

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 2126477 STOUT / LOT A

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E13725487 thru E13725524

My license renewal date for the state of North Carolina is December 31, 2019.

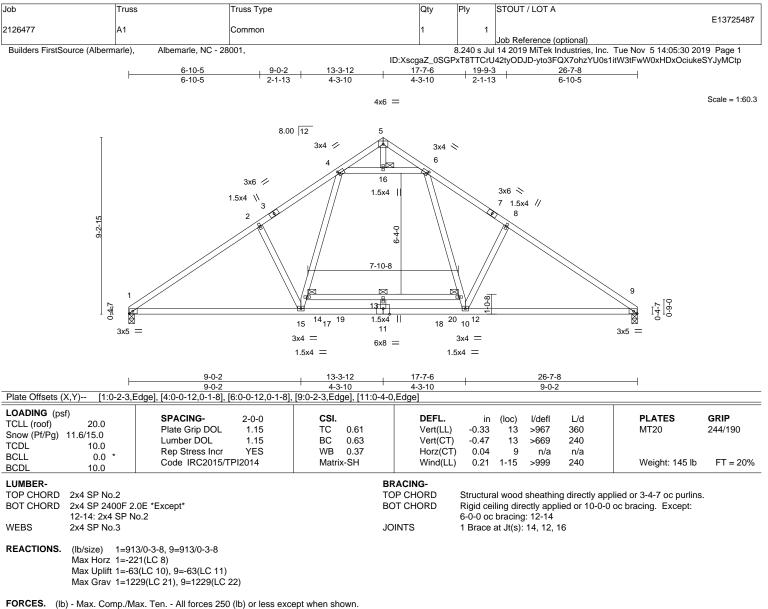
North Carolina COA: C-0844



November 6,2019

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



- TOP CHORD 1-2=-1853/76, 2-4=-1716/139, 6-8=-1717/139, 8-9=-1853/76
- BOT CHORD 1-12=-103/10, 2-4=-1710/139, 0-9=-1717/139, 0-9=-1803/10 1-15=-109/1600, 11-15=0/1266, 10-11=0/1266, 9-10=0/1455
- WEBS 6-12=-69/816, 10-12=-102/704, 8-10=-369/265, 14-15=-102/704, 4-14=-69/816,
 - 2-15=-369/264, 4-16=-1064/122, 6-16=-1064/122
- NOTES-

1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

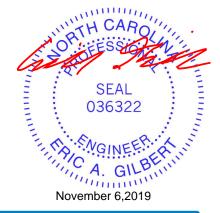
3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

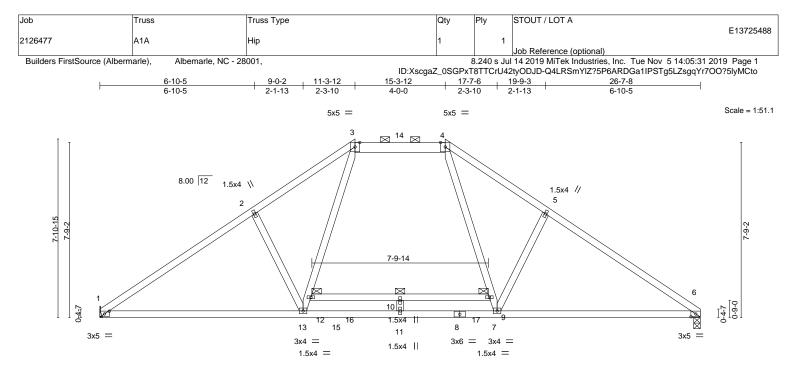
6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 9. This connection is for uplift only and does not consider lateral forces.



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



	9-0-2 9-0-2	13-5-8 4-5-6	17-7-6 4-1-14	-	26-7-8 9-0-2		
LOADING (psf)	D-1-8], [6:0-2-9,0-1-8] SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP
TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	TC 0.65 BC 0.86 WB 0.39	Vert(CT) -	0.36 10 0.53 10 0.05 6	>879 360 >604 240 n/a n/a	MT20	244/190
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-SH	- (-)	0.21 1-13	>999 240	Weight: 140 lb	FT = 20%
LUMBER-		E	BRACING-				

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2 *Except*	TOP CHORD	Structural wood sheathing directly applied or 3-2-2 oc purlins, except
	3-4: 2x6 SP No.2		2-0-0 oc purlins (6-0-0 max.): 3-4.
BOT CHORD	2x4 SP No.1 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
	9-12: 2x4 SP No.2		6-0-0 oc bracing: 9-12
WEBS	2x4 SP No.3		
DELOTIONO	(lb /-i) 4 005/Mbi 0 000/0 0 0		

REACTIONS. (lb/size) 1=935/Mechanical, 6=936/0-3-8 Max Horz 1=-185(LC 8) Max Uplift 1=-72(LC 10), 6=-71(LC 11) Max Grav 1=1185(LC 21), 6=1184(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 1-2=-1818/94, 2-3=-1661/155, 3-4=-1117/128, 4-5=-1651/154, 5-6=-1809/92
- BOT CHORD 1-13=-100/1524, 11-13=0/1176, 7-11=0/1176, 6-7=0/1419
- WEBS 4-9=-72/792, 7-9=-106/672, 5-7=-342/263, 12-13=-108/691, 3-12=-73/811, 2-13=-356/264

NOTES-

1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Provide adequate drainage to prevent water ponding.

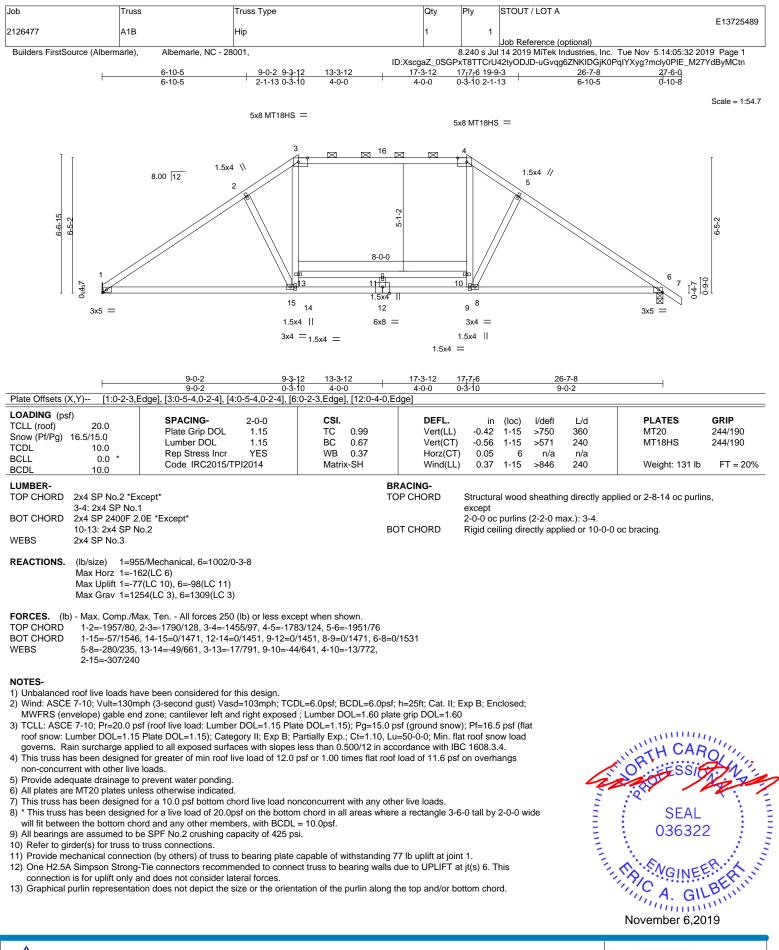
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 1.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This
 connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

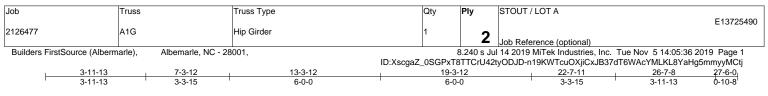


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. ENGINEERING BY REENCED A Mitek Affiliate 818 Soundside Road Edenton, NC 27932

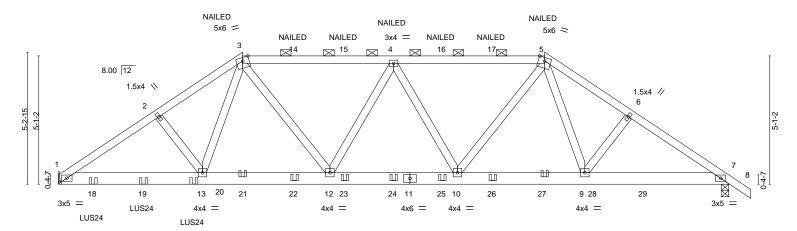


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Scale = 1:45.8



5-8-10	10-9-6	15-10-2	20-10-14		26-7-8	
5-8-10	5-0-12	5-0-12	5-0-12		5-8-10	_
LOADING (psf)					-	
TCLL (roof) 20.0	SPACING- 2-0-0	CSI.	DEFL. in (loc)	l/defl L/d	PLATES	GRIP
Snow (Pf/Pg) 16.5/15.0	Plate Grip DOL 1.15	TC 0.35	Vert(LL) -0.07 10-12	>999 360	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.52 WB 0.22	Vert(CT) -0.13 10-12	>999 240		
BCLL 0.0 *	Rep Stress Incr NO Code IRC2015/TPI2014	WB 0.22 Matrix-SH	Horz(CT) 0.04 7 Wind(LL) 0.08 10-12	n/a n/a >999 240	Weight: 324 lb	FT = 20%
BCDL 10.0	Code IRC2013/1F12014	Matrix-SH	WINd(EE) 0.08 10-12	>999 240	Weight. 324 lb	F1 = 20%
LUMBER-		BRAC	ING-			
TOP CHORD 2x4 SP No.2		TOP C	HORD Structural wood	sheathing directly app	lied or 6-0-0 oc purlins	, except
BOT CHORD 2x6 SP No.2			2-0-0 oc purlins	(6-0-0 max.): 3-5.		
WEBS 2x4 SP No.3		BOT C	HORD Rigid ceiling dire	ctly applied or 10-0-0	oc bracing.	
Max Horz 1=-1 Max Uplift 1=-7	964/Mechanical, 7=1916/0-3-8 28(LC 30) 06(LC 10), 7=-636(LC 11) !53(LC 2), 7=2365(LC 2)					
	lax. Ten All forces 250 (lb) or less exce 78, 2-3=-3611/1096, 3-4=-3269/978, 4-5=					
)9 68, 12-13=-757/2695, 10-12=-938/3414, §	0-10646/2667 7-9752/20	987			
	3, 3-12=-303/1017, 4-12=-352/178, 4-10=	,				
5-9=-227/853	, 0 12 000, 1011, 1 12 002, 1 0, 1 10					
 Top chords connected as follo Bottom chords connected as i Webs connected as follows: 2 2) All loads are considered equa ply connections have been pr 3) Unbalanced roof live loads ha 4) Wind: ASCE 7-10; Vult=130m MWFRS (envelope) gable em 5) TCLL: ASCE 7-10; Pr=20.0 pi roof snow: Lumber DOL=1.15 exposed surfaces with slopes 6) This truss has been designed non-concurrent with other live 7) Provide adequate drainage to 8) This truss has been designed 9) * This truss has been designed 9) * This truss has been designed 10) All bearings are assumed to 12) Provide mechanical connect 13) Two H2.5A Simpson Strong connection is for uplift only a 	follows: 2x6 - 2 rows staggered at 0-9-0 d 2x4 - 1 row at 0-9-0 oc. ally applied to all plies, except if noted as ovided to distribute only loads noted as (ave been considered for this design. hph (3-second gust) Vasd=103mph; TCD d zone; cantilever left and right exposed sf (roof live load: Lumber DOL=1.15 Plats 5 Plate DOL=1.15); Category II; Exp B; Pr less than 0.500/12 in accordance with IE 1 for greater of min roof live load of 12.0 p e loads. b prevent water ponding. I for a 10.0 psf bottom chord live load nor of d ro a live load of 20.0psf on the bottom ord and any other members. be SPF No.2 crushing capacity of 425 p to truss connections. tion (by others) of truss to bearing plate of and does not consider lateral forces.	bc. front (F) or back (B) face in t F) or (B), unless otherwise ir L=6.0psf; BCDL=6.0psf; h=2 ; Lumber DOL=1.60 plate gri e DOL=1.15); Pg=15.0 psf (c artially Exp.; Ct=1.10, Lu=50 3C 1608.3.4. bsf or 1.00 times flat roof load inconcurrent with any other live o chord in all areas where a r si. apable of withstanding 706 l t truss to bearing walls due to	hdicated. 5ft; Cat. II; Exp B; Enclosed; p DOL=1.60 round snow); Pf=16.5 psf (fla -0-0 Rain surcharge applied d of 11.6 psf on overhangs re loads. ectangle 3-6-0 tall by 2-0-0 w b uplift at joint 1. to UPLIFT at jt(s) 7. This	t to all		BERT
	ion does not depict the size or the orienta	ation of the purlin along the to	op and/or bottom chord.		November 6,20	19
Continued on page 2				1		
Design valid for use only with MiTek	neters and READ NOTES ON THIS AND INCLUDED © connectors. This design is based only upon paran fing designer must verify the applicability of design p	neters shown, and is for an individua	I building component, not			

