

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 23211-23211A 150.1910.A - ROOF

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I39970775 thru I39970789

My license renewal date for the state of North Carolina is December 31, 2020.

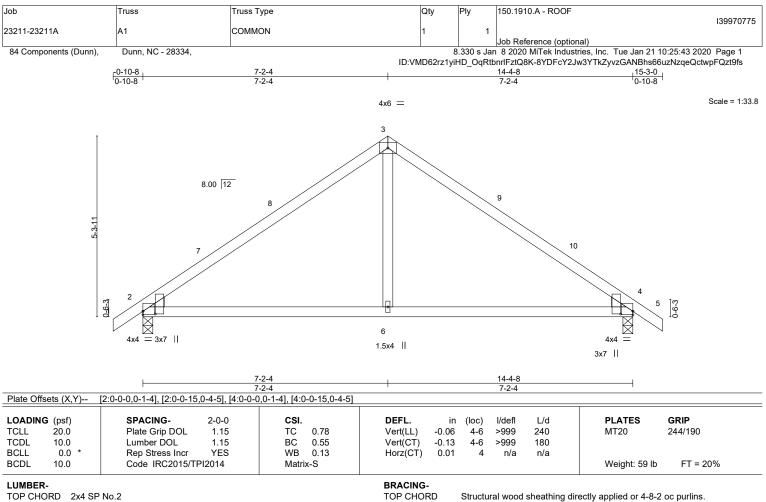
North Carolina COA: C-0844



January 21,2020

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=625/0-3-8, 4=625/0-3-8 Max Horz 2=-133(LC 10) Max Uplift 2=-81(LC 12), 4=-81(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-681/102, 3-4=-681/102

BOT CHORD 2-6=-1/460, 4-6=-1/460 3-6=0/346

WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 7-2-4, Exterior(2) 7-2-4 to 10-2-4, Interior(1) 10-2-4 to 15-3-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

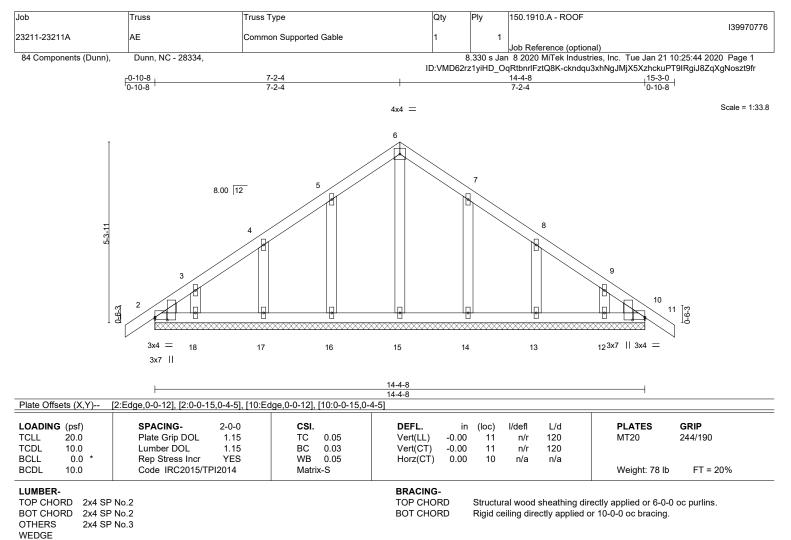
5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid to deviny with with where outputs into used only upon parameters and properly incorporate building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. All bearings 14-4-8.

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 7-2-4, Corner(3) 7-2-4 to 10-2-4, Exterior(2) 10-2-4 to 15-3-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be User Defined crushing capacity of 425 psi.

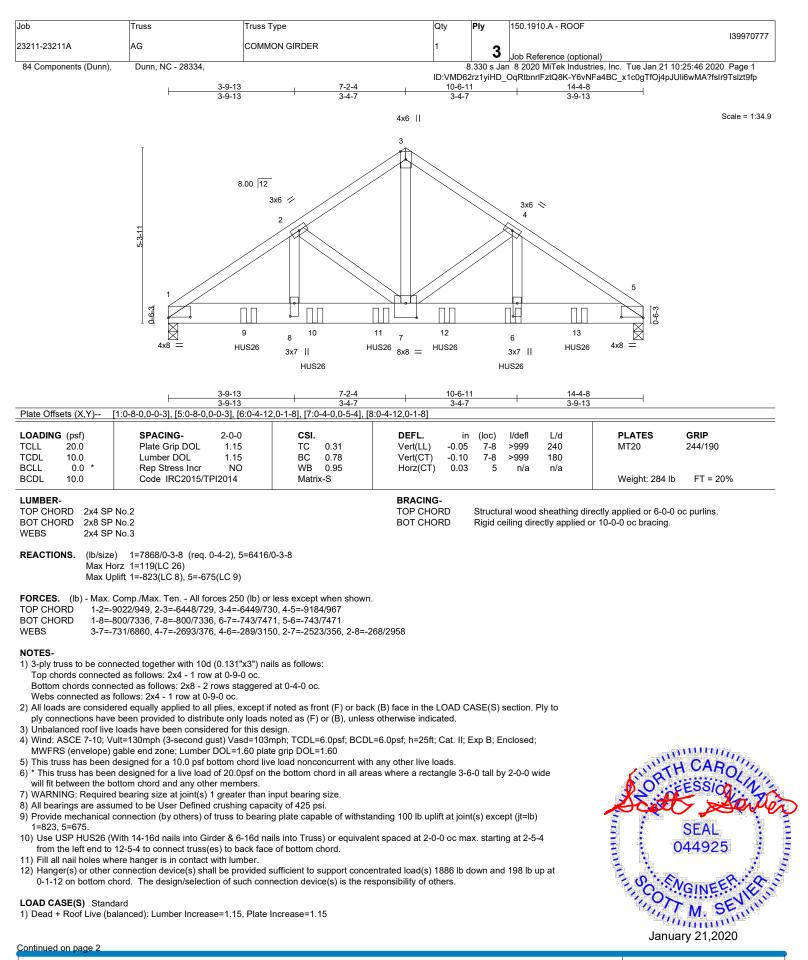


t USE. t uil acing ang Component 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

 ⁽lb) - Max Horz 2=-133(LC 10) Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 16, 17, 18, 14, 13, 12 Max Grav All reactions 250 lb or less at joint(s) 2, 10, 15, 16, 17, 18, 14, 13, 12

¹⁾ Unbalanced roof live loads have been considered for this design.





Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	150.1910.A - ROOF
23211-23211A	AG	COMMON GIRDER	1		139970777
			ľ	3	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.330 s Jar	8 2020 MiTek Industries, Inc. Tue Jan 21 10:25:46 2020 Page 2
			ID:VMD6	2rz1yiHD_	OqRtbnrlFztQ8K-Y6vNFa4BC_x1c0gTfOj4pJUli6wMA?fsIr9Tslzt9fp

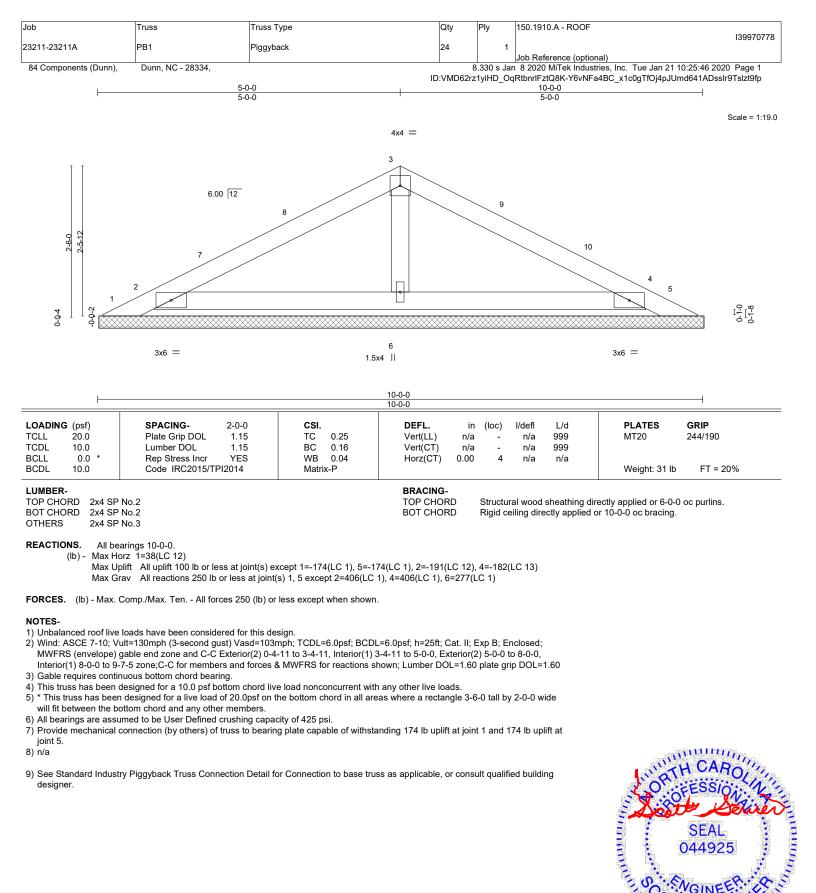
LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20 Concentrated Loads (lb)

