PLANS DESIGNED TO THE

Harnett

COUNTY

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ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or comice vents with the balance of the required ventilation provided by eave or comice vents. As an alternative, the

dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7.

required vertilation provided by eave or conflice vents. As an alternative, the net free cross-ventilation area may be reduced to J300 when a Class I or II vapor retarder is installed on the warmi-newinder side of the ceiling. Exceptions:

1. Enclosed attic/rafter spaces requiring less than 1 selected on 10,929 m2 of vertilation may be vented with continuous soffit vertilation only, 2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only. SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT.

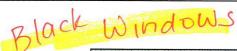
formed where cellings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall

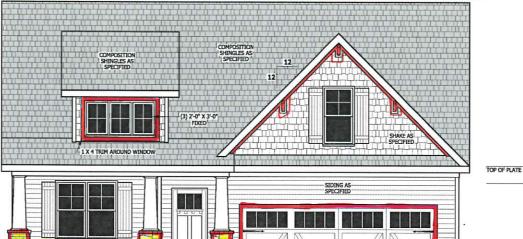
processor against we entones or interest and show, remotern openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire doth screening, herdware doth, or similar material with openings having a least

requirements of Section R802.7.
R866.2. Minimum area. The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by

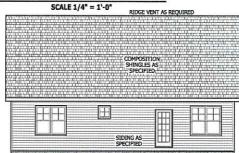
NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0' ABOVE FAVE = 16.51 SOLET. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 8.26 SO.FT.

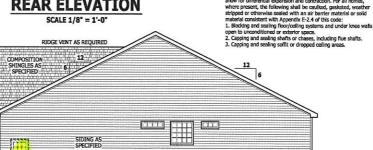




FRONT ELEVATION - A



REAR ELEVATION



RIGHT SIDE ELEVATION

SCALE 1/8" = 1'-0"

CODES AND CONDITIONS HA PARY WITH LOCATION, A LOC DESIGNER, ARCHITECT OR GINEER SHOULD BE CONSULT AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER

⋖ Lauren ELEVATION The

FIRST FLOOR PLATE H

MIND

SUB FLOOR

1791 SQ.FT. 1791 SQ.FT.

148 SQ.FT. 148 SQ.FT.

188 SQ.FT. 469 SO.FT.

160 SQ.FT. 108 SQ.FT. 292 SQ.FT. 560 SQ.FT.

SOUARE FOOTAGE

HEATED OPTIONAL

UNHEATED OPTIONAL

N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit infiltration. The sealing methods between dissimilar materials shall

allow for differential expansion and contraction. For all homes

HEATED

FIRST FLOOR

CAROLINA ROOM TOTAL

UNHEATED FRONT PORCH GARAGE

SCREENED PORCH DECK OR PATIO

THIRD GARAGE

AIR LEAKAGE

20

SQUARE FOOTAGE HEATED HRST ROOR 1791 SQ.FT 1791 90.FT 16 50 FT

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GUARD RAIL NOTES

SECTION NOTE:

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or grade below at any point

Inches (FGZ: mm) measured vertically to the floor or grade below at any politic within 36 inches (914 mm) notrotally to the edge of the open side. Insect screening shall not be considered as a guard.

8112.2 Height. Required guards to open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treass.

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

2. Where the top of the guard also serves as a handrail on the open sides of stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading orders of the tending of the state of the state

not more than as increes you many measured vertically from a line connecting the leading edges of the treads. R31.2.3 Opening limitations. Required guards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter. Exceptions:

1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 Guards on the open sides of stairs shall not have openings which allow

ssage of a sphere 4 3/8 inches (111 mm) in diameter

COMPOSITION SHINGLES AS SIDING AS VENEER AS SPECIFIED

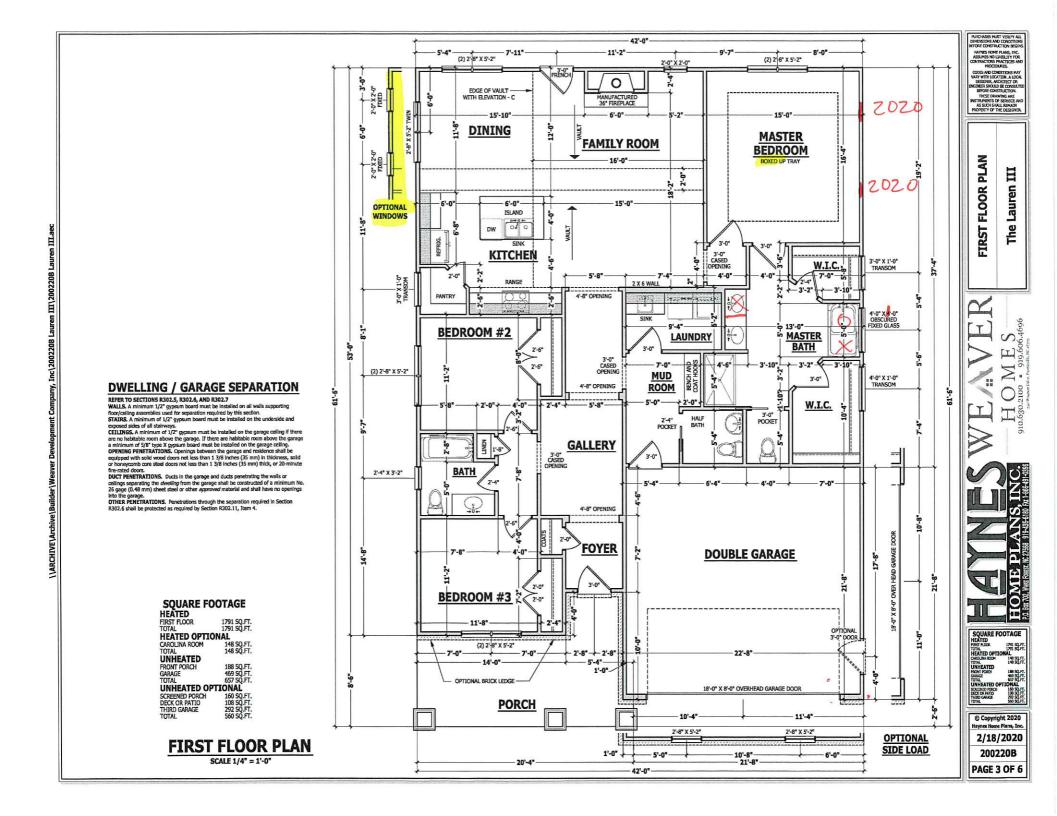
LEFT SIDE ELEVATION

SCALE 1/8" = 1'-0"

