-352(LC 10) Max Uplift 11=-307(LC 13), 2=-312(LC 12)									
FORCES. (lb) Max. TOP CHORD 2-3=- 9-10= BOT CHORD 2-17= WEBS 3-17=	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1677/587, 3-5=-1594/713, 5-6=-1153/554, 6-7=-1159/551, 7-9=-1448/598, 9-10=-1800/657, 10-11=-783/314 BOT CHORD 2-17=-380/1373, 6-15=-198/725, 14-15=-343/1488, 10-14=-343/1488 WEBS 3-17=-481/400, 5-17=-274/554, 15-17=-96/1190, 5-15=138/263, 9-15=-698/401									
 NOTES- (10) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 6) All bearings are assumed to be User Defined crushing capacity of 565 psi. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=307, 2=312. 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any 										

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932

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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.94 BC 0.84 WB 0.77 Matrix-AS	DEFL. Vert(LL) - Vert(CT) - Horz(CT) Wind(LL)	in (loc) //d 0.27 13-14 >9 0.64 13-14 >5 0.22 11 r 0.26 13-14 >9	lefi L/d 199 360 179 240 n/a n/a 199 240	PLATES MT20 Weight: 211 lb	GRIP 244/190 FT = 20%				
LUMBER- TOP CHORD 2x4 SP 5-7,7-8 BOT CHORD 2x4 SP 10-14: : WEBS 2x4 SP WEDGE Left: 2x4 SP No.3	LUMBER- BRACING- FOP CHORD 2x4 SP No.2 *Except* TOP CHORD Structural wood sheathing directly applied, except 5-7,7-8: 2x6 SP No.2, 8-11: 2x6 SP No.1 TOP CHORD Structural wood sheathing directly applied, except 3OT CHORD 2x4 SP No.2 *Except* BOT CHORD Rigid ceiling directly applied. 10-14: 2x6 SP No.2 WEBS 1 Row at midpt 5-14 WEBS 2x4 SP No.3 Left: 2x4 SP No.3 Left: 2x4 SP No.3										
REACTIONS. (lb/size) 11=1236/Mechanical, 2=1290/0-5-8 Max Horz 2=344(LC 11) Max Uplift 11=-283(LC 13), 2=-312(LC 12)											
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1678/588, 3-5=-1594/714, 5-6=-1152/558, 6-7=-1158/555, 7-9=-1450/604, 9-10=-1802/664, 10-11=-784/285 BOT CHORD 2-16=-392/1364, 6-14=-203/722, 13-14=-374/1491, 10-13=-374/1491 WEBS 3-16=-481/400, 5-16=-272/550, 14-16=-110/1181, 5-14=-140/263, 9-14=-695/406 NOTES- (11) 1) Unbalanced roof live loads have been considered for this design.											
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; 1CDL=6.0pst; BCDL=6.0pst; BcDL=6.0pst;											

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May 28,2019



	6-10-5	<u>12-0-15</u> 5-2-9	17-3-	-8	24-0-11 6-9-3		<u>28-5-8</u> <u>30</u> 4-4-13 <u>2</u>	0-11-0 2-5-8	
Plate Offsets (X,Y)	[3:0-6-0,0-2-0], [7:0-6-0	,0-2-0], [8:0-5-4,0	-2-7], [12:0-4-8,0-1-8], [1	14:0-2-12,0-2-12]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/7	2-0-0 1.15 1.15 NO IPI2014	CSI. TC 0.53 BC 0.82 WB 0.32 Matrix-MS	DEFL. Vert(LL) 0 Vert(CT) -0 Horz(CT) 0	in (loc) l/defl .21 13-14 >999 .20 12-13 >999 .11 9 n/a	L/d 240 240 n/a	PLATES MT20 MT20HS Weight: 414 I	GRIP 244/190 187/143 p FT = 20%	
LUMBER- TOP CHORD 2x4 S 7-10: BOT CHORD 2x6 S 5-15: WEBS 2x4 S	P No.2 *Except* 2x6 SP No.2 P No.2 *Except* 2x4 SP No.2, 8-14: 2x6 S P No.2	SP No.1		BRACING- TOP CHORD BOT CHORD	Structural woo 2-0-0 oc purlin Rigid ceiling d 10-0-0 oc brac	d sheathing dir is (6-0-0 max.): irectly applied c ing: 12-13	ectly applied or 6-0- 3-7. or 10-0-0 oc bracing.	0 oc purlins, except Except:	
REACTIONS. (lb/size) 2=2187/0-5-8, 9=2129/0-5-8 Max Horz 2=196(LC 26) Max Uplift 2=-1480(LC 8), 9=-1453(LC 9)									
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2838/2060, 3-4=-2880/2366, 4-5=-3603/2985, 5-7=-3633/3012, 7-8=-3415/2585, 8-9=-2785/1991 BOT CHORD 2-18=-1715/2240, 16-18=-1720/2251, 15-16=-273/369, 5-14=-446/523, 13-14=-2087/2768, 12-13=-2068/2737, 8-12=-2068/2737, 9-11=-1310/1876 WEBS 3-18=-202/498, 3-16=-1078/1188, 4-16=-1184/1192, 14-16=-2167/2735, 4-14=-807/981, 7-14=-1110/1221, 7-13=-625/961, 11-12=-1246/1823, 8-11=-2523/1762									
 Ande-1110/1221, r/13e-625/961, 11-12e-1246/1823, 8-11=-2523/1762 NOTES- (13) 1) 2-ply truss to be connected together with 10d (0.131*x3*) nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been considered for this design. 2) Unbalanced roof live loads have been considered for this design. 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60 5) Provide adequate drainage to prevent water ponding. 6) All plates are MT20 plates unless otherwise indicated. 7) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 9) All bearings are assumed to be User Defined crushing capacity of 565 psi. 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1480, 9=1453. 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 									
Continued on page 2									