Vert: 1=-1886(B) 6=-1879(B) 9=-1879(B) 10=-1879(B) 11=-1879(B) 12=-1879(B) 13=-1879(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

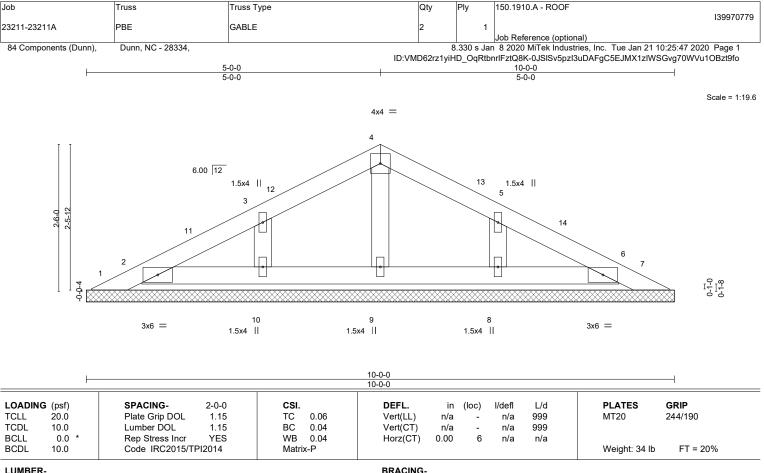




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. TRENGINEERING BY AMITOCATINATA 818 Soundside Road

Edenton, NC 27932

January 21,2020



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS

2x4 SP No.3

REACTIONS. All bearings 10-0-0.

Max Horz 1=38(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 2, 6, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-11 to 3-4-11, Interior(1) 3-4-11 to 5-0-0, Exterior(2) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 9-7-5 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) All bearings are assumed to be User Defined crushing capacity of 425 psi.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7. 10) n/a

11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

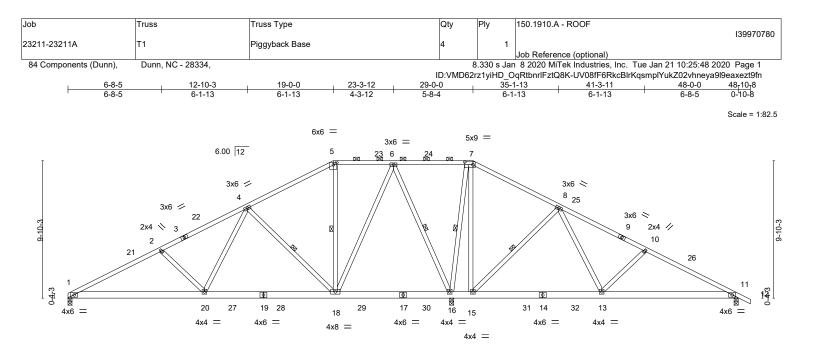


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid to deviny with with where outputs into used only upon parameters and properly incorporate building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





	 	9-9-4	<u>19-0-0</u> 9-2-12			27-5-12 8-5-12	29-0-0 1-6-4		38-2-1 9-2-1		48-0-0	
Plate Offs	sets (X,Y)	[5:0-3-0,0-2-0], [7:0-7-0,0				0-5-12	1-0-4		5-2-12	<u> </u>	9-5-4	
LOADING TCLL TCDL BCLL BCDL	G (psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matrix	0.57 0.47 0.73 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.17 0.02	1-20	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 321 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHC BOT CHC WEBS	ORD 2x4 SF	9 No.2				BRACING TOP CHO BOT CHO	RD	except 2-0-0 c Rigid c	oc purlins eiling dire	(6-0-0 max.) ectly applied	or 6-0-0 oc bracing.	
REACTIC	Max H Max U	e) 1=811/0-3-8, 16=256 orz 1=-174(LC 17) plift 1=-124(LC 12), 16=- irav 1=873(LC 23), 16=25	150(LC 12), 11=	133(LC 13		WEBS		1 Row	at midpt	2	4-18, 5-18, 6-16, 7-16, 8	3-15
FORCES TOP CHC	DRD 1-2=-	Comp./Max. Ten All for 1483/239, 2-4=-1193/204 1=-727/207	()									

 BOT CHORD
 1-20=-290/1256, 18-20=-112/753, 16-18=-278/283, 15-16=-618/281, 13-15=-271/170, 11-13=-92/578

 WEBS
 2-20=-378/226, 4-20=-45/584, 4-18=-726/255, 6-18=-135/982, 6-16=-1313/263,

VEBS 2-20=-378/220, 4-20=-45/584, 4-18=-720/255, 6-18=-135/982, 6-16=-1313/263, 7-16=-1086/192, 7-15=-84/459, 8-15=-730/256, 8-13=-43/605, 10-13=-373/217

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 4-11-6, Interior(1) 4-11-6 to 19-0-0, Exterior(2) 19-0-0 to 25-9-7, Interior(1) 25-9-7 to 29-0-0, Exterior(2) 29-0-0 to 35-9-7, Interior(1) 35-9-7 to 48-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

- 6) WARNING: Required bearing size at joint(s) 16 greater than input bearing size.
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 16, and 11. This connection is for uplift only and does not consider lateral forces.

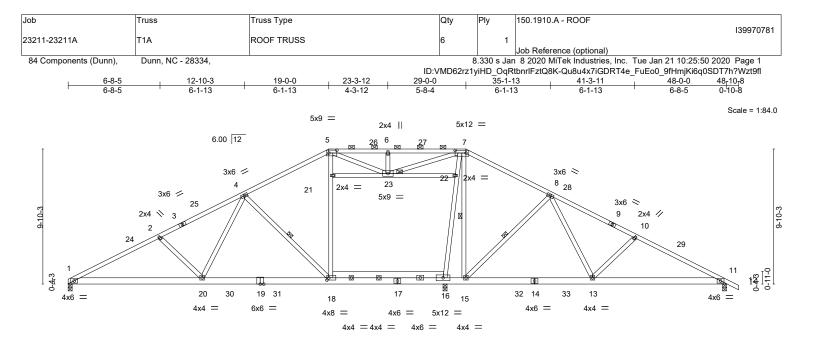
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



TENSINEERING BY REALED AMITEL ATHINATE 818 Soundside Road

Edenton, NC 27932

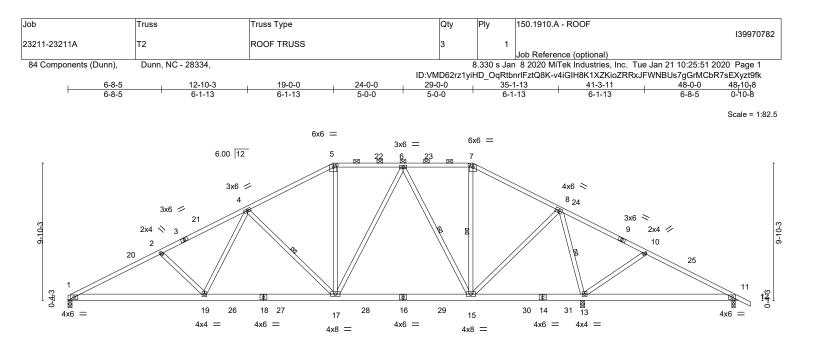
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



