

Trenco 818 Soundside Rd Edenton, NC 27932

Re: Elm_Plan Lamco Custom Homes

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E12952482 thru E12952505

My license renewal date for the state of North Carolina is December 31, 2019.

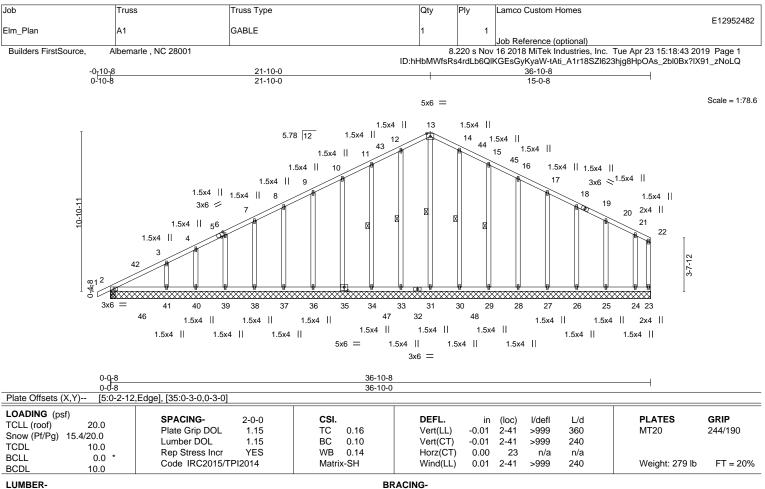
North Carolina COA: C-0844



April 24,2019

Gilbert, Eric

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



WEBS

LUMBER-	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 13-31, 12-33, 11-34, 14-30, 15-29

REACTIONS. All bearings 36-10-8.

- Max Horz 2=224(LC 16) (lb) -
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 33, 34, 35, 36, 37, 38, 39, 40, 41, 30, 29, 28, 27, 26, 25, 24

Max Grav All reactions 250 lb or less at joint(s) 23, 2, 2, 31, 33, 34, 35, 36, 37, 38, 39, 40, 30, 29, 28, 27, 26, 25, 24 except 41=300(LC 34)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 11-12=-89/268, 12-13=-102/301, 13-14=-102/300, 14-15=-89/267

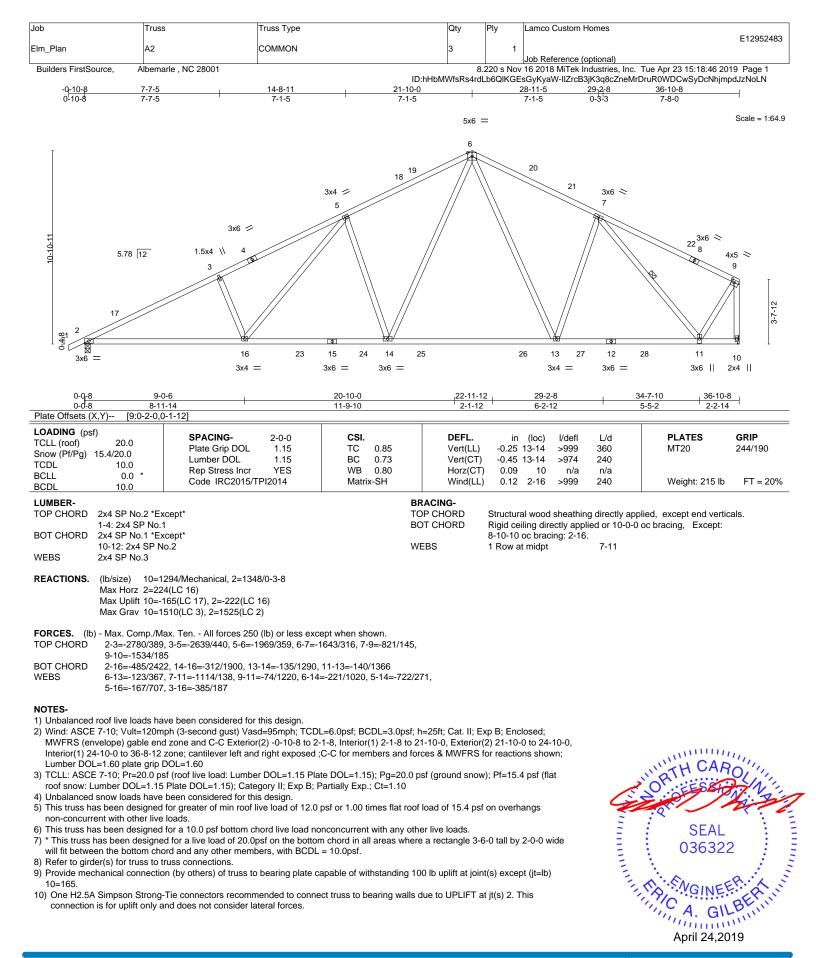
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 21-10-0, Corner(3) 21-10-0 to 24-10-0, Exterior(2) 24-10-0 to 36-8-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 33, 34, 35, 36, 37, 38, 39, 40, 41, 30, 29, 28, 27, 26, 25, 24.

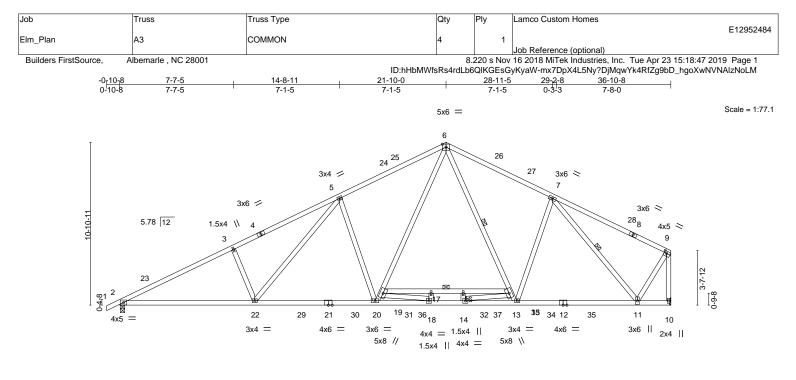


818 Soundside Road Edenton, NC 27932

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not being read to be only with thread outpetting the boots into besign is based only door parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



