

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 22357A 140.1445.C.10x21 CP

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I38918546 thru I38918587

My license renewal date for the state of North Carolina is December 31, 2019.

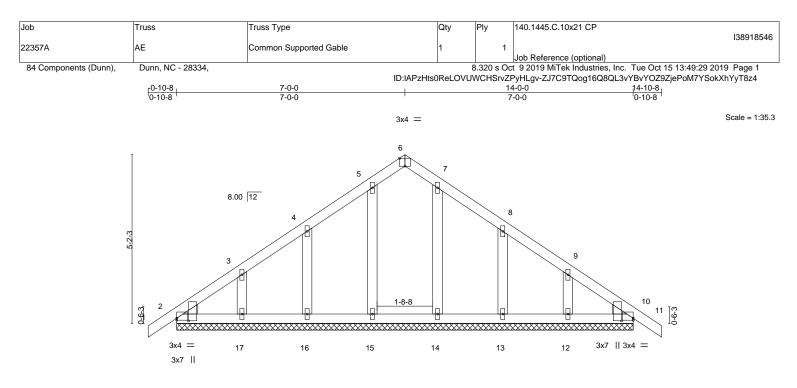
North Carolina COA: C-0844



October 16,2019

# Sevier, Scott

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



14-0-0 14-0-0 Plate Offsets (X,Y)--[2:0-0-15,0-4-5], [2:0-0-0,0-0-12], [6:0-2-0,Edge], [10:Edge,0-0-12], [10:0-0-15,0-4-5] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 тс 0.05 Vert(LL) -0.00 120 MT20 244/190 10 n/r TCDL 10.0 Lumber DOL 1.15 BC 0.03 Vert(CT) -0.00 10 n/r 120 BCLL 0.0 Rep Stress Incr YES WВ 0.04 0.00 10 Horz(CT) n/a n/a Code IRC2015/TPI2014 BCDL 10.0 FT = 20% Matrix-S Weight: 74 lb LUMBER-BRACING-

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

Max Horz 2=130(LC 11) (lb) -Max Uplift All uplift 100 lb or less at joint(s) 2, 12, 13, 14, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 2, 10, 12, 13, 14, 17, 16, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-0-0, Exterior(2) 2-0-0 to 7-0-0, Corner(3) 7-0-0 to 10-0-0, Exterior(2) 10-0-0 to 14-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 12, 13, 14, 17, 16, and 15. This connection is for uplift only and does not consider lateral forces.



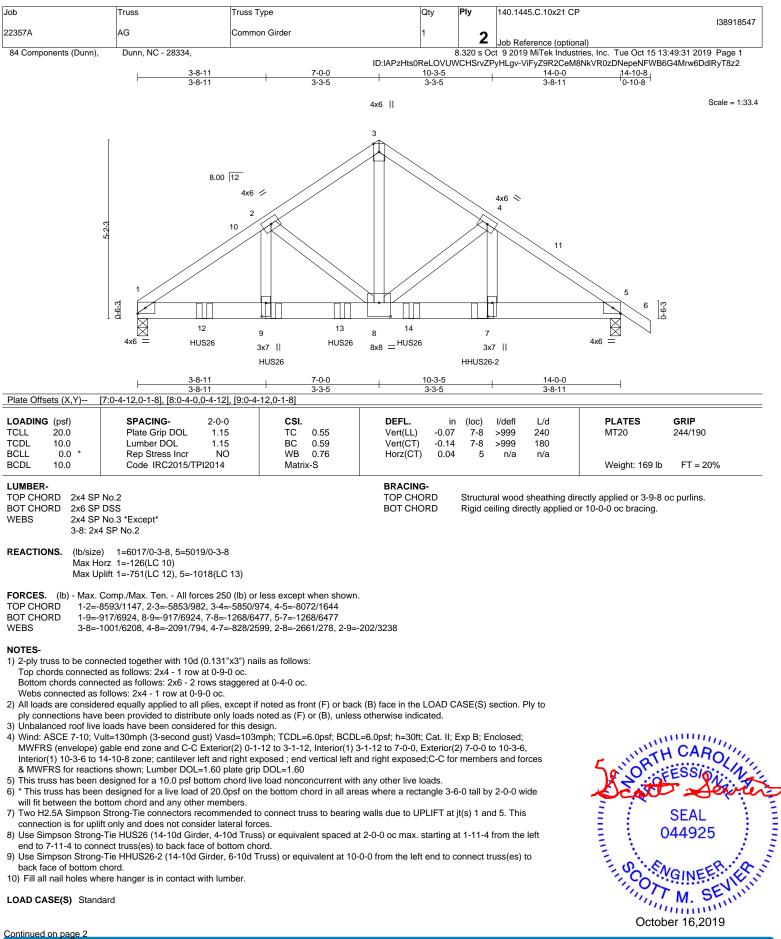
Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS ON TIPS REPRETED FACE PAGE MIT-14/3 refer to 100 Sec. Design valid for use only with MTRK exposure connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