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Job	Truss	Truss Type	Qtv	Ply	H&H/Wilminaton/	
			,	-	3	137223237
654049_130mph	A09	HIP GIRDER	1	2		
				Z	Job Reference (optional)	
Builders FirstSource. S	umter, SC - 29153.		8.2	240 s Mav	13 2019 MiTek Industries, Inc. Tue May 28 10:02:38 2019	Page 2

NOTES- (13)

ID:XOjtQcFjQu8X?XjGN5R0bmzVOFf-EUwgP3bVkfhyoPy8CoZ6bymLoDCAhGG7OXy9dDzC?bl

- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 55 lb down and 84 lb up at 3-6-0, 20 lb down and 45 lb up at 5-6-0, 162 lb down and 199 lb up at 7-5-0, 167 lb down and 199 lb up at 9-5-0, 167 lb down and 199 lb up at 11-5-0, 167 lb down and 199 lb up at 13-5-0, 167 lb down and 199 lb up at 15-5-0, 106 lb down and 103 lb up at 15-5-0, 106 lb down and 103 lb up at 12-5-0, 106 lb down and 103 lb up at 12-5-0, 106 lb down and 103 lb up at 23-5-0, and 84 lb down and 131 lb up at 25-5-0, and 88 lb down and 104 lb up at 27-5-0 on top chord, and 167 lb down and 101 lb up at 1-6-0, 95 lb down and 64 lb up at 3-6-0, 54 lb down and 161 lb up at 5-6-0, 54 lb down and 34 lb up at 7-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 136 lb up at 13-5-0, 120 lb down and 136 lb up at 15-5-0, and 53 lb down and 136 lb up at 19-5-0, 120 lb down and 136 lb up at 21-5-0, 120 lb down and 136 lb up at 23-5-0, and 53 lb down and 24 lb up at 23-5-0, and 136 lb up at 23-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 34 lb up at 13-5-0, 54 lb down and 136 lb up at 13-5-0, 54 lb down and 136 lb up at 23-5-0, and 53 lb down and 136 lb up at 23-5-0, and 136 lb up at 23-5-0, and 53 lb down and 42 lb up at 23-5-0, and 167 lb down and 101 lb up at 29-8-9 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60. 3-7=-60. 7-10=-60. 15-19=-20. 12-14=-20. 11-22=-20

Concentrated Loads (lb)

Vert: 15=-92(F) 5=-8(F) 8=-167(F) 25=-15(F) 27=-68(F) 28=-68(F) 29=-68(F) 30=-68(F) 31=-68(F) 32=-8(F) 33=-8(F) 33=-8(F) 34=-8(F) 35=-44(F) 36=-48(F) 37=-167(F) 38=-95(F) 39=-148(F) 40=-42(F) 41=-42(F) 42=-42(F) 43=-42(F) 45=-92(F) 46=-92(F) 47=-92(F) 48=-57(F) 49=-53(F)

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	9-6-5	17-3-8	21-4-11	28-5-8	₁ 30-11-0			
	9-6-5	7-9-3	4-1-3	7-0-13	2-5-8	1		
Plate Offsets (X,Y)	[2:Edge,0-2-8], [3:0-5-4,0-3-4], [5:0-4-0,0)-2-8], [6:0-5-4,0-3-6], [10:0-4-12,	0-1-8], [12:0-4-12,0-3-8],	, [14:0-5-0,0-4-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. D TC 0.68 V BC 0.71 V WB 0.45 H Matrix-AS W	EFL. in (loc) ert(LL) -0.16 10-11 ert(CT) -0.33 10-11 orz(CT) 0.14 7 <i>l</i> ind(LL) 0.19 10-11	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 219 lb	GRIP 244/190 FT = 20%		
LUMBER- TOP CHORD 2x6 s 3-5: 2 BOT CHORD 2x6 s 4-13, WEBS 2x4 s	SP No.2 *Except* 2x4 SP No.2 SP No.2 *Except* 7-9: 2x4 SP No.2, 6-12: 2x6 SP DSS SP No.3	B Tr	RACING- DP CHORD Structu 2-0-0 c DT CHORD Rigid c 10-0-0	iral wood sheathing direc oc purlins (3-7-5 max.): 3 eeiling directly applied. E oc bracing: 10-11	otly applied, except -5. xcept:			
REACTIONS. (lb/si Max Max	ze) 7=1281/0-5-8, 2=1281/0-5-8 Horz 2=256(LC 11) Uplift 7=-274(LC 13), 2=-274(LC 12)							
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1639/572, 3-4=-1525/686, 4-5=-1510/675, 5-6=-1867/641, 6-7=-1578/620 BOT CHORD 2-14=-296/1192, 13-14=-24/262, 4-12=-366/280, 11-12=-265/1414, 10-11=-266/1425, 6-10=-266/1425, 7-9=-417/1073 WEBS 3-14=0/299, 12-14=-284/949, 3-12=-249/514, 5-12=-284/313, 9-10=-398/1076, 5-11=-63/551, 6-9=-1499/583								
NOTES- (10) 1) Unbalanced roof li 2) Wind: ASCE 7-10; MWFRS (envelope MWFRS for reacti 3) Provide adequate 4) This truss has bee 5) * This truss has be will fit between the	ve loads have been considered for this de: Vult=130mph (3-second gust) Vasd=103r e) gable end zone and C-C Exterior(2) zon ons shown; Lumber DOL=1.60 plate grip E drainage to prevent water ponding. In designed for a 10.0 psf bottom chord live en designed for a live load of 20.0psf on t bottom chord and any other members, wi	sign. nph; TCDL=6.0psf; BCDL=6.0psf e; end vertical left and right expos OL=1.60 e load nonconcurrent with any oth he bottom chord in all areas wher th BCDL = 10.0psf.	; h=25ft; Cat. II; Exp C; E sed;C-C for members an ter live loads. e a rectangle 3-6-0 tall b	Enclosed; d forces & y 2-0-0 wide	AL OPT	CAROL		