Design Valid for use only with Mil RK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	STOUT / LOT A
					E13725490
2126477	A1G	Hip Girder	1	2	
				_	Job Reference (optional)
Builders FirstSource (Alberm	arle), Albemarle, NC - 28	001,	8	3.240 s Ju	14 2019 MiTek Industries, Inc. Tue Nov 5 14:05:36 2019 Page 2
		ID:XscgaZ	_0SGPxT8	3TTCrU42t	yODJD-n19KWTcuOXjiCxJB37dT6WAcYMLKL8YaHg5mmyyMCtj

NOTES-

15) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-4-8 from the left end to 5-4-8 to connect truss(es) to front face of bottom chord.

16) Fill all nail holes where hanger is in contact with lumber.

17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

18) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 172 lb down and 70 lb up at 7-4-8, 172 lb down and 70 lb up at 9-4-8, 172 lb down and 70 lb up at 11-4-8, 172 lb down and 70 lb up at 11-3-12, 172 lb down and 70 lb up at 15-3-0, 172 lb down and 70 lb up at 11-3-0, 172 lb down and 70 lb up at 11-3-0, 172 lb down and 70 lb up at 11-3-0, and 207 lb up at 13-3-12, 172 lb down and 70 lb up at 13-3-0, 172 lb down and 70 lb up at 11-3-0, 172 lb down and 70 lb up at 11-3-0, 172 lb down and 70 lb up at 11-3-0, and 297 lb down and 109 lb up at 23-3-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

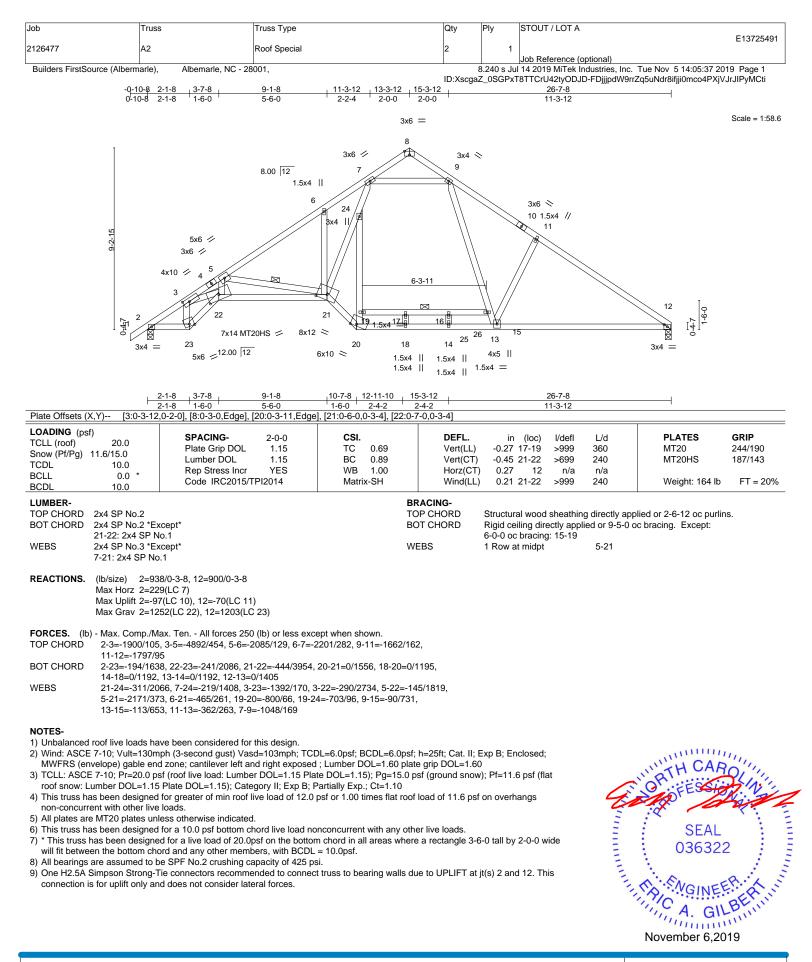
Uniform Loads (plf)

Vert: 1-3=-43, 3-5=-53, 5-8=-43, 1-7=-20

Concentrated Loads (lb)

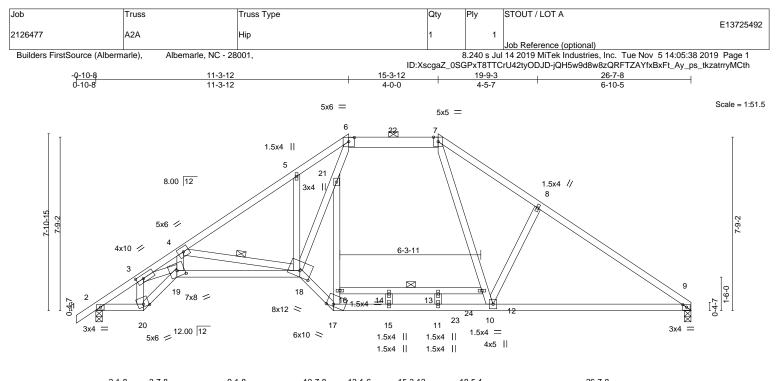
Vert: 3=-13(F) 5=-13(F) 4=-8(F) 14=-8(F) 15=-8(F) 16=-8(F) 17=-8(F) 18=-185(F) 19=-185(F) 20=-185(F) 21=-137 22=-137 23=-137 24=-137 25=-137 26=-137 27=-137 28=-214 29=-251





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L	2-1-8	3-7-8	9-1-8	10-7-8	13-1-6	15-3-12 1	8-5-1 _I	26-7-8		
F	2-1-8	1-6-0	5-6-0	1-6-0	2-5-14	2-2-6 3	i-1-5	8-2-7		
Plate Offsets (X,Y)	[3:0-3-12	,0-2-0], [6:0-3-0,0-2-3	3], [17:0-3-11,Edge	e], [18:0-6-(0,0-3-4], [19:0	-4-0,0-3-4]				
Snow (Pf/Pg) 16.5/1	20.0 5.0 10.0	SPACING- Plate Grip DO Lumber DOL	1.15	CS TC BC		DEFL. Vert(LL) Vert(CT)	-0.26 14 -0.42 18	L/d 360 240	PLATES MT20	GRIP 244/190

BCLL BCDL	0.0 * 10.0	Code IRC2015/TPI2014	Matrix-SH	Wind(LL)	0.25 9 0.19 9-10		n/a 240	Weight: 161 lb	FT = 20%
LUMBER-			BF	RACING-					
TOP CHORD	2x4 SP 2400F 2	.0E *Except*	TC	OP CHORD	Structural woo	d sheathir	ng directly ap	plied or 3-1-13 oc purlins,	,
	6-7: 2x6 SP No.:	2, 7-9: 2x4 SP No.2			except				
BOT CHORD	2x4 SP No.2 *E>	xcept*			2-0-0 oc purlin	s (6-0-0 m	ax.): 6-7.		
	18-19: 2x4 SP N	lo.1	BC	DT CHORD	Rigid ceiling di	rectly app	lied or 9-9-9	oc bracing. Except:	
WEBS	2x4 SP No.3 *Ex	xcept*			6-0-0 oc bracir	ig: 12-16			
	6-18: 2x4 SP No	p.1	W	EBS	1 Row at midp	t	4-18		
REACTIONS.	(lb/size) 9=92	20/0-3-8, 2=958/0-3-8							

Max Horz 2=194(LC 7) Max Uplift 9=-78(LC 11), 2=-105(LC 10) Max Grav 9=1167(LC 23), 2=1216(LC 22)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1838/117, 3-4=-4657/438, 4-5=-2006/137, 5-6=-2069/283, 6-7=-1107/141, 7-8=-1609/171, 8-9=-1765/107

 BOT CHORD
 2-20=-180/1561, 19-20=-223/1990, 18-19=-408/3735, 17-18=0/1476, 15-17=0/1136, 11-15=0/1134, 10-11=0/1134, 9-10=0/1384

 WEBS
 3-20=-1326/158, 3-19=-265/2573, 4-19=-129/1735, 4-18=-2050/352, 5-18=-404/256, 18-21=-292/1933, 6-21=-209/1322, 16-17=-759/58, 16-21=-729/88, 7-12=-82/712,

NOTES-

1) Unbalanced roof live loads have been considered for this design.

10-12=-106/632, 8-10=-328/255

 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-10; PT=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
 5) Browide adequate designed to prevent water pending.
- 5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.

