

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 150\_1773\_A 150.1773.A 10 x 10 CP

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I40052302 thru I40052322

My license renewal date for the state of North Carolina is December 31, 2020.

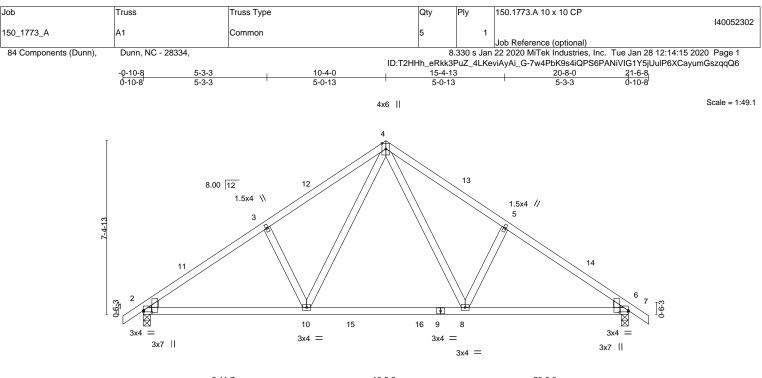
North Carolina COA: C-0844



January 29,2020

# Sevier, Scott

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



		1	6-11-7		1	13-8-9		1		20-8-0		
		Γ	6-11-7		1	6-9-2		1		6-11-7	1	
Plate Offse	ets (X,Y)	[2:0-0-0,0-0-12], [2:0-0-1	5,0-4-5], [6:Ec	lge,0-0-12], [6	6:0-0-15,0-4	5]						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
	20.0	Plate Grip DOL	1.15	TC	0.35	Vert(LL)	-0.09	8-10	>999	240	MT20	244/190
TCDL BCLL	10.0 0.0 *	Lumber DOL Rep Stress Incr	1.15 YES	BC WB	0.49 0.18	Vert(CT) Horz(CT)	-0.13 0.03	2-10 6	>999 n/a	180 n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S						Weight: 106 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

## REACTIONS. (lb/size) 2=876/0-3-8, 6=876/0-3-8 Max Horz 2=185(LC 11) Max Uplift 2=-108(LC 12), 6=-108(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1154/143, 3-4=-1016/201, 4-5=-1016/202, 5-6=-1154/143

BOT CHORD 2-10=-141/974, 8-10=0/637, 6-8=-36/875

WEBS 4-8=-120/502, 5-8=-283/211, 4-10=-120/502, 3-10=-283/211

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 10-4-0, Exterior(2) 10-4-0 to 13-4-0, Interior(1) 13-4-0 to 21-6-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

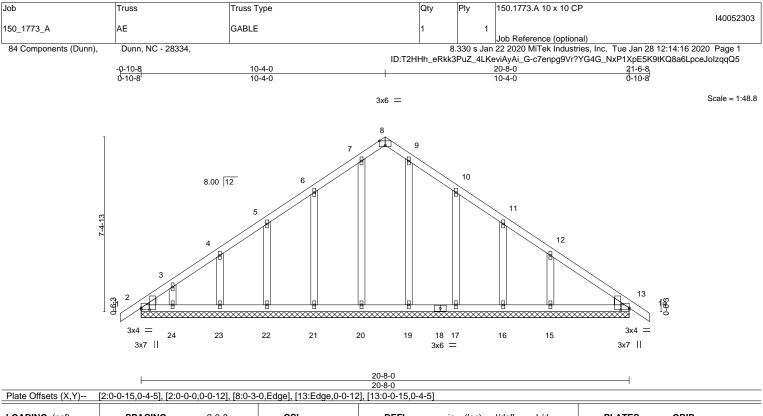
6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 5-1-6 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





LOADING         (psf)           ICLL         20.0           ICDL         10.0           3CLL         0.0           3CDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.12 BC 0.08 WB 0.09 Matrix-S	Vert(CT) (	in (loc) 0.00 14 0.01 14 0.01 13	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 124 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-	2 No 2	BRACING-	Structu	iral wood	sheathing di	rectly applied or 6-0-0	oc purlins	

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

(lb) - Max Horz 2=-185(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 24, 23, 22, 21, 20, 17, 16 except 15=-114(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 24, 23, 22, 21, 20, 19, 17, 16, 13 except 15=279(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

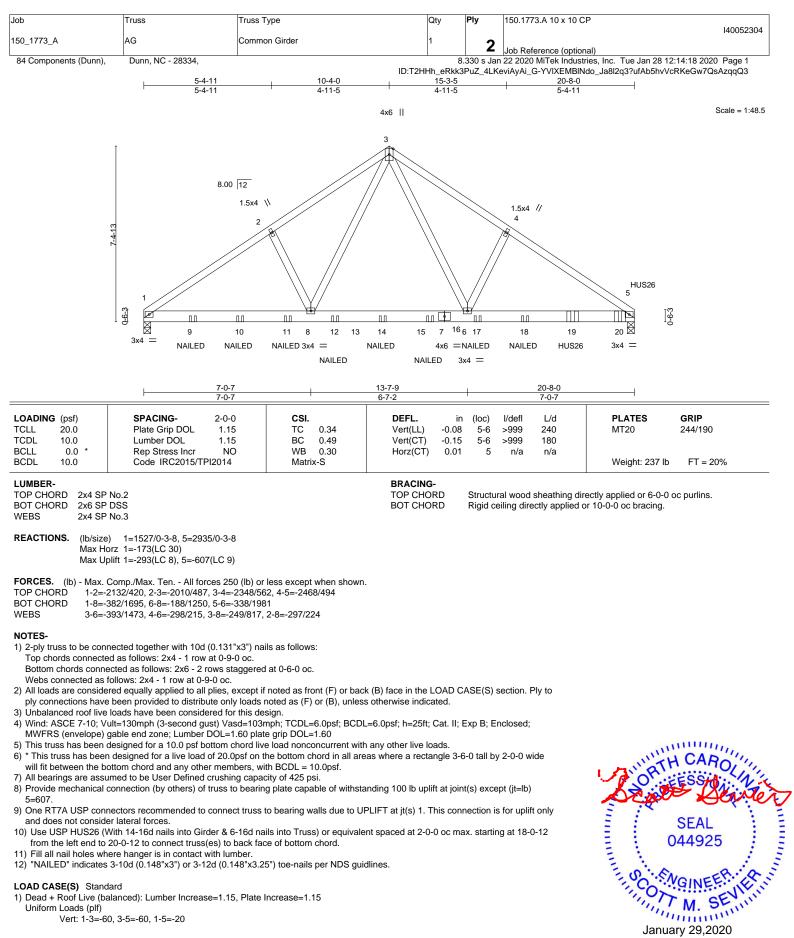
# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 10-4-0, Corner(3) 10-4-0 to 13-4-0, Exterior(2) 13-4-0 to 21-6-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be User Defined crushing capacity of 425 psi.