- 6) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=274, 2=274.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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	8-11-6	17-3-8		28-5-8		30-11-0	
Plate Offsets (X,Y)	[2:0-3-8,Edge], [2:0-0-8,0-3-12], [2:0-0-4	0-0-5], [5:0-4-0,0-1-6], [8:0-5-0,	0-4-8], [9:0-5-7,0	-0-0], [14:0-6-4,0-2	2-8], [15:Edge,0	-1-8], [16:0-3-8,0-3-0	0]
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.93 BC 0.75 WB 0.77 Matrix-AS	DEFL. ir Vert(LL) -0.25 Vert(CT) -0.61 Horz(CT) 0.21 Wind(LL) 0.24	loc) l/defl 13-14 >999 13-14 >610 13-14 >610 10 n/a 13-14 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 217 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP 7-8: 2x4 BOT CHORD 2x4 SP 9-14: 2: WEBS 2x4 SP WEDGE Left: 2x4 SP No.3	No.2 *Except* 5 SP No.2, 8-11: 2x6 SP No.1 No.2 *Except* x6 SP No.2 No.3	, 	SRACING- OP CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling dire 1 Row at midpt	sheathing direc (5-1-9 max.): 5- ctly applied. E: 6-14	tly applied, except 7. xcept: 4	
REACTIONS. (Ib/size Max He Max U	e) 10=1281/0-5-8, 2=1289/0-5-8 prz 2=-325(LC 10) plift 10=-300(LC 13), 2=-304(LC 12)						
FORCES. (lb) - Max. TOP CHORD 2-3=- 8-9=- 8-9=- BOT CHORD 2-16= WEBS 3-16= 8-14= 8-14=	Comp./Max. Ten All forces 250 (lb) or 1685/599, 3-5=-1515/675, 5-6=-1225/58 1834/674, 9-10=-783/317 368/1269, 6-14=-261/177, 13-14=-367/ 425/361, 5-16=-207/460, 14-16=-124/1 704/373	ess except when shown. 2, 6-7=-1233/578, 7-8=-1454/59 1534, 9-13=-367/1534 145, 5-14=-139/377, 7-14=-235,	0, 726,				
 NOTES- (10) 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) MWFRS for reaction 3) Provide adequate dr 4) This truss has been 5) * This truss has been will fit between the b 6) All bearings are assi 7) Provide mechanical 10–300, 2=304. 8) This truss design reconstructs be applied 9) Graphical purlin repr 10) This manufactured 	loads have been considered for this des ult=130mph (3-second gust) Vasd=103n gable end zone and C-C Exterior(2) zon s shown; Lumber DOL=1.60 plate grip D ainage to prevent water ponding. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on th ottom chord and any other members. umed to be User Defined crushing capace connection (by others) of truss to bearing quires that a minimum of 7/16" structural l directly to the bottom chord. esentation does not depict the size or the truss is designed as an individual buildir	ign. ph; TCDL=6.0psf; BCDL=6.0ps e; end vertical left and right exp OL=1.60 load nonconcurrent with any o le bottom chord in all areas whe ity of 565 psi. plate capable of withstanding wood sheathing be applied dire e orientation of the purlin along g component. The suitability ar	f; h=25ft; Cat. II; sed;C-C for mer her live loads. re a rectangle 3- 100 lb uplift at join ctly to the top cho the top and/or bo nd use of this con	Exp C; Enclosed; nbers and forces & 6-0 tall by 2-0-0 w nt(s) except (jt=lb) ord and 1/2" gypsu ttom chord. nponent for any	de m	Cott	SEAL 44925

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

M. SE SE May 28,2019

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10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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SEAL 044925 MGINEEP, HTT May 28,2019



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RENCO



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3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 6) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=312, 10=321.

