

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2000131-2000131A JRT Renee 1502

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I40210533 thru I40210554

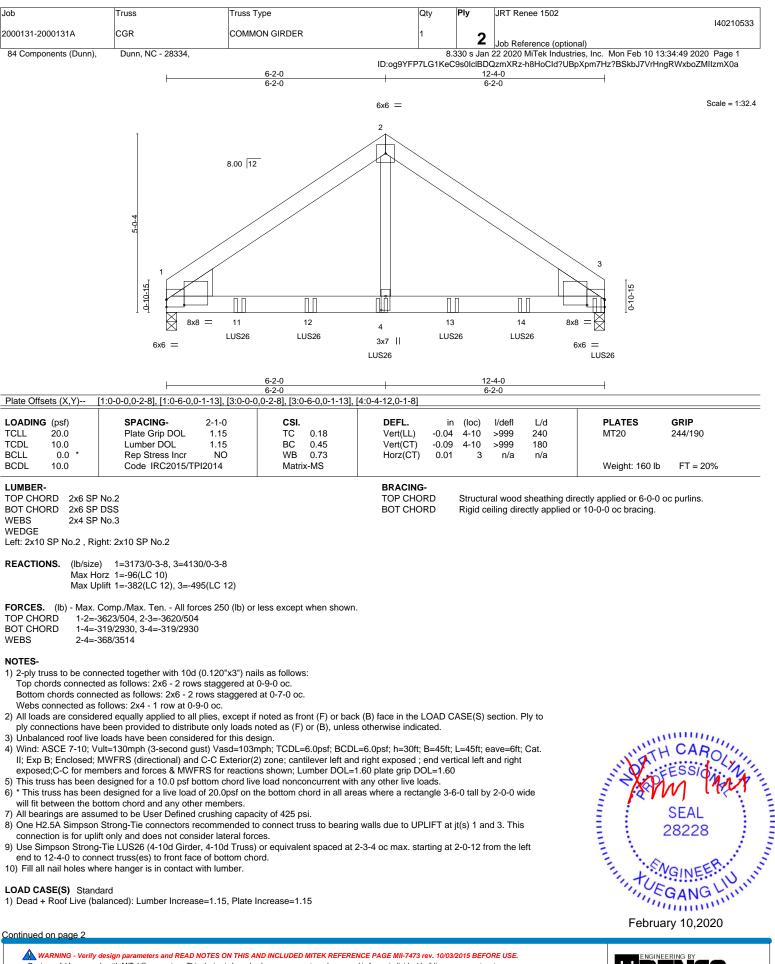
My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



February 10,2020

Liu, Xuegang IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss we band/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	JRT Renee 1502			
	000				140210533			
2000131-2000131A	CGR	COMMON GIRDER	1	2	lab Datawara (antianal)			
				_	Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,		8.	330 s Jan	22 2020 MiTek Industries, Inc. Mon Feb 10 13:34:49 2020 Page 2			
		ID:og9YFP7LG1KeC9s0IcIBDQzmXRz-h8HoCId?UBpXpm7Hz?BSkbJ7VrHngRWxboZMIIzmX0a						

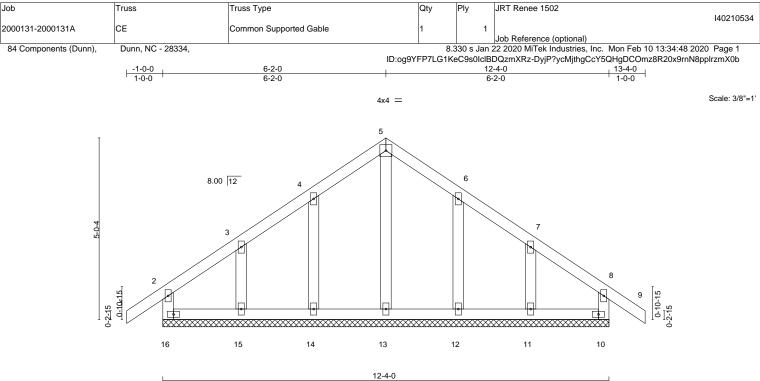
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-62, 2-3=-62, 5-8=-21 Concentrated Loads (lb)

Vert: 4=-1045(F) 10=-1052(F) 11=-1045(F) 12=-1045(F) 13=-1045(F) 14=-1045(F)





			12-4-0		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/de	fl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL) -0.00 9 n	/r 120	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) -0.00 9 n	/r 90	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT) 0.00 10 n/	a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-R	· · ·		Weight: 67 lb FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 12-4-0.

(lb) - Max Horz 16=-130(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 15, 12, 11

Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

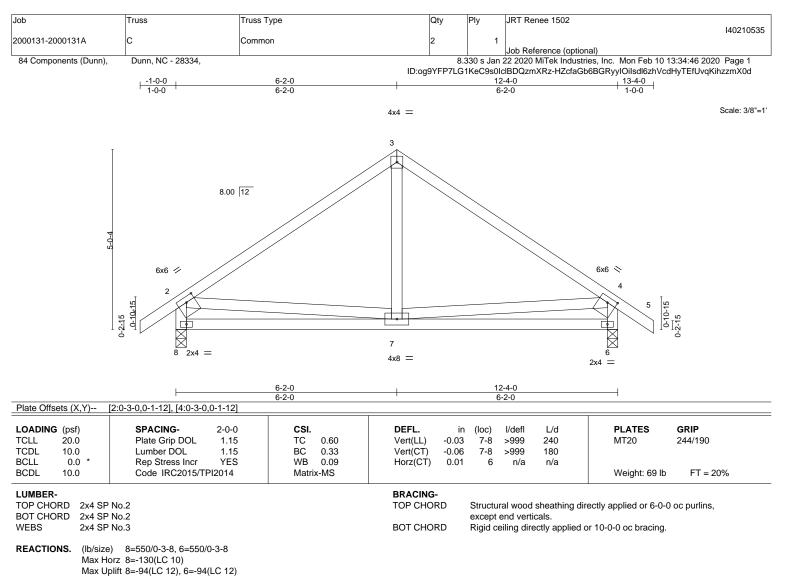
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 10) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 11) Bearing at joint(s) 16, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16, 10, 14, 15, 12, and 11. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. TRENCIO AMITEK Atfiliate 818 Soundside Road