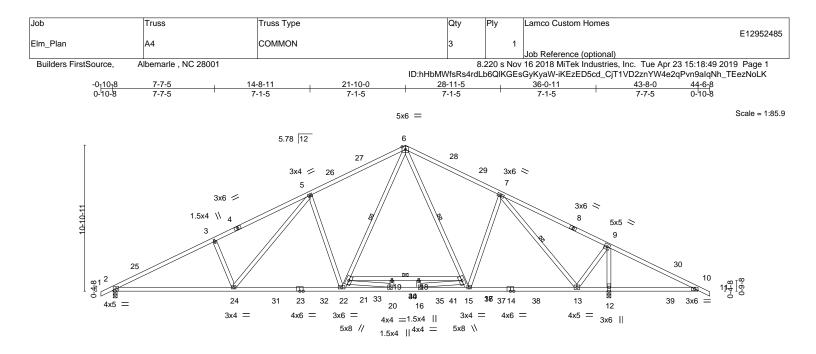
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0-0 <u>-8</u> 0-0-8	9-0-6	20-10-0	22-11-12	29-2-8	34-7-10	36-10-8	
	8-11-14	11-9-10	2-1-12	6-2-12	5-5-2	2-2-14	
Plate Offsets (X,Y) [9:0-2-0,	J-1-12j	1	1				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15	TC 0.90	Vert(LL)	-0.22 20-22	>999 360	MT20	244/190
Snow (Pf/Pg) 15.4/20.0 TCDL 10.0	Lumber DOL 1.15	BC 0.90	Vert(CT)	-0.41 20-22	>999 240		
TCDL 10.0 BCLL 0.0 *	Rep Stress Incr YES	WB 0.80	Horz(CT)	0.10 10	n/a n/a		
BCDL 0.0	Code IRC2015/TPI2014	Matrix-SH	Wind(LL)	0.12 2-22	>999 240	Weight: 238 lb	FT = 20%
LUMBER-	1	Dr					
TOP CHORD 2x4 SP No.2 *E	voort*		RACING- DP CHORD	Structural wood	choothing directly	applied, except end vert	icals
1-4: 2x4 SP No.						-6 oc bracing. Except:	icais.
BOT CHORD 2x4 SP No.2 *E				6-0-0 oc bracing			
2-21: 2x4 SP No		W		1 Row at midpt	6-13, 7	'-11	
WEBS 2x4 SP No.3					, -		
REACTIONS. (lb/size) 10=1							
Max Horz 2=22							
	136(LC 17), 2=-202(LC 16)						
Max Grav 10=1	1685(LC 3), 2=1610(LC 3)						
FORCES. (lb) - Max. Comp./M	lax. Ten All forces 250 (lb) or less exc	ept when shown.					
	3, 3-5=-2913/395, 5-6=-2228/314, 6-7=-						
9-10=-1729/15	53	, , ,					
BOT CHORD 2-22=-444/266	67, 20-22=-271/2136, 18-20=-50/1358, ⁻	4-18=0/2115, 13-14=-93/	/1502,				
11-13=-109/15	549, 17-19=-813/0, 16-17=-813/0, 15-16	=-813/0					
	2, 13-15=-157/271, 7-13=-71/299, 7-11=						
	029, 6-19=-198/1183, 5-20=-725/270, 5-	22=-166/714, 3-22=-382/	188,				
18-19=0/890,	14-15=0/783						
NOTES-							
	ave been considered for this design.						
	nph (3-second gust) Vasd=95mph; TCD	L=6.0psf; BCDL=3.0psf; h	=25ft: Cat. II: Ex	p B: Enclosed:			11.
	d zone and C-C Exterior(2) -0-10-8 to 2				10-0,	W'LL CAL	5-111
Interior(1) 24-10-0 to 36-8-12	zone; cantilever left and right exposed	C-C for members and for	ces & MWFRS fo	or reactions show	/n;	N'ATH ON	10/ 11/
Lumber DOL=1.60 plate grip	DOL=1.60					S.O. EFSSI	J. Mile
	sf (roof live load: Lumber DOL=1.15 Pla		sf (ground snow)	; Pf=15.4 psf (fla		mil 1	No ZA
	5 Plate DOL=1.15); Category II; Exp B; I	Partially Exp.; Ct=1.10				:0	K. 1.
	been considered for this design.					·	: =
	for greater of min roof live load of 12.0	psf or 1.00 times flat roof	load of 15.4 psf	on overhangs	=	SEAL	· · · · · · · · · · · · · · · · · · ·
non-concurrent with other live			ar live leade		E	03632	2 : E
	d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the botto				do	: 00002	- : :
	ord and any other members, with BCDL		e a rectarigie 3-0-	-0 tali by 2-0-0 wi	ue		1 2
8) Refer to girder(s) for truss to		– 10.0psi.			3	· A. En	Rix S
	on (by others) of truss to bearing plate of	apable of withstanding 10	0 lb uplift at ioint	(s) except (it=lb)		SEAL 03632	2 B. K.
10=136.	(,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	.,		(-,		TIC A AL	
10) One H2.5A Simpson Strong	-Tie connectors recommended to conne	ect truss to bearing walls o	due to UPLIFT at	jt(s) 2. This		11, A. GI	LUIII
connection is for uplift only	and does not consider lateral forces.	-					11.
-						April 24,2	019
A							

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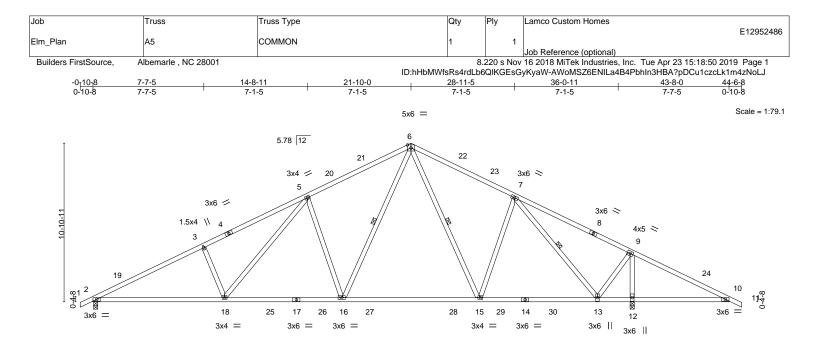
	0-0 <u>-8</u> 0-0-8 8	9-0-6	<u>17-0-15</u> 8-0-8		2-10-0	<u>26-7-1</u> 3-9-1	<u>34-7-10</u> 8-0-8	37-2-0	43-8		
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	if) 20.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI	2-0-0 1.15 1.15 YES	CSI. TC 0.83 BC 0.88 WB 0.80 Matrix-SH	2-0-0	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.22 22-24 -0.39 22-24 0.10 12 0.25 2-24	I/defl L/c >999 360 >999 240 n/a n/a >999 240	d D D a	PLATES MT20 Weight: 261 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.1 2x4 SP No.2 *E 2-23: 2x4 SP No 2x4 SP No.3				TOP	CHORD	Structural wood Rigid ceiling dire 6-0-0 oc bracing 1 Row at midpt	ectly applied or : 17-21		. .	i.
REACTIONS.	Max Horz 2=15 Max Uplift 2=-4	375/0-3-8, 12=1968/0-3-8 56(LC 16) 35(LC 13), 12=-542(LC 13 574(LC 3), 12=2251(LC 3))								
FORCES. (Ib TOP CHORD BOT CHORD	2-3=-2972/186 9-10=-364/685 2-24=-1583/25 15-16=-921/14	lax. Ten All forces 250 (l 50, 3-5=-2830/1907, 5-6=-2 5 593, 22-24=-1276/2061, 20 451, 13-15=-878/1396, 12- 90, 17-18=-823/999	2144/1603, 6-7=-)-22=-826/1289,	1734/1347, 7-9 16-20=-1819/20)58,	,					
WEBS	6-17=-342/319 6-21=-916/118	9, 7-15=-275/349, 7-13=-14 33, 5-22=-725/322, 5-24=-4 398, 16-17=-926/784									
 Wind: ASCE MWFRS (er Interior(1) 2. reactions sh TCLL: ASCI roof snow: L Unbalanced This truss h. non-concurr This truss h. This truss h. 	E 7-10; Vult=120n tvelope) gable en 4-10-0 to 44-6-8 ; town; Lumber DC E 7-10; Pr=20.0 p umber DOL=1.1! I snow loads have as been designer has been designer has been designer	ave been considered for th ph (3-second gust) Vasd= d zone and C-C Exterior(2 zone; cantilever left and rig uL=1.60 plate grip DOL=1.6 sf (roof live load: Lumber I 5 Plate DOL=1.15); Catego been considered for this of tor greater of min roof live b loads. If or a 10.0 psf bottom cho ad for a live load of 20.0psf ord and any other member	e95mph; TCDL=6) -0-10-8 to 2-1-8 ht exposed ; pore 30 DOL=1.15 Plate I ory II; Exp B; Part design. a load of 12.0 psf rd live load noncc f on the bottom cl	, Interior(1) 2-1- ch right exposed DOL=1.15); Pg= ially Exp.; Ct=1. or 1.00 times fla pncurrent with a hord in all areas	-8 to 21-1 d;C-C for =20.0 psf .10 lat roof loa	10-0, Exterior(2 members and (ground snow) ad of 15.4 psf of live loads.) 21-10-0 to 24- forces & MWFR ; Pf=15.4 psf (fla on overhangs	S for	A Charles and a charles and a charles a charle	SEAL 036322	<i>Selle</i>

