

Job: 1 Member Type: Beam | Level: 1st Floor MiTek SAPPHIRE™ Structure Version 8.3.1.215.Update6 Designed by Single Member Design Engine

Member: 2 - 1 3/4" x 9 1/4" 2.0E EverEdge LVL

Label: DB1-0 (Dropped)-

Page: 1 of 1 Date: 11/27/2019 10:00:48 Status: Design Passed



Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

					Maximum Loa	d Magnitudes	
<u>Type</u>	<u>Start</u>	End	Source	<u>Dead</u>	Floor Live	Roof Live	<u>Snow</u>
Self Weight	0'	6'- 9 3/4"	Self Weight	9 lb/ft	-	-	-
Uniform	0'- 4 1/2"	6'- 4 1/2"	Smoothed Load	245 lb/ft	-	237 lb/ft	-
Point	1'- 4 1/2"	1'- 4 1/2"	T1A(c03)	-	-	-2.00 lb	-
Point	3'- 4 1/2"	3'- 4 1/2"	T1A(c02)	-	-	-2.00 lb	-
Point	5'- 4 1/2"	5'- 4 1/2"	T1A(c01)	-	-	-2.00 lb	-

Support Information:

			_		Maximum Ana	lysis Reactions	
Support	<u>Start</u>	End	Source	<u>Dead</u>	Floor Live	Roof Live	<u>Snow</u>
1	0'	0'- 3 1/8"	E2(i3)	776.00 lb	-	719.00/-3.00 lb	-
2	6'- 6 3/4"	6'- 9 3/4"	E3(i14)	759.00 lb	-	703.00/-3.00 lb	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as projected dead loads.

* Calculation of lateral stability factor (CL) is based on width of all plies.

* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.

* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.

- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.