Edenton, NC 27932



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-513/110, 3-4=-513/110, 2-8=-497/172, 4-6=-497/172

BOT CHORD 7-8=-138/372, 6-7=-119/345

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

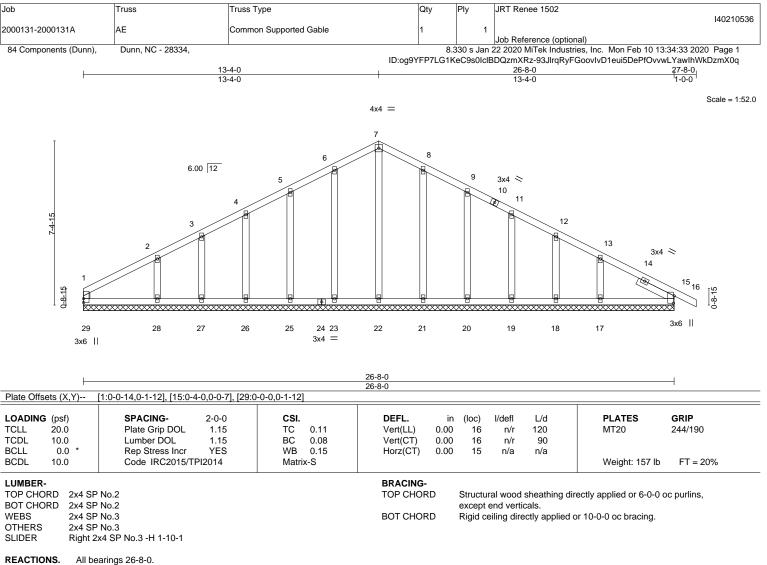
5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) Bearing at joint(s) 8, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 6. This connection is for uplift only and does not consider lateral forces.







(lb) - Max Horz 29=-151(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 23, 25, 26, 27, 28, 21, 20, 19, 18, 17, 15

Max Grav All reactions 250 lb or less at joint(s) 29, 22, 23, 25, 26, 27, 21, 20, 19, 18, 15 except 28=261(LC

17), 17=252(LC 18)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 6-7=-125/255, 7-8=-125/255

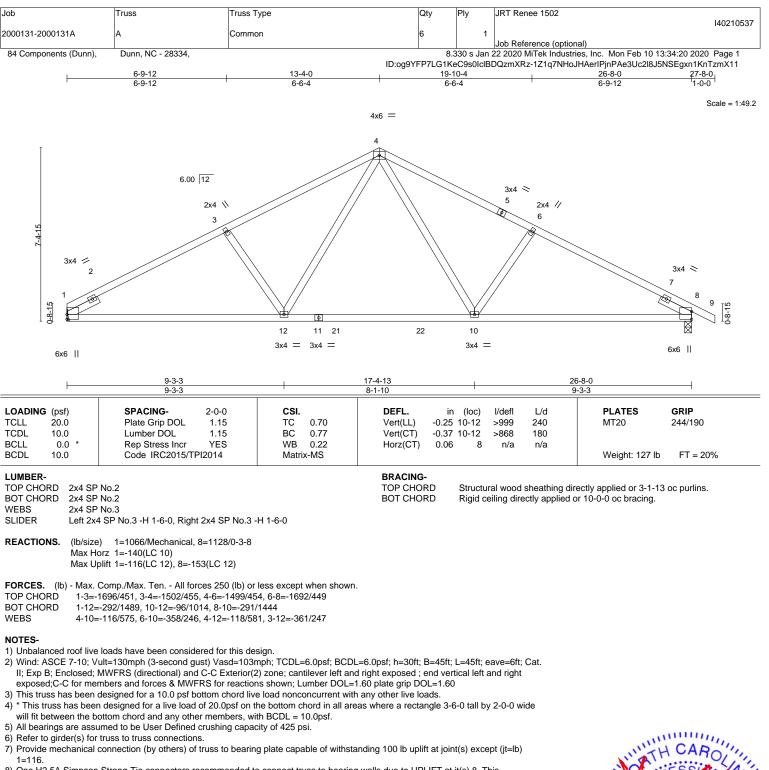
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 23, 25, 26, 27, 28, 21, 20, 19, 18, 17, and 15. This connection is for uplift only and does not consider lateral forces.



February 10,2020

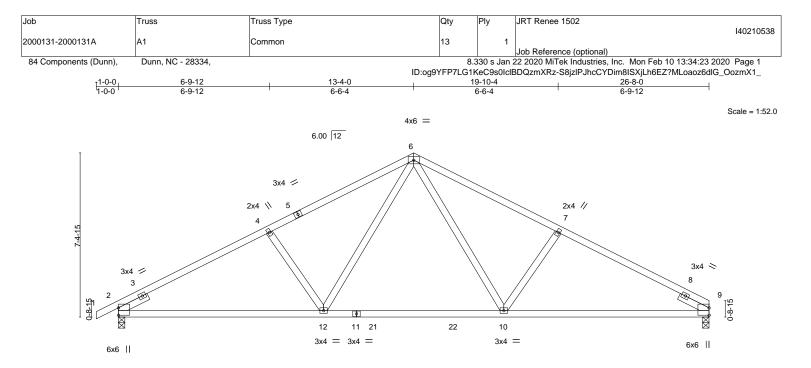




One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This
connection is for uplift only and does not consider lateral forces.