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.



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0-0-	8 9-0-6	17-0-15	26-7-1		34-7-10		37-2-0	43-8-0	
0-0-8	8 8-11-1	4 8-0-8	9-6-3	1	8-0-8	I	2-6-6	6-6-0	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL BCDL	20.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.99 BC 1.00 WB 0.80 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.24 15-16 -0.45 15-16 0.09 12 0.25 2-18	>999 3 >994 2 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 238 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x4 SP No.1 *Ex 1-4,8-11: 2x4 SP 2x4 SP No.2 *Ex 14-17: 2x4 SP No 2x4 SP No.3	No.2 cept*	TC BC	OT CHORD	Structural wood Rigid ceiling dire 1 Row at midpt	ectly applied		c bracing.	
	Max Horz 2=156 Max Uplift 2=-29	17/0-3-8, 12=1871/0-3-8 5(LC 16) 5(LC 13), 12=-342(LC 13) '9(LC 2), 12=2116(LC 2)							
FORCES. (lb) TOP CHORD		ux. Ten All forces 250 (lb) or less exc 2, 3-5=-2553/1290, 5-6=-1882/978, 6-7		5,					
BOT CHORD	2-18=-1031/234 10-12=-522/38	15, 16-18=-714/1822, 15-16=-394/1212 1	2, 13-15=-434/1208, 12-13	3=-522/381,					
WEBS	,	7-15=-112/288, 7-13=-1322/573, 9-13 5-18=-429/708, 3-18=-385/186, 9-12=	,	019,					
 2) Wind: ASCE MWFRS (env Interior(1) 24- reactions sho 3) TCLL: ASCE roof snow: Lu 	7-10; Vult=120mp velope) gable end -10-0 to 44-6-8 zc own; Lumber DOL 7-10; Pr=20.0 ps umber DOL=1.15	ve been considered for this design. bh (3-second gust) Vasd=95mph; TCD zone and C-C Exterior(2) -0-10-8 to 2- one; cantilever left and right exposed ; j =1.60 plate grip DOL=1.60 f (roof live load: Lumber DOL=1.15 Pla Plate DOL=1.15); Category II; Exp B; F been considered for this design.	1-8, Interior(1) 2-1-8 to 2 borch right exposed;C-C f te DOL=1.15); Pg=20.0 p	1-10-0, Exterior(2 or members and) 21-10-0 to 24- forces & MWFR	S for		OR TEST	

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.



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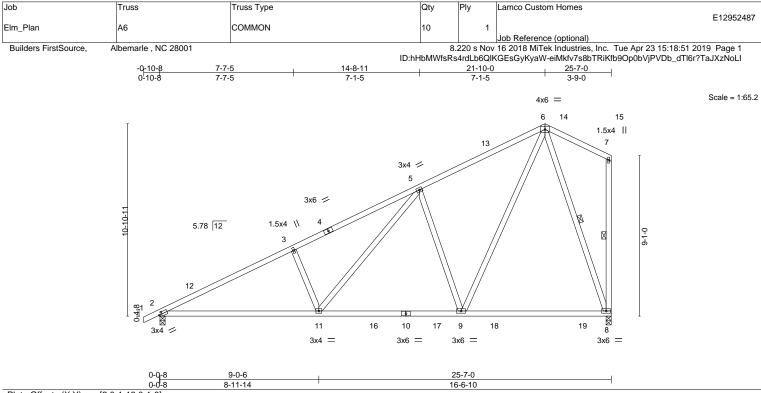


Plate Offsets (X,Y) [2:0-1-1	3,0-1-8]								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.75 BC 0.84 WB 0.80	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.21	(loc) 8-9 2-11 8	l/defl >999 >797 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-SH	Wind(LL)	0.11	2-11	>999	240	Weight: 157 lb	FT = 20%
LUMBER-		BF	ACING-						

LUMBER-		BRACING-		
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing of	lirectly applied or 2-8-14 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.	
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied	l or 8-9-15 oc bracing.
		WEBS	1 Row at midpt	7-8, 6-8
REACTIONS.	(lb/size) 8=894/0-3-8, 2=948/0-3-8			
	Max Horz 2=326(LC 16)			
	Max Liplift 8201(I C 16), 2142(I C 16)			

Max Grav 8=1069(LC 3), 2=1074(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1770/209, 3-5=-1602/261, 5-6=-882/177

BOT CHORD 2-11=-426/1506, 9-11=-249/923, 8-9=-72/302

WEBS 5-9=-728/272, 5-11=-171/724, 6-8=-906/228, 3-11=-398/190, 6-9=-221/1031

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 21-10-0, Exterior(2) 21-10-0 to 24-10-0, Interior(1) 24-10-0 to 25-5-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

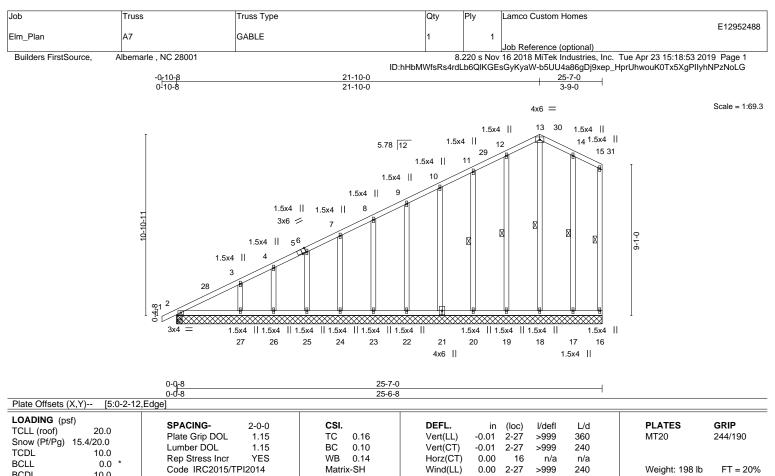
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 2. This connection is for uplift only and does not consider lateral forces.