1	9-9-4	19-0-0	1	27-5-12	29-0-0 ₁	38-2-12	48-0-0	I
	9-9-4	9-2-12	1	8-5-12	1-6-4	9-2-12	9-9-4	
Plate Offsets (X,Y) [5:0-7-0,0-2-8], [7	7:0-9-8,0-2-4], [18:0-1-12	2,0-2-0]					
LOADING (ps TCLL 20. TCDL 10. BCLL 0. BCDL 10.	.0 Plate Grip .0 Lumber D .0 * Rep Stres	DOL 1.15 OL 1.15	CSI. TC 0.88 BC 0.69 WB 0.86 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT		>816 240 >426 180	PLATES MT20 Weight: 339 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x6 SP DSS *Except* 16-18: 2x6 SP No.2 2x4 SP No.3 *Except* 7-16: 2x4 SP No.2			BRACIN TOP CH BOT CH WEBS JOINTS	ORD Struct 2-0-0 ORD Rigid 1 Rov	oc purlins (3-3-3	ning directly applied, except max.): 5-7. pplied or 9-1-14 oc bracing. 4-18, 7-15, 8-15	
REACTIONS.	Max Horz 1=-174(LC 1 Max Uplift 1=-242(LC 1	, ,	-106(LC 12)					
FORCES. (IL TOP CHORD BOT CHORD WEBS	7-8=-1959/455, 8-10=- 1-20=-527/2872, 18-20 11-13=-313/2564 2-20=-360/224, 4-20=- 16-22=-198/795, 7-22=	All forces 250 (lb) or la 048/498, 4-5=-2151/457 2704/462, 10-11=-2940/)=-345/2378, 16-18=-15 60/667, 4-18=-804/267, 205/814, 7-15=-302/13 325/153, 5-23=-226/43	7, 5-6=-2283/633, 6-7= /480 5/1835, 15-16=-129/17 18-21=-38/573, 5-21= /8, 8-15=-696/257, 8-1	:-2282/633, 722, 13-15=-199/; :-40/586,	2131,			
 Wind: ASCE MWFRS (er Interior(1) 2 for reactions Provide ade This truss h This truss will fit betwee All bearings One RT7A I for uplift onl Graphical p 	d roof live loads have beer E 7-10; Vult=130mph (3-su- nvelope) gable end zone a 5-9-7 to 29-0-0, Exterior(2 s shown; Lumber DOL=1. aquate drainage to preven as been designed for a 10 has been designed for a 10 has been designed for a 10 has been designed for a 10 been the bottom chord and a re assumed to be User USP connectors recomment y and does not consider la urlin representation does CE SHOWN IS DESIGNE	econd gust) Vasd=103m and C-C Exterior(2) 0-1-1 2) 29-0-0 to 35-9-7, Interi 60 plate grip DOL=1.60 twater ponding. 0.0 psf bottom chord live ive load of 20.0psf on th any other members, with Defined crushing capaci ended to connect truss to ateral forces.	ph; TCDL=6.0psf; BCE 12 to 4-11-6, Interior(1) or(1) 35-9-7 to 48-10-6 load nonconcurrent wi e bottom chord in all al b BCDL = 10.0psf. ty of 425 psi. b bearing walls due to to c orientation of the purli) 4-11-6 to 19-0-0 8 zone;C-C for m ith any other live reas where a rec UPLIFT at jt(s) 1,	, Exterior(2) 19- embers and forc loads. angle 3-6-0 tall 16, and 11. This	0-0 to 25-9-7, ies & MWFRS by 2-0-0 wide s connection is		SEAL 44925 GINEER REAL

ERGINEERING BY RENCO AMITAK Atiliate 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



	9-9-4	<u>19-0-0</u> 9-2-12		<u>29-0-0</u> 10-0-0		<u>36-10-4</u> 7-10-4		42-1-14	47-9-8 5-7-10	<u>48-0</u> -0 0-2-8
Plate Offsets (X,Y)	[5:0-3-0,0-2-0], [7:0-3-0,0-2-0]			10-0-0		7-10-4		5-5-10	3-7-10	0-2-0
LOADING (psf)	SPACING- 2	-0-0 CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.15 TC	0.73	Vert(LL)	-0.13 15-17	>999	240	MT20	244/1	90
TCDL 10.0	Lumber DOL	1.15 BC	0.61	Vert(CT)	-0.24 15-17	>999	180			
BCLL 0.0 *	Rep Stress Incr	YES WB	0.54	Horz(CT)	0.05 13	n/a	n/a			
BCDL 10.0	Code IRC2015/TPI20	014 Matrix	k-S					Weight: 309	lb FT :	= 20%
LUMBER-				BRACING-						
	P No.2			TOP CHOR		tural wood s	heathing dir	ectly applied or 2-2	-0 oc purlir	ns except
	P No.2						5-0-6 max.):		o oo parm	io, oneopt
	P No.3			BOT CHOR				or 6-0-0 oc bracing.		
				WEBS	0	v at midpt		-17, 6-15, 7-15, 8-1		
REACTIONS. (lb/siz	e) 1=1362/0-3-8, 13=2381/	0-3-8 (req. 0-3-12), 11=	=135/0-3-0							
Max I	Horz 1=-174(LC 13)									
Max l	Jplift 1=-164(LC 12), 13=-169	(LC 13), 11=-86(LC 26)								
Max 0	Grav 1=1362(LC 1), 13=2381	(LC 1), 11=230(LC 25)								
	. Comp./Max. Ten All forces									
	-2558/382, 2-4=-2270/361, 4-		2/350, 6-7=-70	03/266,						
	-862/244, 8-10=-100/841, 10-									
	=-369/2216, 17-19=-190/172			540						
	=-372/226, 4-19=-47/560, 4-1	,	,	518,						
6-15	5=-833/187, 8-15=-117/1245, 8	3-13=-1961/383, 10-13=	-405/228							
NOTES-										
1) Unbalanced roof liv	e loads have been considered	d for this design.								
2) Wind: ASCE 7-10; '	Vult=130mph (3-second gust)	Vasd=103mph; TCDL=	6.0psf; BCDL	=6.0psf; h=25ft; C	at. II; Exp B;	Enclosed;				
MWFRS (envelope) gable end zone and C-C Ext	erior(2) 0-1-12 to 4-11-6	6, Interior(1) 4	-11-6 to 19-0-0, E	xterior(2) 19-	0-0 to 25-9-	7,			
Interior(1) 25-9-7 to	29-0-0, Exterior(2) 29-0-0 to	35-9-7, Interior(1) 35-9-	7 to 48-10-8 z	one;C-C for mem	bers and forc	es & MWFF	RS			
for reactions shown	; Lumber DOL=1.60 plate grip	DOL=1.60								

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) WARNING: Required bearing size at joint(s) 13 greater than input bearing size.

7) All bearings are assumed to be User Defined crushing capacity of 425 psi.

8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 13, and 11. This connection is for uplift only and does not consider lateral forces.

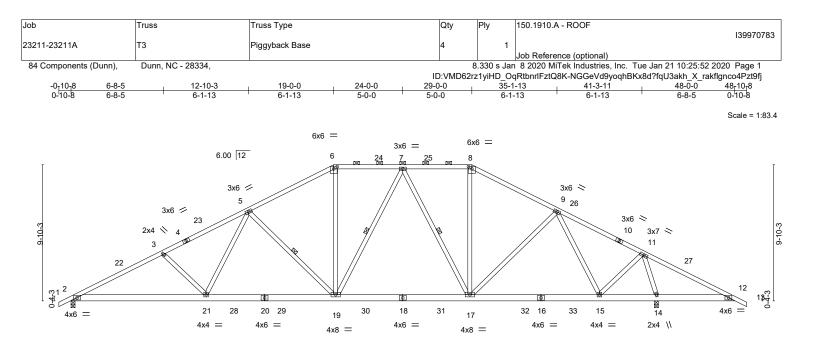
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

SEAL 044925 January 21,2020



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, eleivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



		-0-0 2-12	<u>29-0-0</u> 10-0-0	35-1-13 6-1-13	38-2-12 3-0-15	42-4-4	48-0-0 5-7-12
Plate Offsets (X,Y)	[6:0-3-0,0-2-0], [8:0-3-0,0-2-0]	1					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.64 BC 0.71 WB 0.85 Matrix-S	Vert(LL) -0.1	in (loc) l/defl 7 17-19 >999 2 19-21 >999 8 14 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 314 I	GRIP 244/190 b FT = 20%
BOT CHORD 2x6 S	> No.2 > No.2 > No.3		BRACING- TOP CHORD	Structural wood except 2-0-0 oc purlins	0	ectly applied or 2-5- 6-8.	14 oc purlins,
			BOT CHORD	Rigid ceiling dire 6-0-0 oc bracing		r 10-0-0 oc bracing,	Except:
	e) 2=1716/0-3-8, 14=2226/0-3-8 Horz 2=-166(LC 13) Jplift 2=-206(LC 12), 14=-236(LC 13)		WEBS	1 Row at midpt	5-	19, 7-19, 7-17	