REACTIONS. All bearings 20-8-0.



818 Soundside Road Edenton, NC 27932

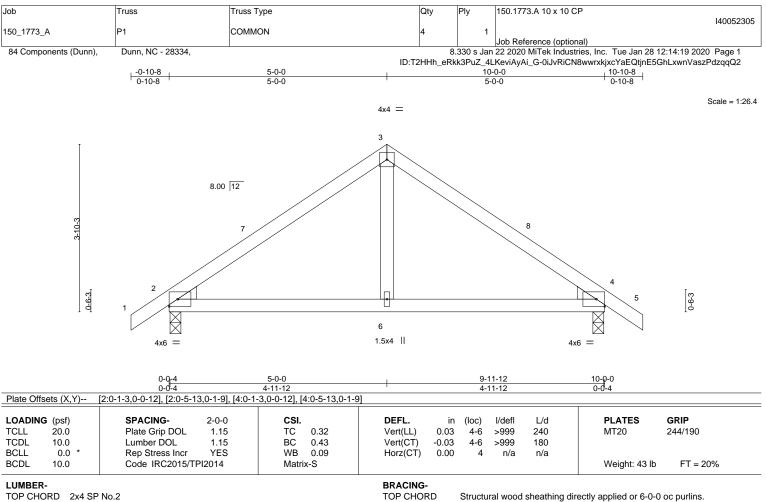
#### Continued on page 2

Job		Truss	Truss Type	Qty	Ply	150.1773.A 10 x 10 CP
150 17	73 /	AG	Common Girder	1		140052304
130_17	/3_X	A0	Common Girder	1	2	Job Reference (optional)
84 Co	mponents (Dunn),	Dunn, NC - 28334,		8.	330 s Jan	22 2020 MiTek Industries, Inc. Tue Jan 28 12:14:18 2020 Page 2
			ID:T2H	Hh_eRkk3	PuZ_4LKe	eviAyAi_G-YVIXEMBINdo_Ja8l2q3?ufAb5hvVcRKeGw7QsAzqqQ3

# LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 9=-130(B) 10=-130(B) 11=-130(B) 12=-130(B) 14=-130(B) 16=-130(B) 17=-130(B) 18=-130(B) 19=-894(B) 20=-899(B)





BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

## REACTIONS. (Ib/size) 2=450/0-3-0, 4=450/0-3-0 Max Horz 2=-97(LC 10) Max Uplift 2=-62(LC 12), 4=-62(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-460/339, 3-4=-460/338

BOT CHORD 2-6=-182/304, 4-6=-182/304

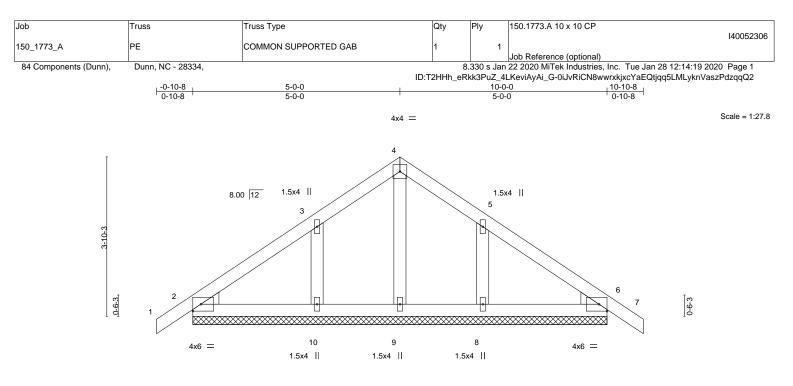
#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-0-0, Exterior(2) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 10-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.







	<u> </u>								
Plate Offsets (X,Y)	[2:0-1-3,0-0-12], [2:0-5-13,0-1-9],	[6:0-1-3,0-0-12], [6:0-5-13	,0-1-9]					1	
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
FCLL 20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL)	0.00	7	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.06	Vert(CT)	0.00	7	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 48 lb	FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

Max Horz 2=-97(LC 10) (lb) -Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-102(LC 12), 8=-100(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 10=261(LC 19), 8=259(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 5-0-0, Corner(3) 5-0-0 to 8-0-0, Exterior(2) 8-0-0 to 10-10-8 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be User Defined crushing capacity of 425 psi.