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BCDL	10.0					
LUMBER-		BR	ACING-			
TOP CHORD	2x4 SP No.2	TO	P CHORD	Structural wood sheathi	ng directly app	lied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2			except end verticals.		
WEBS	2x4 SP No.3	BC	T CHORD	Rigid ceiling directly app	olied or 10-0-0	oc bracing.
OTHERS	2x4 SP No.3	WE	BS	1 Row at midpt	15-16, 13-	18, 12-19, 11-20, 14-17

REACTIONS. All bearings 25-7-0.

(lb) - Max Horz 2=326(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 16, 19, 20, 21, 22, 23, 24, 25, 26, 27, 17 Max Grav All reactions 250 lb or less at joint(s) 16, 2, 2, 18, 19, 20, 21, 22, 23, 24, 25, 26, 17 except 27=300(LC 34)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-329/130, 3-4=-262/90

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 21-10-0, Corner(3) 21-10-0 to 24-10-0, Exterior(2) 24-10-0 to 25-5-4 zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 19, 20, 21, 22, 23, 24, 25, 26, 27, 17.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **NSUFTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



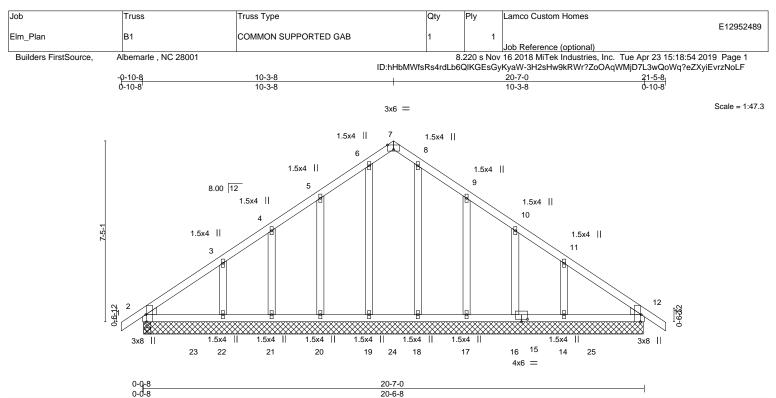


Plate Offsets (X,Y)--[2:0-3-8,Edge], [2:0-1-0,0-5-0], [2:0-0-8,0-0-12], [7:0-3-0,Edge], [12:0-3-8,Edge], [12:0-1-0,0-5-0], [12:0-0-8,0-0-12], [15:0-3-0,0-1-4], [15:0-0-0,0-1-12], [16:0-1-12.0-0]

LOA	DING	(psf)	

LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.10 BC 0.08 WB 0.09 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.00 2-22 -0.01 2-22 0.01 12 0.01 12-14	l/defi L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 122 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3		тс			sheathing directly a actly applied or 10-0	applied or 6-0-0 oc purlins)-0 oc bracing.	5.

WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

- (lb) -Max Horz 2=157(LC 13)
 - Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 17, 16, 12 except 22=-109(LC 14), 14=-108(LC 15)
 - Max Grav All reactions 250 lb or less at joint(s) 2, 2, 19, 20, 21, 18, 17, 16, 12 except 22=270(LC 26), 14=254(LC 31)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 10-3-8, Corner(3) 10-3-8 to 13-3-8, Exterior(2) 13-3-8 to 21-5-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 17, 16, 12 except (jt=lb) 22=109, 14=108.



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REACTIONS. All bearings 20-6-0.

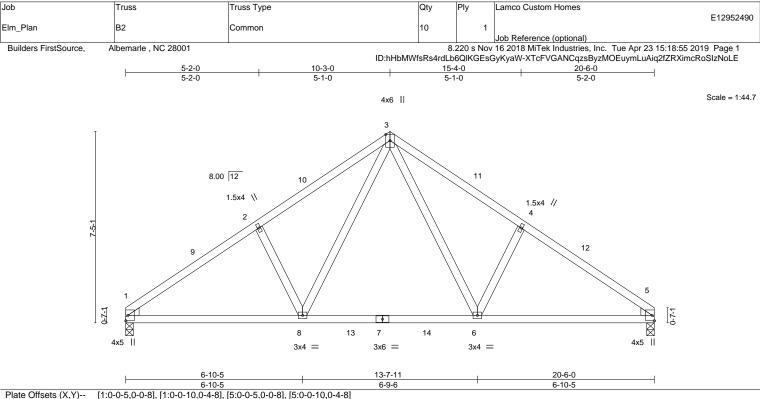


Plate Offsets (X,Y) [1:0-0-5,0	-0-8], [1:0-0-10,0-4-8], [5:0-0-5,0-0-8], [5:0-0-10,0-4-8]						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.36 BC 0.47 WB 0.18 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.09 (-0.13 (0.03	loc) l/defl 6-8 >999 6-8 >999 5 n/a 1-8 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 103 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			RACING- OP CHORD S	Structural w	vood sheath	ing directly ap	oplied or 5-2-1 oc purlins	

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) 1=715/0-3-8, 5=715/0-3-8 Max Horz 1=-150(LC 10) Max Uplift 1=-97(LC 14), 5=-97(LC 15) Max Grav 1=808(LC 2), 5=808(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 1-2=-1124/161, 2-3=-1005/215, 3-4=-1005/215, 4-5=-1124/161
- BOT CHORD 1-8=-149/935, 6-8=-21/616, 5-6=-64/861
- WEBS 3-6=-128/483, 4-6=-257/167, 3-8=-128/483, 2-8=-257/167

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 10-3-0, Exterior(2) 10-3-0 to 13-3-0, Interior(1) 13-3-0 to 20-4-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 5. This connection is for uplift only and does not consider lateral forces.



