

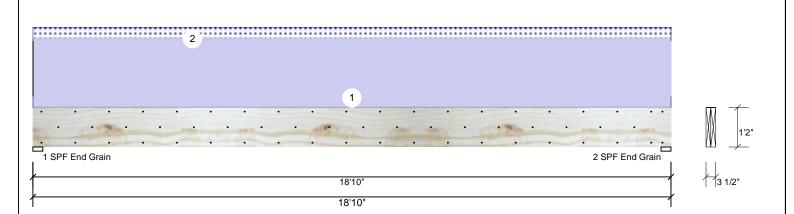
Client: Weaver Development Project: Sinclair (190320B) Address: Sinclair (190320B) Date: 4/15/2020

Input by: Christine Shivy

Job Name: GDH Project #:

1.750" X 14.000" **Kerto-S LVL** 2-Ply - PASSED **GDH**

Level: Level



Member Information								Reactions UNPATTERNED Ib (Uplift)					
Type:	Girder		Applicat	ion: F	loor		Brg	Live	Dea	d Snow	1	Wind	Const
Plies:	2		Design I	Method: A	SD		1	0	259	8 377		0	0
Moisture Cond	ition: Dry		Building	Code: IE	3C 2012		2	0	259	8 377		0	0
Deflection LL:	480		Load Sh	aring: N	0								
Deflection TL:	360		Deck:	N	ot Checked								
Importance:	Normal												
Temperature:	Temp <= 10	0°F											
							Bearings	S					
							Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	3.500"	28%	2598 / 377	2975	L	D+S
							End						
Analysis Res	ults						Grain						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2 - SPF	3.500"	28%	2598 / 377	2975	L	D+S
Moment	11644 ft-lb	9'5"	24299 ft-lb	0.479 (48%) D	Uniform	End Grain						
Unbraced	13332 ft-lb	9'5"	13339 ft-lb	0.999 (100%)	D+S	L							
Shear	2213 lb	1'4 3/4"	9408 lb	0.235 (24%) D	Uniform							

1

Design Notes

1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".

9'5 1/16" 0.459 (L/480) 0.150 (15%) S

9'5 1/16" 0.612 (L/360) 0.880 (88%) D+S

- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top must be laterally braced at a maximum of 7'8 5/8" o.c.
- 6 Bottom braced at bearings.

LL Defl inch 0.068 (L/3239)

TL Defl inch 0.538 (L/410)

7 Lateral slenderness ratio based on single ply width.

H											
	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
l	1	Uniform			Тор	225 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Exterior Siding / Plywood
١	2	Uniform			Тор	40 PLF	0 PLF	40 PLF	0 PLF	0 PLF	2'0" Roof Load
		Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be out or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 2/26/2023

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us ICC-ES: ESR-3633

Manufacturer Info

Comtech, Inc. 1001 S. Reilly Road, Suite #639 Fayetteville, NC USA 28314 910-864-TRUS



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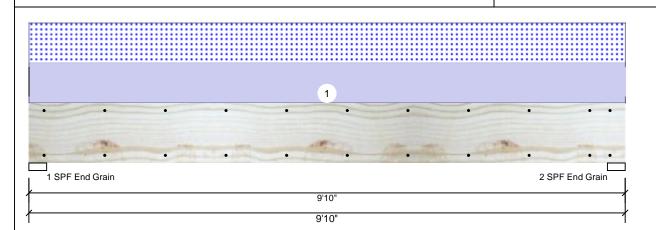
Job Name: GDH-3

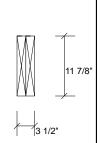
Level: Level

Project #:

GDH-3 **Kerto-S LVL** 1.750" X 11.875"

2-Ply - PASSED





D+S

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Member Information						
Type:	Girder					
Plies:	2					
Moisture Condition:	Dry					
Deflection LL:	480					
Deflection TL:	360					
Importance:	Normal					
Temperature:	Temp <= 100°F					

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reactions UNPAITERNED ID (Uplift)										
	Brg	Live	Dead	Snow	Wind	Const				
	1	0	1422	1377	0	0				
	2	0	1422	1377	0	0				

Analysis Results Capacity Analysis Actual Location Allowed Comb. Case 0.273 (27%) D+S Moment 6254 ft-lb 4'11" 22897 ft-lb L Unbraced 6254 ft-lb 4'11" 9857 ft-lb 0.634 (63%) D+S L 2105 lb 10197 lb 0.206 (21%) D+S Shear 1'2 5/8" ī LL Defl inch 0.058 (L/1928) 4'11" 0.234 (L/480) 0.250 (25%) S L TL Defl inch 0.119 (L/948) 4'11" 0.312 (L/360) 0.380 (38%) D+S L

Bearings Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" 1422 / 1377 2799 L D+S End

Grain 1422 / 1377 2 - SPF 3.500" 2799 L End Grain

Design Notes

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID Trib Width Side Dead 0.9 Load Type Location Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 1 Uniform Top 280 PLF 0 PLF 280 PLF 0 PLF 0 PLF

> Self Weight 9 PLF

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