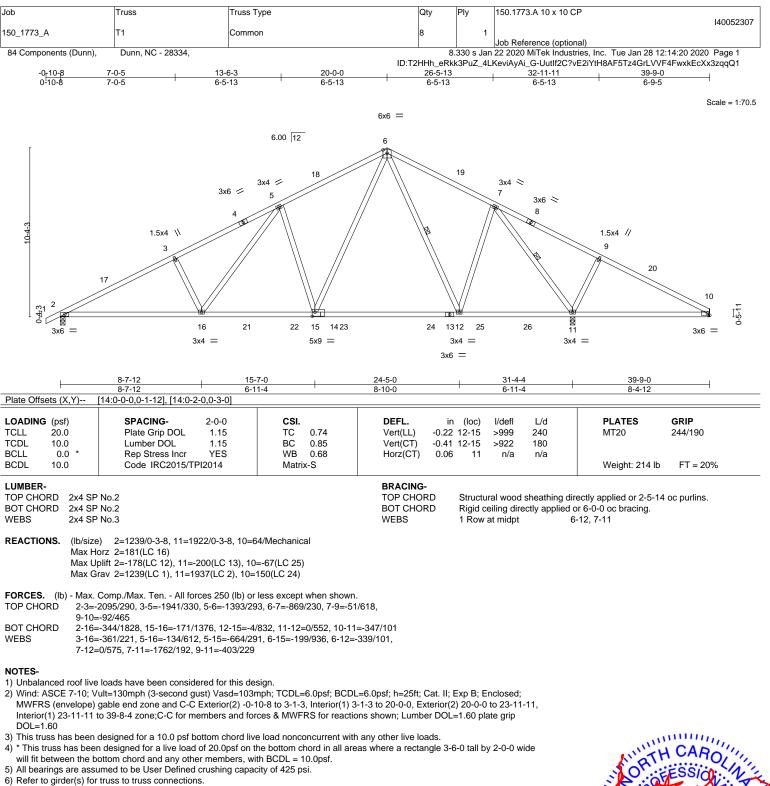


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



REACTIONS. All bearings 10-0-0.

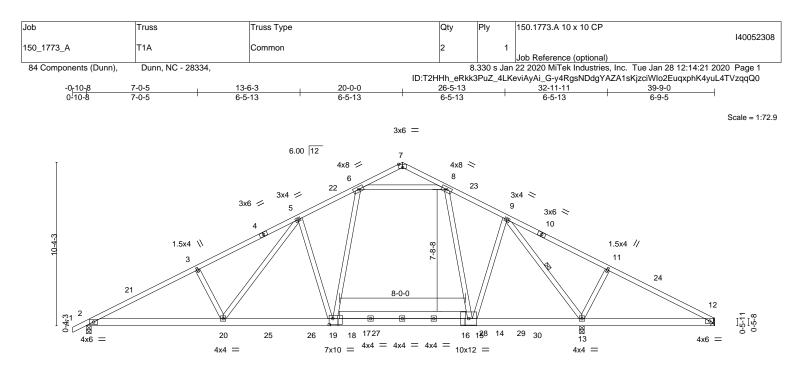


- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 11=200.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



5 BEFORE USE. opent, not the overall anent bracing te SI Building Component B18 Soundside Road Edenton, NC 27932





	8-7-12	15-7-0		24-5-0		31-4-4		39-9-0	
	8-7-12	6-11-4		8-10-0		6-11-4		8-4-12	
Plate Offsets (X,Y)	[7:0-3-0,Edge], [18:0-0-0,0	-2-12], [18:0-2-12,0-4-8]							
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI	2-0-0 <b>CSI</b> 1.15 TC 1.15 BC YES WB 2014 Mat	0.61 0.95	DEFL. Vert(LL) Vert(CT) Horz(CT)	in ( -0.30 19 -0.55 19 0.06	-20 >999	L/d 240 180 n/a	PLATES MT20 Weight: 266 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF REACTIONS. (lb/siz	P No.2	/0-3-8. 12=914/Mechan	cal	BRACING- TOP CHOR BOT CHOR WEBS	D R		ctly applied c	rectly applied or 2-10-4 or 2-2-0 oc bracing. I-13	oc purlins.
Max H Max L Max C	Horz 2=182(LC 16) Jplift 2=-244(LC 12), 13=-3: Grav 2=1465(LC 1), 13=112 . Comp./Max. Ten All forc	22(LC 13), 12=-196(LC 24(LC 24), 12=914(LC 1	12) )						
	-2680/446, 3-5=-2529/483, =-1563/499, 11-12=-1616/4	,	3/91, 8-9=-174	0/411,					
BOT CHORD 2-20 WEBS 3-20	=-483/2379, 19-20=-286/18 =-363/224, 5-20=-168/752, =-784/227, 11-13=-387/239	43, 14-19=-183/1519, 1 5-19=-727/279, 6-19=-1		,	6				
2) Wind: ASCE 7-10; MWFRS (envelope)	e loads have been consider /ult=130mph (3-second gus gable end zone and C-C E to 39-7-4 zone;C-C for mer	t) Vasd=103mph; TCDL xterior(2) -0-10-8 to 3-1	-6, Interior(1) 3	-1-6 to 20-0-0, Ex	erior(2) 2	0-0-0 to 23-11-	14,		

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) Refer to girder(s) for truss to truss connections.

DOI = 1.60

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=196.

8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.