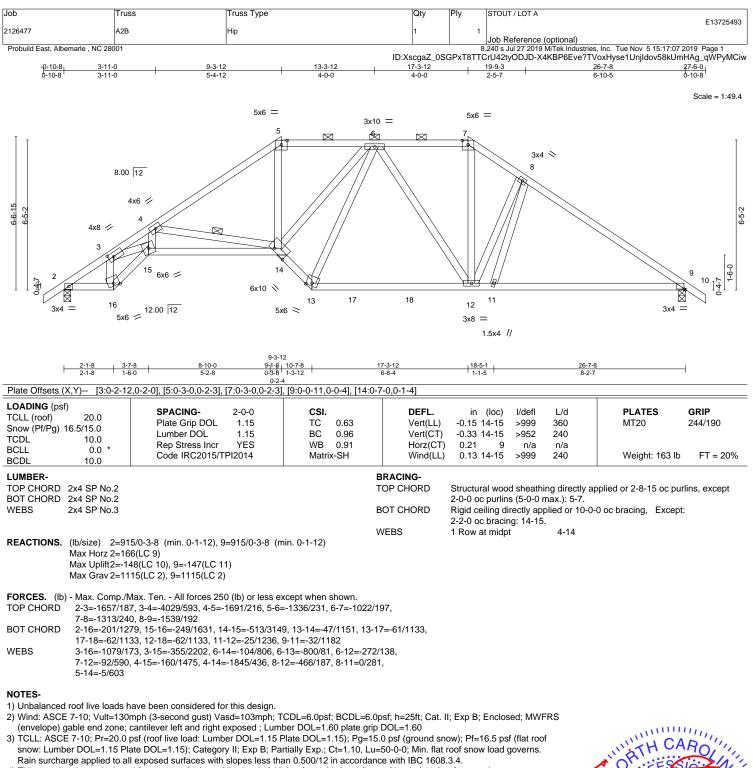
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9 and 2. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
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- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

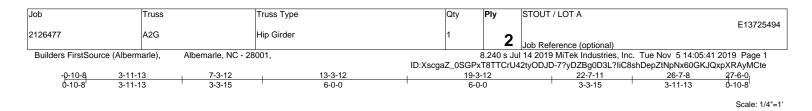
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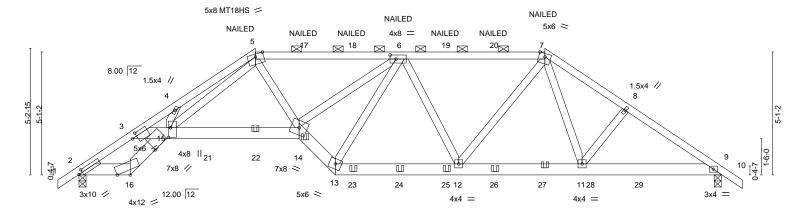
LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP17 Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.









F	2-1-8	3-7-8	9-1-8	10-7-8	-	15-8-14		20-10-4		-	26-7-8	4
Plate Offsets (2-1-8 X Y) [2	1-6-0 2·0-2-0 0-	<u> </u>	<u> 1-6-0</u> 15 0-2-0] [15 [.]	0-4-12 0-1-0	5-1-6	-0 Edge] [16:0-	5-1-6 6-0 Edgel			5-9-4	·
LOADING (ps	, , ,			,	· · · z , · · · · ·	, [10.1		5 5,20 <u>9</u> 0]				
TCLL (roof)	20.0		SPACING- 2-0	-	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
Snow (Pf/Pg)				15	TC 0.76		Vert(LL)	-0.22 14-15	>999	360	MT20	244/190
TCDL	10.0			15	BC 0.99		Vert(CT)	-0.43 14-15	>739	240	MT18HS	244/190
BCLL	0.0) *	Rep Stress Incr I Code IRC2015/TPI201	NO I	WB 0.82 Matrix-SH	2	Horz(CT) Wind(LL)	0.25 9 0.24 14-15	n/a >999	n/a 240	Weight: 345 lb	FT = 20%
BCDL	10.0)		4	Matrix-Si i		WING(LL)	0.24 14-13	>999	240	Weight: 343 lb	FT = 20%
LUMBER-							RACING-					
TOP CHORD			E *Except*			TC	P CHORD				pplied or 5-8-2 oc purlins	, except
		SP No.2						2-0-0 oc purlins			0	
BOT CHORD		No.2 *Exc x6 SP DS				BC	OT CHORD	Rigid ceiling dire	ectly app	lied of 10-0	-0 oc bracing.	
WEBS	2x4 SP I		5									
WEBO	2,4 01 1	10.0										
REACTIONS.	(lb/size)	2=194	2/0-3-8, 9=1928/0-3-8									
		rz 2=135										
			D(LC 10), 9=-593(LC 11)									
	Max Gra	av 2=237	7(LC 2), 9=2368(LC 2)									
FORCES (It) - Max C	Comp /Ma	x. Ten All forces 250 (lb) o	r less excent v	hen shown							
TOP CHORD			, 3-4=-8469/2132, 4-5=-8110			7=-3276	/886,					
	7-8=-3	555/953,	8-9=-3673/935	,	,		,					
BOT CHORD			, 15-16=-716/2791, 3-15=-10	,		,						
			7, 12-13=-919/3431, 11-12=	,			0.007					
WEBS			5-15=-1111/3976, 5-14=-413 7-12=-323/1039, 7-11=-236/		791/2944, 6-	13=-226	2/627,					
	0-12	505/150,	7-12-323/1033, 7-11-230/	000								
 Top chords Bottom chor Webs connet 2) All loads are ply connecti 3) Unbalanced 4) Wind: ASCE MWFRS (er 5) TCLL: ASCI roof snow: L exposed su 6) This truss h non-concurr 7) Provide add 8) All plates ar 9) This truss h 10) * This truss will fit betw 	connected rds conne- ected as fit a consider ions have f roof live I 5 7-10; Vu hvelope) g E 7-10; Pr Lumber DC rfaces with as been d rent with o equate dra e MT20 p as been d s has been	d as follow cted as fo ollows: 2x red equally been pro- loads hav: lit=130mp able end =20.0 psf DL=1.15 F h slopes li h slopes li hesigned fi ther live li inage to p lates unle lesigned fi n designe ottom cho	ether with 10d (0.131"x3") n: vs: 2x4 - 1 row at 0-9-0 oc. Illows: 2x6 - 2 rows staggere 4 - 1 row at 0-9-0 oc. y applied to all plies, except vided to distribute only loads e been considered for this d h (3-second gust) Vasd=103 zone; cantilever left and righ (roof live load: Lumber DOL 21ate DOL=1.15); Category I 2ass than 0.500/12 in accorda or greater of min roof live loa oads. or grevent water ponding. ss otherwise indicated. or a 10.0 psf bottom chord liv d for a live load of 20.0psf o ord and any other members. ie SPF No.2 crushing capaci	d at 0-9-0 oc. if noted as fror noted as (F) c esign. imph; TCDL=6 t exposed ; Lu =1.15 Plate DU t; Exp B; Partia nce with IBC of d of 12.0 psf c	r (B), unless .0psf; BCDL= mber DOL=1 DL=1.15); Pg Illy Exp.; Ct= 608.3.4. r 1.00 times	e6.0psf; .60 plat =15.0 p 1.10, Lu flat roof	se indicated. h=25ft; Cat. II; E grip DOL=1.60 sf (ground snow) =50-0-0 Rain su load of 11.6 psf er live loads.	xp B; Enclosed;); Pf=16.5 psf (fla ircharge applied on overhangs	at to all	Mannan	SEAL 036322 November 6,20	BERT
	- Verify des	ign paramet	ters and READ NOTES ON THIS AN	D INCLUDED MIT	EK REFERENCE	PAGE M	II-7473 rev. 10/03/201	5 BEFORE USE.		Т	ENGINEERING BY	
Design valid f	or use only v	vith MiTek®	connectors. This design is based or	ly upon parameter	s shown, and is	for an indi	vidual building comp	onent, not			TDENC	
building desig	n. Bracing in	ndicated is to	g designer must verify the applicabi o prevent buckling of individual truss	web and/or chord	members only.	Additiona	temporary and perm	nanent bracing				filiata
is always requ	uired for stab	ility and to p	revent collapse with possible persor and bracing of trusses and truss sys	al injury and prop	erty damage. Fo	or general	guidance regarding to aria, DSB-89 and BC	he	nent		A MILIEK A	imate
Safety Inforn	nation avail	able from Tr	uss Plate Institute, 218 N. Lee Stree	t, Suite 312, Alexa			.,	g compo			818 Soundside Road Edenton, NC 27932	

Job	Truss	Truss Type	Qty	Ply	STOUT / LOT A
					E13725494
2126477	A2G	Hip Girder	1	2	
				_	Job Reference (optional)
Builders FirstSource (Alberma	arle), Albemarle, NC - 28	001,	8	3.240 s Jul	14 2019 MiTek Industries, Inc. Tue Nov 5 14:05:41 2019 Page 2
		ID:Xscga	Z_0SGPx	T8TTCrU4	2tyODJD-7?yDZBg0D3L?liC8shDepZtNpNx60GKJQxpXRAyMCte

NOTES-

12) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.

- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 293 lb down and 98 lb up at 3-7-8, 256 lb down and 88 lb up at 5-4-8, 217 lb down and 96 lb up at 7-4-8, 214 lb down and 95 lb up at 9-1-8, 172 lb down and 70 lb up at 11-4-8, 172 lb down and 70 lb up at 13-3-12, 172 lb down and 70 lb up at 15-3-0, 172 lb down and 70 lb up at 15-3-0, 172 lb down and 70 lb up at 17-3-0, 172 lb down and 70 lb up at 13-3-12, 172 lb down and 109 lb up at 23-3-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

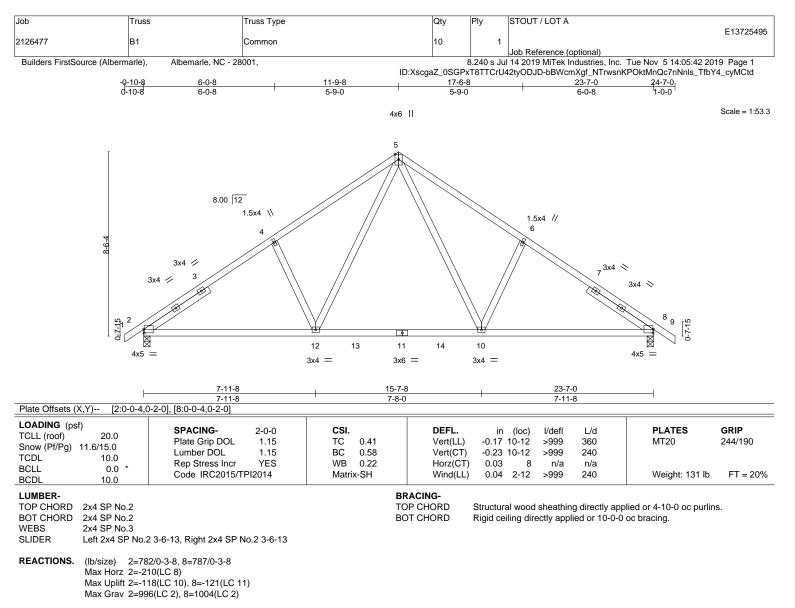
Vert: 1-5=-43, 5-7=-53, 7-10=-43, 2-16=-20, 15-16=-20, 14-15=-20, 13-14=-20, 9-13=-20

Concentrated Loads (lb)

Vert: 7=-13(B) 14=-168 15=-250 6=-8(B) 18=-8(B) 19=-8(B) 20=-8(B) 21=-214 22=-170 23=-137 24=-137 25=-137 26=-137 27=-137 28=-214 29=-251

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-1300/157, 4-5=-1171/233, 5-6=-1170/233, 6-8=-1299/157

BOT CHORD 2-12=-159/1123, 10-12=-1/747, 8-10=-36/999

WEBS 5-10=-139/575, 6-10=-319/237, 5-12=-139/576, 4-12=-319/237

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; PT=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

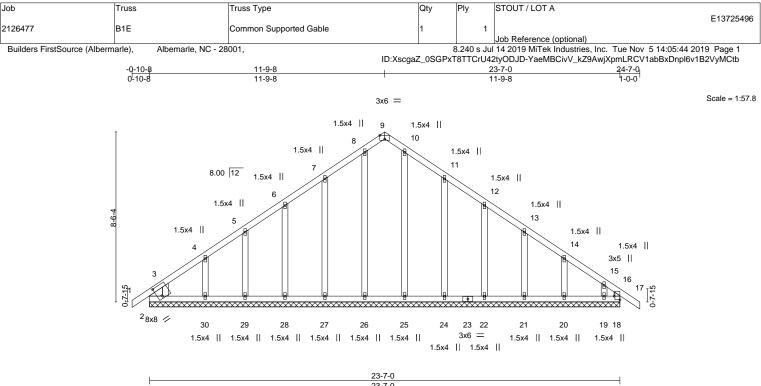
7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.