REACTIONS. All bearings 14-0-0.



818 Soundside Road

Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	140.1445.C.10x21 CP
					I38918547
22357A	AG	Common Girder	1	2	
				<b>_</b>	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	3.320 s Oc	t 9 2019 MiTek Industries, Inc. Tue Oct 15 13:49:31 2019 Page 2

ID:IAPzHts0ReLOVUWCHSrvZPyHLgv-ViFyZ9R2CeM8NkVR0zDNepeNFWB6G4Mrw6DdlRyT8z2

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

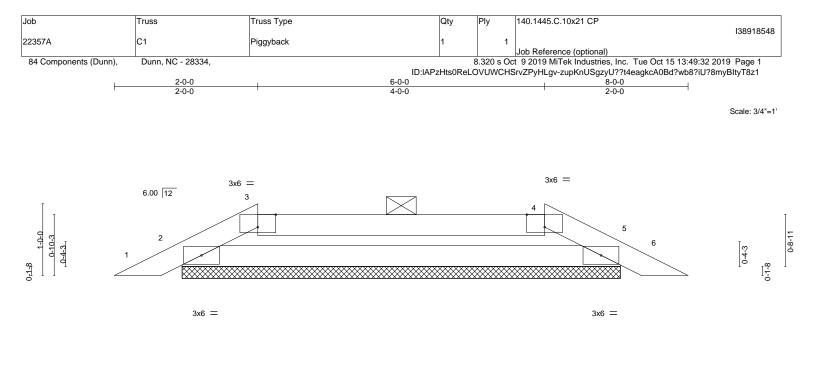
Uniform Loads (plf) Vert: 1-3=-60, 3-6=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-2520(B) 9=-1838(B) 12=-1838(B) 13=-1838(B) 14=-1844(B)

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	L		8-0-0					
			8-0-0					1
Plate Offsets (X,Y)	[3:0-3-0,Edge], [4:0-3-0,Edge]							
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loo	c) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.23	Vert(LL)	0.00	6 n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.35	Vert(CT)	0.01	6 n/r	90		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00	5 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-R					Weight: 22 lb	FT = 20%
UMBER-			BRACING-					
OP CHORD 2x4 S	SP No.2		TOP CHORD	) Stru	ctural wood	sheathing di	rectly applied or 6-0-0	) oc purlins, except
BOT CHORD 2x4 S	P No.2					(6-0-0 max.)		
			BOT CHORD			· /	or 10-0-0 oc bracing.	
REACTIONS. (Ib/siz	ze) 2=282/6-1-6, 5=282/6-1-6				· · · · · · · · · · · · · · · · ·		g	

- Max Horz 2=282/6-1-6, 5=282/6-1-6 Max Horz 2=13(LC 12) Max Uplift 2=-31(LC 9), 5=-31(LC 8)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-476/265, 3-4=-439/249, 4-5=-476/265 BOT CHORD 2-5=-207/439

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

- will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) n/a

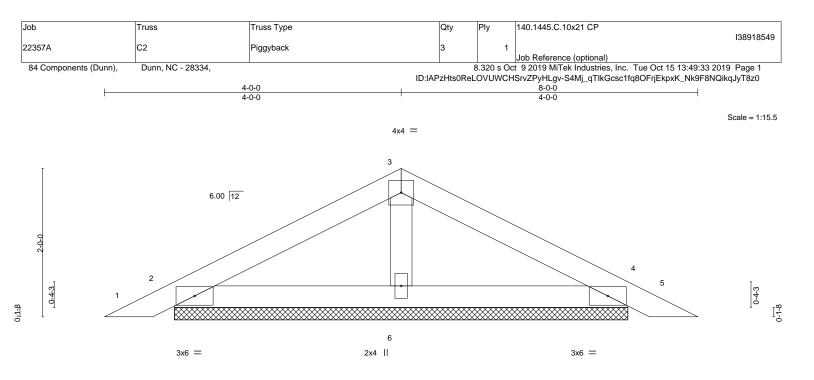
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSIVTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