ŀ	<u>9-3-3</u> 9-3-3		17-4-13 8-1-10		<u>26-8-0</u> 9-3-3		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.70 BC 0.77 WB 0.22	DEFL. in (loc Vert(LL) -0.25 10-1: Vert(CT) -0.37 10-1: Horz(CT) 0.06	2 >999 240	PLATES GRIP MT20 244/190		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS			Weight: 127 lb FT = 20%		
LUMBER-			BRACING-				

TOP CHORD

BOT CHORD

LUMBER-

- TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD
- 2x4 SP No.3 WEBS
- SLIDER Left 2x4 SP No.3 -H 1-6-0, Right 2x4 SP No.3 -H 1-6-0

REACTIONS. (lb/size) 9=1066/0-3-8, 2=1128/0-3-8 Max Horz 2=140(LC 11) Max Uplift 9=-116(LC 12), 2=-153(LC 12)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- 2-4=-1692/449, 4-6=-1499/454, 6-7=-1502/455, 7-9=-1696/451 TOP CHORD
- BOT CHORD 2-12=-309/1478, 10-12=-115/1008, 9-10=-311/1449
- WEBS 6-10=-118/581, 7-10=-361/247, 6-12=-116/575, 4-12=-358/246

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9 and 2. This connection is for uplift only and does not consider lateral forces.



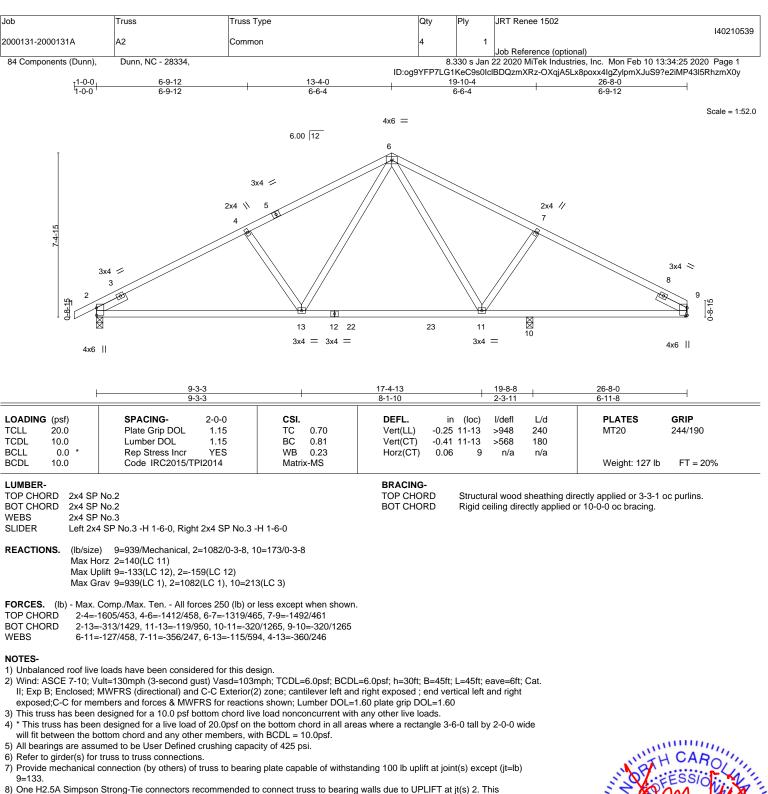
Structural wood sheathing directly applied or 3-1-13 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

February 10,2020





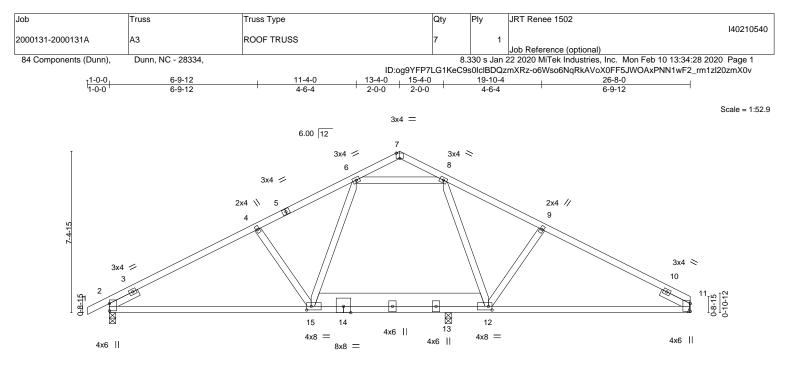


connection is for uplift only and does not consider lateral forces.



February 10,2020

818 Soundside Road Edenton, NC 27932



ł	9-3-3		15-8-8	17-4-13	26-8-		_
Plate Offsets (X,Y)	9-3-3 [7:0-2-0,Edge], [12:0-2-0,0-2-0], [14:0-0		<u>6-5-5</u> 0]	' 1-8-5 '	9-3-:	3	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.65 BC 0.77 WB 0.30	DEFL. Vert(LL) Vert(CT) Horz(CT)		l/defl L/d >999 240 >588 180 n/a n/a	PLATES MT20	GRIP 244/190
3CDL 10.0	Code IRC2015/TPI2014	Matrix-MS	1012(01)	0.04 11	iva iva	Weight: 152 lb	FT = 20%
	P No.2 P No.2 *Except* 2x8 SP No.2		BRACING- TOP CHOR BOT CHOR		al wood sheathing dire iling directly applied or	2 I I	oc purlins.
VEBS 2x4 SP		H 1-6-0					
	a = 11 - 822/Machanical 2 - 960/0.2.8.1	2 401/0 2 8					

REACTIONS. (lb/size) 11=832/Mechanical, 2=960/0-3-8, 13=401/0-3-8 Max Horz 2=140(LC 11) Max Uplift 11=-94(LC 12), 2=-137(LC 12), 13=-37(LC 12) Max Grav 11=832(LC 1), 2=968(LC 22), 13=540(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-4=-1354/370, 4-6=-1180/367, 8-9=-1050/346, 9-11=-1192/347

BOT CHORD 2-15=-237/1242, 13-15=-98/911, 12-13=-106/912, 11-12=-216/1027

WEBS 9-12=-348/206, 6-15=-58/444, 4-15=-360/217, 6-8=-803/338

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11.