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		23-7-0					
Plate Offsets (X,Y) [2:0-1-8,0)-2-4], [2:0-2-12,0-6-11], [3:0-2-2,0-0-0]	, [9:0-3-0,Edge], [16:Edge,0	-3-8], [16:0-1-3	3,0-1-12], [18:0-0-0),0-1-12]		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.09 BC 0.09 WB 0.14 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.00 17 -0.01 17 0.01 18	l/defl L/d n/r 120 n/r 120 n/a n/a	PLATES MT20 Weight: 154 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2		TOP		except end vertica	0 7 11	lied or 6-0-0 oc purlins oc bracing.	5,

WEBS 2x4 SP No.2 OTHERS 2x4 SP No.3

SLIDER Left 2x4 SP No.2 0-9-7

REACTIONS. All bearings 23-7-0.

(lb) - Max Horz 2=-211(LC 8) Max Uplift All uplift 100 lb or less at joint(s) 18, 2, 27, 28, 29, 24, 22, 21, 20 except 30=-108(LC 10), 19=-165(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 18, 2, 26, 27, 28, 29, 30, 25, 24, 22, 21, 20, 19

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

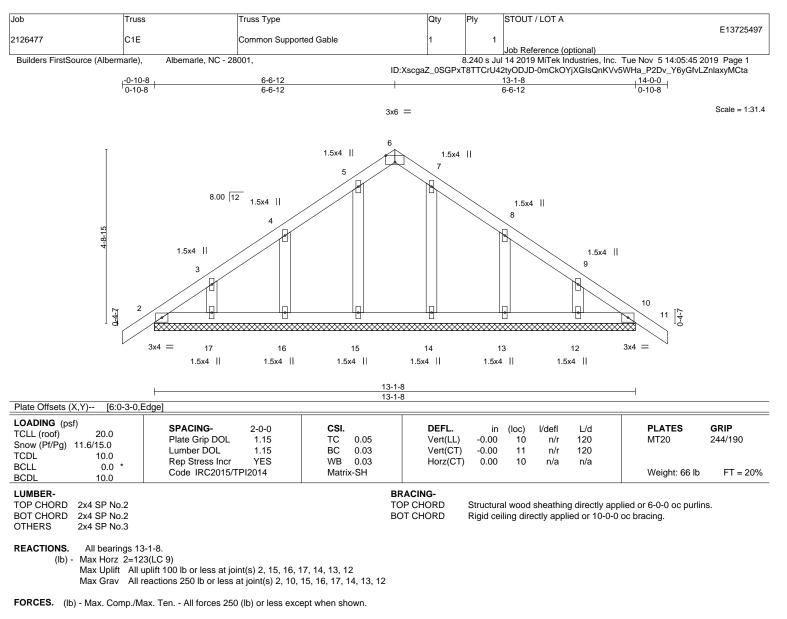
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 18, 2, 27, 28, 29, 30, 24, 22, 21, 20, and 19. This connection is for uplift only and does not consider lateral forces.



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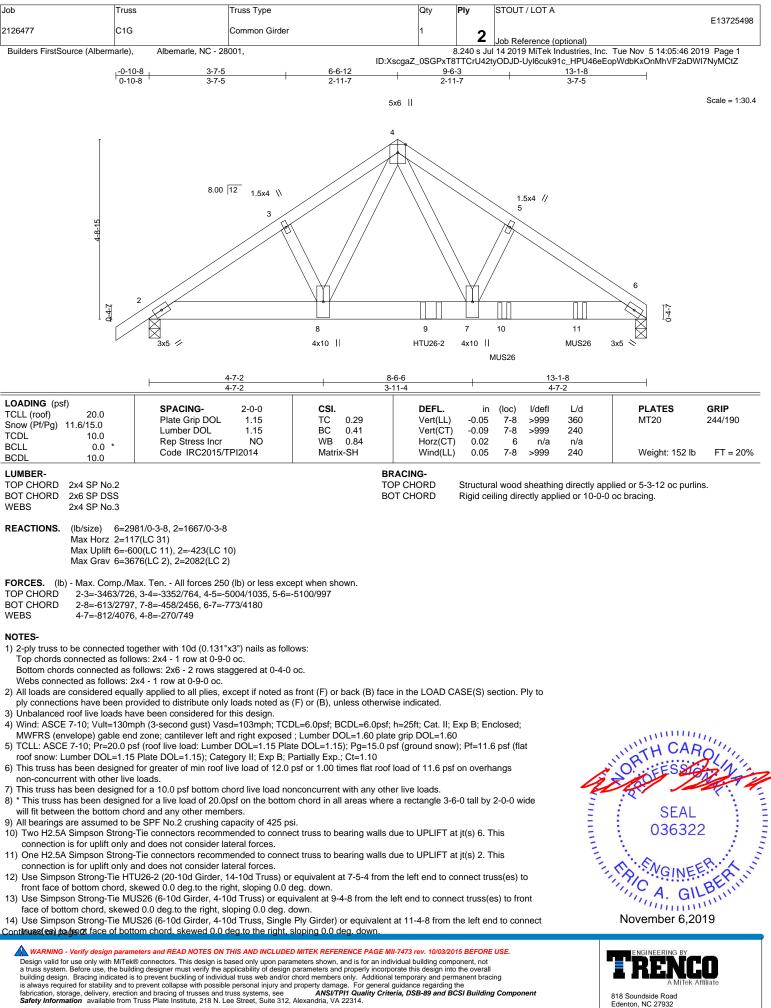
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 15, 16, 17, 14, 13, and 12. This connection is for uplift only and does not consider lateral forces.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 10.





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[Job	Truss	Truss Type	Qty	Ply	STOUT / LOT A
						E13725498
	2126477	C1G	Common Girder	1	2	
					_	Job Reference (optional)
	Builders FirstSource (Alberma	arle), Albemarle, NC - 28	001,	8	3.240 s Jul	14 2019 MiTek Industries, Inc. Tue Nov 5 14:05:46 2019 Page 2
			ID:XscgaZ	_0SGPxT8	TTCrU42t	yODJD-Uyl6cuk91c_HPU46eEopWdbKxOnMhVF2aDWI7NyMCtZ

NOTES-

15) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

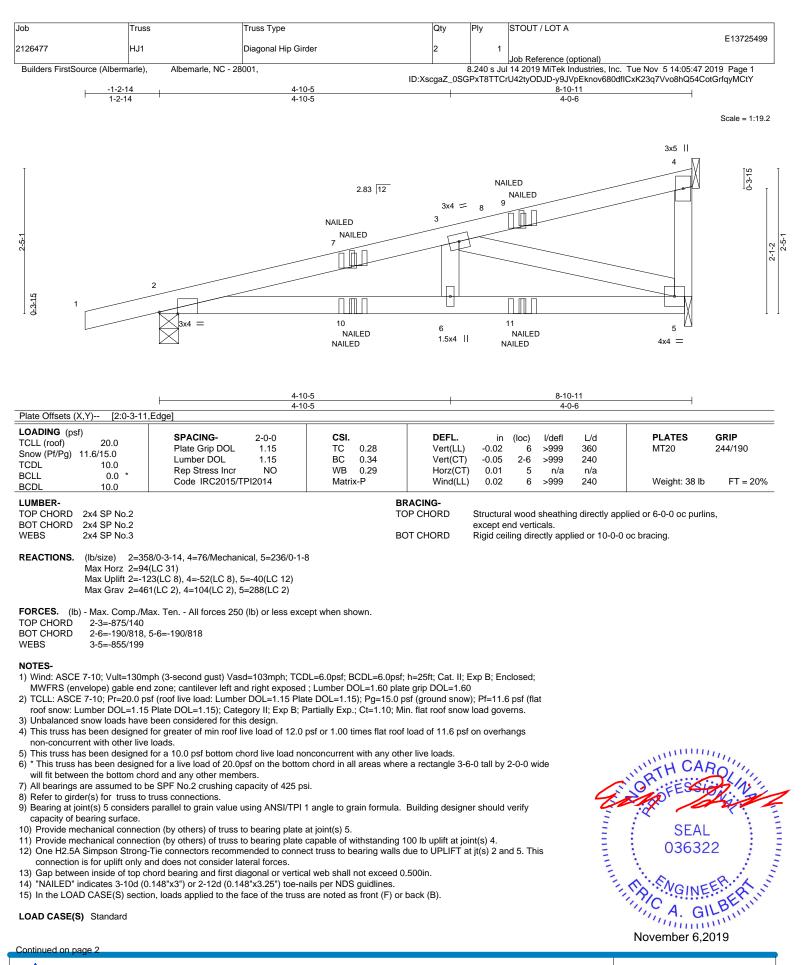
Uniform Loads (plf)

Vert: 1-4=-43, 4-6=-43, 2-6=-20 Concentrated Loads (lb)

Vert: 9=-1944(F) 10=-935(F) 11=-915(F)

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	STOUT / LOT A
					E13725499
2126477	HJ1	Diagonal Hip Girder	2	1	
					Job Reference (optional)
Builders FirstSource (Alberma	arle), Albemarle, NC - 28	001,	8	3.240 s Jul	14 2019 MiTek Industries, Inc. Tue Nov 5 14:05:48 2019 Page 2

ID:XscgaZ_0SGPxT8TTCrU42tyODJD-QLtt1alQZDE?enEUmfrHb1ggeCUw9YJL1X?PBGyMCtX

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

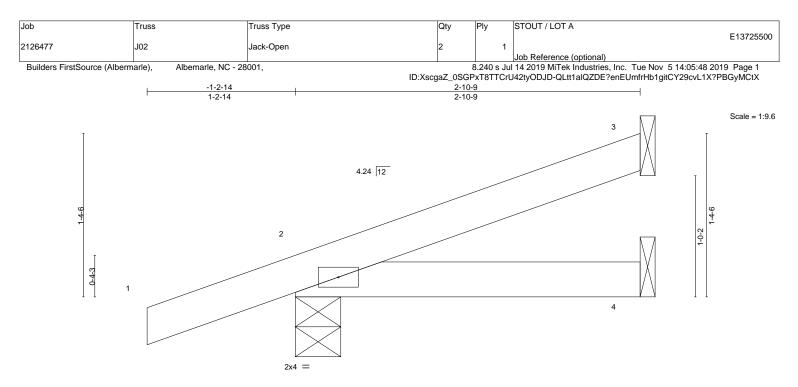
Uniform Loads (plf)

Vert: 1-4=-43, 2-5=-20 Concentrated Loads (lb)

Vert: 9=-45(F=-24, B=-21) 11=-24(F=-13, B=-11)

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			2-10-9 2-10-9					—	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.13 BC 0.08 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.00 -0.01 -0.00 0.00	(loc) 2-4 2-4 3 2	l/defl >999 >999 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 11 lb	GRIP 244/190 FT = 20%



BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-10-9 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 3=40/Mechanical, 2=161/0-4-9, 4=26/Mechanical Max Horz 2=60(LC 8) Max Uplift 3=-34(LC 12), 2=-84(LC 8) Max Grav 3=56(LC 2), 2=214(LC 2), 4=53(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; PI=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