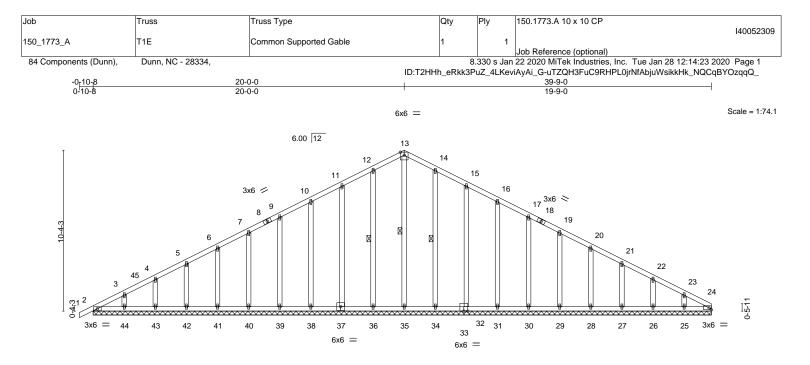


Plate Offsets (X,Y)	[32:0-1-12,0-0-0], [33:0-0-0,0-1-12]		<u>39-9-0</u> 39-9-0			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.09 BC 0.04 WB 0.15 Matrix-S	DEFL.         ii           Vert(LL)         -0.00           Vert(CT)         -0.00           Horz(CT)         0.07	) 1 n/r 120	PLATES MT20 Weight: 270 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP OTHERS 2x4 SP	No.2	I	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathin Rigid ceiling directly apple 1 Row at midpt	g directly applied or 6-0-0 c ied or 10-0-0 oc bracing. 13-35, 12-36, 14-34	c purlins.

# **REACTIONS.** All bearings 39-9-0.

(lb) - Max Horz 2=181(LC 16)

Max Uplift All uplift 100 lb or less at joint(s) 2, 36, 37, 38, 39, 40, 41, 42, 43, 44, 34, 32, 31, 30, 29, 28, 27, 26, 25

Max Grav All reactions 250 lb or less at joint(s) 2, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 34, 32, 31, 30, 29, 28, 27, 26, 25, 24

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 11-12=-100/290, 12-13=-116/331, 13-14=-116/332, 14-15=-100/291

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-1-3, Exterior(2) 3-1-3 to 20-0-0, Corner(3) 20-0-0 to 24-0-0, Exterior(2) 24-0-0 to 39-9-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

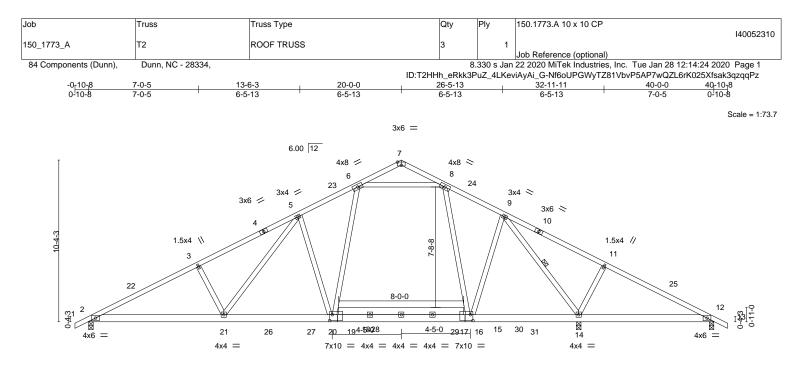
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) All bearings are assumed to be User Defined crushing capacity of 425 psi.







	8-7-12 8-7-12	15-7-0 6-11-4	24-5-0 8-10-0	31-4-4 6-11-4	40-0-0 8-7-12	
Plate Offsets (X,Y)	[7:0-3-0,Edge], [15:0-2-0,0	-4-8], [16:0-0-0,0-2-12], [19	9:0-0-0,0-2-12], [19:0-2-0,0-4-8]			
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI	1.15 BC YES WB	0.91 Vert(CT) - 0.73 Horz(CT)	in (loc) l/defl L/d 0.30 20-21 >999 240 0.56 20-21 >669 180 0.06 12 n/a n/a	PLATES MT20 Weight: 269 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP	No.2		BRACING- TOP CHORD BOT CHORD WEBS	5		oc purlins.
Max He Max U	e) 2=1466/0-3-8, 14=860 orz 2=-174(LC 13) plift 2=-243(LC 12), 14=-3 <sup>-</sup> rav 2=1466(LC 1), 14=113	17(LC 13), 12=-194(LC 12)	)			

- $\label{eq:FORCES.} \textbf{(lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.}$
- TOP CHORD 2-3=-2684/446, 3-5=-2533/483, 5-6=-1873/395, 7-8=-263/92, 8-9=-1736/410,

9-11=-1569/497, 11-12=-1623/461

- BOT CHORD 2-21=-475/2387, 20-21=-278/1848, 15-20=-172/1515, 14-15=-225/1559, 12-14=-351/1432
- WEBS 3-21=-363/224, 5-21=-170/759, 5-20=-732/279, 6-20=-119/758, 8-15=-88/474,
  - 9-14=-763/227, 11-14=-395/230, 6-8=-1314/326

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-1-8, Interior(1) 3-1-8 to 20-0-0, Exterior(2) 20-0-0 to 24-0-0, Interior(1) 24-0-0 to 40-10-8 zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