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-3195/453, 3-5=-2920/436, 5-6=-2157/418, 6-7=-1871/415, 7-8=-1622/399, 8-9=-1880/393, 9-11=-1301/249, 11-12=-224/643 TOP CHORD

	8-9=-1880/393, 9-11=-1301/249, 11-12=-224/643
BOT CHORD	2-21=-400/2780, 19-21=-230/2319, 17-19=-94/1812, 15-17=-132/1456, 14-15=0/252,
	12-14=-495/285
WEBS	3-21=-358/216, 5-21=-42/559, 5-19=-709/256, 6-19=-63/673, 7-17=-542/154,
	8-17=-33/559, 9-17=-19/286, 11-15=-126/1289, 11-14=-2147/394, 9-15=-884/211

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-Č Exterior(2) -0-10-8 to 3-11-2, Interior(1) 3-11-2 to 19-0-0, Exterior(2) 19-0-0 to 25-9-7, Interior(1) 25-9-7 to 29-0-0, Exterior(2) 29-0-0 to 35-9-7, Interior(1) 35-9-7 to 48-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

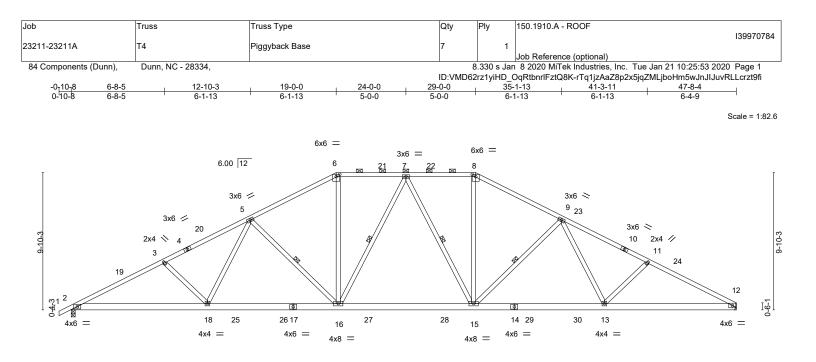
7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 14. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid to deviny with with where outputs into used only upon parameters and properly incorporate building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



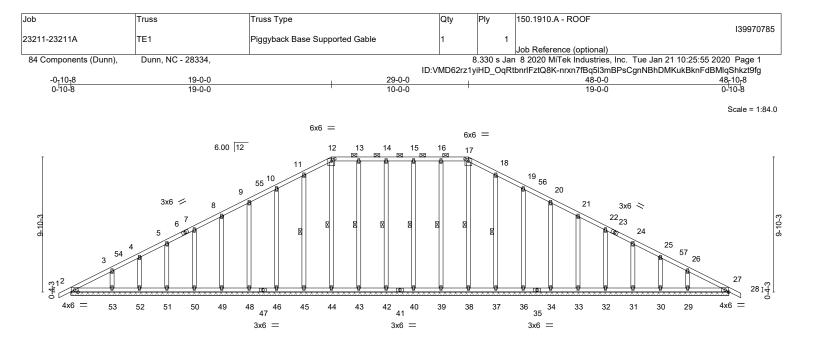
L	9-9-4	19-0-0		29-0-0		38-2-12	47-8-4	
	9-9-4	9-2-12	I	10-0-0	1	9-2-12	9-5-8	
Plate Offsets (X,Y)	[6:0-3-0,0-2-0], [8:0-3-0,0-2-	.0]		1				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15 YES 014	CSI. TC 1.00 BC 0.80 WB 0.38 Matrix-S		in (loc) -0.27 15-16 -0.52 15-16 0.14 12	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 307 lb	GRIP 244/190 FT = 20%
Max H	No.2			BRACING- TOP CHORI BOT CHORI WEBS	2-0-0 o Rigid c	ral wood sheathing di c purlins (3-7-7 max.): eiling directly applied at midpt 5	: 6-8.	
TOP CHORD 2-3=- 8-9=- BOT CHORD 2-18: WEBS 3-18:	Comp./Max. Ten All force 3734/533, 3-5=-3452/522, 5 -2676/498, 9-11=-3394/519, -421/3261, 16-18=-328/278 -356/216, 5-18=-42/554, 5- -358/151, 8-15=-95/903, 9-	-6=-2685/504, 6- 11-12=-3657/540 9, 15-16=-200/24 16=-706/256, 6-16	7=-2347/498, 7-8=-2 07, 13-15=-309/275 5=-93/908, 7-16=-34	2338/493, 9, 12-13=-399/317 13/151,	3			
 2) Wind: ASCE 7-10; MWFRS (envelope) 25-8-15, Interior(1) // MWFRS for reactior 3) Provide adequate di 4) This truss has been 5) * This truss has beee will fit between the b 6) All bearings are ass 	a loads have been considered (ult=130mph (3-second gust gable end zone and C-C Ex 25-8-15 to 29-0-0, Exterior(2 is shown; Lumber DOL=1.6(rainage to prevent water por designed for a 10.0 ps bott in designed for a live load of bottom chord and any other r umed to be User Defined cru) Vasd=103mph; terior(2) -0-10-8 t) 29-0-0 to 35-8-1) plate grip DOL= iding. om chord live load 20.0psf on the bo nembers, with BC	o 3-10-12, Interior(1 5, Interior(1) 35-8-1 1.60 d nonconcurrent with ttom chord in all are CDL = 10.0psf.) 3-10-12 to 19-0-0 5 to 47-7-8 zone;C n any other live load	, Exterior(2) 19 C for member	9-0-0 to rs and forces &	i another	CAROLINA ESSIO

- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 12=185.9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





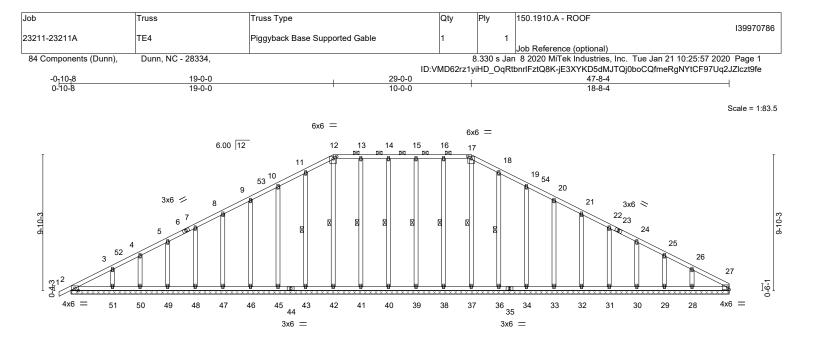
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