 This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	10-2-15	14-2-3	21-1-1	30-11-0	1	
	10-2-15	3-11-4	6-10-15	9-9-15		
Plate Offsets (X,Y)	[2:0-0-0,0-0-4], [5:0-3-12,0-1-12], [6:0-3-	12,0-1-12]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.58 BC 0.71 WB 0.44 Matrix-AS	DEFL. in Vert(LL) -0.22 Vert(CT) -0.31 Horz(CT) 0.04 Wind(LL) 0.07	(loc) I/defl L/d 12-14 >999 360 12-14 >999 240 10 n/a n/a 15-21 >999 240	PLATES MT20 Weight: 219 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF 5-12,10 SLIDER Right 2 REACTIONS. (Ib/sizt Max H Max II	² No.2 ² No.2 ³ No.3 *Except* 6-17: 2x4 SP No.2 2x6 SP No.2 1-11-12 e) 2=1288/0-5-8, 10=1315/0-5-8 lorz 2=-355(LC 10) lorif 2=-312(LC 12) 10=-321(LC 13)		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing dir 2-0-0 oc purlins (5-6-0 max.): Rigid ceiling directly applied. 1 Row at midpt 5-	ectly applied, except 5-6. -12, 16-17	
FORCES. (lb) - Max. TOP CHORD 2-3=- BOT CHORD 2-15= WEBS 3-15= NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10: V	rav 2=1314(LC 19), 10=1322(LC 20) Comp./Max. Ten All forces 250 (lb) or :1729/597, 3-5=-1548/659, 5-6=-1056/56 =-386/1507, 14-15=-124/1066, 12-14=-12 =-491/399, 15-16=-224/753, 5-16=-221/7 b loads have been considered for this deal (ult=130mph (3-second gust) Vasd=1037	less except when shown. 1, 6-8=-1464/639, 8-10=-1 24/1066, 10-12=-288/1227 85, 6-12=-161/637, 8-12= sign. 2001: TCDI =6 00sf; BCDI =	1605/582 , -440/375 =6 0nsf: h=25ft: Cat II: I	-yn C. Enclosed		

Wind: ASCE 7-10, Volie Tompin (Second gust) Vasue Tompin, ToDL=0.0pst, BCDL=0.0pst, BCDL=0.0pst, Te251, Cal. II, EXP C, Enclosed, MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=312, 10=321.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 5-6=-60, 6-11=-60, 19-22=-20



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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=183, 7=183.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces &

MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 565 psi.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=189, 8=189.

7) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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NOTES- (7)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 565 psi.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=167, 7=188.

7) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) All plates are MT20 plates unless otherwise indicated.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 565 psi.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=203, 10=203.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
					13	7223257
654049_130mph	C01	GABLE	3	1		
					Job Reference (optional)	
Builders FirstSource,	Sumter, SC - 29153,		8.2	40 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:03:11 2019 Pa	age 2
		ID:XOjtQo	FiQu8X?	(jGN5R0b	mzVOFf-mrQ9LL?BzorHFT UeEfrQve YWaGwDn zASPIpz0	C?bE

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-4=-75(F=-15), 4-6=-75(F=-15), 6-9=-60, 20-26=-35(F=-15), 23-26=-20, 7-11=-45(F)

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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
654049 130mph	C02	Hip Girder	3	2	13	7223258
				Z	Job Reference (optional)	
Builders FirstSource S	umter SC - 29153		82	240 s May	13 2019 MiTek Industries Inc. Tue May 28 10:03:12 2019 P	age 2

ID:XOjtQcFjQu8X?XjGN5R0bmzVOFf-E1_XYh0pk5z8tdZgCxA4z7BE8wnMfcN7CpByqFzC?bD 12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-6=-60, 12-15=-20

Concentrated Loads (lb)

Vert: 18=-1218(B) 19=-1216(B) 20=-1216(B) 21=-1216(B) 22=-1216(B) 23=-1216(B) 24=-1216(B) 25=-1216(B) 26=-1216(B)

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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component** 818 Soundside Road

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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
					137	7223259
654049_130mph	C03	GABLE	5	1		
					Job Reference (optional)	
Builders FirstSource,	Sumter, SC - 29153,		8.2	240 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:03:14 2019 Pa	age 2
		ID:XOjtQ	cFjQu8X?	XjGN5R0b	mzVOFf-BQ6HzM14GjDr6xi2JMCY2YGUrkdZ7aWQf7h3v8zC	C?bB

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-4=-75(F=-15), 4-27=-75(F=-15), 7-27=-60, 21-28=-35(F=-15), 24-28=-20, 5-10=-45(F)

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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
						137223260
654049_130mph	C04	Common Girder	5	2		
				Z	Job Reference (optional)	
Builders FirstSource, S	Sumter, SC - 29153,		8.2	240 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:03:15 2019	Page 2
		ID:	XOjtQcFj0	Qu8X?XjG	N5R0bmzVOFf-fcggAi2i10Lik5HFt3jnblokg8pMsu0ZunQcRa	azC?bA

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 10-13=-20

Concentrated Loads (lb)

Vert: 7=-1216(B) 6=-1216(B) 15=-1222(B) 16=-1216(B) 17=-1216(B) 18=-1216(B) 19=-1216(B) 20=-1216(B) 21=-1216(B)

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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 4-5=-487/138, 5-6=-845/49, 2-3=-598/63, 3-4=-533/35, 6-8=-802/168, 2-14=-476/0
- BOT CHORD 13-14=-113/433, 12-13=-113/433, 11-12=-113/433, 9-11=-53/669, 8-9=-418/535
- WEBS 11-15=-517/304, 5-15=-536/302, 5-9=0/251, 6-9=-34/516, 3-12=-669/468