Job	Truss		Truss Type		Qty	Ply	Tiffany Weaver Hou	use-Ro	of		
Q-1902247-1	T1		Roof Special		8	1	Job Poforonoo (ont	ional)			
Peak Truss Builders LLC, New	Hill, user			Run: 8.31 S	Sep 9 2019	Print: 8.310) S Sep 9 2019 MiTek Indus	tries, Ind	c. Wed Nov 27 09:2	23:52 F	Page: 1
					ID:SS	SQdbvpjz7F	LKnRogQyVpAycLyo-J21m	hJPw2R	qQ7YRQODW2fgL	_jP6eus2eigdAmv9	∂yF1q5
<u>-1-0-0</u> 1-0-0	<u>6-2-4</u> 6-2-4		<u>11-8-14</u> 5-6-10	<u>17-1-12</u> 5-4-14	+	<u>22-8-1</u> 5-6-5	<u>28-2-6</u> 5-6-5	\rightarrow	<u>34-3-8</u> 6-1-2	<u>35-3-</u> 8 1-0-0	8
9 9 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	23 B1 1	3x5≠ 3 4 4 5 5x6=	3x4 = 3x4 = 4 - 5 8	4: 24 72 44 44 44 44 44 44 44 44 44 44 44 44 44	x5= 6 4	25 13 5x8=	3x4 3x4 10 8 10 8 64 1 3	2x4 17 2 2 2 44=	9	26 10 Sx5=	
Scale = 1:62.5	<u>5-4-0</u> 5-4-0	<u>6-4-0</u> 1-0-0	<u>13-3-0</u> 6-11-0	<u>20-2</u> 6-11	<u>2-0</u> 1-0		<u>26-11-5</u> 6-9-5		<u>34-0-0</u> 7-0-11	<u>34-</u> 3-8 0-3-8	
Loading TCLL (roof) TCDL BCLL RCDL	(psf) Spacing 20.0 Plate Gri 10.0 Lumber 0.0* Rep Street	p DOL DOL ess Incr	2-0-0 1.15 1.15 YES	CSI TC BC WB Matrix MS	0.77 V 0.65 V 0.65 H	DEFL /ert(LL) /ert(CT) lorz(CT)	in (loc) l/defl -0.14 12-13 >999 -0.31 12-13 >999 0.17 10 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190	
	10.0 Code									11 - 2078	
LUMBER TOP CHORD 2x4 SP N BOT CHORD 2x4 SP N	10.2 10.2			E 1 E	BRACING TOP CHOF BOT CHOF	RD RD	Structural wood sheath Rigid ceiling directly ap	ning dire	ectly applied or 3 or 5-0-15 oc brac	3-4-8 oc purlins. ing.	
WEBS 2x4 SP N REACTIONS (lb/size) Max Horiz Max Uplift	№.3 2=-419/0-3-8, (min. 15=2270/(0-3-8 + b 2=-222 (LC 9) 2=-501 (LC 21), 10	0-1-8), 10 earing blo =-166 (LC	0=1012/0-3-8, (min. 0-1- ck), (min. 0-3-9) 11), 15=-204 (LC 11)	-9),			MiTek recommends th installed during truss e Installation guide.	at Stab erection	ilizers and requi a, in accordance	red cross bracir with Stabilizer	ng be
FORCES (Ib) - 1	2=-38 (LC 11), 10= Max. Comp./Max. To	1012 (LC en All for	1), 15=2270 (LC 1) rces 250 (lb) or less exc	cept when shown.	6 04- 05	0/150 0 0	E- 1226/105				
TOP CHORD 2-23= 7-25= 80T CHORD 2-16=	-05/1213, 3-23=-28 -1367/172, 7-8=-14 -1016/125, 15-16=-	36/162, 8- 1016/125,	+=-025/152, 4-5=-950/12 -9=-2329/317, 9-26=-24 , 14-15=-1158/156, 13-1	22, 5-24=-932/136, 42/294, 10-26=-25 14=-43/363, 12-13=	, 0-24=-853 533/271 =0/1530, 10	0-12=-146	5/2164				
WEBS 3-15= 9-12=	-1754/220, 3-14=-1 -274/172	1/1565, 4-	-14=-715/96, 4-13=0/52	6, 6-13=-76/996, 8	-13=-528/2	228, 8-12=	148/715,				
NOTES 1) 2x4 SP No.2 bearing 2) Unbalanced roof live 3) Wind: ASCE 7-10; Vu and C-C Exterior (2) vertical left and right	block 12" long at jt loads have been co ult=120mph (3-seco -1-0-0 to 2-5-2, Inte	. 15 attach onsidered ond gust) V erior (1) 2-{	ned to front face with 2 for this design. /asd=95mph; TCDL=6.0 5-2 to 17-1-12, Exterior of forces & MW/ERS for	rows of 10d (0.131 0psf; BCDL=6.0psf; (2) 17-1-12 to 20-6 reactions shown: I	"x3") nails ; h=30ft; B: 5-14, Interio	spaced 3 =20ft; L=3 or (1) 20-6	" o.c. 8 Total fasteners. I 94ft; eave=4ft; Cat. II; Ex 6-14 to 35-3-8 zone; can 1ate grin DOI =1 60	Bearing p B; Er tilever l	is assumed to b nclosed; MWFRS left and right exp	be SPF No.2. S (directional) bosed ; end	

vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and 4) any other members.

5)

Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 501 lb uplift at joint 2, 204 lb uplift at joint 15 and 166 lb uplift at joint 10. This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1. 6) 7)