 			<u>48-0-0</u> 48-0-0		
Plate Offsets (X,Y)	[12:0-3-0,0-2-0], [17:0-3-0,0-2-0]		40-0-0		
OADING (psf) "CLL 20.0 "CDL 10.0 3CLL 0.0 3CDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.06 WB 0.13 Matrix-S	DEFL. vert(LL) 0.0 Vert(CT) 0.0 Horz(CT) 0.0	0 28 n/r 120	PLATES GRIP MT20 244/190 Weight: 350 lb FT = 20%
UMBER- OP CHORD 2x4 SP OT CHORD 2x4 SP ITHERS 2x4 SP	No.2	1	BRACING- TOP CHORD BOT CHORD WEBS	2-0-0 oc purlins (6-0-0 m	ng directly applied or 6-0-0 oc purlins, except hax.): 12-17. lied or 10-0-0 oc bracing. 17-38, 16-39, 15-40, 14-42, 13-43, 12-44, 11-45, 18-37
(lb) - Max Ho Max Up Max G ORCES. (lb) - Max. 0 OP CHORD 10-11	arings 48-0-0. orz 2=-166(LC 13) plift All uplift 100 lb or less at joint(s) 2, 33, 32, 31, 30, 29 rav All reactions 250 lb or less at joint(37, 36, 34, 33, 32, 31, 30, 29, 27 Comp./Max. Ten All forces 250 (lb) or =-94/284, 11-12=-112/331, 12-13=-98/3	s) 2, 38, 39, 40, 42, 43, 4 less except when shown 15, 13-14=-98/315, 14-1	14, 45, 46, 48, 49, 50, 5		
IOTES-) Unbalanced roof live) Wind: ASCE 7-10; Vi MWFRS (envelope) (Exterior(2) 23-9-10 to MWFRS for reactions) Truss designed for w Gable End Details as) Provide adequate dra) All plates are 2x4 MT) Gable requires contir) Gable studs spaced) This truss has been will fit between the bo	=-98/315, 16-17=-98/315, 17-18=-112/3 loads have been considered for this de ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Corner(3) -0-1 o 29-0-0, Corner(3) 29-0-0 to 33-9-10, E s shown; Lumber DOL=1.60 plate grip D ind loads in the plane of the truss only. s applicable, or consult qualified building ainage to prevent water ponding. F20 unless otherwise indicated. nuous bottom chord bearing. at 2-0-0 oc. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on t ottom chord and any other members. sumed to be User Defined crushing cap.	sign. nph; TCDL=6.0psf; BCDI 0-8 to 3-11-2, Exterior(2) ixterior(2) 33-9-10 to 48-1 OOL=1.60 For studs exposed to wi g designer as per ANSI/TI e load nonconcurrent with he bottom chord in all are	3-11-2 to 19-0-0, Corne 10-8 zone;C-C for memil nd (normal to the face), Pl 1.	r(3) 19-0-0 to 23-9-10, pers and forces & see Standard Industry	SEAL 044925
12) Graphical purlin rep	presentation does not depict the size or	the orientation of the purl	in along the top and/or l	pottom chord.	MGINEER IRAN

onent 818 Soundside Road Edenton, NC 27932

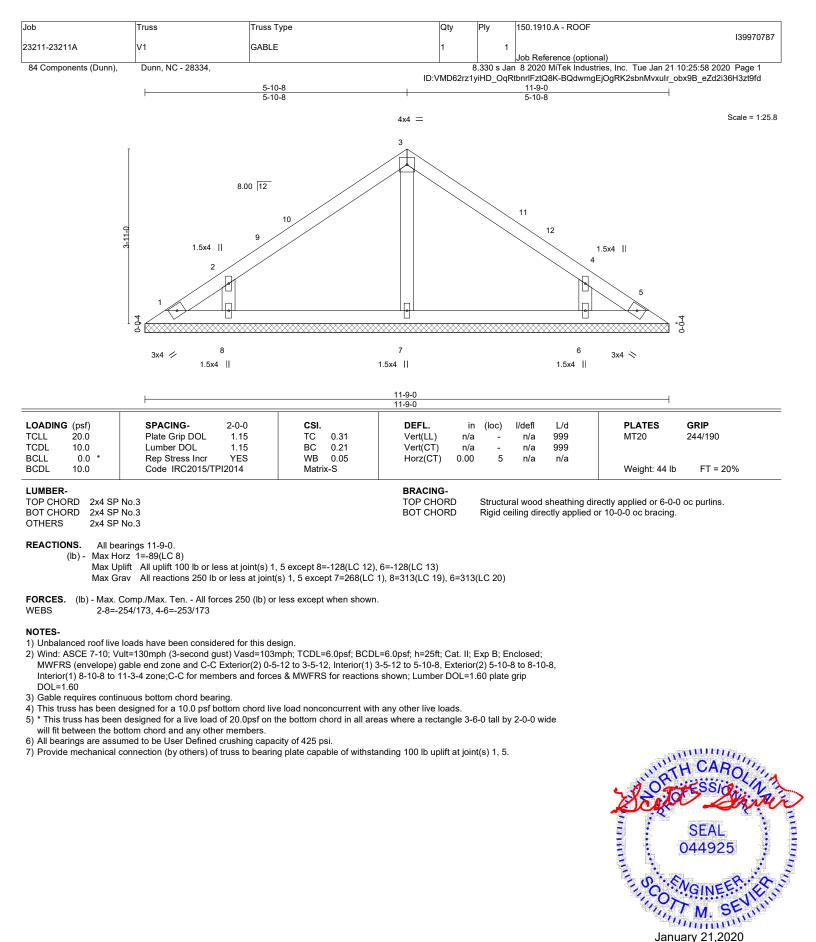
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



F			47-8-4 47-8-4				
Plate Offsets (X,Y)	[12:0-3-0,0-2-0], [17:0-3-0,0-2-0]		47-0-4				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.06 WB 0.13 Matrix-S	DEFL. i Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) 0.0	0 1 n/r	L/d 120 120 n/a	PLATES MT20 Weight: 347 lb	GRIP 244/190 FT = 20%
UMBER- COP CHORD 2x4 SF SOT CHORD 2x4 SF DTHERS 2x4 SF	P No.2		BRACING- TOP CHORD BOT CHORD WEBS	2-0-0 oc purlins (6-0-0 max.): 1 ctly applied or 17-	2-17. 2-17. 10-0-0 oc bracing. 37, 16-38, 15-39, 14 43, 18-36	
(Ib) - Max H Max U	earings 47-8-4. lorz 2=173(LC 12) Jplift All uplift 100 lb or less at joint(s) 2, 32, 31, 30, 29, 28 Grav All reactions 250 lb or less at joint(36, 34, 33, 32, 31, 30, 29, 28, 27						
OP CHORD 10-1 15-10	Comp./Max. Ten All forces 250 (lb) or 1=-92/276, 11-12=-110/323, 12-13=-97/3 6=-97/308, 16-17=-97/308, 17-18=-110/3	308, 13-14=-97/308, 14-1 326, 18-19=-92/279					
) Wind: ASCE 7-10; MWFRS (envelope) Exterior(2) 23-9-4 to	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Corner(3) -0-1 o 29-0-0, Corner(3) 29-0-0 to 33-9-4, Ext ; Lumber DOL=1.60 plate grip DOL=1.60	mph; TCDL=6.0psf; BCD 0-8 to 3-10-12, Exterior(2 erior(2) 33-9-4 to 47-8-4	2) 3-10-12 to 19-0-0, Coi	rner(3) 19-0-0 to 23-			
Gable End Details a 4) Provide adequate d 5) All plates are 2x4 M	wind loads in the plane of the truss only. as applicable, or consult qualified building rainage to prevent water ponding. T20 unless otherwise indicated. inuous bottom chord bearing.		(),	see Standard Indus	try	Current Print	A CAROLY
8) This truss has been 9) * This truss has bee will fit between the b	designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on t bottom chord and any other members. ssumed to be User Defined crushing cap	he bottom chord in all are		-6-0 tall by 2-0-0 wid	de	Acata (SEAL 044925
12) Graphical purlin re	presentation does not depict the size or	the orientation of the purl	in along the top and/or I	bottom chord.		11,01	M. SEVIER

TRENGINEERING BY REPART Attiliate AMITER Attiliate B18 Soundside Road Edenton, NC 27932

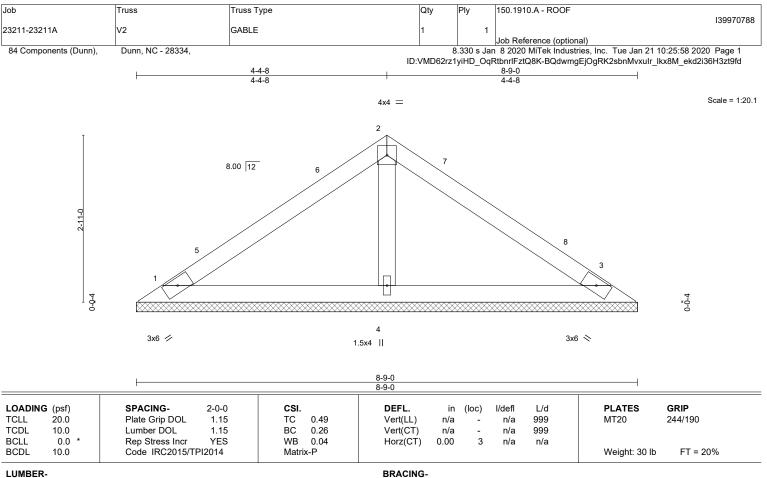
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932



TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD OTHERS

2x4 SP No.3

REACTIONS. 1=167/8-9-0, 3=167/8-9-0, 4=289/8-9-0 (lb/size) Max Horz 1=64(LC 11) Max Uplift 1=-37(LC 12), 3=-46(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-4-8, Exterior(2) 4-4-8 to 7-4-8, Interior(1) 7-4-8 to 8-3-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid to deviny with with where outputs into used only upon parameters and properly incorporate building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



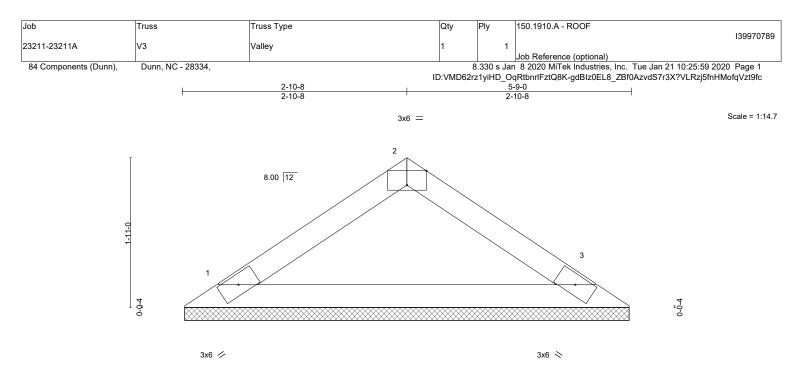


Plate Offsets (X,Y)	0-9 <u>-6</u> 0-0-6 [2:0-3-0,Edge]		5-9-0 5-8-10			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.17 BC 0.49 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	1012(01)	0.00 0	11/4 11/4	Weight: 17 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-9-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (lb/size) 1=191/5-8-4, 3=191/5-8-4 Max Horz 1=-39(LC 8) Max Uplift 1=-21(LC 12), 3=-21(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

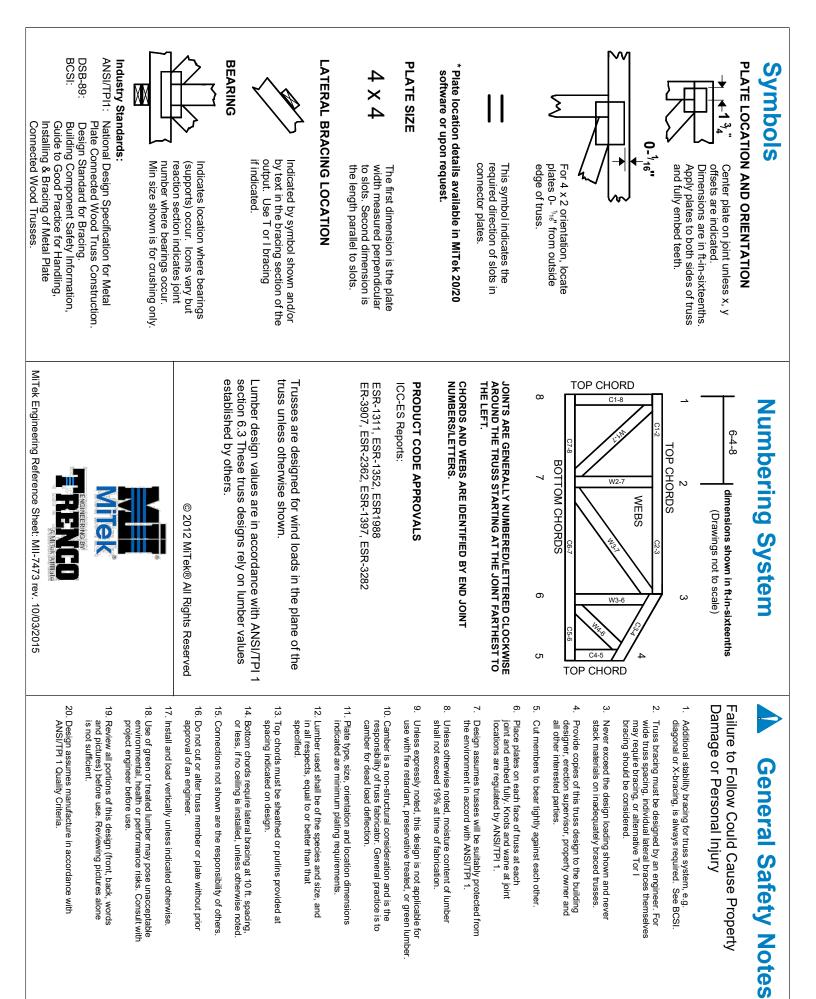
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, eleivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







Trenco 818 Soundside Rd Edenton, NC 27932

Re: 23211-23211A 150.1910.EXT PORCH

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I39970901 thru I39970910

My license renewal date for the state of North Carolina is December 31, 2020.

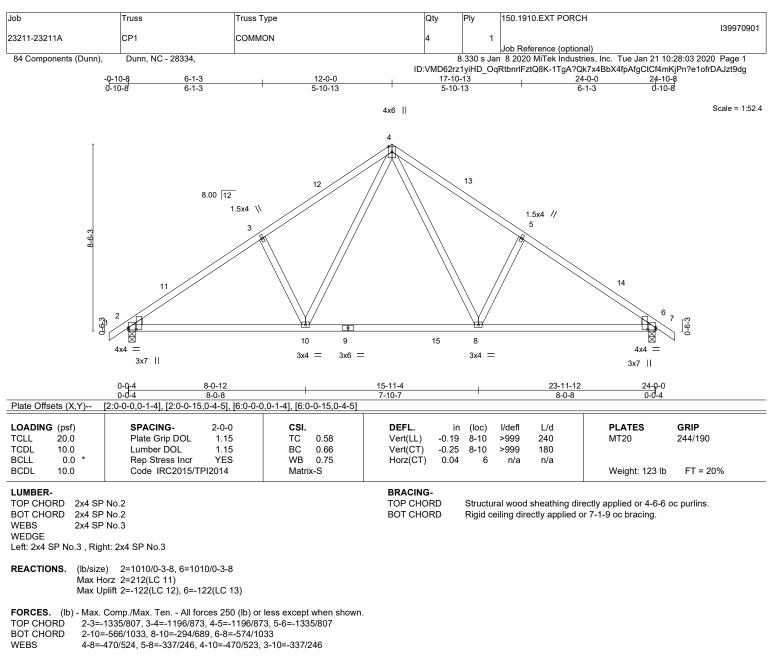
North Carolina COA: C-0844



January 21,2020

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 12-0-0, Exterior(2) 12-0-0 to 15-0-0, Interior(1) 15-0-0 to 24-10-8 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

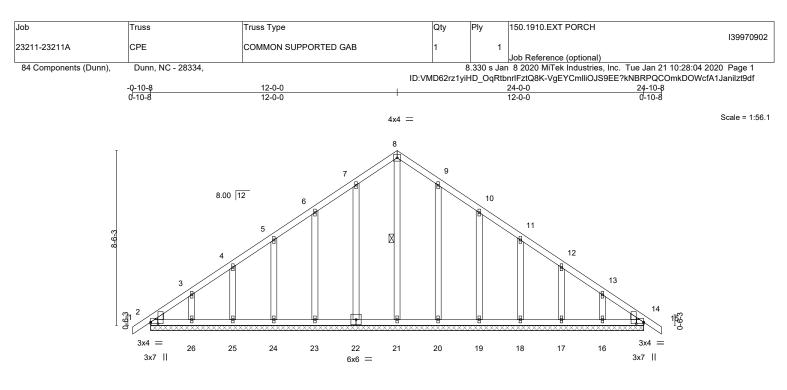
5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





			<u>24-0-0</u> 24-0-0					
Plate Offsets (X,Y)	[2:0-0-0,0-0-12], [2:0-0-15,0-4-5], [14:0	-0-0,0-0-12], [14:0-0-15,0-	-4-5]				1	
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL) -0.00	1 4	n/r	120	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) -0.00	14	n/r	120		
3CLL 0.0 *	Rep Stress Incr YES	WB 0.12	Horz(CT) 0.01	14	n/a	n/a		
3CDL 10.0	Code IRC2015/TPI2014	Matrix-S					Weight: 154 lb	FT = 20%

BOT CHORD

WEBS

Rigid ceiling directly applied or 10-0-0 oc bracing.