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 8 except (jt=lb) 12=433, 13=387.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
						137223261
654049_130mph	C05	GABLE	2	1		
					Job Reference (optional)	
Builders FirstSource,	Sumter, SC - 29153,		8.	240 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:03:17 2019	Page 2
		ID:XOjtQ	cFjQu8X?)	(jGN5R0b	mzVOFf-b?nQbO4yZebQzORd?UmFgAu_cxYLKtYsM5vjVT	zC?b8

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 4-5=-76(F=-16), 5-6=-60, 6-7=-60, 1-2=-60, 2-4=-76(F=-16), 14-23=-36(F=-16), 8-23=-20, 5-11=-46(F)

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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
					13	37223262
654049_130mph	C06	Common Girder	1	2		
				_	Job Reference (optional)	
Builders FirstSource,	Sumter, SC - 29153,		8.	240 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:03:18 2019 P	Page 2
		ID:XO	QcFjQu8X?	XiGN5R0	bmzVOFf-3BLopk4aKxkHbY0gYCHUDOQCuLgz3Gn0alfG2vz	C?b7

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 6-11=-20

Concentrated Loads (lb)

Vert: 8=-1215(B) 7=-1215(B) 6=-1223(B) 12=-1215(B) 13=-1215(B) 14=-1215(B) 15=-1215(B) 16=-1215(B) 17=-1215(B)

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Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
			-			137223263
654049_130mph	C07	Common Girder	1	2		
				–	Job Reference (optional)	
Builders FirstSource, S	umter, SC - 29153,		8.2	240 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:03:20 2019	Page 2
		ID:XOjtQ	cFjQu8X?	XiGN5R0	omzVOFf-?aTZEQ6rrZ_?qsACgdJylpWYN9WTXAFI238N6c	zC?b5

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 6-11=-20

Concentrated Loads (lb)

Vert: 8=-1216(B) 7=-1216(B) 6=-1224(B) 12=-1216(B) 13=-1216(B) 14=-1216(B) 15=-1216(B) 16=-1216(B) 17=-1216(B)

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-696/941, 3-4=-696/941

BOT CHORD 2-6=-779/609, 4-6=-779/609 WEBS 3-6=-384/247

NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=222. 4=222.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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TILD0

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 565 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=135, 6=135.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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May 28,2019





TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

TOP CHORD BOT CHORD

TOP CHORD

BOT CHORD

REACTIONS.

WEBS

2x6 SP No.2

2x4 SP No.2

2x4 SP No.3

BOT CHORD 2-6=-340/1080, 4-6=-340/1080 WEBS 3-6=0/404

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2-3=-1235/530, 3-4=-1235/530

(lb/size) 2=836/0-5-8, 4=836/0-3-8 Max Horz 2=-100(LC 13)

Max Uplift 2=-235(LC 12), 4=-235(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be User Defined crushing capacity of 565 psi.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=235, 4=235.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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 	9-11-8				19-11-0		
Plate Offsets (X,Y)	[2:Edge,0-2-0], [3:0-4-0,0-2-2], [4:0-4-0,	0-2-2], [5:Edge,0-2-0], [7:0-5-	0,0-4-8]		3-11-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrNOCode IRC2015/TPI2014	CSI. TC 0.52 BC 0.56 WB 0.11 Matrix-MS	DEFL. in Vert(LL) 0.17 Vert(CT) -0.15 Horz(CT) -0.02	(loc) l/defl 7-10 >999 7-10 >999 5 n/a	L/d 240 240 n/a	PLATES MT20 Weight: 100 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SI 3-4: 2: BOT CHORD 2x6 SI WEBS 2x4 SI REACTIONS. (lb/siz	P No.2 *Except* k6 SP No.2 P No.2 P No.2 e) 2=921/0-5-8, 5=921/0-3-8		BRACING- TOP CHORD BOT CHORD	Structural wood : 2-0-0 oc purlins (Rigid ceiling dire	sheathing directly 5-2-5 max.): 3-4 ctly applied or 7-	y applied or 4-1-0 o 2-11 oc bracing.	c purlins, except
Max H Max L	lorz 2=66(LC 31) Jplift 2=-607(LC 8), 5=-604(LC 9)						
FORCES. (lb) - Max TOP CHORD 2-3= BOT CHORD 2-7= WEBS 3-7=	Comp./Max. Ten All forces 250 (lb) or -1491/1096, 3-4=-1523/1228, 4-5=-1491, -947/1379, 5-7=-938/1328 -278/394, 4-7=-283/394	less except when shown. 1094					
 NOTES- (11) 1) Unbalanced roof liv 2) Wind: ASCE 7-10; ' MWFRS (envelope) 3) Provide adequate d 4) This truss has beer will fit between the l 6) All bearings are ass 7) Provide mechanica 2=607, 5=604. 8) Graphical purlin rep 9) Hanger(s) or other of 6-2-6, 95 lb down and 66 lb up at 6-3 38 lb down and 66 lb up at and 66 lb up at 6-3 38 lb down and 66 l The design/selectio 10) In the LOAD CASI 11) This manufactured particular building 	e loads have been considered for this de Vult=130mph (3-second gust) Vasd=1037) gable end zone; end vertical left and rig rainage to prevent water ponding. In designed for a 10.0 psf bottom chord liv en designed for a live load of 20.0psf on t bottom chord and any other members. Sumed to be User Defined crushing capac I connection (by others) of truss to bearin presentation does not depict the size or th connection device(s) shall be provided su nd 56 lb up at 7-10-4, 95 lb down and 56 at 13-8-10 on top chord, and 42 lb down -12, 38 lb down and 66 lb up at 7-10-4, 55 lb up at 13-7-4, and 45 lb down and 96 ll n of such connection device(s) is the res E(S) section, loads applied to the face of d truss is designed as an individual buildin is the responsibility of the building design	sign. nph; TCDL=6.0psf; BCDL=6.0 nt exposed; Lumber DOL=1.6 e load nonconcurrent with any ne bottom chord in all areas v city of 565 psi. g plate capable of withstandir e orientation of the purlin alor fficient to support concentrate lb up at 9-10-4, and 95 lb do and 59 lb up at 2-3-12, 45 lb 88 lb down and 66 lb up at 9- o up at 15-7-4, and 42 lb dow ponsibility of others. the truss are noted as front (F ig component. The suitability ier per ANSI TPI 1 as referen	Opsf; h=25ft; Cat. II; 50 plate grip DOL=1. y other live loads. where a rectangle 3- ng 100 lb uplift at joir ng the top and/or boi ed load(s) 89 lb dow own and 56 lb up at down and 96 lb up at 10-4, 38 lb down an <i>in</i> and 59 lb up at 1; =) or back (B). y and use of this corn iced by the building of	Exp C; Enclosed; 60 6-0 tall by 2-0-0 wi ht(s) except (jt=lb) ttom chord. n and 56 lb up at 11-10-4, and 89 lb at 4-3-12, 38 lb dc d 66 lb up at 11-10 7-7-4 on bottom ch ponent for any code.	de wn)-4, ord.	THE REPORT	CAROLINE ESSION SEAL 44925
LOAD CASE(S) Star 1) Dead + Roof Live (I	dard balanced): Lumber Increase=1.15, Plate	ncrease=1.15				The TT	M. SEVILIN