RAIL AS NEEDED

BRICK OR STONE

DOOR DOOR



Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	-	-
Guardrail In-fill components	50		-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		-

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS: Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x105 PSI
Parallel strand lumber (PSI) = Fb=2900 PSI, Fv=290 PSI, E=2.0x105 PSI
Laminated strand lumber (LSI) Fb=2250 PSI, Fv=400 PSI, Fe=1.55x105 PSI
Laminated strand lumber (LSI) Fb=2250 PSI, Fv=400 PSI, Fe=1.55x105 PSI

Lauren III\200220B

TRUSS AND 1-JOIST MEMBERS: All roof truss and I-loist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or 1-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel

angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 7 see angle with 5 see angle with 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on ter for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist specing, minimum #Inimum 1/2 discrete to the control of the specing and minimum 3/4"

F: Portal fame per figure R602.10.1 thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum

CONCRETE AND SOILS: See foundation notes

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Pan, Inc. attention before construction begins. KINEE WALL AND CELLING HEIGHTS. All finished lines was heights and ceiling heights are shown furred down 10° from rof dedding for insulation. If for any reason the truss manufacturer falls to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage m vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the sonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics.

BEARING, All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

and floor system thicknesses

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless

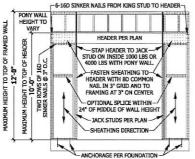
GYPSUM: All interior sides of exterior walls and both sides Interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute per racie ROU.1.1.3. Methods CS-WSP and CS-SHS contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length. HD: 800 lbs hold down hold down device fastened to the edge

of the brace wall panel closets to the corner. Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7° on center at edges and 7° on center at Intermediate supports with minimum 5d cooler pails or #6 screws



PORTAL FRAME AT OPENING

(METHOD PF PER FIGURE AND SECTION R602,10.1) SCALE 1/4" = 1'-0"

EXTERIOR HEADERS

(2) 2 X 6 WITH 1 JACK STUD EACH END **UNI ESS NOTED OTHERWISE** - KING STUDS EACH END PER TABLE BELOW

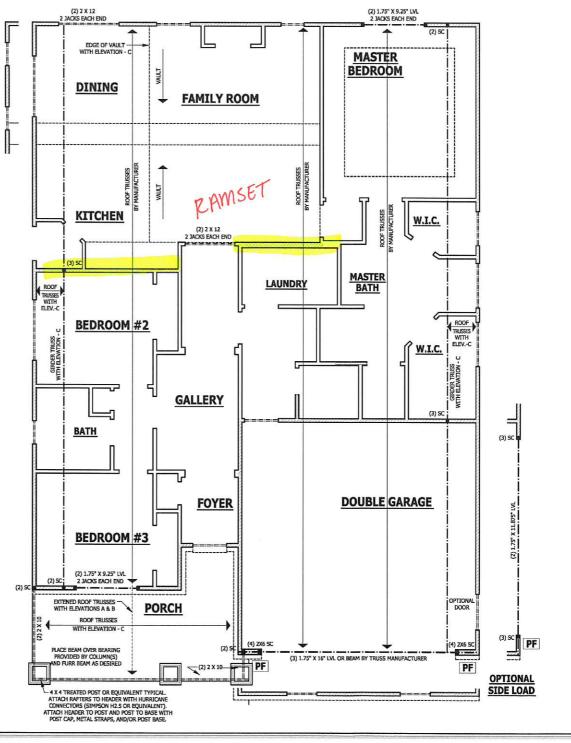
HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6 INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE NON LOAD BEARING HEADERS TO BE

FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0"

LADDER FRAMED



ODES AND CONDITIONS MA RY WITH LOCATION, A LOC DESIGNER, ARCHITECT OR

STRUCTURAL Lauren FLOOR The FIRST

SQUARE FOOTAGE HEATED FIRST BOOK 1781 SQUEE 1791 SQ.FT. 1791 SQ.FT. HEATED OPTIO 148 SQ.FT. UNHEATED 188 SQ.FT 469 SQ.FT 657 SQ.FT UNHEATED OPTIONAL

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PAGE 4 OF 6

ROOF TRUSS REQUIREMENTS

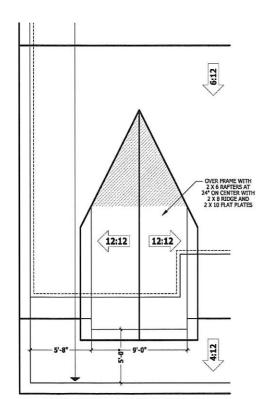
TRUSS DESIGN. Trusses to be designed and engineered in ecoordance with these drawings, any variation with these drawings must be brought to Haynes Home Plan, Jin. attention before construction begins. RNEE WALL AND CEILUNE REIGHTS. All finished know wall heights and calling heights are shown furmed down 10° from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished time wall heights, or finished ceiling heights shown on these derwings the finished square footoge may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the MANCHIBMER of the output of the product.

ANCHORAGE, All required anchors for trusses due to uplift or boaring shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledges; upless noted ethorage.

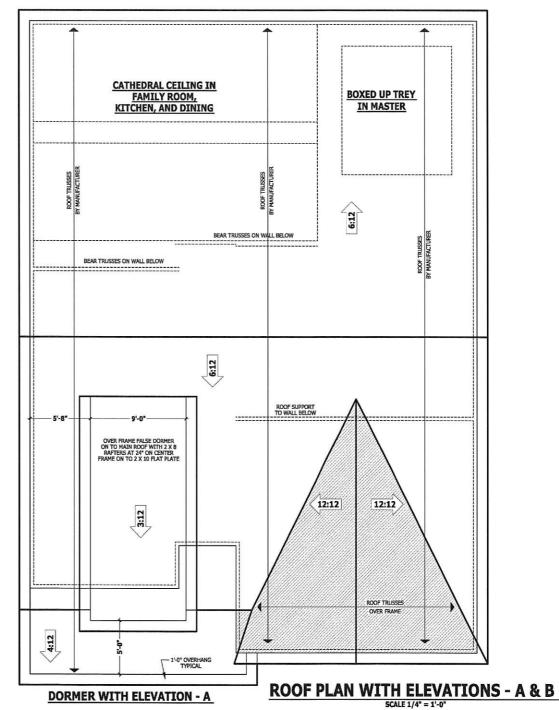
ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE HEEL HEIGHT ABOVE SECOND FLOOR PLATE



DORMER WITH ELEVATION - B



PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION BEGINS HAYNES HOME FLANS, INC. ASSUMES NO LIABILITY FOR

PROCEDURES.