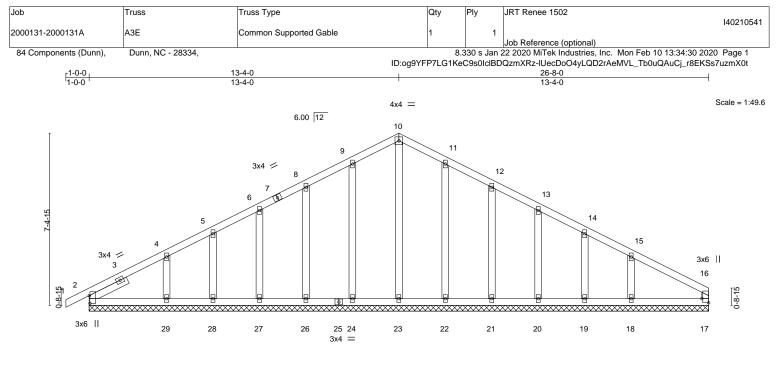
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.

9) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



February 10,2020





L					26-8-0						
1					26-8-0						I
late Offsets (X,Y)	[2:0-4-0,0-0-7], [16:Edge	,0-3-8], [16:0-0)-14,0-1-12],	[17:0-0-0,0-1	-12]						
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	0.00	1	n/r	90		
CLL 0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(CT)	0.00	17	n/a	n/a		
3CDL 10.0	Code IRC2015/T	PI2014	Matri	x-S						Weight: 157 lb	FT = 20%
UMBER-					BRACING-						
TOP CHORD 2x4 SP	No 2				TOP CHOP		Structu	ral wood	sheathing di	rectly applied or 6-0-0	oc nurlins
BOT CHORD 2x4 SP					. 51 01101			end verti			50 pa

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

SLIDER Left 2x4 SP No.3 -H 1-10-1

REACTIONS. All bearings 26-8-0. (lb) - Max Horz 2=151(LC

Max Horz 2=151(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18 Max Grav All reactions 250 lb or less at joint(s) 17, 2, 23, 24, 26, 27, 28, 22, 21, 20, 19 except 29=257(LC

17). 18=253(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 9-10=-127/253, 10-11=-127/253

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) All bearings are assumed to be User Defined crushing capacity of 425 psi.

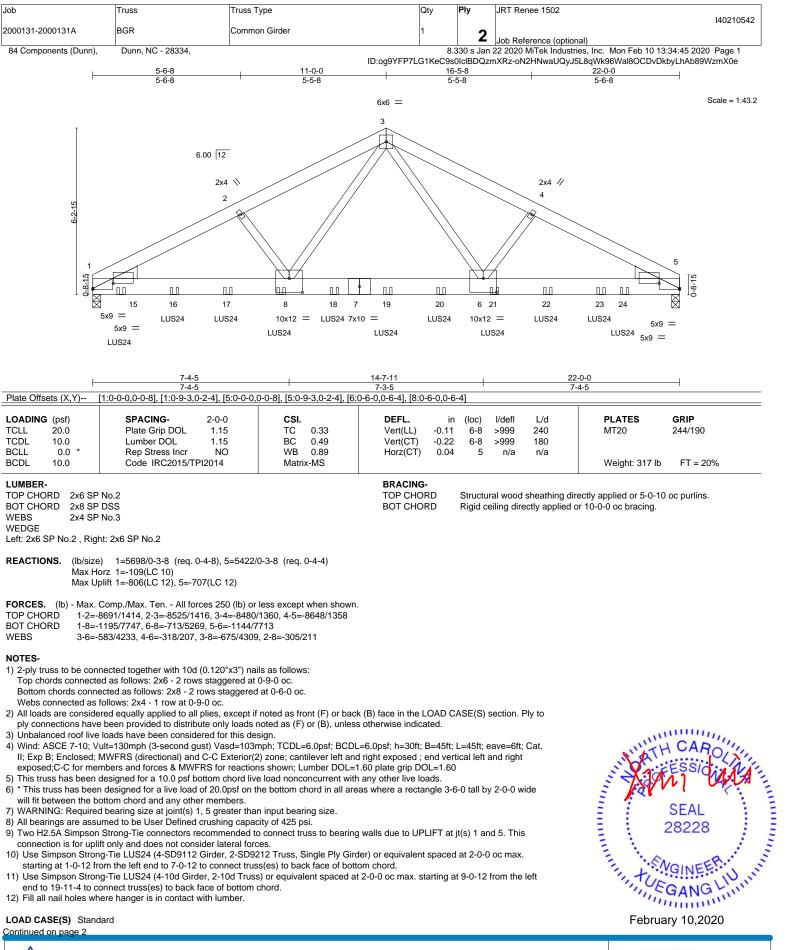
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 24, 26, 27, 28, 29, 22, 21, 20, 19, and 18. This connection is for uplift only and does not consider lateral forces.