			8-0-0 8-0-0						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.16	Vert(LL)	0.00	5	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT)	0.01	5	n/r	90		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 24 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 2=164/6-1-6, 4=164/6-1-6, 6=234/6-1-6 (lb/size) Max Horz 2=31(LC 12) Max Uplift 2=-44(LC 12), 4=-50(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

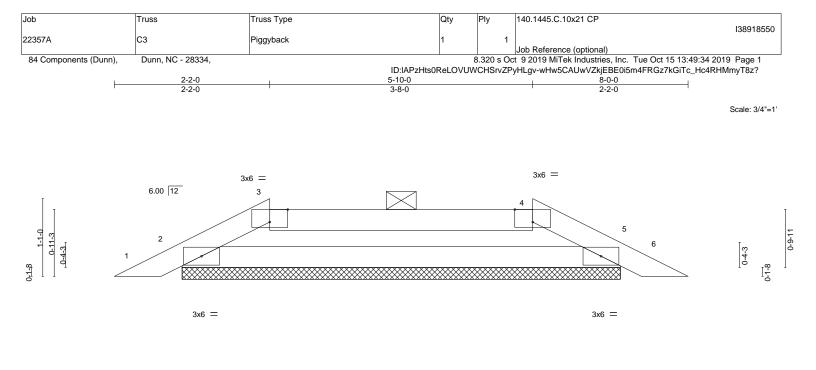
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) n/a
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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F			8-0-0						
Plate Offsets (X,Y)	[3:0-3-0,Edge], [4:0-3-0,Edge]		8-0-0						
	[3.0-3-0,Edge], [4.0-3-0,Edge]								
LOADING (psf) TCLL 20.0	SPACING-2-0-0Plate Grip DOL1.15	<b>CSI.</b> TC 0.19	DEFL. Vert(LL)	0.00	(loc) 6	l/defl n/r	L/d 120	PLATES MT20	<b>GRIP</b> 244/190
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.34 WB 0.00 Matrix-R	Vert(CT) Horz(CT)	0.01 0.00	6 5	n/r n/a	90 n/a	Weight: 22 lb	FT = 20%
Max H			BRACING- TOP CHOR BOT CHOR	2	2-0-0 0	c purlins	(6-0-0 max.):	rectly applied or 6-0-0 : 3-4. or 10-0-0 oc bracing.	oc purlins, except
TOP CHORD 2-3=-	Comp./Max. Ten All forces 250 (lb) or 441/249, 3-4=-401/234, 4-5=-441/249 187/401	less except when shown.							

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) n/a

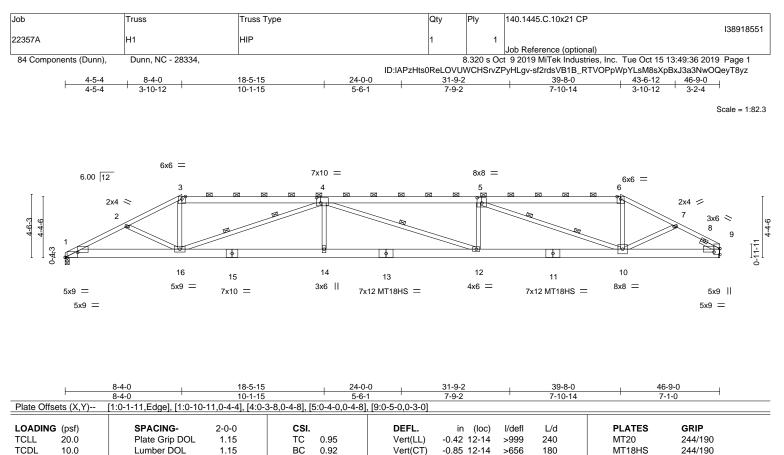
 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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A MITek Attiliate 818 Soundside Road Edenton, NC 27932



BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.86 Matrix-MS	Horz(CT) 0.1	6 9 n/a n/a	Weight: 326 lb FT = 20%
	No.2 *Except* : 2x4 SP No.2		BRACING- TOP CHORD	Structural wood sheathi	ing directly applied or 1-9-15 oc purlins,
BOT CHORD 2x8 SP				2-0-0 oc purlins (2-2-0 r	,
4-16,5-1	No.3 *Except* 10: 2x4 SP DSS x4 SP No.3 -H 1-6-0		BOT CHORD WEBS	Rigid ceiling directly ap 1 Row at midpt 2 Rows at 1/3 pts	plied or 2-2-0 oc bracing. 4-12 4-16. 5-10

REACTIONS. (lb/size) 1=1864/0-3-8, 9=1864/Mechanical Max Horz 1=79(LC 12) Max Uplift 1=-213(LC 9), 9=-222(LC 8)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-3804/826, 2-3=-3633/762, 3-4=-3209/715, 4-5=-5435/1159, 5-6=-2808/632, 6-7=-3163/664, 7-9=-3115/685

 BOT CHORD
 1-16=-701/3362, 14-16=-1055/5578, 12-14=-1055/5578, 10-12=-1024/5417, 9-10=-551/2673

 WEED
 0.400/1402, 144, 0/1402, 1440, 0044/577, 540, 0(1402, 540, 0040/570)

WEBS 3-16=-149/1197, 4-14=0/420, 4-16=-2614/527, 5-12=0/483, 5-10=-2849/576, 6-10=-102/963, 7-10=-62/319

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=222.