CODES AND CONDITIONS
VARY WITH LOCATION, A L
DESIGNER, ARCHITECT (
ENGINEER SHOULD BE CONS

DESIGNER, ARCHITECT C ENGINEER SHOULD BE COME BEFORE CONSTRUCTOR THESE DRAWING ARE INSTRUMENTS OF SERVICE AS SUCH SHALL REHAD

ROOF PLAN I ELEVATIONS - A & B The Lauren III

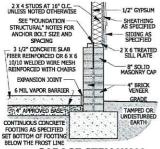
MES WITH

HOME PLANS, INC.

SQUARE FOOTAGE
HEATED
H

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200220B PAGE 5 OF 6 SCALE 3/4" = 1'-0"



GARAGE STEM WALL D SCALE 3/4" = 1'-0"

DECK STAIR NOTES

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer If used, suspended headers shall shall be attached with 3/8 Inch galvanized bolts with nuts and washers to securely support stringers at the too.

DECK BRACING

SECTION AM109

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to provide lateral stability.

AM109.1.1. When the deck floor height is less than 4'-0" above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

AM109.1.2. 4 x 4 wood knee braces may be provided on

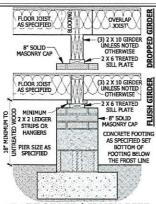
each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be boited to the post and the girder/double band with one 5/8 inch hot dipped nalvanized bolt with our and washer at both ends of the brace per Figure AM109 1

AM109.1.3. For freestanding decks without knee braces or diagonal bracing, lateral stability may be provided by embedding the post in accordance with Figure AM109.2

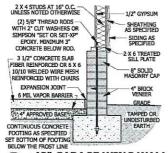
POST	TRIBUTARY	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE
4X4	48 SF	4'-0"	2'-6"	1'-0"
6 X 6	120 SF	6'-0"	3'-6"	11-8"

be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 v 6's shall be attached to the posts with one 5/8 Inch hot dipped gaivanized bolt with nut and washer at each end of each bracing member per Figure AM109.3.

AM109.1.5. For embedment of piles in Coastal Regions, see Chanter 45.



DROPPED/ FLUSH PIER B SCALE 3/4" = 1'-0"



<48" GARAGE WING WALL SCALE 3/4" = 1'-0"

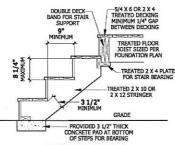


FIGURE AM110 TYPICAL DECK STAIR DETAIL

SHEATHING-

LATH

SEE FOUNDATION

FOR FOUNDATION

DETAILS

WEEP SCREED

SCALE 3/4" = 1'-0"

SCALE 3/4" = 1'-0"

STONE VEENER

AS SPECIFIED

APOR BARRIER

WEEP SCREED

MINIMUM 4" TO

GROUND OR 2 -TO PAVEMENT

GRADE

All weep screeds and stone veneer to be Installed per manufactures instructions and per the 2012 North Carolina Residential

R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvantzed sheet gage) corrosion-resistant weep screed or plastic ween screed, with a minimum vertical chment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 Inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier sha lan the attachment flange. The exterior lath

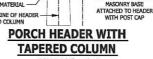
WEEP SCREEDS

shall cover and terminate on the attachment flange of the weep screed.

2 X 4 STUDS AT 16" O.C. -UNLESS NOTED OTHERWISE 1/2" GYPSUM 12 SUB FLOOR AS-PLAN OR ELEVATION SPECIFIED FOR PITCH 2 X RIM JOIST FLOOR JOIST MASONRY CAP ROOF TRUSSES BY MANUFACTURER 4" CONCRETE PORCH HEADER PER -BRICK VENEER SEE "FOUNDATION PLAN INSTALLED OVER EXPANSION 10INT STRUCTURAL" NOTES FOR CENTER OF COLUMN BASE ANCHOR BOLT SIZE AND - 6 MIL VAPOR BARRIER BLOCKING INSTALLED-ON BOTH SIDES & UNDER 3 1/2" SLAB HEADER AS DESIRED CONTINUOUS CONCRETE 1 X MATERIAL TAMPED OR FOOTING AS SPECIFIED CENTER LINE OF HEADER SET BOTTOM OF FOOTING UNDISTURBED AND COLUMN EARTH

CRAWL SPACE AT GARGE SCALE 3/4" = 1'-0"

SMOKE ALARMS



- EDGED OR PORCH FLOOR

SHINGLES AS SPECIFIED

SHEATHING AS SPECIFIED

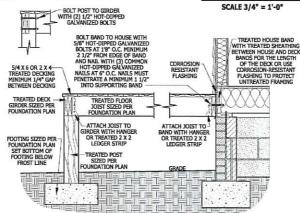
- 15# BUILDING FELT

- 2 X 6 SUB FASCIA

-VINYL OR HARDIE SOFFIT

INSTALLED PER MANUFACTURERS

TAPERED COLUMN OVER



DECK ATTACHMENT DETAIL TO FRAMED WALL

SCALE 3/4" TO 1'-0"

ion and notification. All smoke alarms shall be

listed in accordance with UL 217 and installed in accordance with

per mixed. The nobelednative dearns system shall provide use sam level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification

device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an approved supervising station and be maintained in accordance with

requirements of Section R314.4.
R314.3 Location. Smoke alarms shall be installed in the following

2. Dutside each separate sleeping area in the immediate vicinity of

the becorous.

3. On each additional story of the dwelling, including basements and habitable attics (finished) but not including crawl spoces, uninhabitable (unfinished) attics and uninhabitable (unfinished) attics and uninhabitable (unfinished) attics and uninhabitable (unfinished) attics of uninhabitable (unfinished) attics with split tevels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent.

ower level provided that the lower level is less than one full story

below the upper level.