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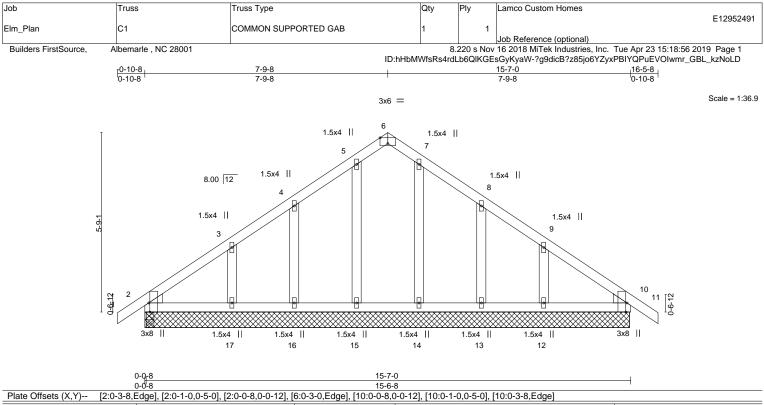


Plate Offsets (X,Y) [2:0-3-8,I	zagej, [2:0-1-0,0-5-0], [2:0-0-8,0-0-12],	, <u></u> , <u></u> , <u></u> , <u></u> _, <u></u> , <u>_</u> , <u></u>	, <u>_</u>	,] ,[, -9-1			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.07 BC 0.05 WB 0.05 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc -0.00 2-17 -0.00 2-17 0.00 10 -0.00 10-12	/ >999 / >999) n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 85 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			RACING- DP CHORD	Structural woo	od sheathir	ng directly ap	plied or 6-0-0 oc purlir	IS.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

- REACTIONS. All bearings 15-6-0.
 - (lb) Max Horz 2=122(LC 13) Max Uplift All uplift 100 lb or less at joint(s) 15, 16, 17, 14, 13, 12 Max Grav All reactions 250 lb or less at joint(s) 2, 2, 10, 15, 16, 17, 14, 13, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 7-9-8, Corner(3) 7-9-8 to 10-9-8, Exterior(2) 10-9-8 to 16-5-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

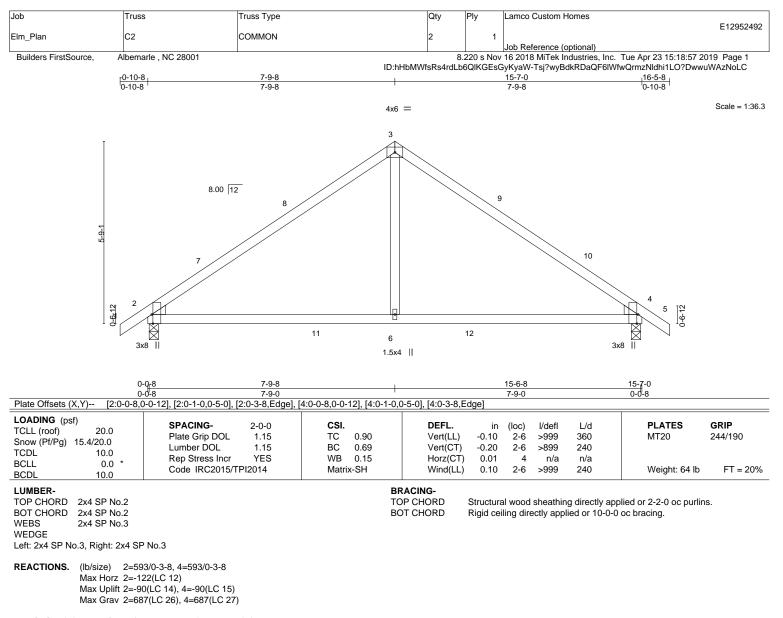
4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 16, 17, 14, 13, 12.



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 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-797/120, 3-4=-797/120

 BOT CHORD
 2-6=-21/587, 4-6=-21/587

 WEBS
 3-6=0/380

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 7-9-8, Exterior(2) 7-9-8 to 10-9-8, Interior(1) 10-9-8 to 16-5-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



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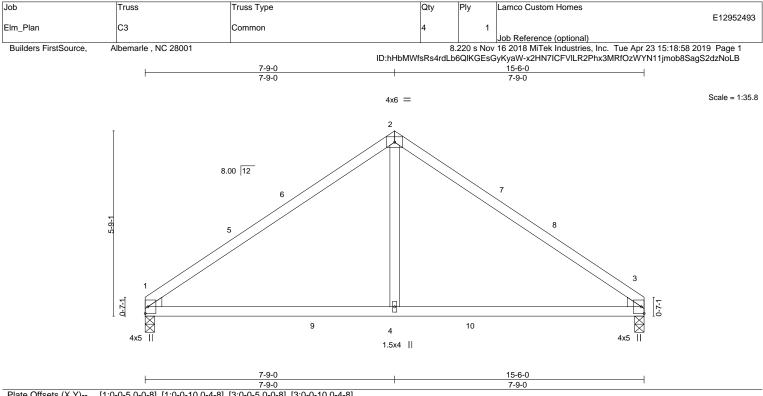


Plate Offsets (X,Y) [1:0-0-5,0	7-9-0 D-0-8], [1:0-0-10,0-4-8], [3:0-0-5,0-0-8], [3:0-0-10,0-4-8]		7-9-0			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.90 BC 0.70 WB 0.16 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	-0.21 1-4 0.01 3	l/defl L/d >999 360 >867 240 n/a n/a >999 240	PLATES MT20 Weight: 60 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3, Right: 2x4 SP	P No.3	TC			sheathing directly app ctly applied or 10-0-0	blied or 2-2-0 oc purlir oc bracing.	IS.

REACTIONS. (lb/size) 1=538/0-3-8, 3=538/0-3-8 Max Horz 1=-115(LC 12) Max Uplift 1=-73(LC 14), 3=-73(LC 15)

Max Grav 1=627(LC 25), 3=627(LC 26)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-799/125, 2-3=-799/125