8-21

1 Row at midpt

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

(lb) - Max Horz 2=212(LC 11)
 Max Uplift All uplift 100 lb or less at joint(s) 2, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16, 14
 Max Grav All reactions 250 lb or less at joint(s) 2, 21, 22, 23, 24, 25, 26, 20, 19, 18, 17, 16, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-0-0, Exterior(2) 2-0-0 to 12-0-0, Corner(3) 12-0-0 to 15-0-0, Exterior(2) 15-0-0 to 24-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 22.
- 11) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 23, 24, 25, 26, 20, 19, 18, 17, 16, and 14. This connection is for uplift only and does not consider lateral forces.

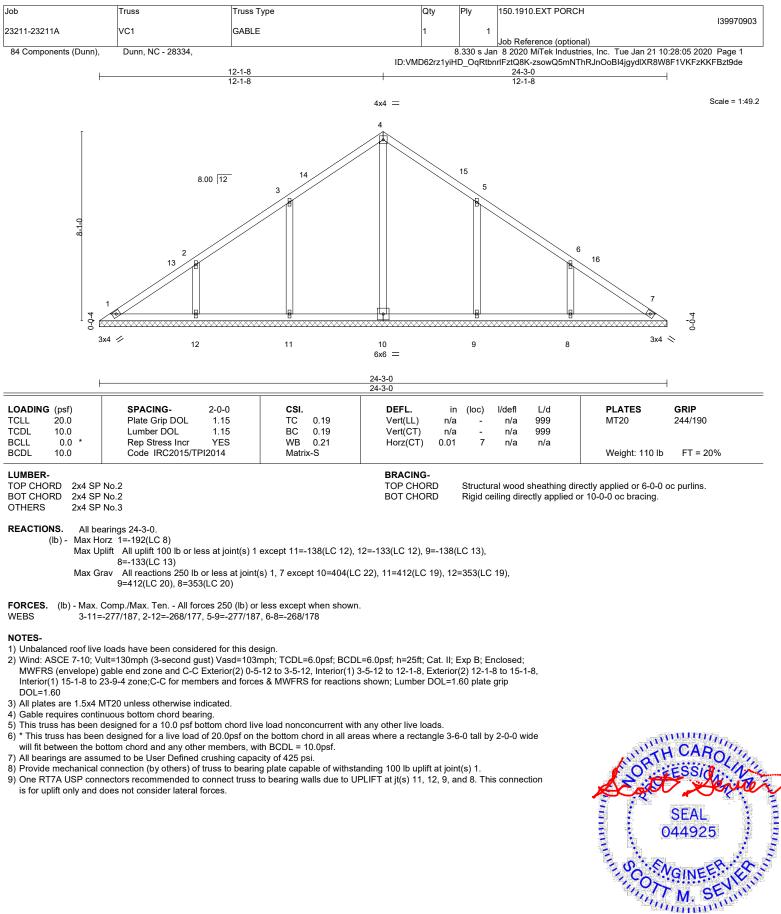


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



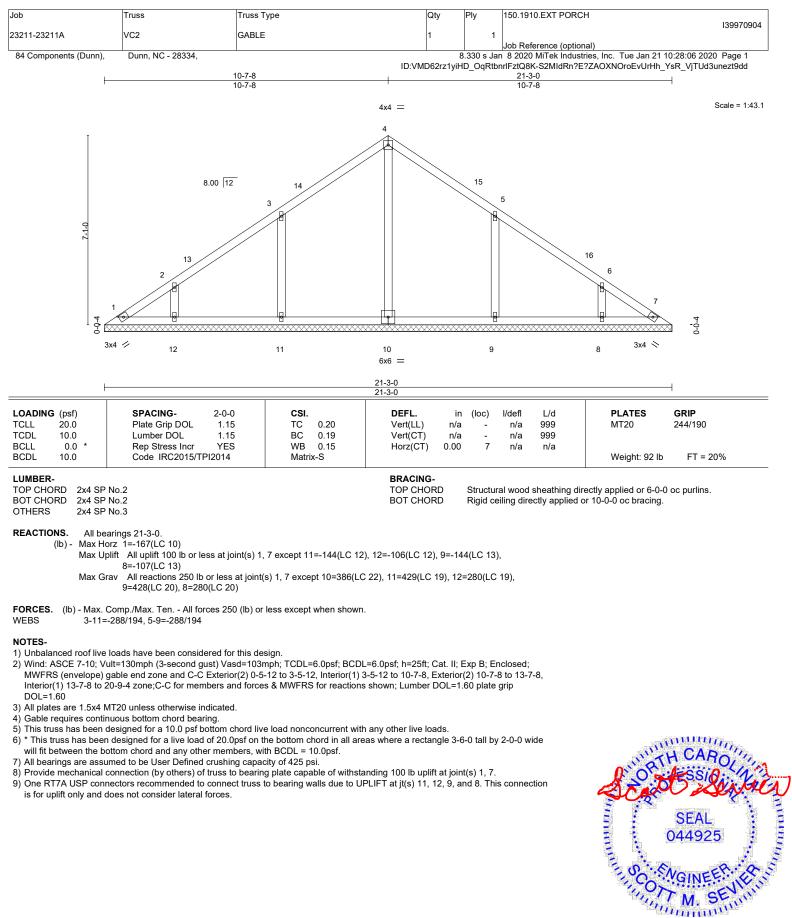
REACTIONS. All bearings 24-0-0.

¹⁾ Unbalanced roof live loads have been considered for this design.



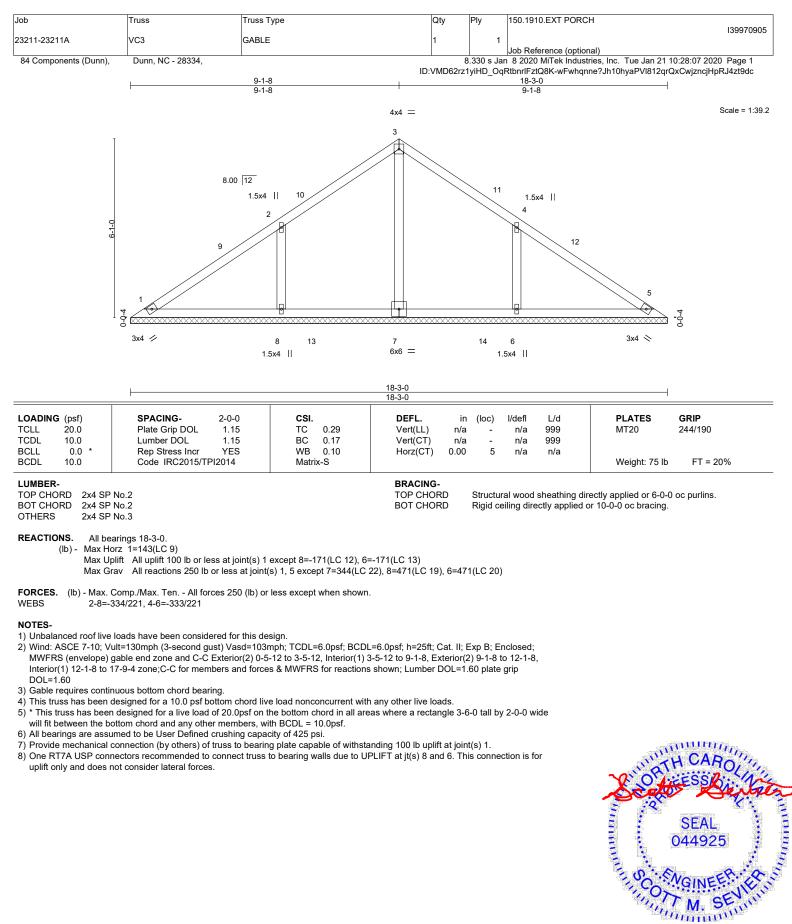
TENSINEEERING BY EREENCO AMITOL At Made 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocllapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