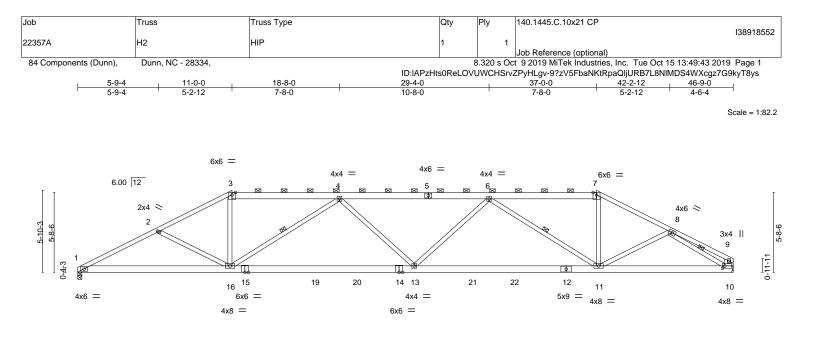
One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This
connection is for uplift only and does not consider lateral forces.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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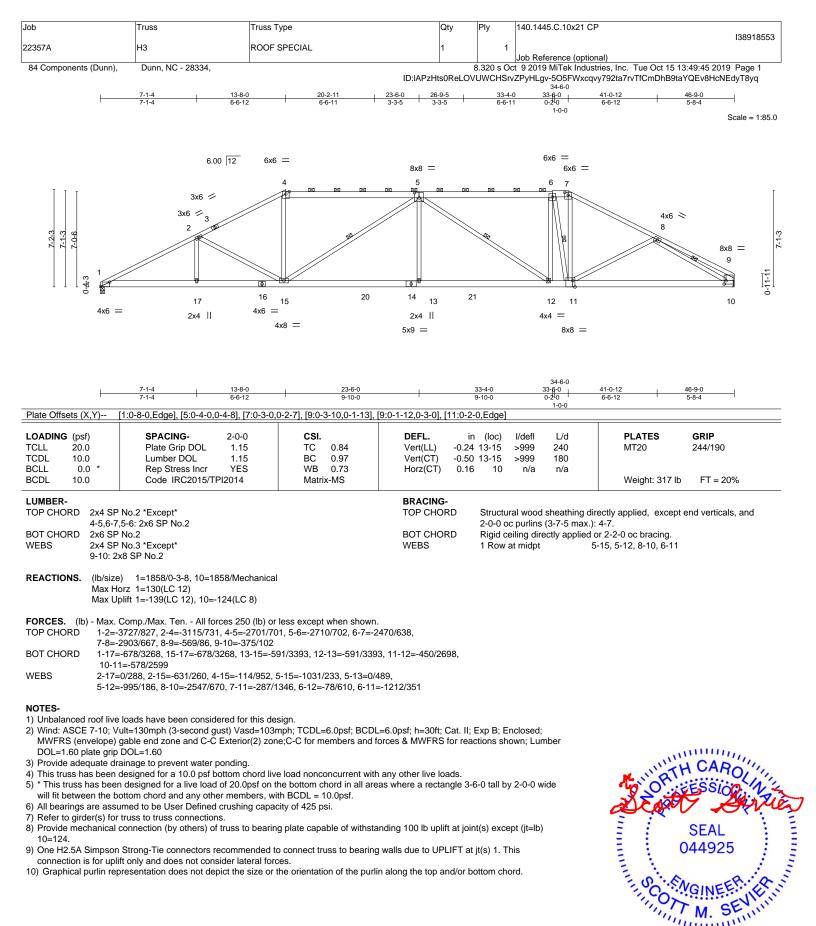
5-9			24-0-0		37-0-0		46-9-0	
5-9 Plate Offsets (X,Y)	-4 <u>5-2-12</u> [10:0-2-0.0-2-0]		13-0-0		13-0-0		9-9-0	
	[10.0-2-0,0-2-0]							
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TI	2-0-0 1.15 1.15 YES PI2014	<b>CSI.</b> TC 0.71 BC 0.88 WB 0.75 Matrix-MS	Vert(CT)	in (loc) l/defl -0.29 13-16 >999 -0.62 13-16 >902 0.14 10 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 293 lb	<b>GRIP</b> 244/190 FT = 20%
3-5,5-7	No.2 *Except* : 2x6 SP No.2			BRACING- TOP CHORD	except end verti	cals, and 2-0-0	ctly applied or 2-7-7 c ) oc purlins (2-8-12 ma	
14-15:	DSS *Except* 2x6 SP No.2			BOT CHORD WEBS	<ul> <li>Rigid ceiling dire</li> <li>1 Row at midpt</li> </ul>		8-2-13 oc bracing. 16, 6-11, 8-10	
	No.3 *Except* x8 SP No.2							
Max H	e) 1=1858/0-3-8, 10=18 orz 1=108(LC 12) plift 1=-165(LC 9), 10=-1							
TOP CHORD 1-2=-		3, 3-4=-2952/68	less except when shown. 9, 4-6=-4090/873, 6-7=-20					
BOT CHORD 1-16= WEBS 2-16=	-710/3281, 13-16=-778/2	4021, 11-13=-7 71, 4-16=-1390	50/3888, 10-11=-542/2522 '375, 4-13=0/313, 6-13=0/ 0, 8-10=-2610/604					
<ol> <li>Wind: ASCE 7-10; W MWFRS (envelope) DOL=1.60 plate grip</li> <li>Provide adequate di</li> <li>This truss has been</li> </ol>	gable end zone and C-C DOL=1.60 ainage to prevent water p designed for a 10.0 psf b	ust) Vasd=103r Exterior(2) zor conding. ottom chord liv	sign. nph; TCDL=6.0psf; BCDL e;C-C for members and fo e load nonconcurrent with he bottom chord in all area	any other live load	r reactions shown; Lur Is.		W. ATF	CARO

- will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=174.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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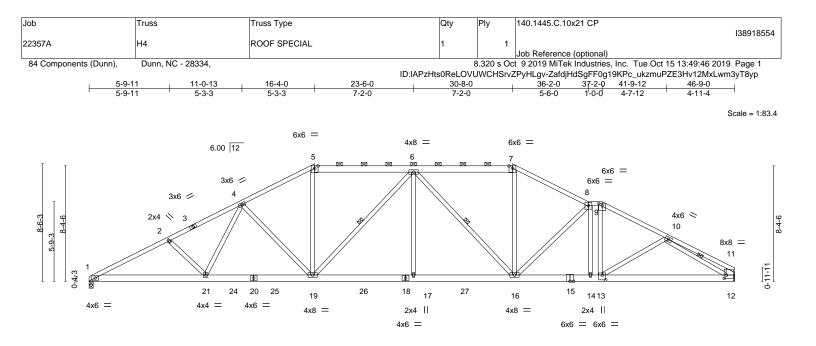