11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



February 10,2020





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	JRT Renee 1502
					140210542
2000131-2000131A	BGR	Common Girder	1	2	Job Reference (optional)
84 Components (Dunn),	Dunn. NC - 28334.		8		22 2020 MiTek Industries, Inc. Mon Feb 10 13:34:45 2020 Page 2
04 Components (Dunin),	Dunn, NO - 20004,				nXRz-oN2HNwaUQyJ5L8gWk96Wal8OCDvDkbyLhAb89WzmX0e

LOAD CASE(S) Standard

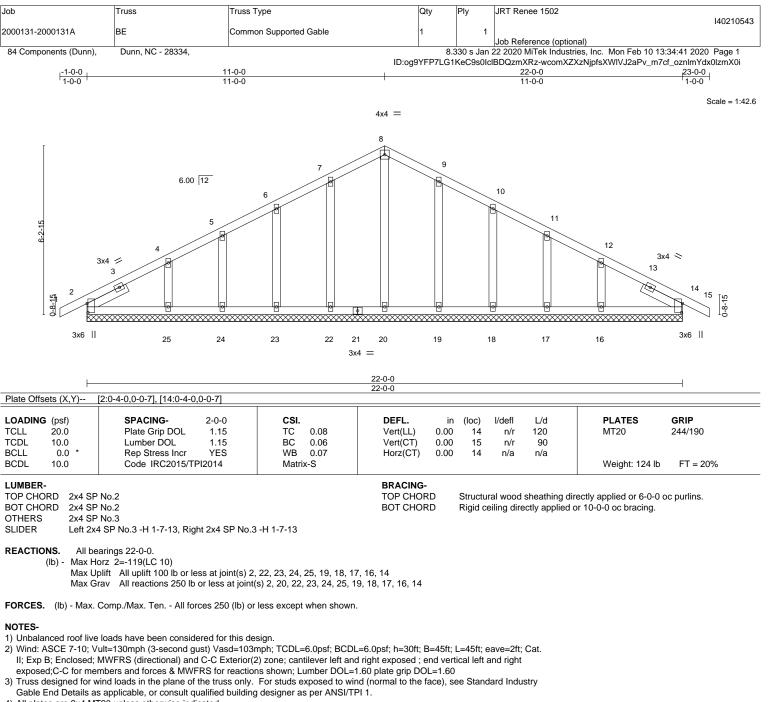
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 9-12=-20

Concentrated Loads (lb)

Vert: 8=-919(B) 15=-919(B) 16=-919(B) 17=-919(B) 18=-812(B) 19=-812(B) 20=-812(B) 21=-812(B) 22=-812(B) 23=-812(B) 24=-812(B) 24=-81





All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) All bearings are assumed to be User Defined crushing capacity of 425 psi.

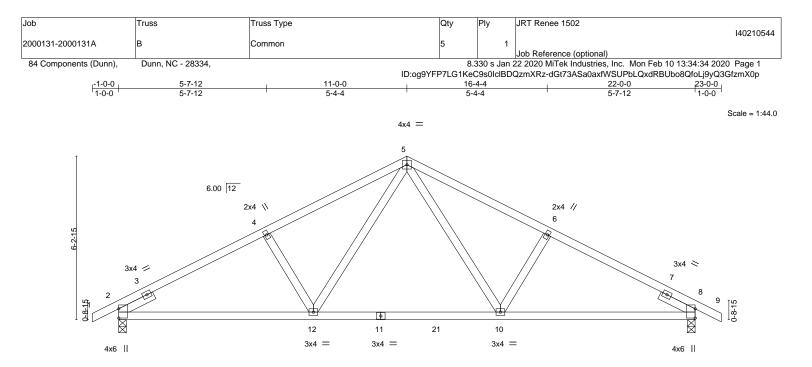
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 22, 23, 24, 25, 19, 18, 17, 16, and 14. This connection is for uplift only and does not consider lateral forces.

11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 14.



February 10,2020





F	7-5-3 7-5-3		14-6-13 7-1-11	<u>22-0</u> 7-5-	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.48 BC 0.57 WB 0.18	DEFL. in (loc) Vert(LL) -0.13 10-12 Vert(CT) -0.22 10-12 Horz(CT) 0.04 8	>999 240 >999 180	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS		3 n/a n/a	Weight: 108 lb FT = 20%
LUMBER-			BRACING-		

TOP CHORD

BOT CHORD

LUMBER-

- TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD
- 2x4 SP No.3 WEBS
- SLIDER Left 2x4 SP No.3 -H 1-6-0, Right 2x4 SP No.3 -H 1-6-0

REACTIONS. (lb/size) 2=940/0-3-8, 8=940/0-3-8 Max Horz 2=-119(LC 10) Max Uplift 2=-132(LC 12), 8=-132(LC 12)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-4=-1357/358, 4-5=-1221/375, 5-6=-1221/375, 6-8=-1357/358
- BOT CHORD 2-12=-221/1169, 10-12=-74/816, 8-10=-224/1155
- WEBS 5-10=-98/460, 6-10=-277/198, 5-12=-98/457, 4-12=-277/198

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

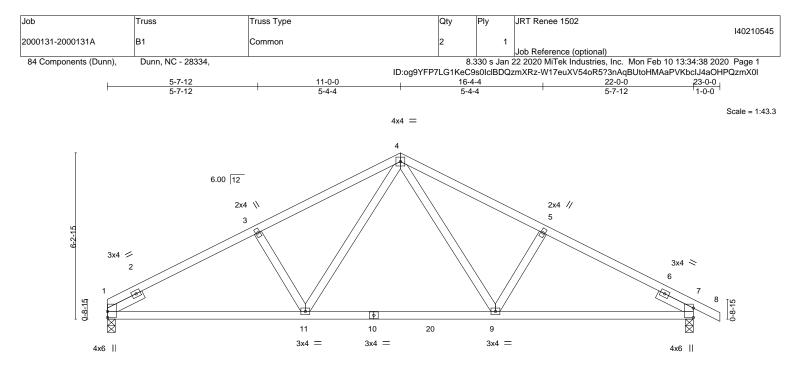
6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 4-2-6 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing





H	7-5-3 7-5-3		14-6-13 7-1-11	22-0-0 7-5-3	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.48 BC 0.57 WB 0.18	DEFL. in (loc) Vert(LL) -0.13 9-11 Vert(CT) -0.22 9-11 Horz(CT) 0.04 7	>999 240	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS			Weight: 106 lb FT = 20%
LUMBER-			BRACING-		

TOP CHORD

BOT CHORD

LUMBER-

- TOP CHORD 2x4 SP No.2
- 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS
- SLIDER Left 2x4 SP No.3 -H 1-6-0, Right 2x4 SP No.3 -H 1-6-0

REACTIONS. (lb/size) 1=879/0-3-8, 7=941/0-3-8 Max Horz 1=-117(LC 10) Max Uplift 1=-95(LC 12), 7=-133(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 1-3=-1364/361, 3-4=-1228/379, 4-5=-1224/377, 5-7=-1359/360
- BOT CHORD 1-11=-227/1176, 9-11=-76/818, 7-9=-226/1157
- WEBS 4-9=-98/459, 5-9=-277/199, 4-11=-100/464, 3-11=-281/200

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 7. This connection is for uplift only and does not consider lateral forces.



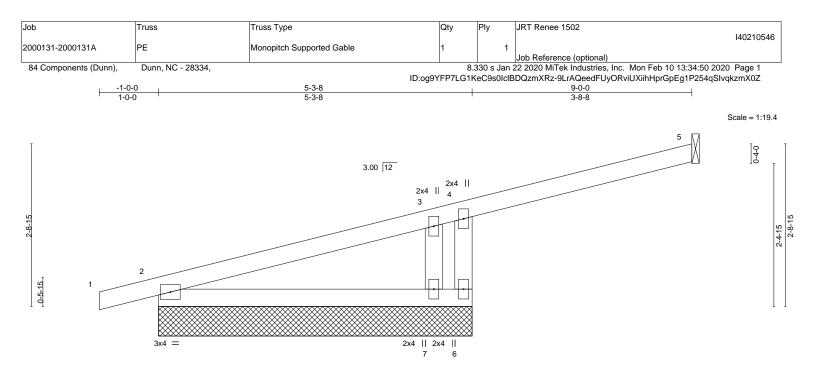
Structural wood sheathing directly applied or 4-2-5 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

February 10,2020







LOADING (psf) SPACING- 2-0-0	CSI. DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0 Plate Grip DOL 1.15 TCDL 10.0 Lumber DOL 1.15	TC 0.27 Vert(LL) -0.01 2-7 >999 240 BC 0.19 Vert(CT) -0.03 2-7 >999 180	MT20 244/190
BCLL 0.0 * Rep Stress Incr YES BCDL 10.0 Code IRC2015/TPI2014	WB 0.07 Horz(CT) -0.00 5 n/a n/a Matrix-P	Weight: 27 lb FT = 20%

BOT CHORD

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3OTHERS2x4 SP No.3

Structural wood sheathing directly applied or 5-3-8 oc purlins except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=90/Mechanical, 6=-12/5-3-8, 2=215/5-3-8, 7=407/5-3-8 Max Horz 2=72(LC 9) Max Uplift 5=-34(LC 12), 6=-158(LC 3), 2=-45(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

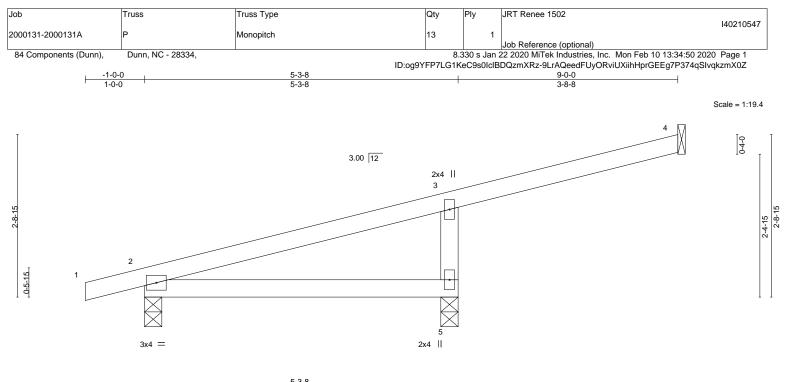
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.







				5-3-8								
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.31	Vert(LL)	-0.03	5-8	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.06	5-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.01	2	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MP						Weight: 25 lb	FT = 20%

```
LUMBER-
```

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-3-8 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=76/Mechanical, 5=380/0-3-8, 2=244/0-3-8 Max Horz 2=71(LC 9) Max Uplift 4=-32(LC 12), 5=-85(LC 12), 2=-43(LC 12)

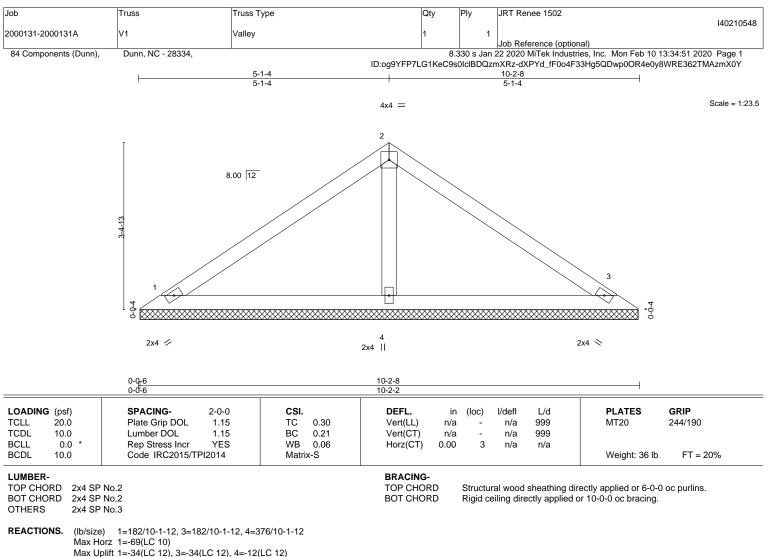
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-5=-324/258

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 2. This connection is for uplift only and does not consider lateral forces.