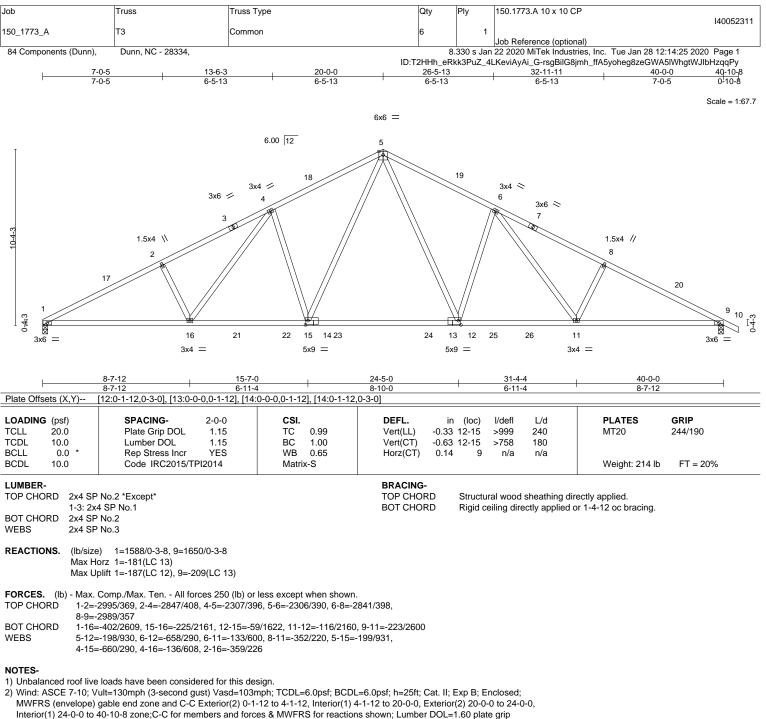
6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 12. This connection is for uplift only and does not consider lateral forces.

7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



DOI = 1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

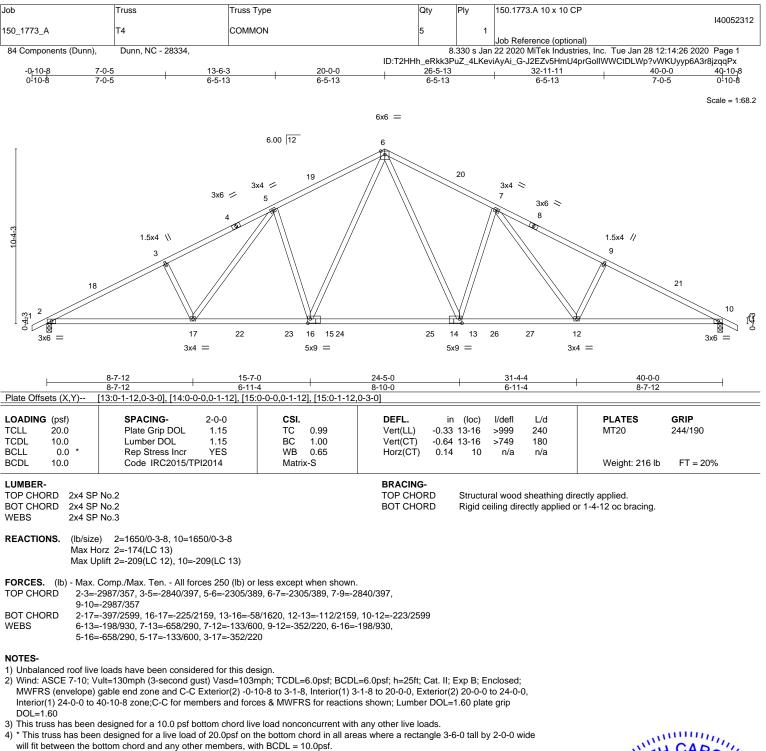
6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 9. This connection is for uplift only and does not consider lateral forces.



🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

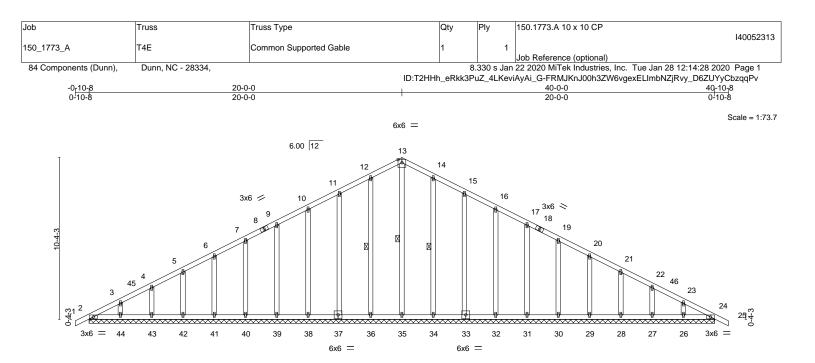


5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.







			40-0-0 40-0-0					
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	n (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.09	Vert(LL) -0.0	) 2́4	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) -0.0	) 24	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.15	Horz(CT) 0.0	1 24	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 272 lb	FT = 20%

## LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

BRACING-TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing 1 Row at midpt 13-35, 12-36, 14-34

REACTIONS. All bearings 40-0-0. Max Horz 2=174(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 36, 37, 38, 39, 40, 41, 42, 43, 44, 34, 33, 32, 31, 30, 29, 28, 27, 26

Max Grav All reactions 250 lb or less at joint(s) 2, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 34, 33, 32, 31, 30, 29, 28, 27, 26, 24

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 11-12=-102/297, 12-13=-118/338, 13-14=-118/341, 14-15=-102/299

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-1-8, Exterior(2) 3-1-8 to 20-0-0, Corner(3) 20-0-0 to 24-0-0, Exterior(2) 24-0-0 to 40-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 1.5x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