When more than one smoke alarm is required to be installed within

an Individual dwelling unit the elarm devices shall be interconnected

in such a manner that the actuation of one alarm will activate all of

In such a manner that the actuation of one alarm will activate all or the alarms in It includious Incl. R314.4 Power source. Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commendal source, and when primary power is interrupted, shall receive power from a bettery. Wiring shall be personnent and without a disconnecting switch other than those required for constructions of the primary power is the property of the property of the primary power is the property of the primary power in the property of the property o

rrent protection. Smoke alarms shall be interconnected

Exception: Where smoke alarms are provided meeting the

1. In each sleeping room.

STAIRWAY NOTES

R311.7.2 Headroom. The minimum headroom in all parts of the stairway R311.7.2 Headroom. The firm minimum headroom in all parts of the stainway shall not be less that feet 8 Inches (2003 mm) measured vertically from the stoped line adjoining the tread noising or from the floor surface of the landing or platform on that portion of the stainway. R311.7.4 Stair treade of risers. Stort purpose and risers shall meet the requirements of this section, For the turpose or this section all dimensions.

and dimensioned surfaces shall be exclusive of carpets, ruos or runners. (210 mm). The riser shall be measured vertically between leading edges of

the provisions of this code and the household fire warning equipment provisions of NPA 27. R334.2 Smoka detection systems. Household fire alarm systems installed in accordance with NPA 27 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same. the adjacent treads R311.7.4.2 Tread deoth. The minimum tread deoth shall be 9 Inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 Inches (220 mm) measured as above at a point 12 Inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

minimum tread depth of 4 inches (102 mm) at any point. R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greate than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stal

R311.7.7 Handrails. Handrails shall be provided on at least one side of each

continuous run of treads or flight with four or more risers.

R311.7.1 Height. Handral height, measured vertically from the sloped plane adjoining the tread nosing, or flinks surface of ramp slope, shall be not less than 34 inches (864 mm) and not more than 38 inches (965 mm).

1. The use of a volute, turnout or starting easing shall be allowed over the

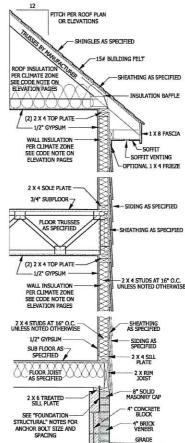
lowest tread.

2. When handrall fittings or bendings are used to provide continuous transition between flights, the transition from handrall to guardrall, or used at the start of a flight, the handrall height at the fittings or bendings shall

between the wall and the handralls.

Handralls shall be permitted to be interrupted by a newel post.
 The use of a volute, turnout, starting easing or starting newel shall be

allowed over the lowest tread. 3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.

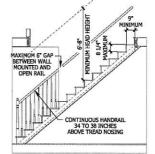


BELOW THE FROST LINE TYPICAL WALL DETAIL SCALE 3/4" = 1'-0

CONTINUOUS CONCRETE

FOOTING AS SPECIFIED

SET BOTTOM OF FOOTING



TYPICAL STAIR DETAIL

HAYNES HOME PLANS, INC ODES AND CONDITIONS MAY DESIGNER, ARCHITECT OR IGNEER SHOULD BE CONSULT BEECOSE CONSTRUCTION THESE DRAWING ARE STRUMENTS OF SERVICE AN AS SUCH SHALL REMAIN ROPERTY OF THE DESIGNER

Ħ DETAIL Lauren TYPICAL The

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SQUARE FOOTAGE HEATED FIRST ROOK 1791 SQ.FT 1791 SO.F HEATED OPTION 148 SOLF UNHEATED