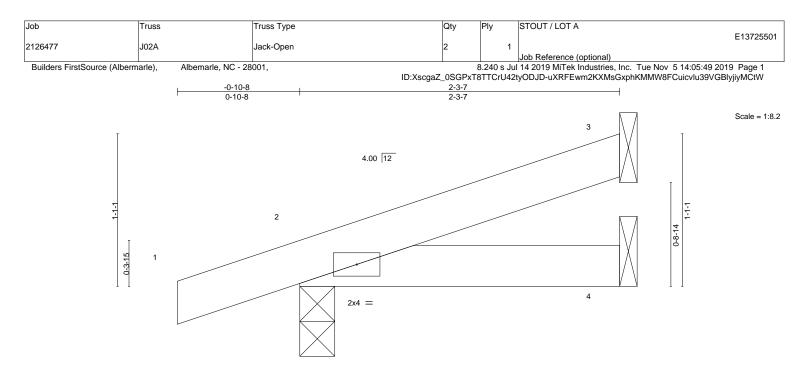
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This
- connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





		⊢			<u>2-3-7</u> 2-3-7						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI:	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matri	0.07 0.05 0.00 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.00 -0.00 -0.00 0.00	(loc) 2-4 2-4 3 2	l/defl >999 >999 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 8 lb	GRIP 244/190 FT = 20%

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LUMBER-
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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-3-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=35/Mechanical, 2=120/0-3-0, 4=21/Mechanical Max Horz 2=43(LC 8) Max Uplift 3=-28(LC 12), 2=-60(LC 8) Max Grav 3=49(LC 2), 2=158(LC 2), 4=42(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

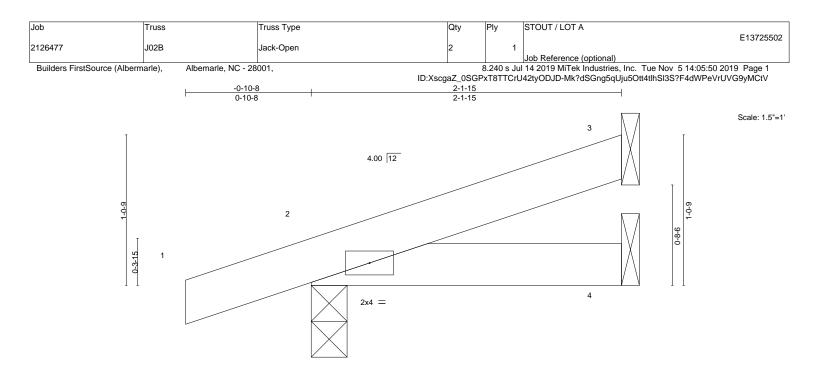
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This
 - connection is for uplift only and does not consider lateral forces.



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	F		<u>2-1-15</u> 2-1-15						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.04 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.00 -0.00 -0.00 0.00	(loc) 2 2-4 3 2	l/defl >999 >999 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 8 lb	GRIP 244/190 FT = 20%

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LUMBER-
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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-1-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 3=32/Mechanical, 2=116/0-3-0, 4=20/Mechanical Max Horz 2=42(LC 8) Max Uplift 3=-26(LC 12), 2=-60(LC 8) Max Grav 3=44(LC 2), 2=154(LC 2), 4=39(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- 2) TCLL: ASCE 7-10; PI=20.0 psi (root live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psi (ground show); PI=11.6 psi (in roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

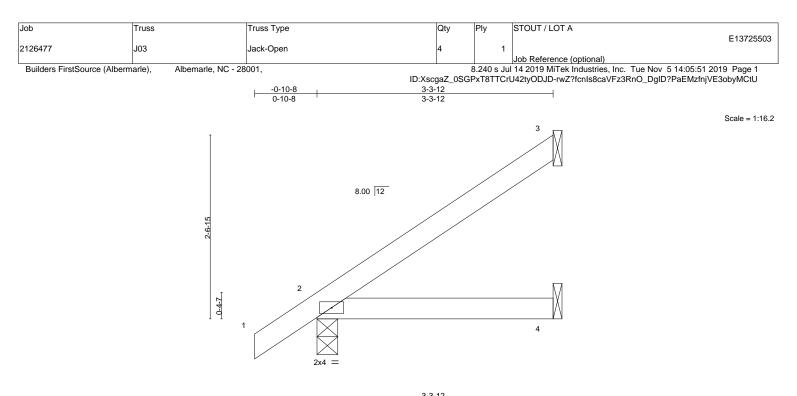
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This
 - connection is for uplift only and does not consider lateral forces.



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			3-3-12			—			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.14 BC 0.11	DEFL. Vert(LL) Vert(CT)	in -0.01 -0.01	(loc) 2-4 2-4	l/defl >999 >999	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL 0.0 BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Horz(CT) Wind(LL)	-0.00 0.00	3	n/a ****	n/a 240	Weight: 13 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

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LUMBER-
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REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

> (lb/size) 3=60/Mechanical, 2=149/0-3-8, 4=31/Mechanical Max Horz 2=103(LC 10) Max Uplift 3=-67(LC 10), 2=-19(LC 10)

Max Grav 3=92(LC 22), 2=195(LC 2), 4=62(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

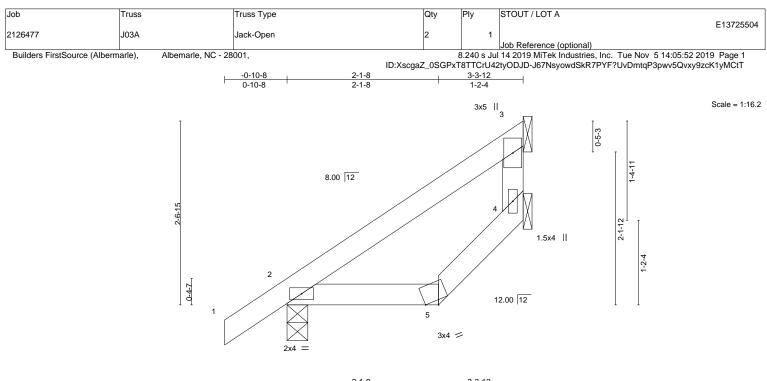


Structural wood sheathing directly applied or 3-3-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





	H	2-1-8	1-2-4		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.12 BC 0.08 WB 0.00 Matrix-P	Vert(CT) -0.01 Horz(CT) -0.01	c) l/defl L/d 5 >999 360 5 >999 240 4 n/a n/a 5 >999 240	PLATES GRIP MT20 244/190 Weight: 15 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 3-3-12 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=56/Mechanical, 2=147/0-3-8, 4=31/Mechanical Max Horz 2=101(LC 10) Max Uplift 3=-56(LC 10), 2=-19(LC 10) Max Grav 3=85(LC 22), 2=192(LC 2), 4=56(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

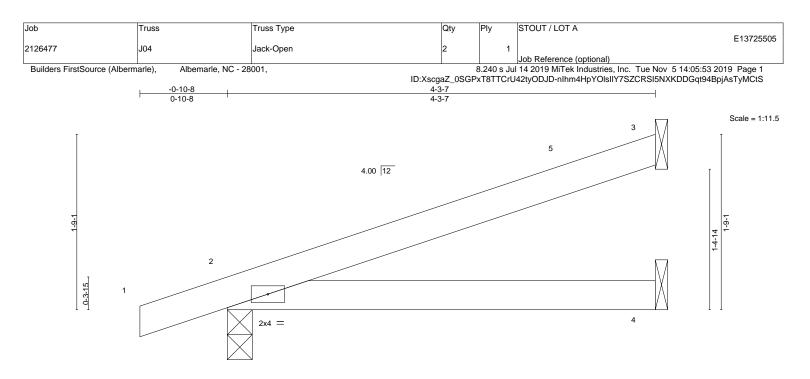
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.
- 10) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.





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			4-3-7						
LOADING (psf)	SPACING- 2-0-0	CSI.	4-3-7 DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.28 BC 0.20	Vert(LL) Vert(CT)	-0.02 -0.03	2-4 2-4	>999 >999	360 240	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Horz(CT) Wind(LL)	-0.00 0.00	3 2	n/a ****	n/a 240	Weight: 15 lb	FT = 20%
LUMBER-		BF	RACING-						



TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-3-7 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 3=83/Mechanical, 2=178/0-3-0, 4=41/Mechanical Max Horz 2=69(LC 8) Max Uplift 3=-61(LC 12), 2=-65(LC 8) Max Grav 3=116(LC 2), 2=231(LC 2), 4=82(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

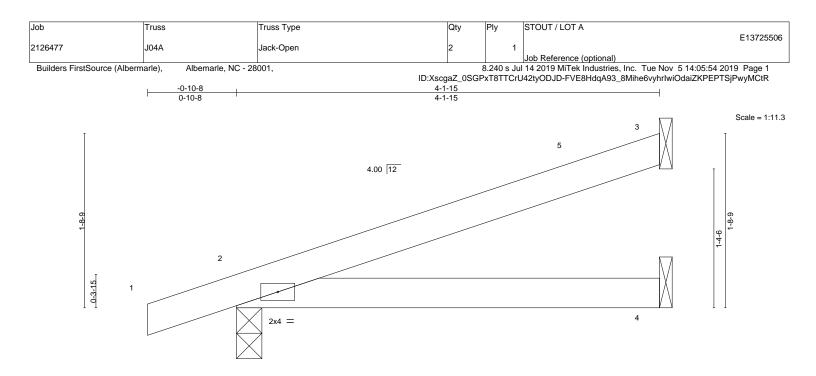
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 - connection is for uplift only and does not consider lateral forces.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED MITER REFERENCE PAGE MIT-14's rev. Invozoris beroke use. Design valif for use only with MiTeR's connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





			4-1-15 4-1-15						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.26 BC 0.19 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.03 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.00	2	****	240	Weight: 14 lb	FT = 20%

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LUMBER-
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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-1-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 3=80/Mechanical, 2=174/0-3-0, 4=40/Mechanical Max Horz 2=67(LC 8) Max Uplift 3=-59(LC 12), 2=-65(LC 8) Max Grav 3=112(LC 2), 2=227(LC 2), 4=79(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; PI=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

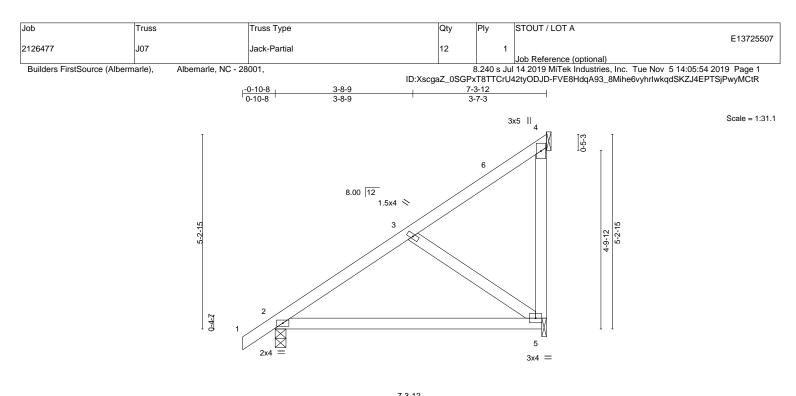
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This
 - connection is for uplift only and does not consider lateral forces.