October 16,2019

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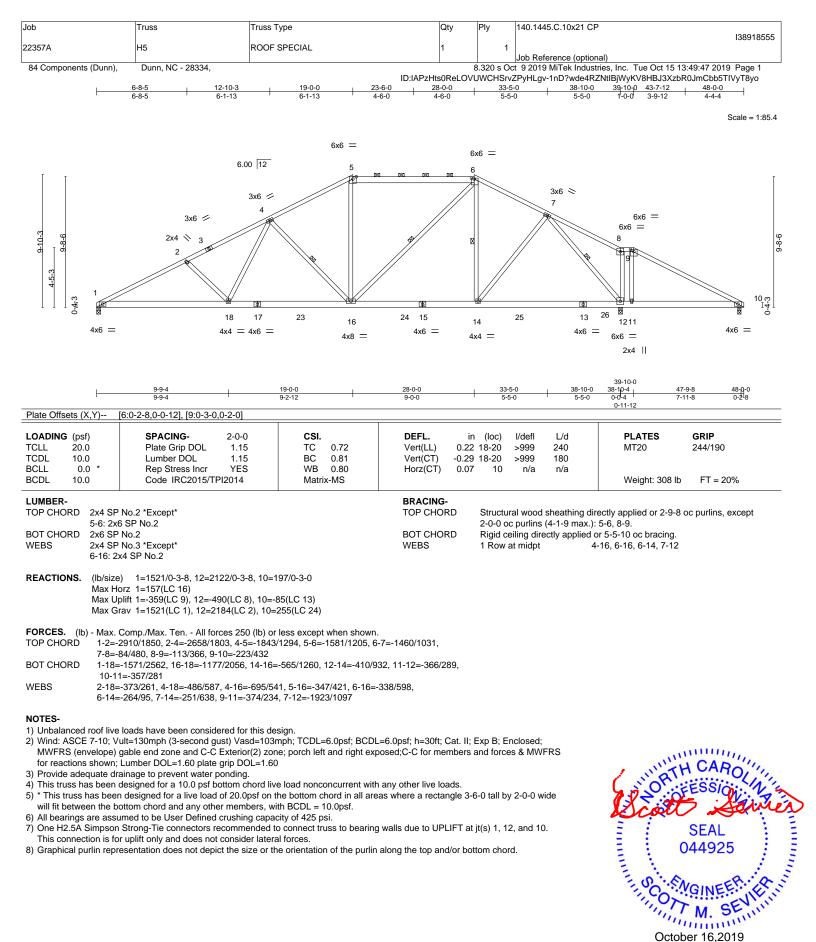
<b> </b>	8-5-4	<u> </u>	23-6-0		<u>30-8-0</u> 7-2-0	<u>36-2-0</u> 5-6-0	<u>37-2-0</u> 1-0-0	<u>46-9-0</u> 9-7-0	
Plate Offsets (X,Y)			1-12,0-3-0], [13:0-3-0,0-4	-4]	7-2-0	5-0-0	1-0-0	3-1-0	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 YES Pl2014	<b>CSI.</b> TC 0.75 BC 0.89 WB 0.64 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.22 17-19 -0.44 17-19 0.14 12	l/defl L/d >999 240 >999 180 n/a n/a		PLATES MT20 Weight: 329 lb	<b>GRIP</b> 244/190 FT = 20%
5-7: 2 BOT CHORD 2x6 S WEBS 2x4 S	SP No.2 *Except* 2x6 SP No.2 SP No.2 SP No.3 *Except* 2: 2x8 SP No.2	I		BRACING TOP CHC BOT CHC WEBS	RD Structu except RD Rigid c	end verticals, a	and 2-0-0 of pplied or 8-	y applied or 2-2-0 c purlins (3-4-0 ma 6-10 oc bracing. 6-16, 10-12	
Max	ze) 1=1858/0-3-8, 12=1 Horz 1=152(LC 12) Uplift 1=-165(LC 12), 12=-								
TOP CHORD 1-2: 7-8: BOT CHORD 1-2: 13: WEBS 2-2: 6-1:	x. Comp./Max. Ten All fo =-3729/865, 2-4=-3516/83 =-2768/720, 8-9=-2664/67 1=-729/3302, 19-21=-557// -14=-479/2664, 12-13=-57 1=-315/204, 4-21=-67/501 7=0/371, 6-16=-695/145, 7	1, 4-5=-2827/733 8, 9-10=-3011/71 2873, 17-19=-45 6/2556 , 4-19=-585/251, -16=-141/884, 8	5-6=-2466/693, 6-7=-24 1, 10-11=-436/76, 11-12: 1/2774, 16-17=-451/2774 5-19=-168/956, 6-19=-61	=-312/94 -, 14-16=-487/2  4/156,	592,				
<ol> <li>NOTES-</li> <li>Unbalanced roof lin</li> <li>Wind: ASCE 7-10; MWFRS (envelope DOL=1.60 plate gr</li> <li>Provide adequate 4</li> <li>This truss has bee</li> <li>* This truss has be will fit between the</li> <li>All bearings are as</li> <li>Refer to girder(s) fr</li> <li>Provide mechanica 12=160.</li> <li>One H2.5A Simpson</li> </ol>	3=-211/1041, 10-12=-2666 Vulta=130mph (3-second g a) gable end zone and C-C ip DOL=1.60 drainage to prevent water in designed for a 10.0 psf b sen designed for a live loac bottom chord and any oth sourdet to be User Defined or truss to truss connection al connection (by others) o on Strong-Tie connectors r plift only and does not con	ered for this des ust) Vasd=103m Exterior(2) zone ponding. bottom chord live of 20.0psf on th er members, with crushing capaci is. f truss to bearing ecommended to	ph; TCDL=6.0psf; BCDL= ;C-C for members and fo load nonconcurrent with e bottom chord in all area n BCDL = 10.0psf. ty of 425 psi. plate capable of withstar connect truss to bearing	any other live lo as where a rectand	for reactions should be adds. Ingle 3-6-0 tall b	own; Lumber y 2-0-0 wide ept (jt=lb)		A CONTRACT	SEAL