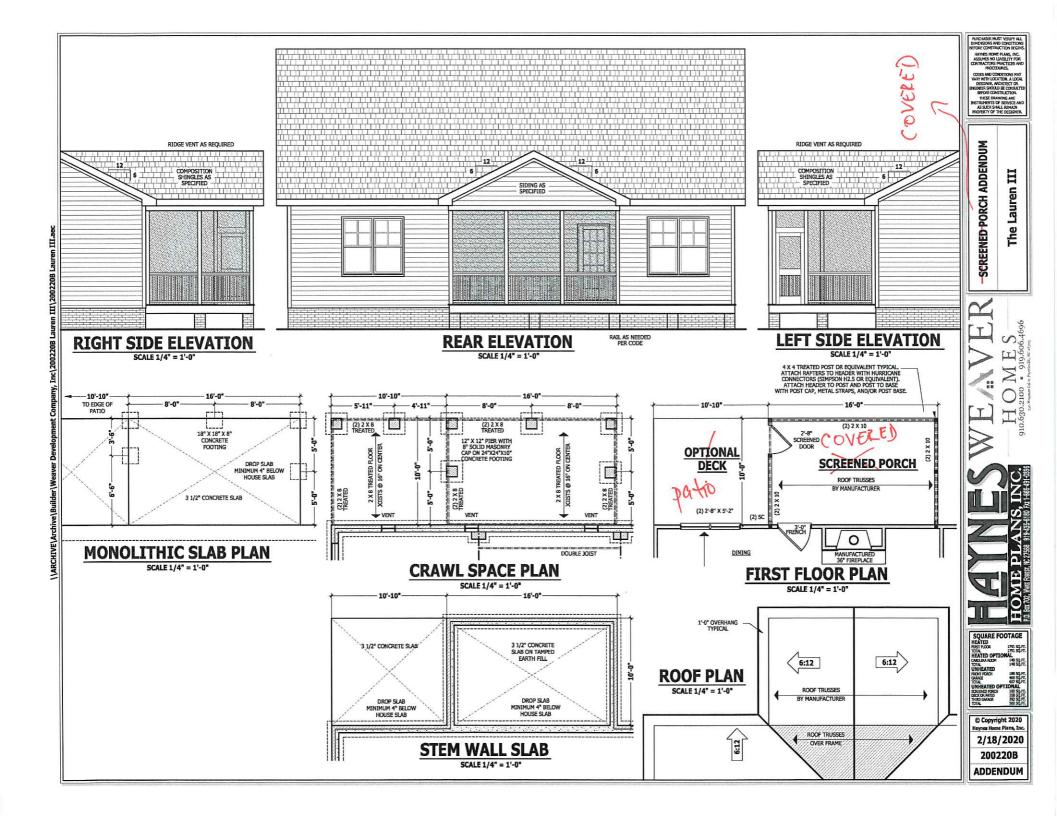
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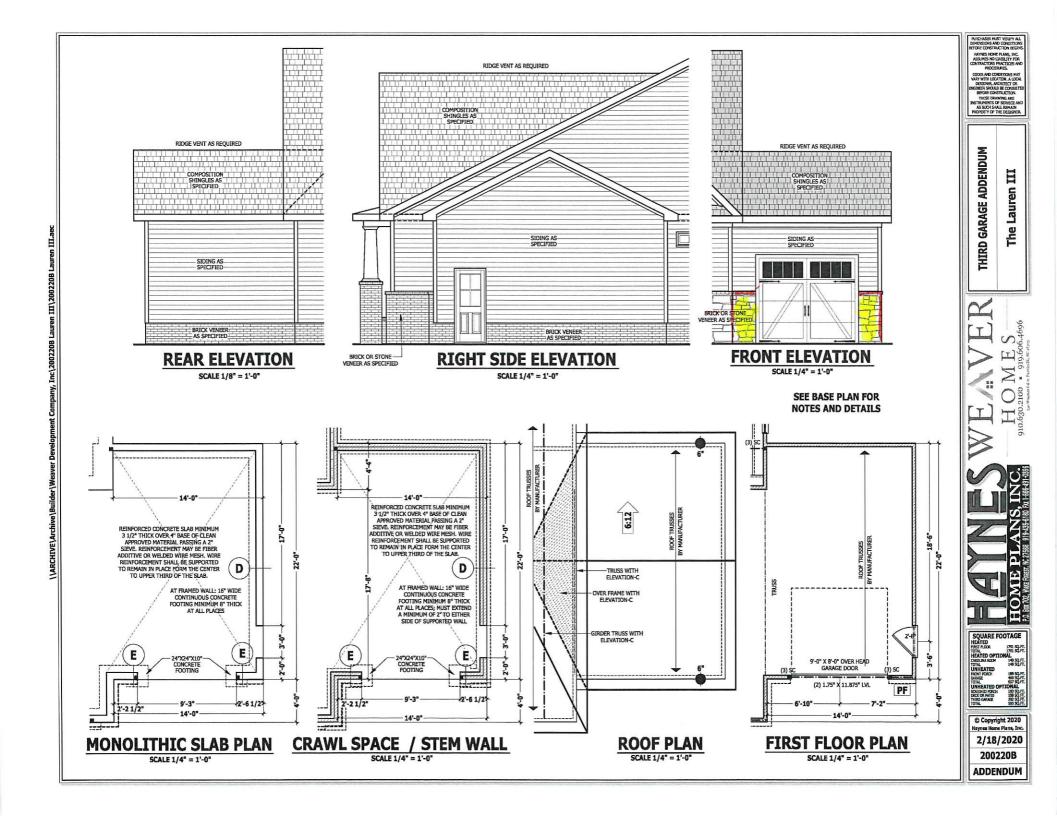
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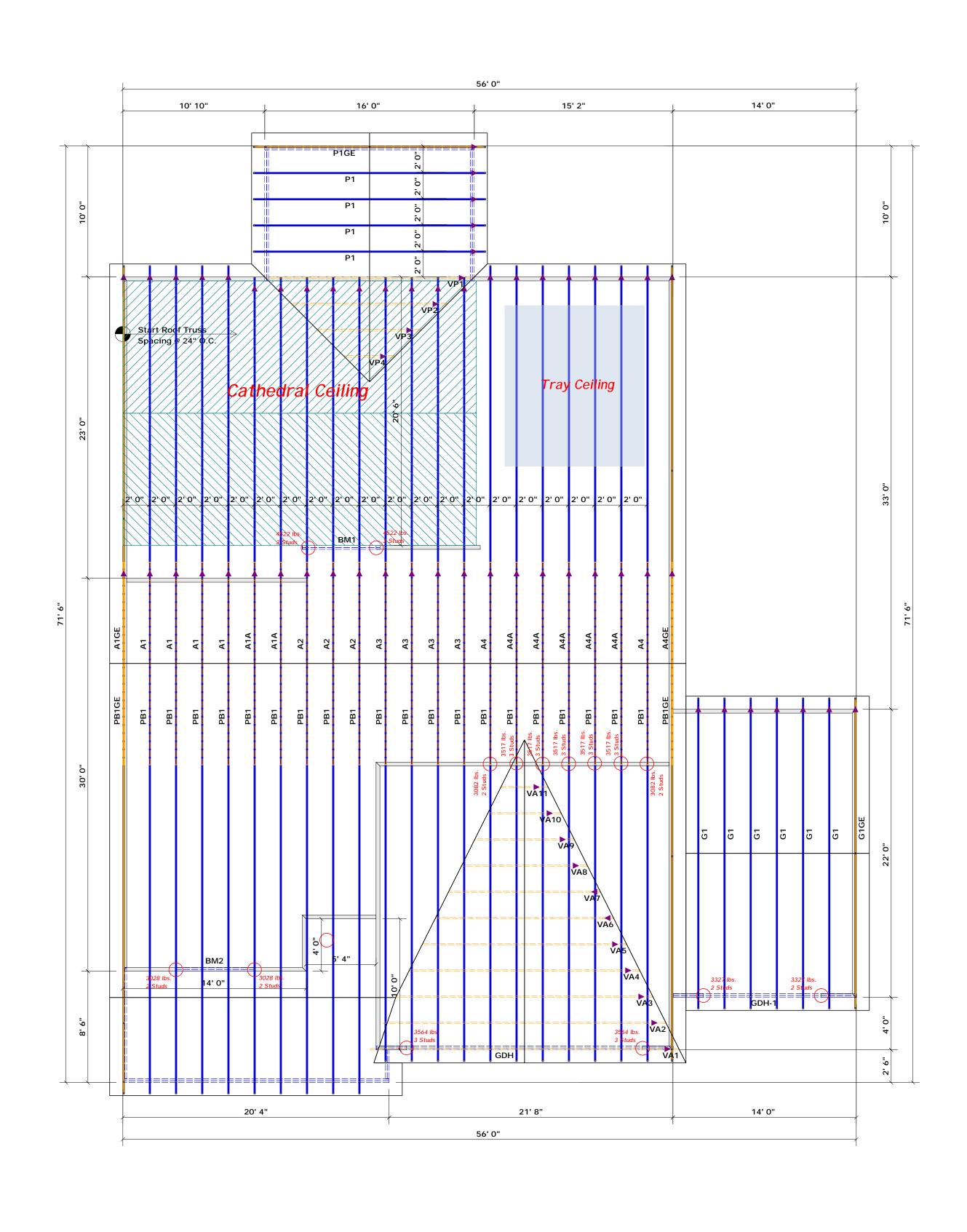
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III.ae