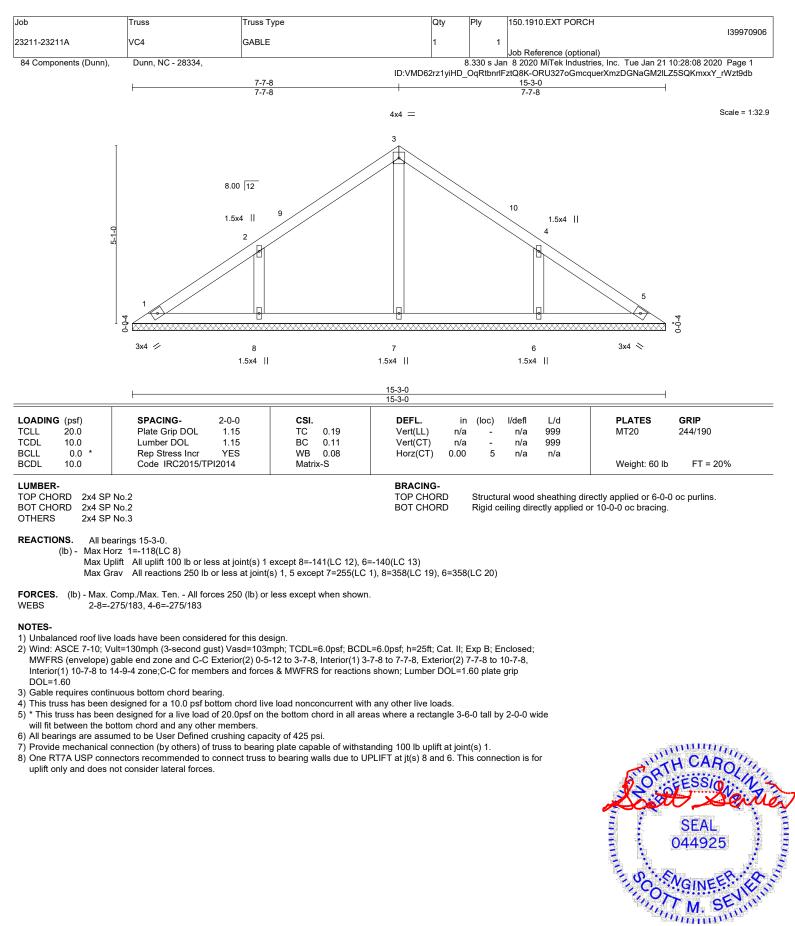




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

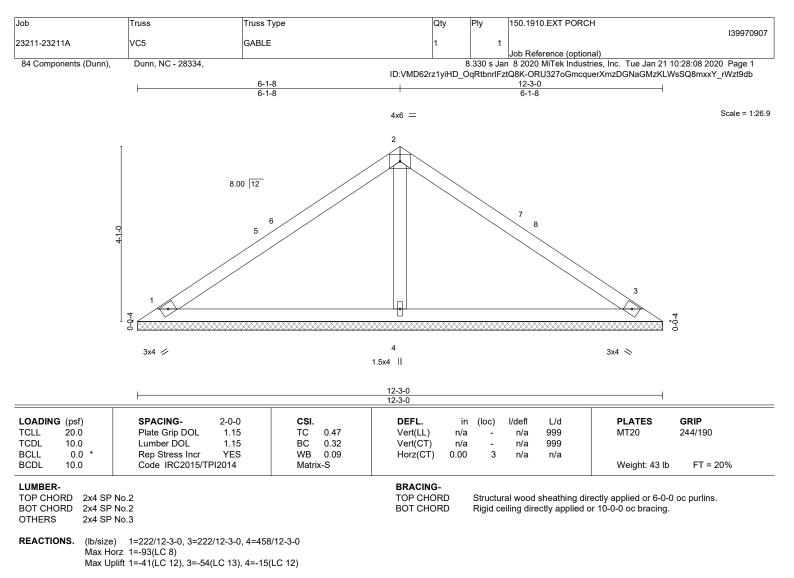
BIRENCO AMITEK Atlifiate 818 Soundside Road

Edenton, NC 27932



PERSINEERING BY CREECING BY AMITEK Atiliate 818 Soundside Road Edenton, NC 27932

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-4=-292/88

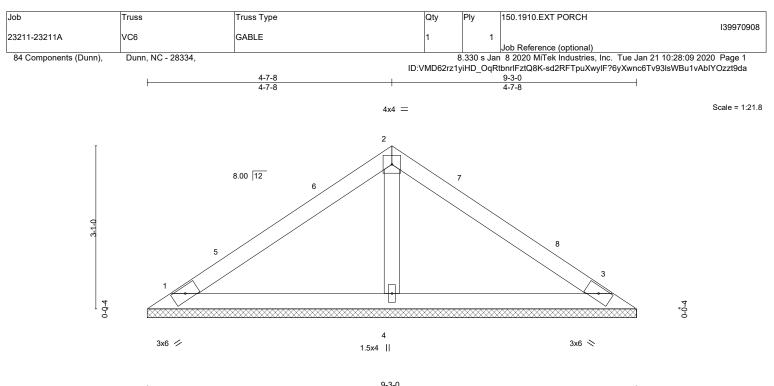
NOTES.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-1-8, Exterior(2) 6-1-8 to 9-1-8,
- Interior(1) 9-1-8 to 11-9-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid to deviny with with where outputs into used only upon parameters and properly incorporate building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





						9-3-0					1	
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	тс	0.40	Vert(LL)	n/a	-	n/a	999	MT20	244/190
FCDL	10.0	Lumber DOL	1.15	BC	0.29	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matriz	k-S						Weight: 32 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

REACTIONS. (lb/size) 1=162/9-3-0, 3=162/9-3-0, 4=340/9-3-0 Max Horz 1=-68(LC 8)

Max Uplift 1=-30(LC 12), 3=-39(LC 13), 4=-12(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-7-8, Exterior(2) 4-7-8 to 7-7-8, Interior(1) 7-7-8 to 8-9-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.



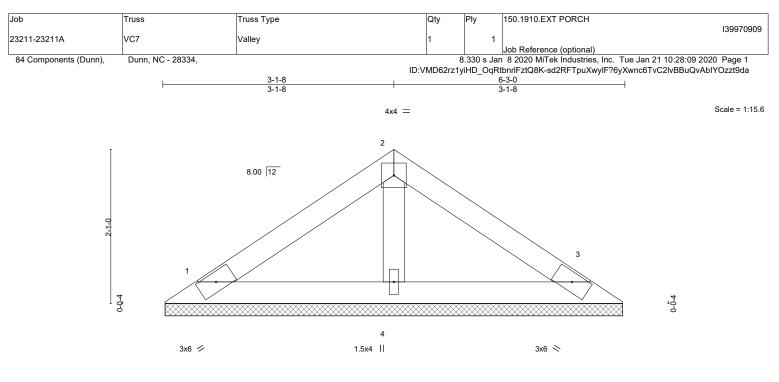
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



LUMBER-



	0-0 <u>-</u> 6 0-0-6							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.21 BC 0.12 WB 0.03 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 21 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 1=113/6-2-4, 3=113/6-2-4, 4=196/6-2-4 Max Horz 1=44(LC 9) Max Uplift 1=-25(LC 12), 3=-31(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

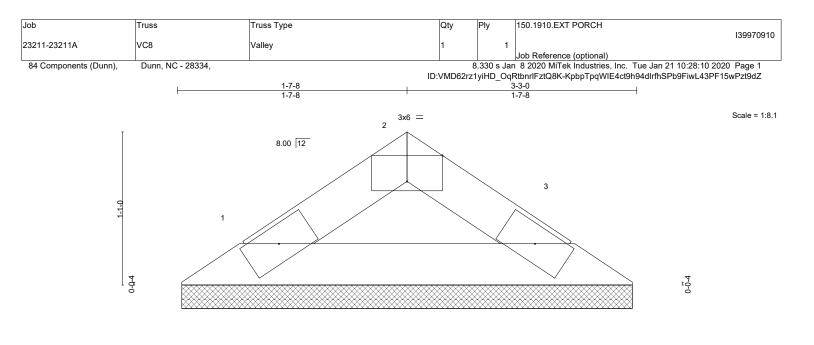


818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid to deviny with with where outputs into used only upon parameters and properly incorporate building design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



3x6 🥢

3x6 🚿

Structural wood sheathing directly applied or 3-3-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	<u>3-3-0</u> 3-2-10										
Plate Offsets (X	') [2:0-3-0,Edge]										
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.03	Vert(LL)	n/a	-	n/a	999	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	n/a	-	n/a	999		
3CLL 0.0	 Rep Stress Incr 	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matrix	κ- Ρ						Weight: 9 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (Ib/size) 1=91/3-2-4, 3=91/3-2-4 Max Horz 1=-19(LC 8) Max Uplift 1=-10(LC 12), 3=-10(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, eleivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