Continued on page 2

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May 28,2019

Job	Truss	Truss Type	Qty	Ply	H&H/Wilmington/	
						137223271
654049_130mph	G03	Hip Girder	1	1		
					Job Reference (optional)	
Builders FirstSource, S	umter, SC - 29153,		8.2	240 s May	13 2019 MiTek Industries, Inc. Tue May 28 10:03:29 2019	Page 2

ID:XOjtQcFjQu8X?XjGN5R0bmzVOFf-EIWy6VDUkK6jPEMxh0z39iO2?ng88NBd6zpLwmzC?ay

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-6=-60, 8-11=-20 Concentrated Loads (lb) Vert: 17=-42(B) 18=-30(B) 23=-30(B) 24=-42(B)

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```
LUMBER-
```

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 OTHERS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

- REACTIONS. (lb/size) 5=14/6-0-0, 2=190/6-0-0, 6=317/6-0-0 Max Horz 2=92(LC 9) Max Uplift 5=-6(LC 9), 2=-92(LC 8), 6=-127(LC 12)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. WEBS 3-6=-234/285

NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2 except (jt=lb) 6=127.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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	0-6-8		6-0-0	
	0-6-8		5-5-8	
Plate Offsets (X,Y)	[2:0-0-0,0-0-12], [2:0-2-9,Edge]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.39 BC 0.39 WB 0.00 Matrix-AS	DEFL. in (loc) l/defl L/d Vert(LL) 0.11 4-9 >624 240 Vert(CT) -0.07 4-9 >999 240 Horz(CT) -0.01 2 n/a n/a	PLATES GRIP MT20 244/190 Weight: 23 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 WEDGE
 Left: 2x4 SP No.3

REACTIONS. (Ib/size) 2=320/0-3-0, 4=201/0-1-8 Max Horz 2=92(LC 8) Max Uplift 2=-216(LC 8), 4=-144(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be User Defined crushing capacity of 425 psi.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216, 4=144.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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			-6-8				5-0-0					
			-6-8 deel				4-5-8					
Plate Olise	els (X, Y)	[2:0-0-0,0-1-0], [2:0-2-9,E	agej	1								
LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.24 0.25 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.05 -0.03 -0.01	(loc) 4-9 4-9 2	l/defl >999 >999 n/a	L/d 240 240 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2015/TF	912014	Matri	k-AS						Weight: 20 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2WEDGELeft: 2x4 SP No.3

REACTIONS. (Ib/size) 2=283/0-3-0, 4=158/0-1-8 Max Horz 2=78(LC 8) Max Uplift 2=-194(LC 8), 4=-113(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be User Defined crushing capacity of 425 psi.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=194, 4=113.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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Max Uplift 4=-56(LC 12), 2=-106(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=106.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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			4-10-12					
LOADING (psf)	SPACING- 2-0-0 Plate Grip DOI 1 15	CSI. TC 0.20	DEFL. in	(loc) 4-7	l/defl ⊳999	L/d 240	PLATES	GRIP 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.17 WB 0.00 Matrix AS	Vert(CT) -0.01 Horz(CT) -0.01	4-7 3	>999 n/a	240 n/a	Woight: 22 lb	ET - 20%

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