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			7-3-12				-1			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Lumber DOL 1.	15 TC 15 BC ES WB	0.17 0.66 0.09	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.15 -0.29 0.00 0.00	(loc) 2-5 2-5 5 2	l/defl >578 >289 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 38 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=61/Mechanical, 2=269/0-3-8, 5=157/0-1-8 Max Horz 2=195(LC 10) Max Uplift 4=-45(LC 10), 2=-16(LC 10), 5=-50(LC 10) Max Grav 4=92(LC 22), 2=347(LC 2), 5=204(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-270/11

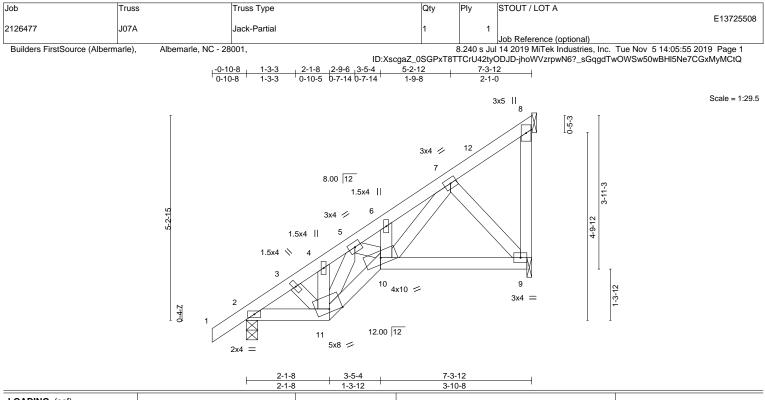
NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



	2-1-8	3 1-3-12	3-10-8			1			
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.07 BC 0.17 WB 0.16		-0.01	(loc) 9-10 9-10 9	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 0.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL)	0.01	10	>999	240	Weight: 47 lb	FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 8=30/Mechanical, 2=269/0-3-8, 9=188/0-1-8 Max Horz 2=195(LC 10) Max Uplift 8=-20(LC 12), 2=-16(LC 10), 9=-75(LC 10) Max Grav 8=46(LC 22), 2=347(LC 2), 9=249(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-353/0, 3-4=-320/0, 4-5=-348/15, 5-6=-507/94, 6-7=-504/131
- BOT CHORD 2-11=-130/266, 10-11=-197/484
- WEBS 7-10=-148/424, 7-9=-283/128

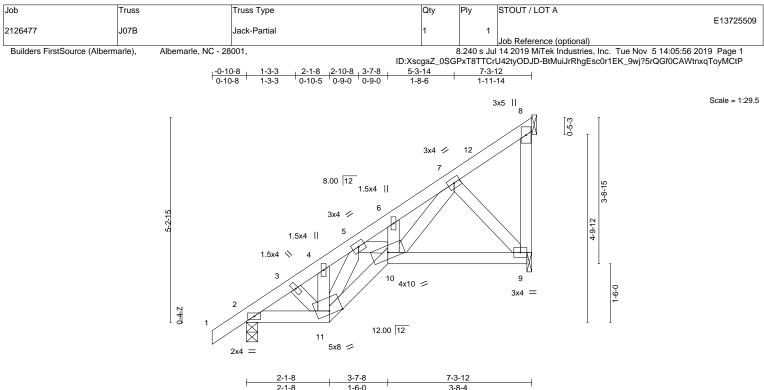
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs
- non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 9.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.
- 12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.





🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and KEAD NOTES ON THIS AND INCLODED INTER REFERENCE FACE INTERVISED. INCLOSE DEL ONE OCL Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



	2-1-8	1-6-0	3-8-4					
Show (Pf/Pg) 11.6/15.0 Lumb TCDL 10.0 Rep S	Cling- 2-0-0 Grip DOL 1.15 er DOL 1.15 Stress Incr YES IRC2015/TPI2014 IRC2015/TPI2014	CSI. TC 0.07 BC 0.16 WB 0.17 Matrix-P	DEFL. Vert(LL) -0.0 Vert(CT) -0.0 Horz(CT) 0.0 Wind(LL) 0.0	02 9-10 01 9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 46 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

 REACTIONS.
 (lb/size)
 8=28/Mechanical, 2=269/0-3-8, 9=190/0-1-8

 Max Horz
 2=195(LC 10)

 Max Uplift
 8=-19(LC 12), 2=-16(LC 10), 9=-76(LC 10)

 Max Grav
 8=43(LC 22), 2=347(LC 2), 9=252(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-353/0, 3-4=-321/0, 4-5=-350/17, 5-6=-539/110, 6-7=-531/145
- BOT CHORD 2-11=-130/266, 10-11=-208/507
- WEBS 7-10=-160/454, 7-9=-291/128

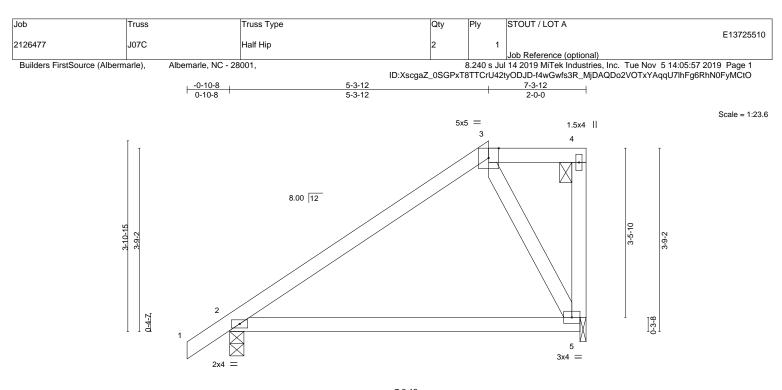
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 9.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This
 connection is for uplift only and does not consider lateral forces.
- 12) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

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	+		3-12 3-12						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 16.5/15.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.50 BC 0.66 WB 0.06 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.15 -0.29 0.00 0.00	(loc) 2-5 2-5 5	l/defl >578 >289 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 35 lb	GRIP 244/190 FT = 20%
BCDL 10.0			ACING-	0.00	_		2.0		

TOP CHORD

BOT CHORD

TOP CHORD

2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS REACTIONS.

(lb/size) 2=271/0-3-8, 5=234/0-1-8 Max Horz 2=153(LC 10) Max Uplift 2=-37(LC 10), 5=-56(LC 10)

Max Grav 2=347(LC 2), 5=276(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 9) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



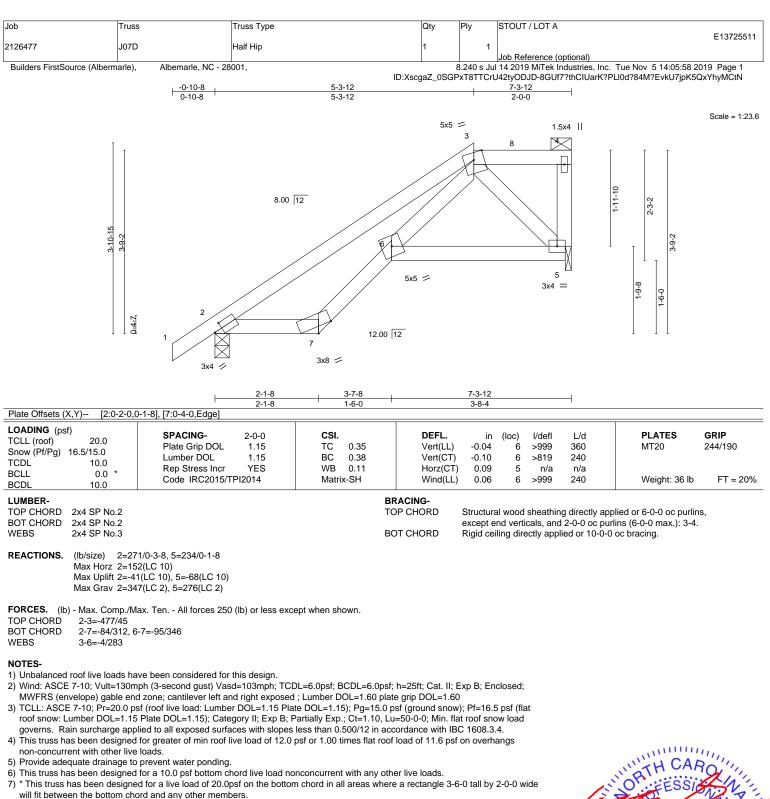
818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.

9) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This 11) connection is for uplift only and does not consider lateral forces.

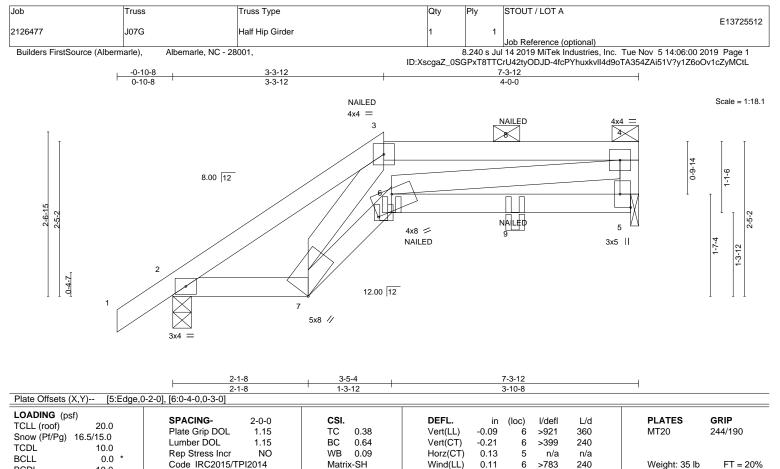
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Edenton, NC 27932

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BCDL	10.0			0	0 1100	2.0	troigini co is
LUMBER-		BF	ACING-				
TOP CHOP	RD 2x4 SP No.2	TC	P CHORD S	tructural w	ood sheathir	ng directly app	blied or 6-0-0 oc purlins,
BOT CHOF	RD 2x4 SP No.2		e	xcept end v	verticals, and	d 2-0-0 oc pur	lins (6-0-0 max.): 3-4.
WEBS	2x4 SP No.3	BC	T CHORD R	igid ceiling	directly app	lied or 10-0-0	oc bracing.
							-

REACTIONS. (lb/size) 2=295/0-3-8, 5=270/0-1-8

Max Horz 2=101(LC 10) Max Uplift 2=-73(LC 10), 5=-78(LC 7)

Max Grav 2=371(LC 2), 5=313(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-535/85, 3-4=-333/133

BOT CHORD 2-7=-96/377, 6-7=-152/280

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This
 connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Continued on page 2

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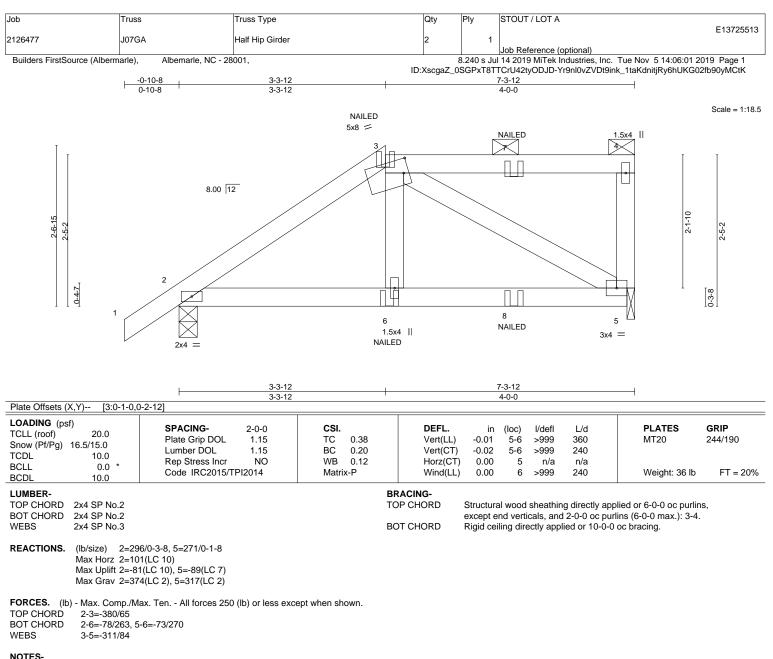
lob	Truss	Truss Type	Qty	Ply	STOUT / LOT A	
					E13725512	
2126477	J07G	Half Hip Girder	1	1		
					Job Reference (optional)	
Builders FirstSource (Alberm	001,	8	3.240 s Jul	14 2019 MiTek Industries, Inc. Tue Nov 5 14:06:00 2019 Page 2		
ID:XscgaZ_0SGPxT8TTCrU42tyODJD-4fcPYhuxkvII4d9oTA354ZAi51V?y1Z6oOv1cZyMC						