PAGE 6 OF 6







▲ = Denotes Left End of Truss(Reference Engineered Truss Drawing)Do Not Erect Trusses Backwards

3400 !

6600 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(0.45% ON HARDS 85025(), 4-6))

MARICA OF JACK STUDO ACCUME(D. 9) CA CAS OF FEADER 675003

BYO DEACTION (LP TO) (ACT STUDS FOR

2550 1 5100 2

7650 3

10200 4 12750 5

15300 6

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

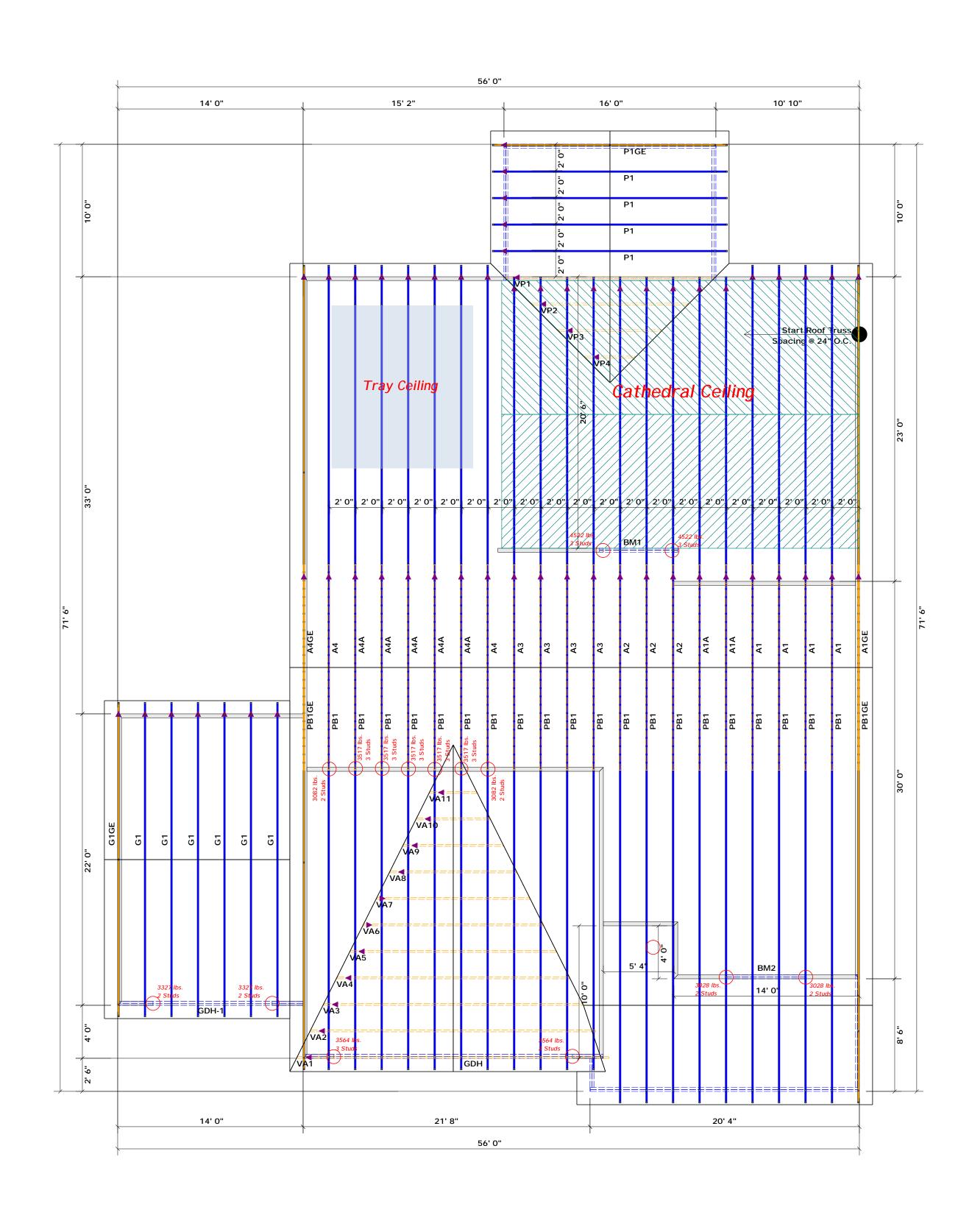
Beam Legend PlotID Length Product Plies Net Qty 6' 0" BM1 1-3/4"x 9-1/4" LVL Kerto-S 2 2 6' 0" 2 BM2 1-3/4"x 9-1/4" LVL Kerto-S GDH-1 14' 0" 1-3/4"x 11-7/8" LVL Kerto-S 2 1-3/4"x 16" LVL Kerto-S 2 2 GDH 23' 0"

ISS	es Backwards			SCALE: 3/16" = 1'	23°0° 1-3/4"X 16° LVL Kei
	BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incomposed the building designer. See individual sheets for each truss design identified on the placement drawing. The building designer.
2000	JOB NAME	Lot 1-E Murray Farm	ADDRESS	Lot 1-E Murray Farm	is responsible for temporary and permanent bracing of the roof and floor the overall structure. The design of the truss support structure including walls, and columns is the responsibility of the building designer. For gen regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss of
398%	PLAN	Lauren III / 3rd Car / CP	MODEL	Model	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comp prescriptive Code requirements. The contractor shall refer to the a
	SEAL DATE	11/7/18	DATE REV.	02/11/20	(derived from the prescriptive Code requirements) to determine the foundation size and number of wood studs required to support reach than 3000# but not greater than 15000#. A registered design profes be retained to design the support system for any reaction that exc
	QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional retained to design the support system for all reactions that exceed
	JOB #	J0220-0596	SALESMAN	Lenny Norris	signature Curtis Quick