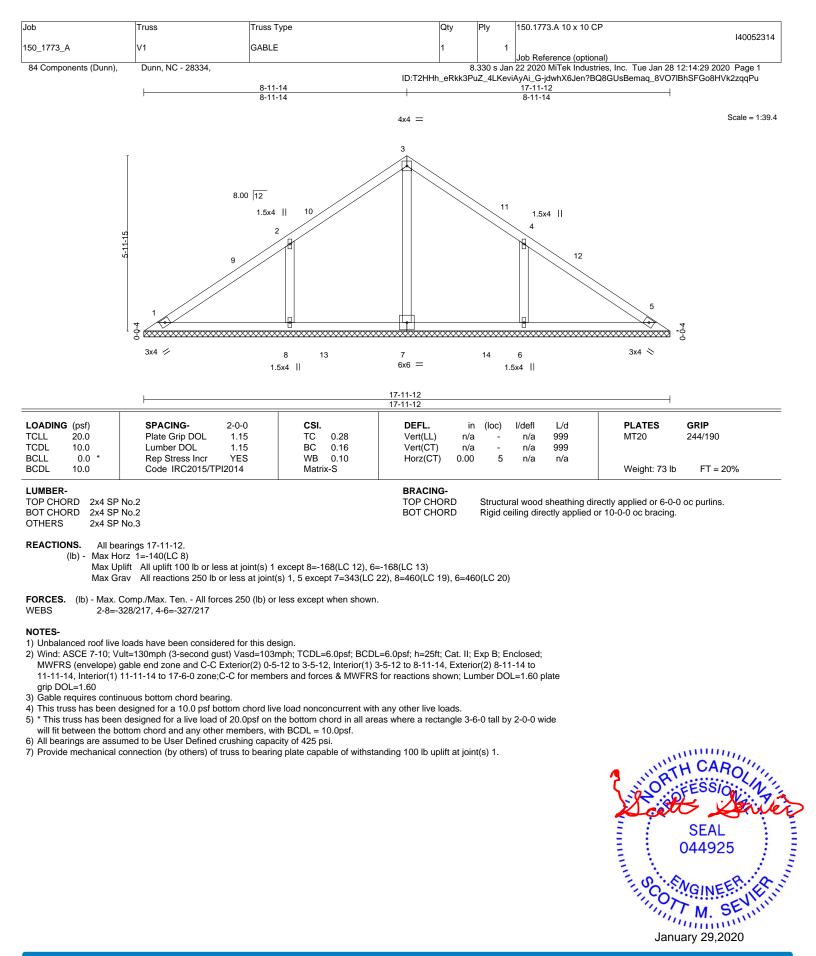
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) All bearings are assumed to be User Defined crushing capacity of 425 psi.