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-43, 3-4=-53, 2-7=-20, 6-7=-20, 5-6=-20 Concentrated Loads (lb) Vert: 3=-8(F) 6=-11(F) 8=-8(F) 9=-11(F)

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- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
 MM/202 (survival) while and access and interval to the survival based of the
- MWFRS (envelope) gable end zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=16.5 psf (flat roof snow: Lumber DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 9) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This
 connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	STOUT / LOT A
					E13725513
2126477	J07GA	Half Hip Girder	2	1	
					Job Reference (optional)
Builders FirstSource (Alberm	arle), Albemarle, NC - 28	001,	8	3.240 s Jul	14 2019 MiTek Industries, Inc. Tue Nov 5 14:06:01 2019 Page 2

8.240 s Jul 14 2019 Mi Lek Industries, Inc. Tue Nov 5 14:06:01 2019 Page 2 ID:XscgaZ_0SGPxT8TTCrU42tyODJD-Yr9nl0vZVDt9ink_1taKdnitjRy6hUKG02fb90yMCtK

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

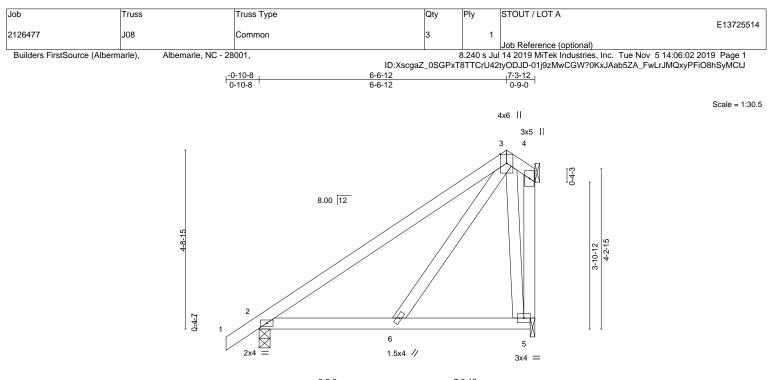
Uniform Loads (plf) Vert: 1-3=-43, 3-4=-53, 2-5=-20

Concentrated Loads (lb)

Vert: 3=-11(B) 6=-11(B) 7=-8(B) 8=-11(B)

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		<u>3-8-9</u> 3-8-9	+ 7-3-12 3-7-3				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.84 BC 0.14 WB 0.09 Matrix-P	DEFL. in Vert(LL) -0.01 Vert(CT) -0.01 Horz(CT) -0.00 Wind(LL) 0.00	5-6 > 5-6 > 4	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 45 lb	GRIP 244/190 FT = 20%

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3 *Except*

4-5: 2x4 SP No.2

BRACING-TOP CHORD

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=269/0-3-8, 4=13/Mechanical, 5=205/0-1-8 Max Horz 2=180(LC 10) Max Uplift 2=-26(LC 10), 4=-13(LC 11), 5=-82(LC 10)

Max Grav 2=347(LC 2), 4=20(LC 23), 5=272(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

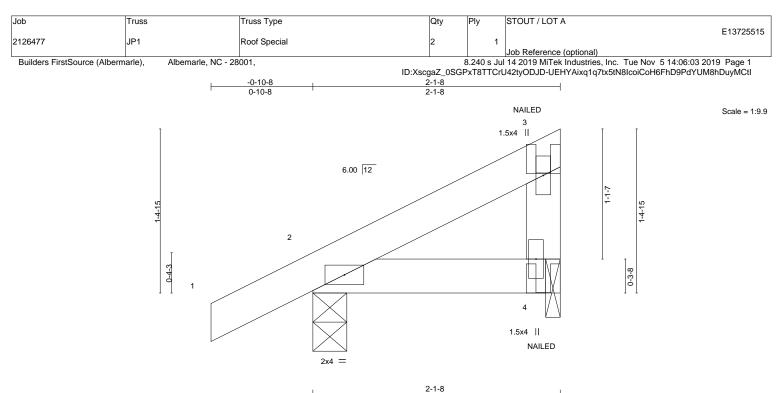
- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; PI=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 13) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



TRENGINEERING BY A MITEK Atfiliate 818 Soundside Road

Edenton, NC 27932

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			2-1-8						
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.04 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.00 -0.00 0.00 0.00	(loc) 2 2-4 2	l/defl >999 >999 n/a	L/d 360 240 n/a 240	PLATES MT20 Weight: 9 lb	GRIP 244/190 FT = 20%
LUMBER-		BF	ACING-						

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=114/0-3-8, 4=72/0-1-8 Max Horz 2=53(LC 12) Max Uplift 2=-31(LC 12), 4=-26(LC 12) Max Grav 2=152(LC 2), 4=87(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 11) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 2-4=-20

Concentrated Loads (lb) Vert: 3=-13(F) 4=-14(F)



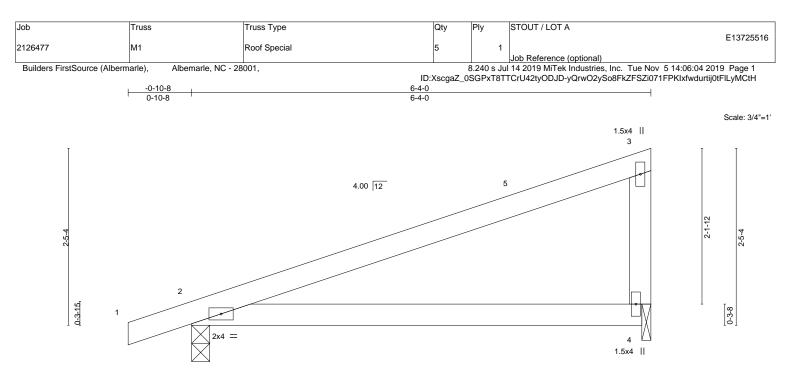
Structural wood sheathing directly applied or 2-1-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and KEAD NOTES ON THIS AND INCLODED INTER REFERENCE FACE INTERVISED. INCLOSE DEL ONE OCL Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



	L		6-4-0						
	1		6-4-0					-	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.70 BC 0.47	DEFL. Vert(LL) Vert(CT)	-0.08 -0.16	(loc) 2-4 2-4	l/defl >898 >449	L/d 360 240	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Horz(CT) Wind(LL)	0.00 0.00	2	n/a ****	n/a 240	Weight: 23 lb	FT = 20%
LUMBER-		BF	RACING-						

TOP CHORD

BOT CHORD

TOP CHORD

2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=238/0-3-0, 4=422/0-1-8 Max Horz 2=94(LC 8) Max Uplift 2=-73(LC 8), 4=-104(LC 12)

Max Grav 2=307(LC 2), 4=513(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 276 lb down and 53 lb up at 6-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 2-4=-20

Concentrated Loads (lb) Vert: 4=-234



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



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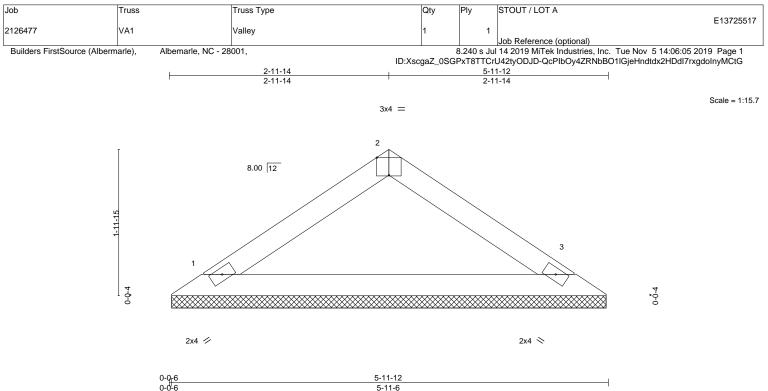


Plate Offsets (X,Y) [2:0-2-0,E	Edge]								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.11 BC 0.32 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l n/a n/a 0.00	-	defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 18 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			RACING- DP CHORD S	Structural v	wood she	eathing	directly app	plied or 5-11-12 oc pu	rlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 1=158/5-11-0, 3=158/5-11-0 Max Horz 1=-41(LC 6) Max Uplift 1=-22(LC 10), 3=-22(LC 11) Max Grav 1=201(LC 2), 3=201(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

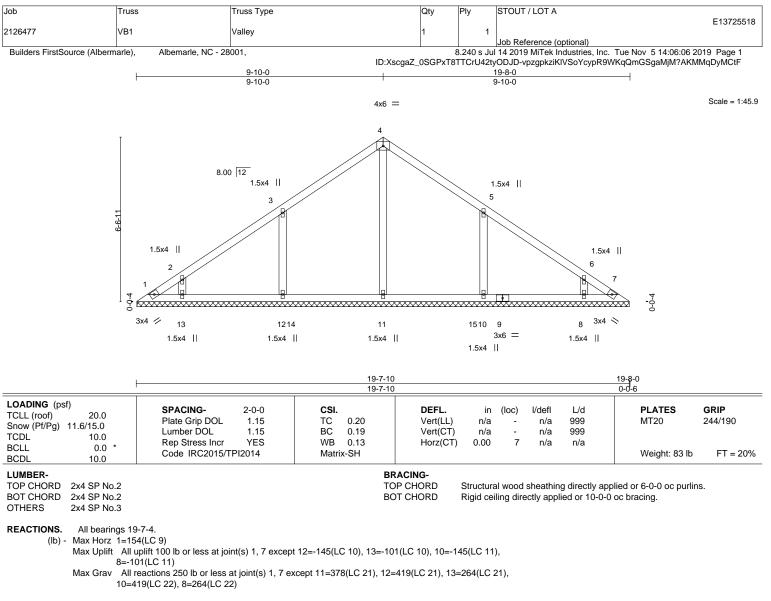
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 3-12=-288/194, 5-10=-288/194

NOTES-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; PT=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.