Truss Placement Plan



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444



▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BANFO ON 1 MBERS (2502-51)) & (b)()
NUMBER OF JACK STUDG REQUIRE(DIR) (A CND OF FEADER/STEGER

2550 1 5100 2 7650 3

10200 4 12750 5 15300 6

| Total | Tota

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan

PlotID	Length	Product	Plies	Net Qty
BM1	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH	23' 0"	1-3/4"x 16" LVL Kerto-S	2	2

BUILDER	Weaver Development	COUNTY	Harnett	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer
JOB NAME	Lot 1-E Murray Farm	ADDRESS	Lot 1-E Murray Farm	is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package
PLAN	Lauren III / 3rd Car / CP	MODEL	Model	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables
SEAL DATE	11/7/18	DATE REV.	02/11/20	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those
QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.
JOB #	J0220-0596	SALESMAN	Lenny Norris	Signature Curtis Quick

SCALE: 3/16" = 1'



Fayetteville, N.C. 28309

Phone: (910) 864-8787 Fax: (910) 864-4444

Client: Weaver Development

Project: Address:

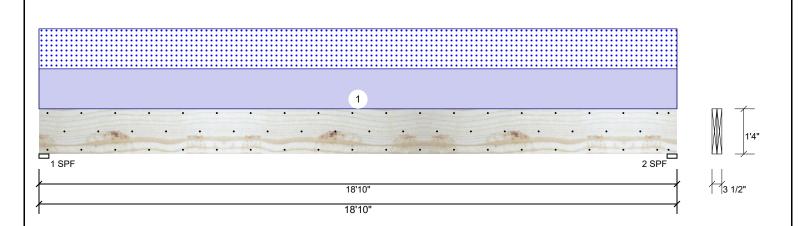
Date: 2/11/2020

Designer: Curtis Quick Job Name: The Lauren III Beams Page 1 of 8

Project #:

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL**

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Live Wind Const Type: Floor Dead Snow Plies: 2 Design Method: ASD 0 1840 1723 0 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 0 1840 1723 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal Temp <= 100°F Temperature: **Bearings** Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" D+S 1840 / 1723 3564 L 2 - SPF 3.500" 68% 1840 / 1723 3564 L D+S

Analysis Results

-						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16009 ft-lb	9'5"	39750 ft-lb	0.403 (40%)	D+S	L
Unbraced	16009 ft-lb	9'5"	16016 ft-lb	1.000 (100%)	D+S	L
Shear	2976 lb	17'3 3/8"	13739 lb	0.217 (22%)	D+S	L
LL Defl inch	0.213 (L/1035)	9'5 1/16"	0.460 (L/480)	0.460 (46%)	S	L
TL Defl inch	0.441 (L/501)	9'5 1/16"	0.613 (L/360)	0.720 (72%)	D+S	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 7'4 1/2" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	183 PLF	0 PLF	183 PLF	0 PLF	0 PLF	A4A
	Self Weight				12 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 3071 Commerce Dr, Suite E Fort Gratiot, MI 48059

(800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





Client:

Project: Address: Weaver Development

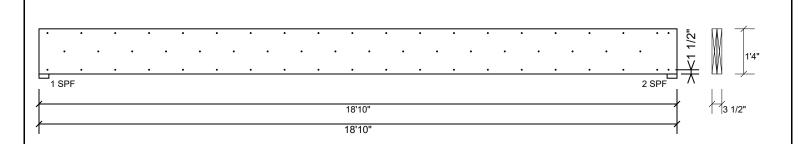
2/11/2020 Designer:

Curtis Quick Job Name: The Lauren III Beams Page 2 of 8

Project #:

1.750" X 16.000" 2-Ply - PASSED **Kerto-S LVL**

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

1 3		•	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	245.6 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood

3071 Commerce Dr, Suite E Fort Gratiot, MI 48059 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





Client:

Project: Address: Weaver Development

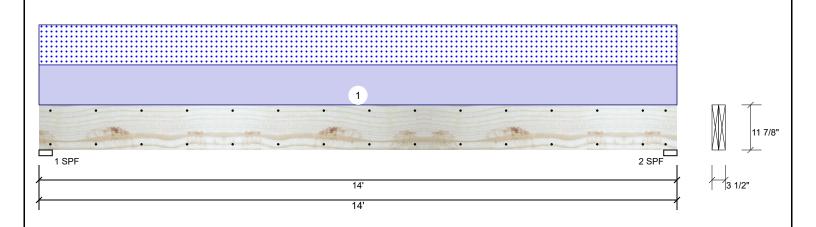
2/11/2020 Designer:

Curtis Quick Job Name: The Lauren III Beams Page 3 of 8

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL**

Level: Level



Member Infor	mation			Reactio	ns UNPAT	TERNED I	(Uplift)		
Туре:	Girder	Application:	Floor	Brg	Live	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	0	1696	1631	0	0
Moisture Condition	n: Dry	Building Code:	IBC 2012	2	0	1696	1631	0	0
Deflection LL:	480	Load Sharing:	No						
Deflection TL:	360	Deck:	Not Checked						
Importance:	Normal								
Temperature:	Temp <= 100°F								
				Bearing	js				
				Bearing	Length	Cap. Rea	ct D/L lb	Total Ld. Case	e Ld. Comb.
				1 - SPF	3.500"	64% 169	96 / 1631	3327 L	D+S
				2 - SPF	3.500"	64% 169	96 / 1631	3327 L	D+S

Analysis Results

Analysis Actual Location Allowed Capacity Comb.	Case
Moment 10893 ft-lb 7' 22897 ft-lb 0.476 (48%) D+S	L
Unbraced 10893 ft-lb 7' 10911 ft-lb 0.998 D+S (100%)	L
Shear 2747 lb 1'2 5/8" 10197 lb 0.269 (27%) D+S	L
LL Defl inch 0.195 (L/832) 7' 1/16" 0.339 (L/480) 0.580 (58%) S	L
TL Defl inch 0.398 (L/408) 7' 1/16" 0.451 (L/360) 0.880 (88%) D+S	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 8'2 5/8" o.c.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	233 PLF	0 PLF	233 PLF	0 PLF	0 PLF	G1
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- IARIGUING & INSTALLATION

 LVL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beams trength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained.