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



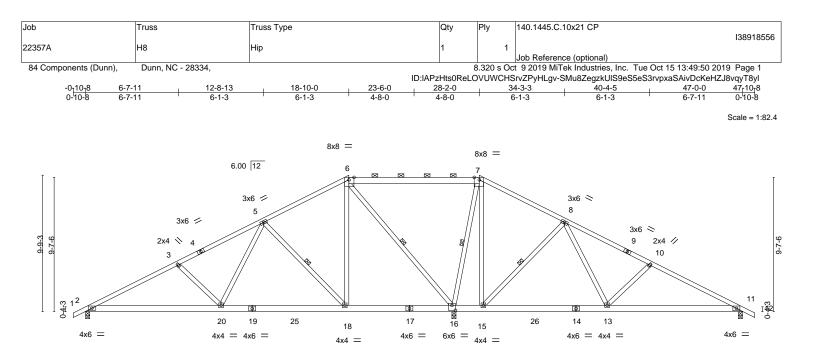
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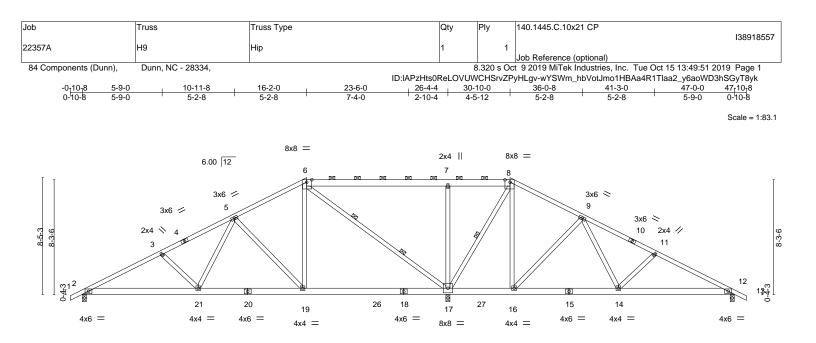
F	<u>9-8-4</u> 9-8-4		-10-0	<u>26-4-4</u> 7-6-4	28-2-0	<u>37-3-12</u> 9-1-12	47-0-0	
Plate Offsets (X,				7-0-4	1-9-12	9-1-12	9-0-4	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	* SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 YES 'PI2014	<b>CSI.</b> TC 0.75 BC 0.51 WB 1.00 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.14 20-22 -0.17 20-22 0.02 16	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 312 lb	<b>GRIP</b> 244/190 FT = 20%
6 BOT CHORD 2	x4 SP No.2 *Except* -7: 2x6 SP No.2 x6 SP No.2 x4 SP No.3			BRACING TOP CHO BOT CHO WEBS	RD Structu 2-0-0 c RD Rigid c	oc purlins (10-0-0 max. eiling directly applied o		oc purlins, except
n N N	Ib/size) 2=795/0-3-8, 16=25 Max Horz 2=163(LC 16) Max Uplift 2=-187(LC 9), 16=-1 Max Grav 2=866(LC 23), 16=2 Max. Comp./Max. Ten All fc	580(LC 9), 11=- 2584(LC 1), 11=	134(LC 13) 584(LC 24)		86/0-3-8			
TOP CHORD	2-3=-1321/716, 3-5=-1071/66 8-10=-440/214, 10-11=-690/2	9, 5-6=-256/156						
BOT CHORD	2-20=-534/1141, 18-20=-167, 11-13=-131/576		/293, 15-16=-632/667, 1	3-15=-285/276,				
WEBS	3-20=-369/258, 5-20=-488/59 7-16=-1283/811, 7-15=-434/5	,	, ,	,				
<ol> <li>Wind: ASCE 7 MWFRS (enver for reactions si</li> <li>Provide adequ</li> <li>This truss has</li> <li>* This truss has</li> <li>* This truss has</li> <li>will fit between</li> <li>All bearings ar</li> </ol>	of live loads have been consid- 10; Vult=130mph (3-second g lope) gable end zone and C-C hown; Lumber DOL=1.60 plate ate drainage to prevent water been designed for a 10.0 psf s been designed for a live load the bottom chord and any oth e assumed to be User Defined Strong-Tie connectors recom	gust) Vasd=103r C Exterior(2) zor e grip DOL=1.60 ponding. bottom chord liv d of 20.0psf on t her members, wi d crushing capad	mph; TCDL=6.0psf; BCD e; porch left and right ex e load nonconcurrent wit he bottom chord in all ar th BCDL = 10.0psf. city of 425 psi.	kposed;C-C for me th any other live lo eas where a recta	embers and force bads. Ingle 3-6-0 tall b	es & MWFRS y 2-0-0 wide	Stort	ESSION N