```
LUMBER-
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TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=128/Mechanical, 2=242/0-5-8, 4=62/Mechanical Max Horz 2=286(LC 12) Max Uplift 3=-194(LC 12), 4=-14(LC 12)

Max Grav 3=168(LC 19), 2=242(LC 1), 4=92(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=194.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Max Grav 4=30(LC 1), 2=236(LC 1), 6=186(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 6=141.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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LUMBER-				BRACING-	_	_							
BCDL	10.0	Code IRC2015/TP	912014	Matrix	<-AS	Wind(LL)	0.00	6-9	>999	240	Weight: 31 lb	FT = 20%	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	4	n/a	n/a			
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	-0.01	6-9	>999	240			
TCLL	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	-0.01	6-9	>999	360	MT20	244/190	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.2 *Except*	TOP CHORD	Structural wood sheathing directly applied, except
	3-4: 2x4 SP No.2		2-0-0 oc purlins: 3-4.
BOT CHORD	2x6 SP No.2	BOT CHORD	Rigid ceiling directly applied.
WEBS	2x4 SP No.3		

REACTIONS. (lb/size) 4=75/Mechanical, 2=242/0-5-8, 5=115/Mechanical Max Horz 2=164(LC 12) Max Uplift 4=-50(LC 8), 2=-38(LC 12), 5=-44(LC 12) Max Grav 4=75(LC 1), 2=242(LC 1), 5=119(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2, 5.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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4-10-12
4-10-12

Plate Offse	ets (X,Y)	[3:0-4-4,0-1-12]										
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.33	Vert(LL)	-0.00	5 -8	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	-0.01	5-8	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.03	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	<-MP	Wind(LL)	0.00	5-8	>999	240	Weight: 31 lb	FT = 20%
LUMBER-						BRACING						

LUMBER-

 TOP CHORD
 2x6 SP No.2 *Except*

 3-4: 2x4 SP No.2

 BOT CHORD
 2x6 SP No.2

 WEBS
 2x4 SP No.2

 TOP CHORD
 Structural wood sheathing directly applied or 4-10-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=240/0-5-8, 5=187/Mechanical

Max Horz 2=86(LC 8) Max Uplift 2=-73(LC 8), 5=-81(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearings are assumed to be: Joint 2 User Defined crushing capacity of 565 psi, Joint 5 User Defined crushing capacity of 425 psi.7) Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 61 lb down and 30 lb up at 0-9-15, and 71 lb down and 28 lb up at 2-11-8 on top chord, and 10 lb down and 13 lb up at 0-11-8, and 10 lb down and 13 lb up at 2-11-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-6=-20 Concentrated Loads (lb)

Vert: 10=-1(B) 11=-1(B)





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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.07 BC 0.04 WB 0.00 Matrix-MP	DEFL. in (loc) l/defl L/d Vert(LL) 0.00 7 >999 240 Vert(CT) -0.00 7 >999 240 Horz(CT) 0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 7 lb FT = 20%
LUMBER-			BRACING-	

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-5-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEDGE

Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=28/Mechanical, 2=126/0-5-8, 4=13/Mechanical Max Horz 2=85(LC 12) Max Uplift 3=-36(LC 12), 2=-26(LC 12), 4=-6(LC 12) Max Grav 3=36(LC 19), 2=126(LC 1), 4=23(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.
- 7) This manufactured truss is designed as an individual building component. The suitability and use of this component for any
- particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

All bearings are assumed to be User Defined crushing capacity of 425 psi.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=103, 5=116.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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0.05

0.04

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

n/a

>999

2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied.

4

5-6

n/a

240

Weight: 26 lb

Structural wood sheathing directly applied, except end verticals, and

FT = 20%

FORCES.	(lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.

0.0

10.0

TOP CHORD 2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

BCDL

LUMBER-

WEBS

BOT CHORD

REACTIONS.

NOTES-(12)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-AS

3) Provide adequate drainage to prevent water ponding.

Max Horz 8=150(LC 12)

4) All plates are MT20 plates unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6) will fit between the bottom chord and any other members.

7) All bearings are assumed to be User Defined crushing capacity of 425 psi.

Code IRC2015/TPI2014

Max Uplift 8=-43(LC 12), 4=-65(LC 9), 5=-22(LC 12) Max Grav 8=255(LC 1), 4=108(LC 1), 5=82(LC 3)

(Ib/size) 8=255/0-5-8, 4=108/Mechanical, 5=73/Mechanical

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 4, 5.

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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 IMI	REF	> _
 , 1411		·-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD WEBS

2x4 SP No.2 *Except* 2-5: 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (Ib/size) 6=152/0-5-8, 3=38/Mechanical, 4=18/Mechanical Max Horz 6=125(LC 12) Max Uplift 3=-63(LC 12), 4=-55(LC 12) Max Grav 6=152(LC 1), 3=53(LC 19), 4=50(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(6)

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
- 6) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Plate Offsets (A, f)	[3.0-2-0,Euge]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.14 BC 0.10 WB 0.00 Matrix-MR	DEFL. in (loc) l/defl L/d Vert(LL) -0.00 6 >999 240 Vert(CT) -0.00 6 >999 240 Horz(CT) -0.00 5 n/a n/a Weight: 13 lb FT = 20%
LUMBER- TOP CHORD 2x4 S	P No.2		BRACING- TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins,

 TOP CHORD
 2x4 SP No.2
 TOP CHORD
 Structural wood sheatning directly applied of 2-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.