 BOT CHORD
 1-4=-27/584, 3-4=-27/584

 WEBS
 2-4=0/381

NOTES-

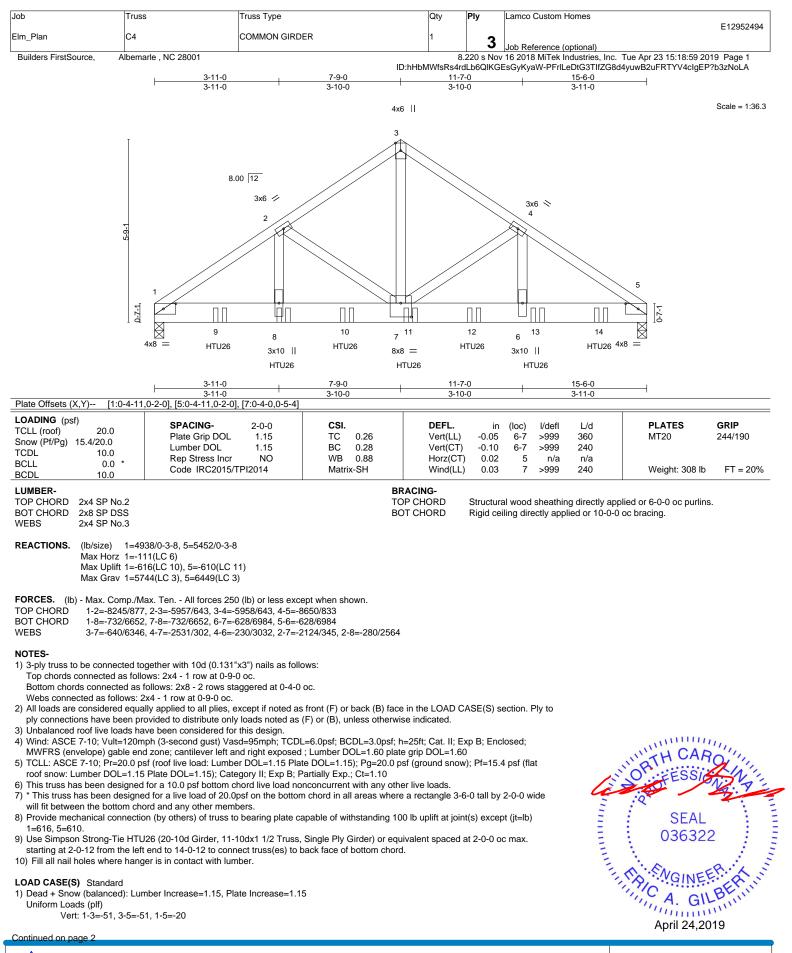
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 7-9-0, Exterior(2) 7-9-0 to 10-9-0, Interior(1) 10-9-0 to 15-4-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



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Job	Truss	Truss Type	Qty	Ply	Lamco Custom Homes	
						E12952494
Elm_Plan	C4	COMMON GIRDER	1	2		
				3	Job Reference (optional)	
Builders FirstSource,	Albemarle, NC 28001			8.220 s No	v 16 2018 MiTek Industries, Inc. Tue Apr 23 15	:19:00 2019 Page 2
			ID:hHbMWfsRs4r	dLb6QIKGE	sGyKyaW-tRP8Y_EV1Mb9HjrKBnT7TOb3?ron	EXsRvu9Z7VzNoL9

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 8=-1274(B) 9=-1274(B) 10=-1274(B) 11=-1373(B) 12=-1373(B) 13=-1373(B) 14=-1373(B)

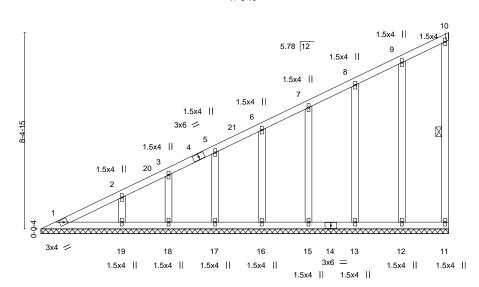
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







Scale = 1.494



17-5-10

	I	17-5-10			I		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.16 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)		l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 107 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3		TC BC	ACING- OP CHORD OT CHORD EBS	except end vert	icals. ectly applied or 10-0-0	lied or 6-0-0 oc purlins	5,

REACTIONS. All bearings 17-5-10.

- (lb) Max Horz 1=222(LC 16)
 - Max Uplift All uplift 100 lb or less at joint(s) 11, 12, 13, 15, 16, 17, 18, 19
 - Max Grav All reactions 250 lb or less at joint(s) 11, 1, 12, 13, 15, 16, 17, 18, 19
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-338/118, 2-3=-255/92

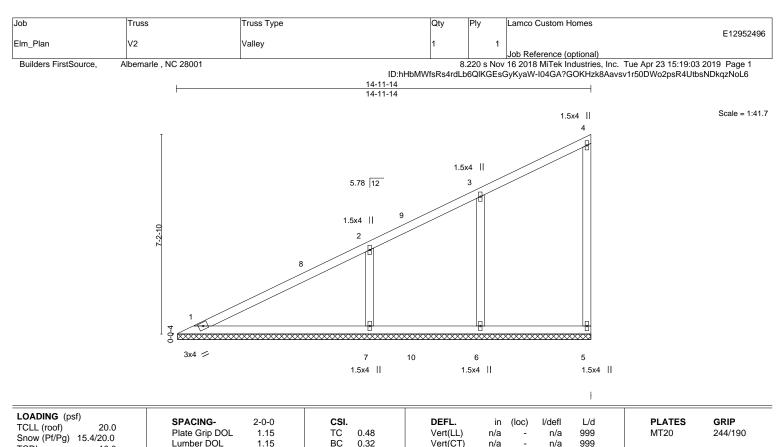
NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 17-3-14 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 12, 13, 15, 16, 17, 18, 19.



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BCLL BCDL	10.0 0.0 * 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.12 Matrix-SH	Horz(CT)	0.00	5	n/a	n/a	Weight: 67 lb	FT = 20%
LUMBER-				ACING-						
TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.2		TC		Structural except end			g directly app	lied or 6-0-0 oc purlins,	
WEBS OTHERS	2x4 SP No.3 2x4 SP No.3		BC	T CHORD	Rigid ceilir	ng direo	ctly appl	ied or 10-0-0	oc bracing.	

REACTIONS. All bearings 14-11-6.

(lb) - Max Horz 1=195(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 5, 6 except 7=-140(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=379(LC 5), 7=515(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-256/109

WEBS 2-7=-360/250

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 14-10-2 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

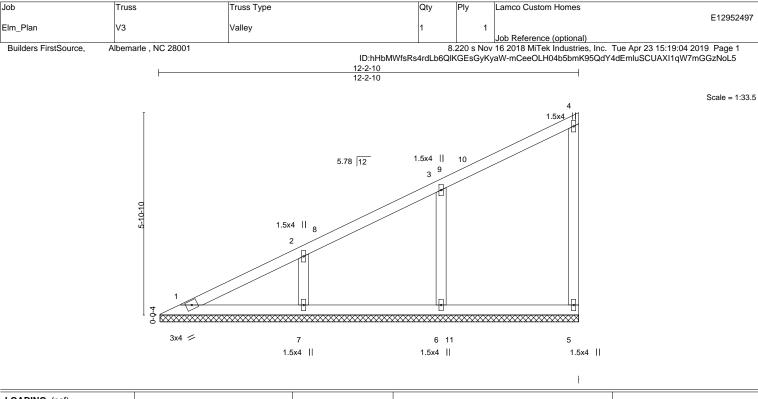
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6 except (jt=lb) 7=140.



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.20 BC 0.17 WB 0.08	DEFL. Vert(LL) Vert(CT) Horz(CT)		l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-SH					Weight: 52 lb	FT = 20%
LUMBER-		BR	ACING-					
TOP CHORD 2x4 SP No.2		TO	P CHORD	Structural woo	d sheathin	g directly ap	oplied or 6-0-0 oc purlins	З,
BOT CHORD 2x4 SP No.2				except end ver	ticals.			
WEBS 2x4 SP No.3		BC	OT CHORD	Rigid ceiling di	rectly appl	ied or 10-0-	0 oc bracing.	
OTHERS 2x4 SP No.3								

REACTIONS. All bearings 12-2-2.