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood 3071 Commerce Dr, Suite E Fort Gratiot, MI 48059 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





Client: Weaver Development

Project:

Address:

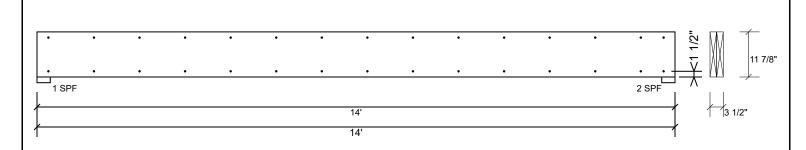
2/11/2020

Designer: Curtis Quick Job Name: The Lauren III Beams Page 4 of 8

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL**

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

		`	,
Capacity	0.0 %		
Load	0.0 PLF		
Yield Limit per Foot	163.7 PLF		
Yield Limit per Fastener	81.9 lb.		
Yield Mode	IV		
Edge Distance	1 1/2"		
Min. End Distance	3"		
Load Combination			
Duration Factor	1.00		

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood

3071 Commerce Dr, Suite E Fort Gratiot, MI 48059 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633







Client:

Project: Address: Weaver Development

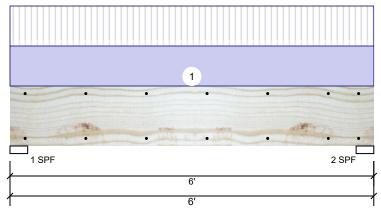
2/11/2020 Designer: Curtis Quick

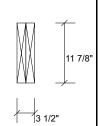
Job Name: The Lauren III Beams

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL**







Page 5 of 8

Member	Information
Type:	Girder

Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance:

Normal Temperature: Temp <= 100°F

Building Code: Load Sharing: Deck:

Application: Design Method:

Not Checked

ASD

No

IBC 2012

Reactions	UNPAT	TERNED	lb	(Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	2247	2275	0	0	0
2	2247	2275	0	0	0

Bearings

Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF 3.500"	87% 2275 / 2247	4522 L	D+L
2 - SPF 3500"	87% 2275 / 2247	4522 I	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5786 ft-lb	3'	19911 ft-lb	0.291 (29%)	D+L	L
Unbraced	5786 ft-lb	3'	14445 ft-lb	0.401 (40%)	D+L	L
Shear	2685 lb	1'2 5/8"	8867 lb	0.303 (30%)	D+L	L
LL Defl inch	0.024 (L/2743)	3'	0.139 (L/480)	0.170 (17%)	L	L
TL Defl inch	0.049 (L/1363)	3'	0.185 (L/360)	0.260 (26%)	D+L	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Top	749 PLF	749 PLF	0 PLF	0 PLF	0 PLF	A2	

Self Weight 9 PLF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood 3071 Commerce Dr, Suite E Fort Gratiot, MI 48059 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





Client:

Project: Address:

Weaver Development

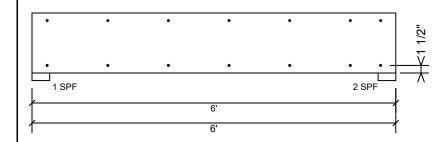
2/11/2020

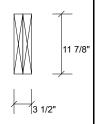
Designer: Curtis Quick Job Name: The Lauren III Beams

Project #:

Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level





Page 6 of 8

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

rasterrain pries asing 2 rows	or roa box rians (. 120x5) at
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 3071 Commerce Dr, Suite E Fort Gratiot, MI 48059 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info





Client:

Project: Address: Weaver Development

Date: 2/11/2020 Designer:

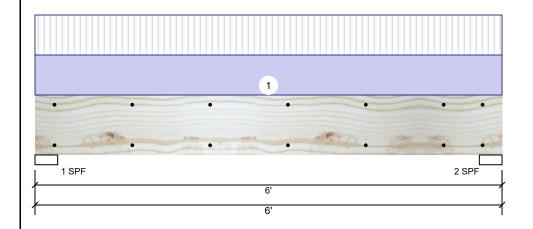
Curtis Quick

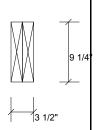
Job Name: The Lauren III Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL**

Level: Level





Page 7 of 8

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal Temperature:

Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind	Const
1	1503	1525	0	0	0
2	1503	1525	0	0	0

Bearings

Bearing Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF 3.500"	58% 1525 / 1503	3028 L	D+L
2 SDE 3500"	58% 1525 / 1503	3028 I	D+I

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3874 ft-lb	3'	12542 ft-lb	0.309 (31%)	D+L	L
Unbraced	3874 ft-lb	3'	10359 ft-lb	0.374 (37%)	D+L	L
Shear	2018 lb	5'	6907 lb	0.292 (29%)	D+L	L
LL Defl inch	0.030 (L/2226)	3'	0.185 (L/360)	0.160 (16%)	L	L
TL Defl inch	0.060 (L/1105)	3'	0.277 (L/240)	0.220 (22%)	D+L	L

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width

, <u></u> a.o.a.	ololiacilloco latto bacca cil	omigio più maun									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Top	501 PLF	501 PLF	0 PLF	0 PLF	0 PLF	a1	

Self Weight 7 PLF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding Manufacturer Info

Metsä Wood 3071 Commerce Dr, Suite E Fort Gratiot, MI 48059 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633





Client: Weaver Development

Project: Address:

Date: 2/11/2020

Designer: Curtis Quick Job Name: The Lauren III Beams

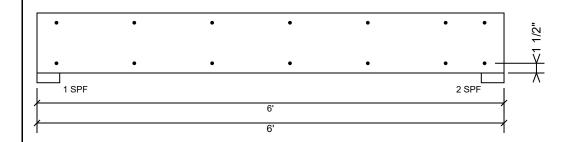
Project #:

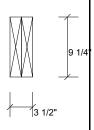
Kerto-S LVL

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 8 of 8

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

	` ,
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood 3071 Commerce Dr, Suite E Fort Gratiot, MI 48059 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633