 WEBS
 2x4 SP No.2 *Except*
 BOT CHORD
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

 4-5: 2x4 SP No.3
 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=50/Mechanical, 6=148/0-5-8 Max Horz 6=123(LC 9) Max Uplift 5=-79(LC 9), 6=-39(LC 12) Max Grav 5=65(LC 19), 6=148(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearings are assumed to be: Joint 6 User Defined crushing capacity of 565 psi.

- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any

particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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REACTIONS. (Ib/size) 5=50/Mechanical, 6=140/0-5-8 Max Horz 6=78(LC 11) Max Uplift 5=-42(LC 9), 6=-49(LC 12)

Max Grav 5=62(LC 24), 6=140(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 1-10-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: Joint 6 User Defined crushing capacity of 565 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.19 BC 0.13 WB 0.08 Matrix-P	DEFL. in Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) 0.00	(loc) 1 1 5	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 23 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No 2			BRACING-	Structu	iral wood	sheathing di	rectly applied or 6-0-0	oc purlins

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.2 OTHERS 2x4 SP No.3 BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlin except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

- REACTIONS. (lb/size) 5=14/6-0-0, 2=190/6-0-0, 6=317/6-0-0 Max Horz 2=92(LC 9) Max Uplift 5=-6(LC 9), 2=-92(LC 8), 6=-127(LC 12)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. WEBS 3-6=-234/285

NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2 except (jt=lb) 6=127.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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0-6-8				6-0-0					1			
		0-6-8					5-5-8					1
Plate Offse	ets (X,Y)	[2:0-0-0,0-0-12], [2:0-2-9,E0	dge]									
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	0.11	4-9	>624	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.39	Vert(CT)	-0.07	4-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	2	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2	2014	Matrix	x-AS						Weight: 23 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 WEDGE
 Left: 2x4 SP No.3

REACTIONS. (Ib/size) 2=320/0-3-0, 4=201/0-1-8 Max Horz 2=92(LC 8) Max Uplift 2=-216(LC 8), 4=-144(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be User Defined crushing capacity of 425 psi.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=216. 4=144.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.



	0-6-8		<u>5-0-0</u> 4-5-8	
Plate Offsets (X,Y)	[2:0-0-0,0-1-0], [2:0-2-9,Edge]		+00	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.24 BC 0.25 WB 0.00 Matrix-AS	DEFL. in (loc) l/defl L/d Vert(LL) 0.05 4-9 >999 240 Vert(CT) -0.03 4-9 >999 240 Horz(CT) -0.01 2 n/a n/a	PLATES GRIP MT20 244/190 Weight: 20 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 WEDGE
 Left: 2x4 SP No.3

REACTIONS. (Ib/size) 2=283/0-3-0, 4=158/0-1-8 Max Horz 2=78(LC 8) Max Uplift 2=-194(LC 8), 4=-113(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be User Defined crushing capacity of 425 psi.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=194, 4=113.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.



REACTIONS. (lb/size) 4=148/4-0-0, 2=213/4-0-0 Max Horz 2=65(LC 9) Max Uplift 4=-56(LC 12), 2=-106(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (9)

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=106.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







2x4 =

_

2x4 =

Rigid ceiling directly applied or 10-0-0 oc bracing

			4-6-11				-
Plate Offsets (X,Y) [[3:0-3-0,0-0-1], [4:0-3-0,0-0-1]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.04 BC 0.07 WB 0.00 Matrix-R	DEFL. i Vert(LL) n/ Vert(CT) n/ Horz(CT) 0.0	n (loc) l/def a - n/a a - n/a) 6 n/a	l L/d a 999 a 999 a n/a	PLATES MT20 Weight: 13 lb	GRIP 244/190 FT = 20%
LUMBER- FOP CHORD 2x4 SP 3OT CHORD 2x4 SP	No.2 No.2		BRACING- TOP CHORD	Structural wo except 2-0-0 oc purli	od sheathing di ns: 3-4.	rectly applied or 4-6-1	1 oc purlins,

4-6-11

BOT CHORD

REACTIONS. All bearings 4-6-11.

(lb) - Max Horz 1=-37(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 6, 2, 5

Max Grav All reactions 250 lb or less at joint(s) 1, 6, 2, 5

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 2, 5.
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.00

5

n/a

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 4-6-11 oc purlins.

NO	TES-	(11)	

BCLL

BCDL

LUMBER-

BOT CHORD

REACTIONS.

0.0

10.0

TOP CHORD 2x4 SP No.2

(lb) -

2x4 SP No.2

All bearings 4-6-11. Max Horz 1=-51(LC 8)

1) Unbalanced roof live loads have been considered for this design.

Rep Stress Incr

Code IRC2015/TPI2014

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 2, 4 Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4 FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WΒ

Matrix-P

0.00

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 2, 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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FT = 20%

Weight: 13 lb



BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

REACTIONS. All bearings 4-6-11.

Max Horz 1=-51(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 2, 4 Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 2, 4.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be User Defined crushing capacity of 565 psi.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 2=142, 4=125.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







- will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 2=142, 4=125.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 4 except (jt=lb) 2=112.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







- will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be User Defined crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 4 except (jt=lb) 2=112.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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