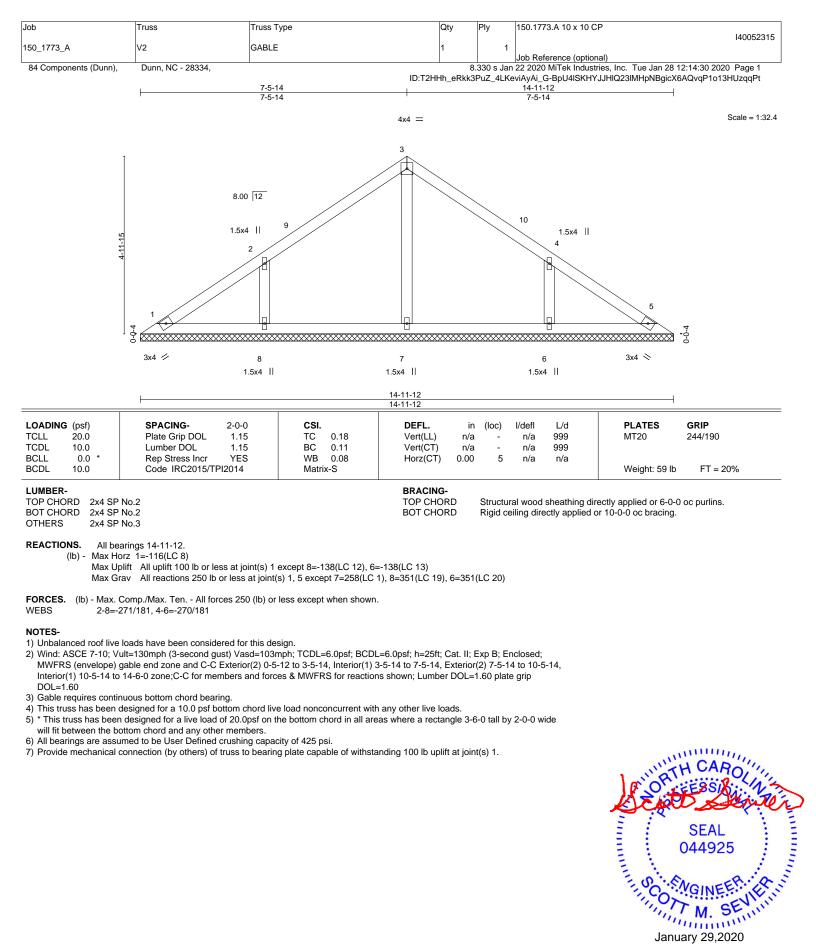






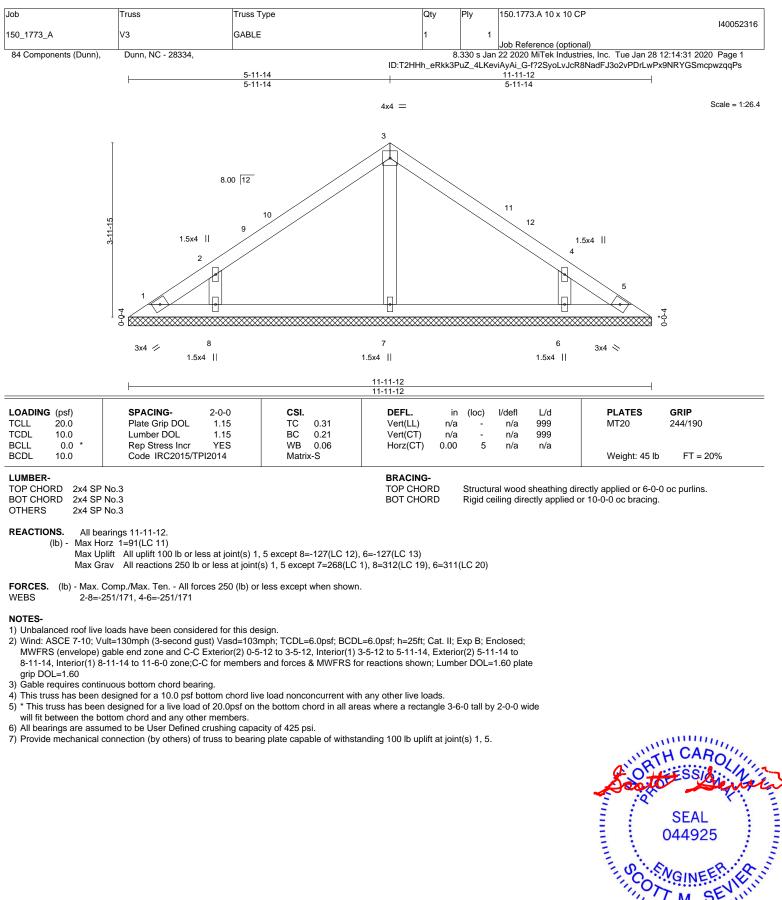
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. ENGINEERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932



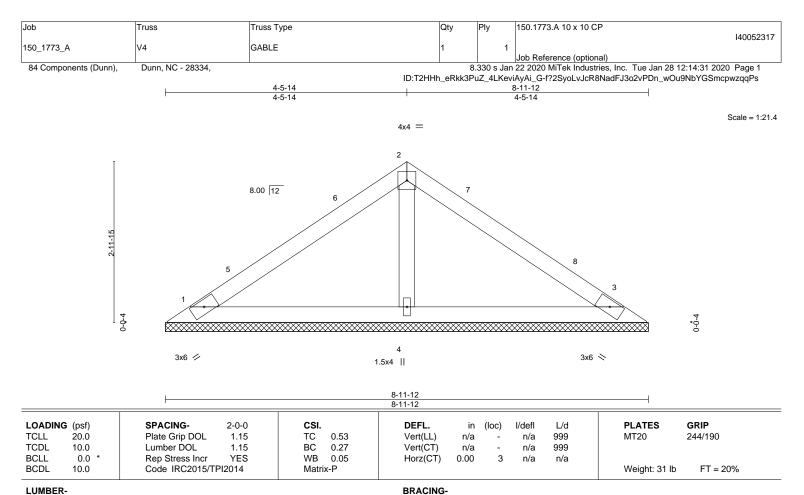
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818 Soundside Road Edenton, NC 27932





January 29,2020



TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

2x4 SP No.3 OTHERS

REACTIONS. 1=172/8-11-12, 3=172/8-11-12, 4=298/8-11-12 (lb/size) Max Horz 1=-66(LC 8) Max Uplift 1=-38(LC 12), 3=-47(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-5-14, Exterior(2) 4-5-14 to 7-5-14, Interior(1) 7-5-14 to 8-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

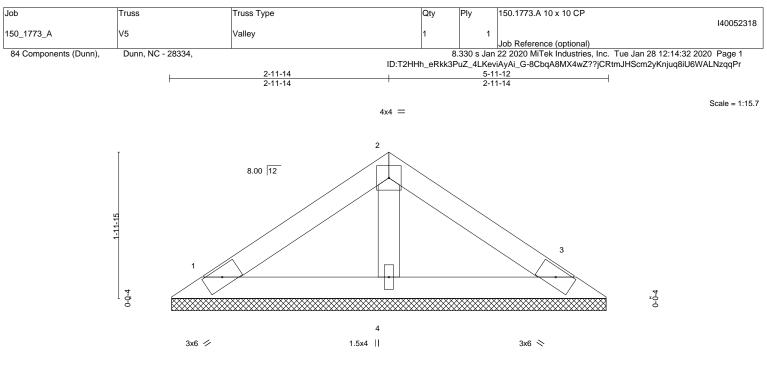
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.







	0-0 <u>-6</u> 0-0-6		5-11-12 5-11-6					
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc		L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.19 BC 0.11	Vert(LL) Vert(CT)	n/a - n/a -	11/04	999 999	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.03 Matrix-P	Horz(CT)	0.00	3 n/a	n/a	Weight: 20 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 1=107/5-11-0, 3=107/5-11-0, 4=186/5-11-0 Max Horz 1=41(LC 9) Max Uplift 1=-24(LC 12), 3=-29(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

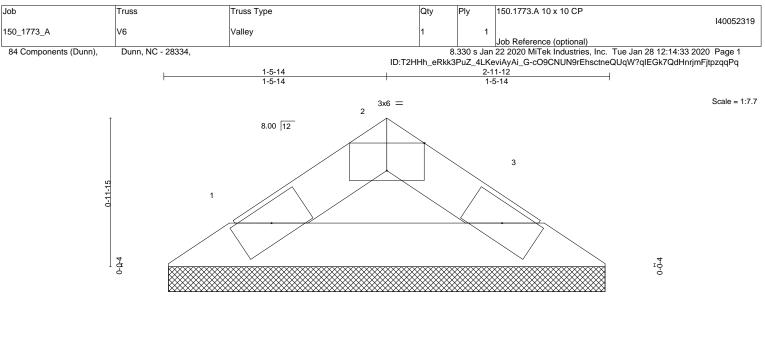
6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