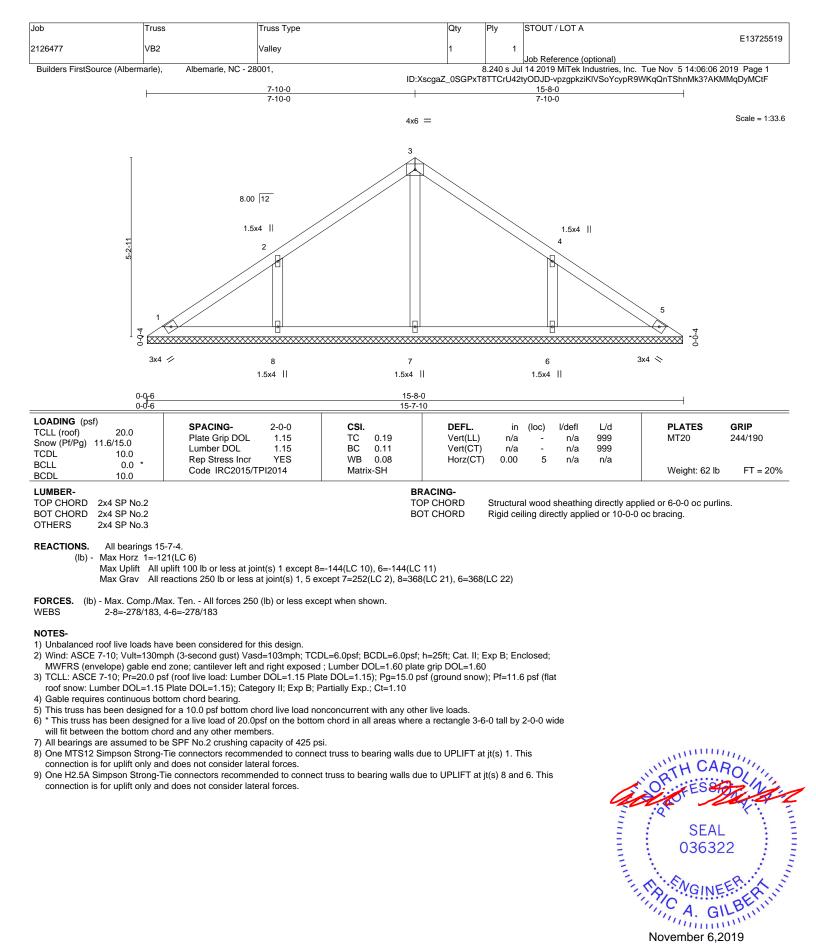
8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 7. This connection is for uplift only and does not consider lateral forces.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12, 13, 10, and 8. This connection is for uplift only and does not consider lateral forces.



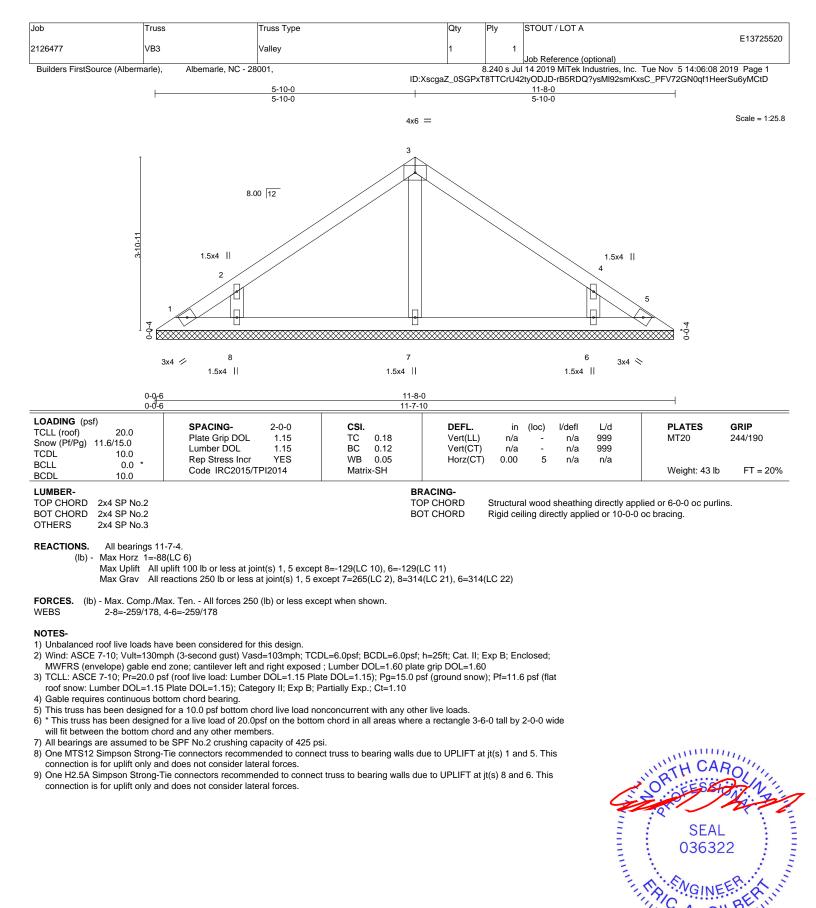
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 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 ANSITPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

¹⁾ Unbalanced roof live loads have been considered for this design.



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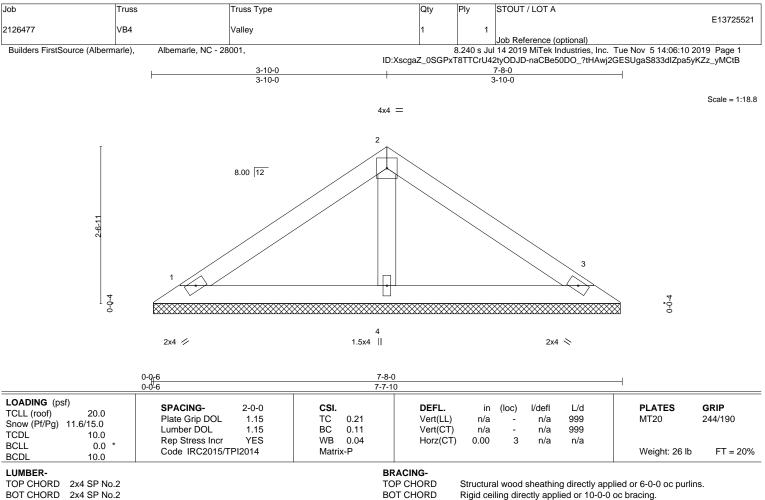


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek@ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss systems, see **ANSI/TPI Ouality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

November 6,2019



BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 1=111/7-7-4, 3=111/7-7-4, 4=201/7-7-4 Max Horz 1=-55(LC 6) Max Uplift 1=-32(LC 10), 3=-40(LC 11) Max Grav 1=145(LC 2), 3=145(LC 2), 4=247(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

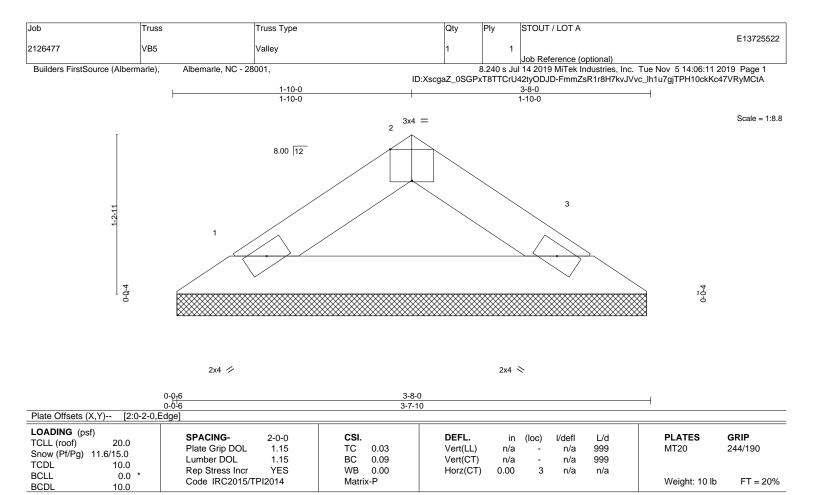
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-10; PT=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 1=85/3-7-4, 3=85/3-7-4 Max Horz 1=22(LC 7) Max Uplift 1=-12(LC 10), 3=-12(LC 11) Max Grav 1=108(LC 2), 3=108(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

 TCLL: ASCE 7-10; PT=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.

8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

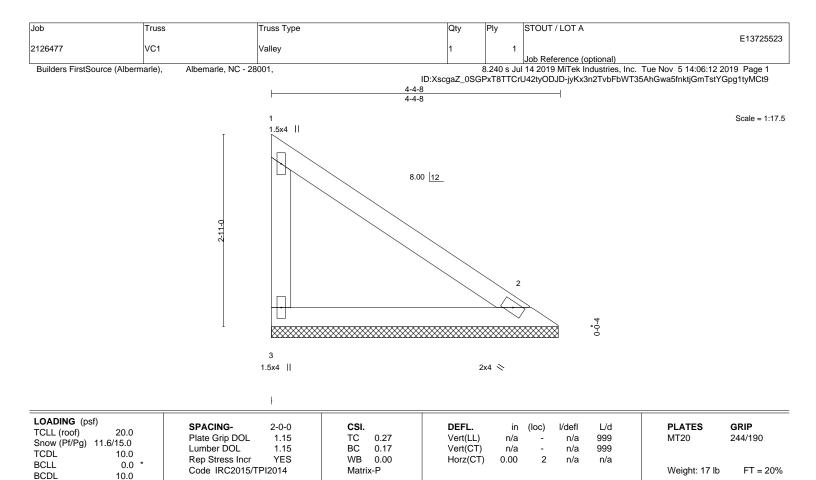
MILLIN C VIIIIII WWWWW SEAL 036322 GI "IIIIIIIIIIIII November 6,2019

Structural wood sheathing directly applied or 3-8-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



LUMBER-	

2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-4-8 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=118/4-4-2, 2=118/4-4-2 Max Horz 3=-97(LC 11) Max Uplift 3=-60(LC 11) Max Grav 3=159(LC 22), 2=150(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

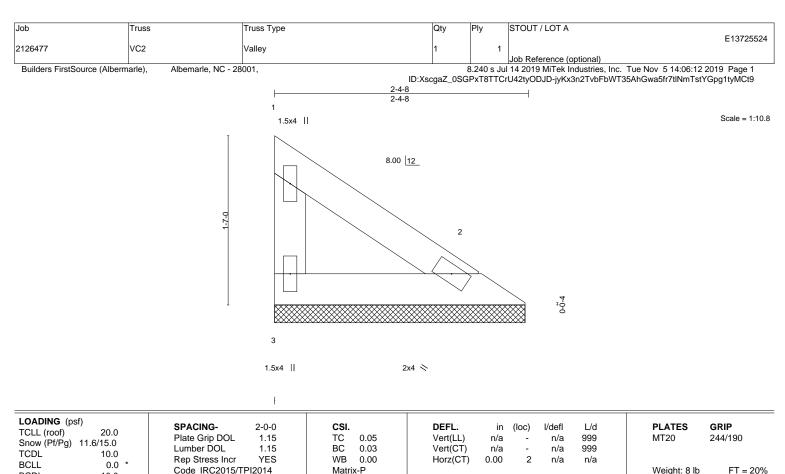
3) Gable requires continuous bottom chord bearing. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi. 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 3. This connection is for uplift only and does not consider lateral forces.



📣 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REPERIENCE PAGE mit-14/3 at 900, 1002/015 BEPORE 052. Design valid for use only with MITeR works connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BCDL	10.0	Code 11(C2013/11/12014	Matrix-1			Weight: 0 lb
LUMBER-			BR	ACING-		
TOP CHORD	2x4 SP No.2		TC	P CHORD	Structural wood sheathing directly app	ied or 2-4-8 oc purlins,
BOT CHORD	2x4 SP No.2				except end verticals.	
WEBS	2x4 SP No.3		BC	T CHORD	Rigid ceiling directly applied or 10-0-0	oc bracing.

REACTIONS. (lb/size) 3=55/2-4-2, 2=55/2-4-2 Max Horz 3=-45(LC 11) Max Uplift 3=-28(LC 11) Max Grav 3=74(LC 22), 2=70(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

- MWFRS (envelope) gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-10; PT=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=15.0 psf (ground snow); Pf=11.6 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SPF No.2 crushing capacity of 425 psi.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 3. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safey Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