 $\frac{1}{12} = \frac{1}{12} = \frac{1}{12}$

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

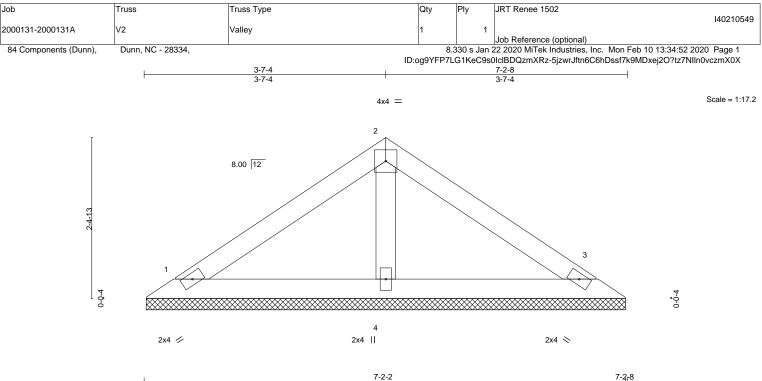
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.







			7-2-2							0-0-6		
LOADIN TCLL	G (psf) 20.0	SPACING- Plate Grip DC	2-0-0 DL 1.15	CSI. TC	0.18	DEFL. Vert(LL)	in n/a	(loc)	l/defl n/a	L/d 999	PLATES MT20	GRIP 244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	n/a	-	n/a	999	WIT20	244/190
BCLL BCDL	0.0 * 10.0	Rep Stress Ir Code IRC20		WB Matrix	0.03 -P	Horz(CT)	0.00	3	n/a	n/a	Weight: 24 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS

2x4 SP No.3

REACTIONS. (lb/size) 1=135/7-1-12, 3=135/7-1-12, 4=230/7-1-12 Max Horz 1=-46(LC 10) Max Uplift 1=-31(LC 12), 3=-31(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



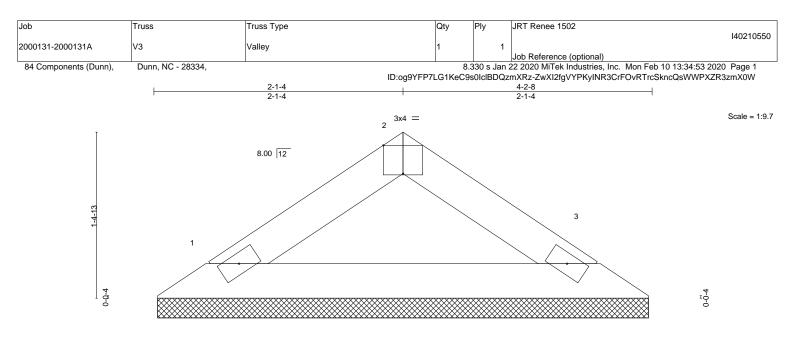
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

February 10,2020







2x4 🥢

2x4 📎

Structural wood sheathing directly applied or 4-2-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y) [2:0	0-0-6 0-0-6 0-2-0,Edge]		4-2-8 4-2-2			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.04 BC 0.13 WB 0.00 Matrix-P	Vert(CT)	in (loc) n/a - n/a -).00 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES GRIP MT20 244/190 Weight: 12 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

REACTIONS. (Ib/size) 1=130/4-1-12, 3=130/4-1-12 Max Horz 1=-24(LC 10) Max Uplift 1=-14(LC 12), 3=-14(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

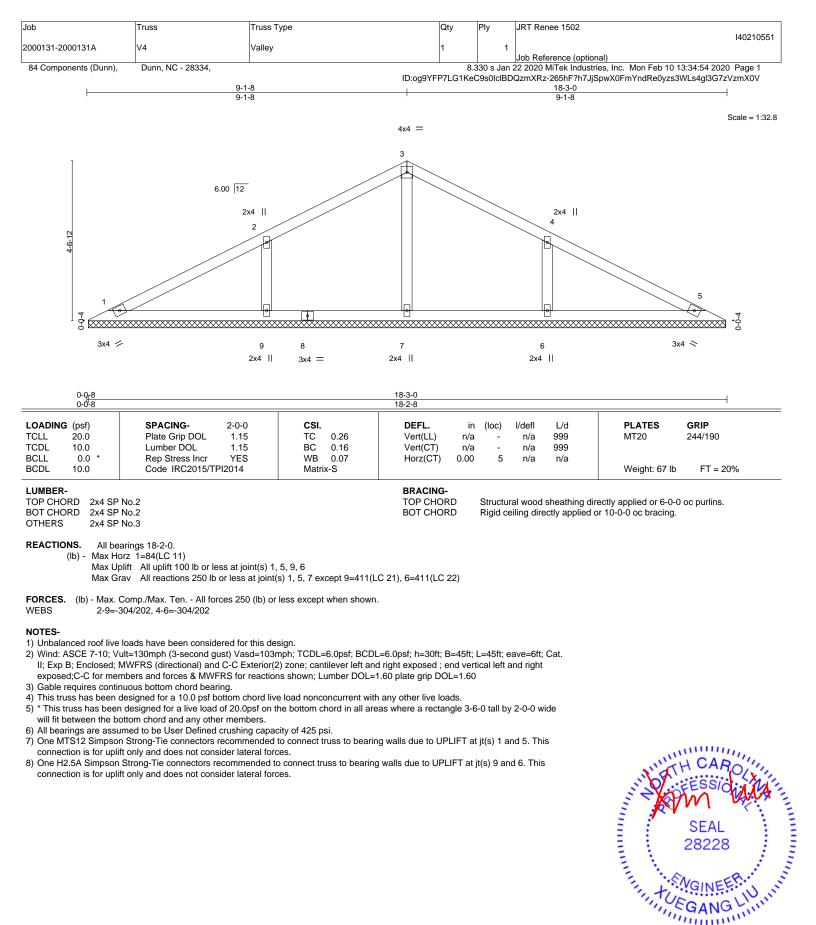
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