Structural wood sheathing directly applied or 5-11-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





3x6 🥢

3x6 🚿

Structural wood sheathing directly applied or 2-11-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

2-11-12 0-0-6 2-11-6 2-11-6 Plate Offsets (X,Y)--[2:0-3-0,Edge] SPACING-DEFL. PLATES GRIP LOADING (psf) 2-0-0 CSI. in (loc) l/defl L/d Plate Grip DOL 244/190 TCLL 20.0 1.15 тс 0.03 Vert(LL) n/a 999 MT20 n/a TCDL 10.0 Lumber DOL 1.15 BC 0.08 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a Code IRC2015/TPI2014 BCDL 10.0 Matrix-P Weight: 8 lb FT = 20% BRACING-LUMBER-TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

REACTIONS. (lb/size) 1=81/2-11-0, 3=81/2-11-0 Max Horz 1=-17(LC 8) Max Uplift 1=-9(LC 12), 3=-9(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

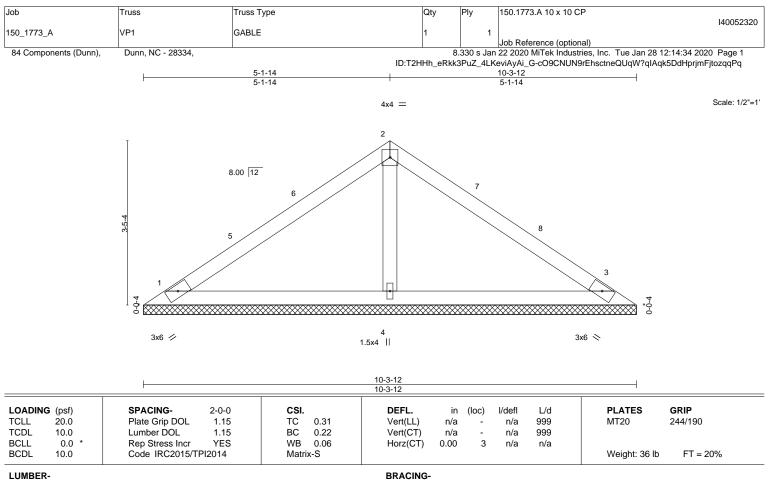
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.







TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS

2x4 SP No.3

REACTIONS. 1=184/10-3-12, 3=184/10-3-12, 4=380/10-3-12 (lb/size) Max Horz 1=77(LC 11) Max Uplift 1=-34(LC 12), 3=-44(LC 13), 4=-12(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-1-14, Exterior(2) 5-1-14 to 8-1-14, Interior(1) 8-1-14 to 9-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5)

will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

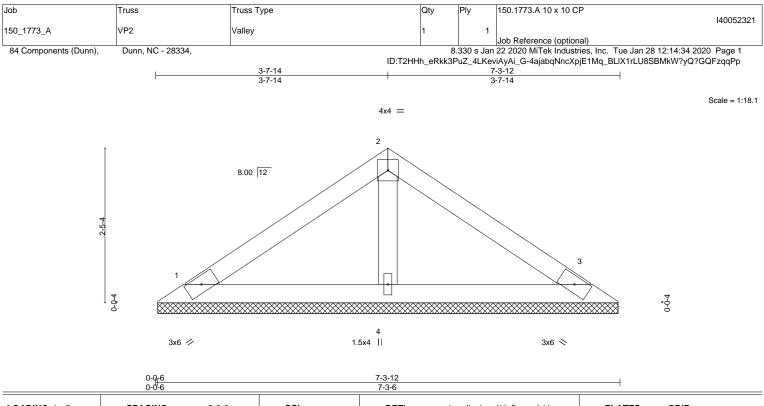


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing



LUMBER-



LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.32	Vert(LL)	n/a -	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(CT)	n/a -	n/a	999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT) 0	.00 3	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 25 lb	FT = 20%
LUMBER-			BRACING-					

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD OTHERS 2x4 SP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=136/7-3-0, 3=136/7-3-0, 4=236/7-3-0 Max Horz 1=-52(LC 10) Max Uplift 1=-30(LC 12), 3=-37(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

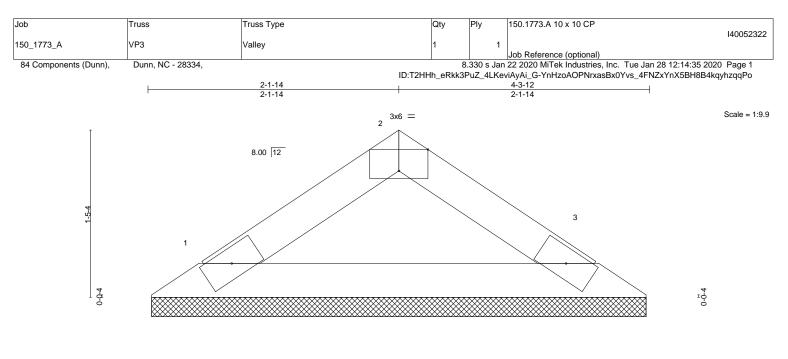
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.







3x6 🥢

3x6 📎

Structural wood sheathing directly applied or 4-3-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

		0-q <sub>1</sub> 6				4-3-12						
		0-0-6				4-3-6					I	
Plate Offse	ets (X,Y)	[2:0-3-0,Edge]										
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-P						Weight: 12 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (Ib/size) 1=134/4-3-0, 3=134/4-3-0 Max Horz 1=-28(LC 8) Max Uplift 1=-14(LC 12), 3=-14(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.