- (lb) Max Horz 1=164(LC 16)
 - Max Uplift All uplift 100 lb or less at joint(s) 5, 6, 7
 - Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=358(LC 3), 7=333(LC 2)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

WEBS 3-6=-258/163

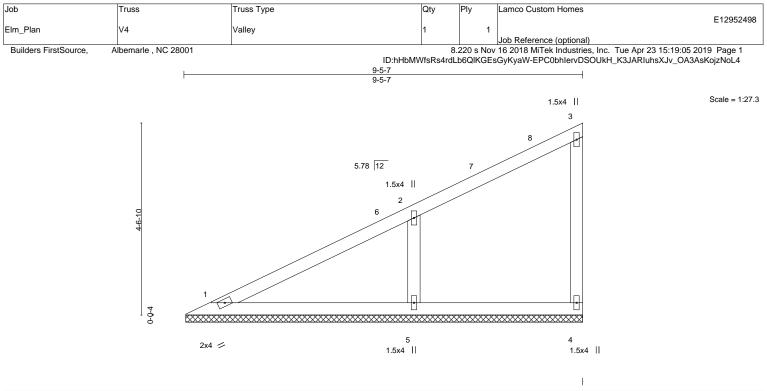
NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 12-0-14 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6, 7.



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.32 BC 0.19 WB 0.09 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	n/a -	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 37 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3		TC	ACING- OP CHORD OT CHORD	except end verti	sheathing directly app cals. cctly applied or 10-0-0	·	ns,

REACTIONS. (lb/size) 1=134/9-4-15, 4=92/9-4-15, 5=386/9-4-15

Max Horz 1=134(LC 16) Max Uplift 4=-21(LC 16), 5=-122(LC 16)

Max Grav 1=152(LC 2), 4=111(LC 22), 5=437(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-5=-327/298

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 9-3-11 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.

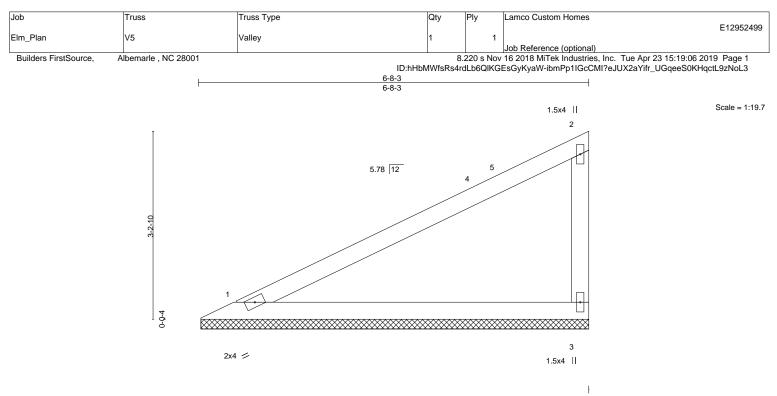
4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 5=122.



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A MiTek Affili 818 Soundside Road Edenton, NC 27932



LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.70 BC 0.44 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 24 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3		то		Structural wood except end vert Rigid ceiling din	icals.	, , ,	blied or 6-0-0 oc purlir	NS,

REACTIONS. (lb/size) 1=208/6-7-11, 3=208/6-7-11 Max Horz 1=94(LC 16) Max Uplift 1=-21(LC 16), 3=-67(LC 16) Max Grav 1=235(LC 2), 3=235(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 6-6-7 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

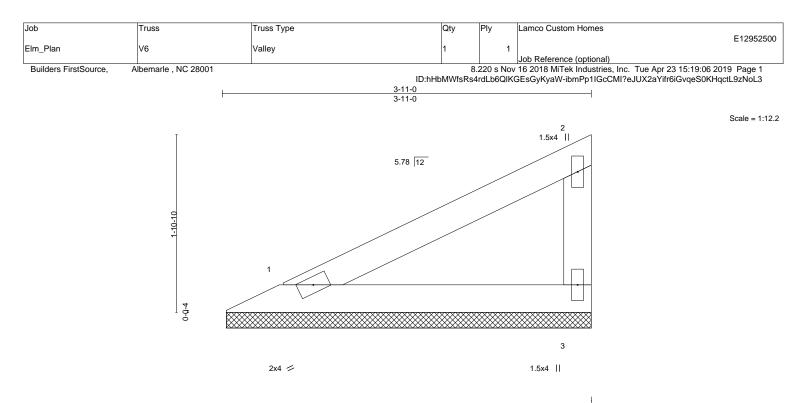
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 DCLL 0.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.18 BC 0.11 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 13 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2				Structural wood		g directly app	plied or 3-11-0 oc purl	ins,
WEBS 2x4 SP No.3		B		Rigid ceiling di		ied or 10-0-0	oc bracing.	

REACTIONS. (lb/size) 1=110/3-10-8, 3=110/3-10-8 Max Horz 1=50(LC 16) Max Uplift 1=-11(LC 16), 3=-35(LC 16) Max Grav 1=125(LC 2), 3=125(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

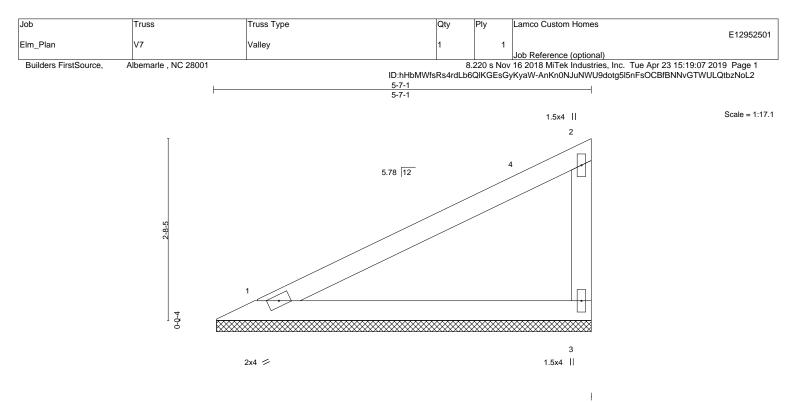
4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.45 BC 0.28 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	n/a n/a	(loc) - -	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 19 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2			BRACING- TOP CHORD	Structural except en			g directly ap	plied or 5-7-1 oc purlir	ns,
WEBS 2x4 SP No.3			BOT CHORD	Rigid ceili	ng dire	ectly app	ied or 10-0-0) oc bracing.	