February 10,2020

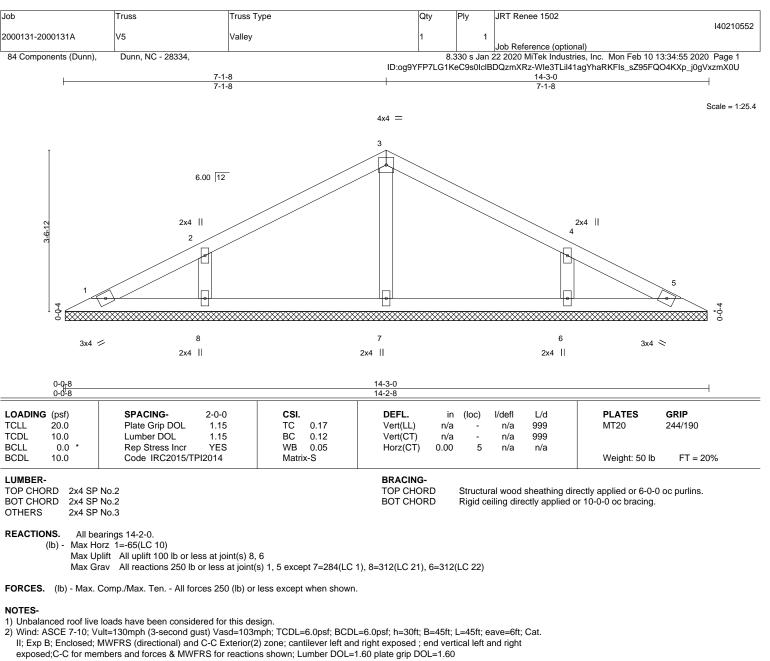


ENGINEERING BY A MITEK Atfiliate B18 Soundside Road Edenton, NC 27932



February 10,2020





3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

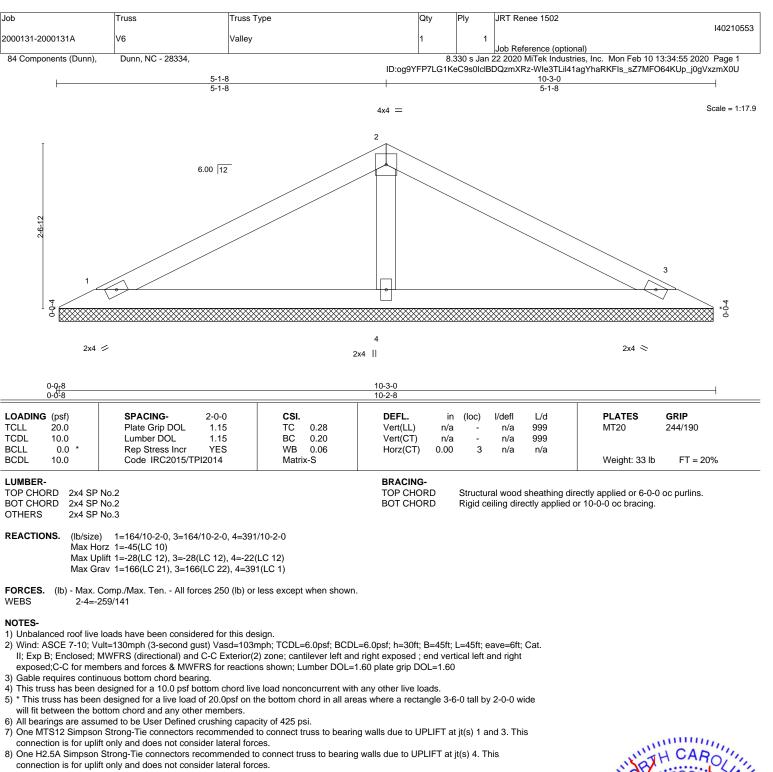
will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 6. This
connection is for uplift only and does not consider lateral forces.



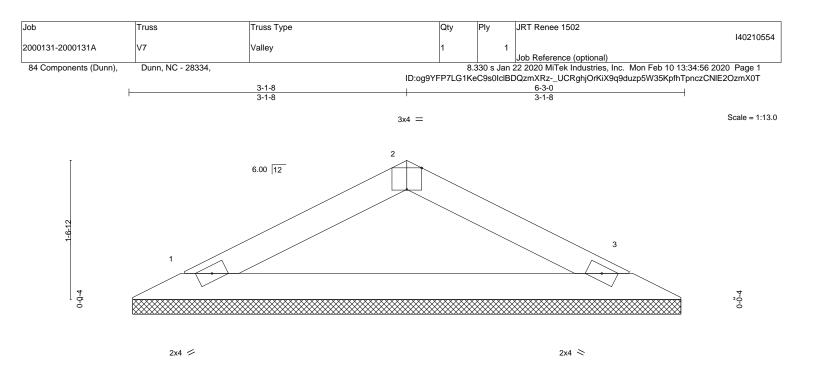






February 10,2020

818 Soundside Road Edenton, NC 27932



			6-2-8 6-2-8			<u>6-</u> 3-0 0-0-8	
Plate Offsets (X,Y) [2:0-2-0,Edge]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.11 BC 0.32 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	n/a - i n/a - i	defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 17 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.				oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 1=200/6-2-0, 3=200/6-2-0 Max Horz 1=25(LC 11) Max Uplift 1=-22(LC 12), 3=-22(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road Edenton, NC 27932