Job	Truss	Truss Type		Qty	Ply	Tiffany Weaver House-Roof	
Q-1902247-1	T1A	Roof Special		3	1	Job Reference (optional)	
Peak Truss Builders LLC, New H	lill, user		Run: 8.31 S Sep	9 2019 Pri ID:SSQ	int: 8.310 S dbvpjz7FLK	Sep 9 2019 MiTek Industries, Inc. Wed Nov 27 09:23:52 nRogQyVpAycLyo-J21mhJPw2RqQ7YRQODW2fgLjP6ees2ei	Page: 1 gdAmv9yF1q5
<u>-1-0-0</u> 1-0-0	<u>6-2-4</u> 6-2-4	<u>11-8-14</u> 5-6-10	<u>17-1-12</u> 5-4-14	ŀ	<u>22-8-1</u> 5-6-5	<u>28-2-6</u> <u>34-3-8</u> 5-6-5 6-1-2	
G	3x5= 3 3 15 4 5x6=	3x4 3x4 8 8 12 4 5 1 2 8 5	4 = 23 	x5= 6 1 1 1 1 1 1 1 1 1	344 12 5x8=	3x4x 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	5-2-4 10 3x5=
Scale = 1:61	<u>5-4-0 6-4-0</u> 5-4-0 1-0-0	<u>13-3-0</u> 6-11-0	<u>20-2</u> 6-11	<u>2-0</u> -0		26-11-5 34-0-0 6-9-5 7-0-11	<u>34-</u> 3-8 0- 3 -8
Loading TCLL (roof) TCDL BCLL BCDL	(psf)Spacing20.0Plate Grip DOL10.0Lumber DOL0.0*Rep Stress Incr10.0Code	2-0-0 1.15 1.15 YES IBC2015/TPI2014	CSI TC BC WB Matrix-MS	0.77 Ver 0.67 Ver 0.65 Hor	FL t(LL) · t(CT) · z(CT)	in (loc) I/defi L/d PLATES GRIP 0.14 11-12 >999 240 MT20 244/190 0.31 11-12 >999 180 0.17 10 n/a n/a Weight: 185 lb FT = 20	%
LUMBER TOP CHORD 2x4 SP N BOT CHORD 2x4 SP N WEBS 2x4 SP N REACTIONS (Ib/size) 2	0.2 0.2 0.3 /=-419/0-3-8, (min. 0-1-8), 10 4=2271/(0.3-8 + bearing blo	=951/0-3-8, (min. 0-1-8)	BR TO BO	ACING P CHORD T CHORD	S R I I	tructural wood sheathing directly applied or 3-3-1 oc p igid ceiling directly applied or 5-0-15 oc bracing. AiTek recommends that Stabilizers and required cross nstalled during truss erection, in accordance with Stat nstallation quide.	urlins. bracing be illizer
Max Horiz 2 Max Uplift 2 Max Grav 2	=218 (LC 10) =-501 (LC 21), 10=-125 (LC =-15 (LC 11), 10=951 (LC 1)	11), 14=-233 (LC 11) . 14=2271 (LC 1)			L	<u> </u>	
FORCES (lb) - M TOP CHORD 2-22=- 7-24=- - BOT CHORD 2-15=- WEBS 3-14=- 9-11=-2 9-11=-2	lax. Comp./Max. Ten All for 101/1213, 3-22=-68/1338, 3- 1370/193, 7-8=-1440/183, 8- 1016/128, 14-15=-1016/128, 1755/248, 3-13=-42/1567, 4- 277/174	ces 250 (lb) or less exce 4=-626/147, 4-5=-952/1: 9=-2342/345, 9-25=-245 13-14=-1158/159, 12-13 13=-715/113, 4-12=0/52	ept when shown. 34, 5-23=-934/147, (51/322, 10-25=-2547 3=-41/359, 11-12=0/ 7, 6-12=-95/999, 8-1	6-23=-855/ 7/303 1535, 10-1 12=-530/23	/170, 6-24 1=-206/21 30, 8-11=-	=-1339/216, 78 155/725,	
 NOTES 1) 2x4 SP No.2 bearing to 2) Unbalanced roof live to 3) Wind: ASCE 7-10; Vul and C-C Exterior (2) - vertical left and right e 4) * This truss has been and the second second	block 12" long at jt. 14 attach oads have been considered f t=120mph (3-second gust) V 1-0-0 to 2-5-2, Interior (1) 2-5 xposed;C-C for members an designed for a live load of 20	ed to front face with 2 rd for this design. asd=95mph; TCDL=6.0p -2 to 17-1-12, Exterior (d forces & MWFRS for r .0psf on the bottom cho	ows of 10d (0.131"x osf; BCDL=6.0psf; h 2) 17-1-12 to 20-6-1 eactions shown; Lur rd in all areas where	3") nails sp =30ft; B=2 4, Interior mber DOL= a rectang	oaced 3" o 0ft; L=34fi (1) 20-6-1 =1.60 plat le 3-06-00	.c. 8 Total fasteners. Bearing is assumed to be SPF N ; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (direction 4 to 34-3-8 zone; cantilever left and right exposed ; er e grip DOL=1.60 1 tall by 2-00-00 wide will fit between the bottom chord	o.2. nal) d and

any other members. Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 501 lb uplift at joint 2, 233 lb uplift at joint 14 and 125 lb uplift at joint 10. This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1. 5)

6) 7)



7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 213 lb uplift at joint 2, 206 lb uplift at joint 10 and 213 lb uplift at joint 2.

8) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.