REACTIONS. (lb/size) 1=170/5-6-9, 3=170/5-6-9 Max Horz 1=76(LC 16) Max Uplift 1=-17(LC 16), 3=-54(LC 16) Max Grav 1=192(LC 2), 3=192(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 5-5-5 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

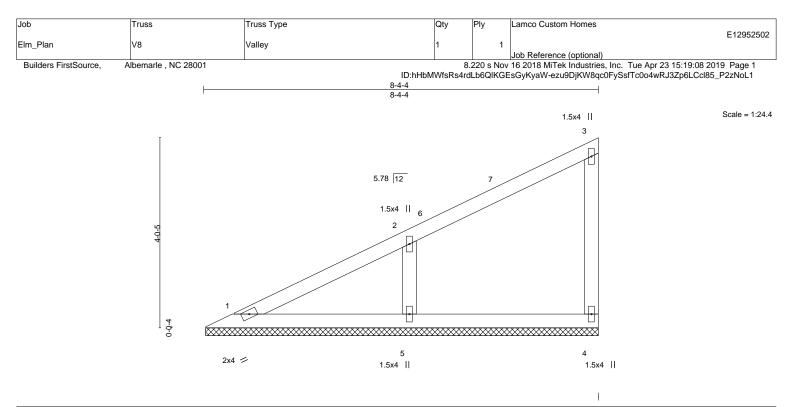
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REPERENCE PAGE MIL-14's rev. Invozens Derrore USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.14 WB 0.08 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 32 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3		TC		Structural wood except end vert Rigid ceiling dir	icals.	, , , ,	olied or 6-0-0 oc purlir) oc bracing.	ns,

REACTIONS. (lb/size) 1=97/8-3-12, 4=104/8-3-12, 5=334/8-3-12

Max Horz 1=120(LC 16) Max Uplift 4=-33(LC 16), 5=-107(LC 16)

Max Grav 1=110(LC 2), 4=118(LC 22), 5=378(LC 2)

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-5=-283/283
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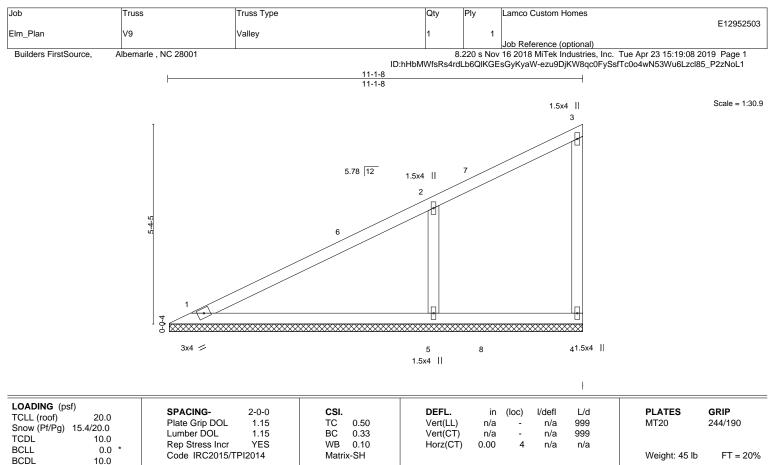
VLD0

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 8-2-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 5=107.



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 BUDL
 10.0

 LUMBER BRACING

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

REACTIONS. (lb/size) 1=185/11-1-0, 4=63/11-1-0, 5=482/11-1-0

Max Horz 1=151(LC 16) Max Uplift 4=-9(LC 18), 5=-141(LC 16)

Max Grav 1=210(LC 2), 4=125(LC 5), 5=545(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-5=-383/290

NOTES-

- Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 10-11-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

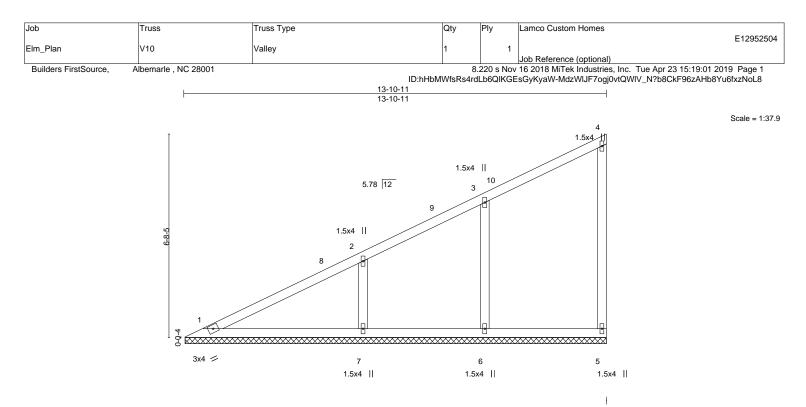
4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 5=141.



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LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.32 BC 0.21 WB 0.10 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl n/a - n/a n/a - n/a 0.00 5 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 61 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			RACING- OP CHORD	Structural wood sheathin	g directly app	lied or 6-0-0 oc purlir	IS,
BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3		В	OT CHORD	except end verticals. Rigid ceiling directly appl	ied or 10-0-0	oc bracing.	

 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-10-3.

(lb) - Max Horz 1=183(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 5, 6 except 7=-124(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=337(LC 5), 7=436(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-251/101

WEBS 2-7=-307/234

NOTES-

OTHERS

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-10-12, Exterior(2) 4-10-12 to 13-8-15 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

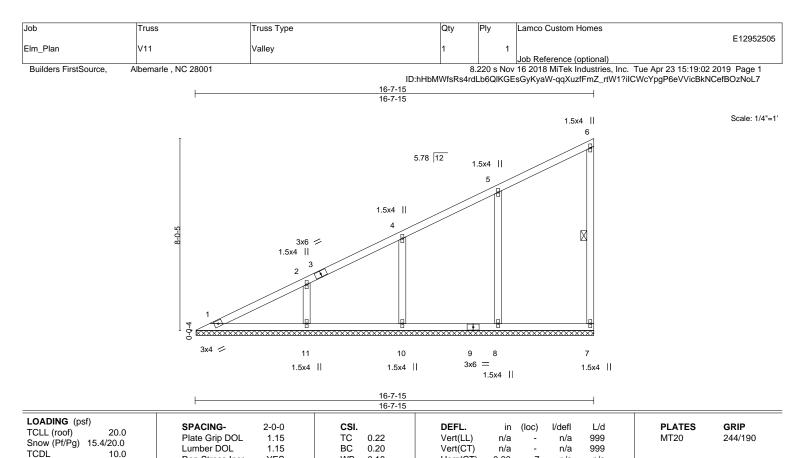
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6 except (jt=lb) 7=124.



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BCLL BCDL	0.0 * 10.0	Code IRC2015/TPI2014	Matrix-SH	Horz(CT)	0.00	1	n/a	n/a	Weight: 78 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS			TC	OT CHORD	except end	vertica g direc	als.	g directly app ed or 10-0-0 6-7	lied or 6-0-0 oc purlins, oc bracing.	

REACTIONS. All bearings 16-7-7.

(lb) - Max Horz 1=214(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 10 except 11=-106(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 7, 1 except 8=454(LC 5), 10=322(LC 3), 11=365(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-307/115

WEBS 5-8=-277/147, 2-11=-262/200

NOTES-

 Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-7-13 to 4-7-15, Exterior(2) 4-7-15 to 16-6-3 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 10 except (jt=lb) 11=106.



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