				-									
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.71	Vert(LL)	-0.36	10-12	>931	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.87	Vert(CT)	-0.46	10-12	>739	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.28	Horz(CT)	0.05	8	n/a	n/a			
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS							Weight: 142 lb	FT = 20%	

BRACING

LUMBER

TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 3-7-13 oc purlins. BOT CHORD 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.3 WEBS MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer **REACTIONS** (lb/size) 2=1190/0-3-8, (min. 0-1-14), 8=1190/0-3-8, (min. 0-1-14) Installation guide. Max Horiz 2=181 (LC 10) Max Uplift 2=-174 (LC 11), 8=-174 (LC 11) (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES

TOP CHORD 2-19=-1628/206, 3-19=-1562/247, 3-20=-1482/304, 4-20=-1372/304, 4-5=-1357/324, 5-6=-1357/324, 6-21=-1372/304, 7-21=-1482/304, 7-22=-1562/247, 8-22=-1628/206

BOT CHORD

2-12=-74/1399, 12-23=0/904, 11-23=0/904, 11-24=0/904, 10-24=0/904, 8-10=-74/1300

WEBS 5-10=-112/735, 7-10=-414/231, 5-12=-112/735, 3-12=-414/231

NOTES

Unbalanced roof live loads have been considered for this design. 1)

Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=20ft; L=28ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) 2) and C-C Exterior (2) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 14-1-8, Exterior (2) 14-1-8 to 17-1-8, Interior (1) 17-1-8 to 29-3-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and 3) any other members, with BCDL = 10.0psf.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 2 and 174 lb uplift at joint 8. 4)

5) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

Job	Truss		Truss Type		Qty	Ply		Tiffany We	eaver Ho	ouse-Ro	of	
Q-1902247-1	T2A		Roof Special		7	1		loh Refer	ence (or	tional)		
Peak Truss Builders LLC, I	New Hill, user			Run: 8.3	1 S Sep 9 20 ⁻	19 Print: 8.31	10 S Se	p 9 2019 N	/iTek Indu	istries, In	nc. Wed Nov 27 09:	23:53 Pag
						ID:SSQdbvp	jz7FLKı	nRogQyVp	AycLyo-ni	Eb8ufQY	plyHlh0cyw2HCuu	v6Vy_bUzrvGwKRbyF
<u>-1</u> .	-0-0	7-4-3		<u>14-1-8</u> 6-9-5		20-	<u>11-13</u> 10-5		_		28-3-0 7-3-3	29-3-0
11-	0-0	140	Ι	000	ļ	0	10 0		1		100	1-0-0
10 5 3	15 2 3x5=	B1 	8 12 8 5x10 ≠ 3 12 12 5x8 B2	3x5≠ 4 20 10	4x5= 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3x5 ¢	1 Wit		3x5≈ 7 ₩ 5 10 2x4॥	B3	2	2 8 9 3x4=
Scale = 1:53.5	0 <u>3-8</u> 03-8	7-0-0 6-8-8	13- 6-5	5-1 <u>5</u> i-15	<u>13-</u> 8-8 0-2-9	<u>20-1</u> 7-3	<u>1-13</u> i-5		1		<u>28-3-0</u> 7-3-3	
], [11.0-3-4,0-2-6], [1]	2.0-3-11,0-2-8]				_					
Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15	TC	0.71	DEFL Vert(LL)	-0.1	in (loc) 6 11-12) I/defl >999	L/d 240	PLATES MT20	GRIP 244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.79	Vert(CT)	-0.3	37 11-12	>925	180 p/o		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS	0.71		0.4	21 C) n/a	n/a	Weight: 146 lb	FT = 20%
LUMBER TOP CHORD 2x4 3 BOT CHORD 2x4 3 WEBS 2x4 3 REACTIONS (lb/size) Max Ho Max Up FORCES (lt TOP CHORD 2- 7- BOT CHORD 2- WEBS 3-	SP No.2 SP No.2 SP No.3) 2=1190/C riz 2=-181 (I ulift 2=-174 (I o) - Max. Con .19=-3071/26 .21=-1139/23 .12=-139/261 .12=-119/261)-3-8, (min. 0-1-14), 8 _C 9) _C 11), 8=-174 (LC 1 np./Max. Ten All for 7, 3-19=-2962/302, 7 4, 7-22=-1592/233, 8 7, 11-12=-135/2373, 3-11=-1723/250. 5-	3=1190/0-3-8, (min. 0-1- 1) cces 250 (lb) or less exc 3-20=-1173/242, 4-20=- 3-22=-1690/197 10-11=-62/1325, 8-10= 11=-132/777, 7-11=-594	-14) cept when sho 1049/242, 4-5 :-62/1325 4/180	BRACIN TOP CHO BOT CHO WEBS wn. =-1029/262,	G DRD DRD 5-6=-991/2	Stru Rigi 1 Ro MiT Inst Inst	ctural woo d ceiling o <u>ow at mid</u> ek recom alled duri allation g 21=-1011/	od sheat directly a pt mends t ng truss uide. /234,	thing dir ipplied c hat Stat erection	rectly applied or or 10-0-0 oc brao 3-11, 7-11 bilizers and requ n, in accordance	2-5-10 oc purlins. cing. ired cross bracing with Stabilizer
NOTES 1) Unbalanced roof 2) Wind: ASCE 7-10	live loads ha	ve been considered	for this design. /asd=95mph: TCDI =6.0)nsf: BCDI =6	Onsf h=30ft	B=20ft·I=	28ft∙ ≏	ave=4ft· (Cat II·⊑	vn B· ⊏	nclosed: MW/FR	S (directional)

Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; 1CDL=6.0pst; BCDL=6.0pst; h=30ft; B=20ft; L=26ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior (2) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 14-1-8, Exterior (2) 14-1-8 to 17-1-8, Interior (1) 17-1-8 to 29-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and

3) any other members.

Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 2 and 174 lb uplift at joint 8. 4)

5)

6) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.



26, 27, 28, 29, 30, 31, 32, 2, 18

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=20ft; L=28ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner (3) -1-0-0 to 2-1-8, Exterior (2) 2-1-8 to 14-1-8, Corner (3) 14-1-8 to 17-1-8, Exterior (2) 17-1-8 to 29-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20, 2.

9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 18.

10) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.



Max Grav All reactions 250 (lb) or less at joint(s) 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 2, 18

FORCES (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=20ft; L=28ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner (3) -1-0-0 to 2-1-8, Exterior (2) 2-1-8 to 14-1-8, Corner (3) 14-1-8 to 17-1-8, Exterior (2) 17-1-8 to 29-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 27, 28, 29, 30, 31, 32, 25, 24, 23, 22, 21, 20, 2.

9) This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

THIS LAYOUT IS TO BE USED AS A TRUSS PLACEMENT GUIDE ONLY. PLEASE REFER TO BUILDING PLANS FOR BUILDING CONSTRUCTION AND DETA SUCH AS PLUMBING OR DUCT DROPS.



Roof framed by others.

Tiff

AILS,	PROPOSED DESIGN- NOT FOR CONSTRUCTION Notes: 1. Exterior dimensions shown are assumed to be: □ Out-to-out of stud ☑ Out-to-out of sheathing 2. Adjust truss locations as needed for plumbing and mechanical clearance Unless	3	Job #		1477081-20	
	otherwise noted, trusses may b shifted as long as O.C. spacing shown is not exceeded. 3. Do not cut, drill, or otherwise damage any part of any truss without prior approval from Peer Truss. 4. Do not approve drawings if any information herein is unclear. Once ordered trusses will be fabricated as approved. 5. Please contact Peak Truss Builders with any questions. W are available to help any way we can. We can be reached a 919-545-5555 or sales@peaktruss.com Roof Truss Loading per 2018 NC Residential Code Top Chord Live Load 20# P Pop Chord Dead Load 10# P Bottom Chord Live Load 0# P Bottom Chord Live Load 04 # P	De D	Tiffanv Weaver House	8 Main St	Lillington NC	
	Trusses are designed for additional storage load wherever a 42"x24" box will fit between the webs. △ - This symbol denotes left end truss as shown on truss drawi ● - Approximate location of toile drop. Builder please confirm. Truss connections by others: ⟨N⟩-Nailed (L) -Ledger	al d of t t	Date Quoted:		Designer:	Devon Thompson
fany Wea Roof T ' OC, 1' (aver House russes Overhang		Southeastern Interiors	228 Airport Rd	Erwin,NC	28339
				Peak Truss	Builders, LLC	PO Box 340, New Hill, NC 27562



Book Trace	Southeastern Interiors	Date Quoted:	Tiffany Weaver House	Job #
PO Box 340, New Hill, NC 27562	228 Airport Rd Erwin,NC 28339	Designer: Devon Thompson	Lillington NC	Q-1902247