- 7) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16. This connection is for uplift only and does not consider lateral forces.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

> annum anna an CHILLING WINDOW SEAL 044925 S S Μ. "minin October 16,2019

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	8-4-4 16-2-0	21-3-2	26-4-4	30-10-0	38-7-12	47-0-0	)
	8-4-4 7-9-12	5-1-2	5-1-2	4-5-12	7-9-12	8-4-4	
Plate Offsets (X,Y)	[6:0-4-6,Edge], [8:0-4-6,Edge]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.45 WB 0.81 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.10 21-23 -0.15 17-19 0.02 12	l/defl L/d >999 240 >999 180 n/a n/a	<b>PLATES</b> MT20 Weight: 315 lb	<b>GRIP</b> 244/190 FT = 20%
						0	
6-8: 2: BOT CHORD 2x6 SI WEBS 2x4 SI	P No.2 *Except* x6 SP No.2 P No.2 P No.3 *Except* 2x4 SP No.2		BRACING TOP CHOI BOT CHOI WEBS	RD Structur 2-0-0 oc RD Rigid ce 1 Row a	ral wood sheathing direc c purlins (10-0-0 max.): siling directly applied or at midpt 8-1 at 1/3 pts 6-1	6-8. 6-0-0 oc bracing. 7	oc purlins, except
Max H Max I	te) 2=820/0-3-8, 17=2533/(0-3-8 + H Horz 2=-141(LC 13) Jplift 2=-200(LC 9), 17=-677(LC 9), 12 Grav 2=879(LC 23), 17=2533(LC 1), 1	=-131(LC 13)	req. 0-4-0), 12=5	12/0-3-8			
TOP CHORD 2-3= 8-9= BOT CHORD 2-21 WEBS 3-21 7-17	. Comp./Max. Ten All forces 250 (lb) -1409/799, 3-5=-1196/759, 5-6=-523/3 -159/466, 9-11=-544/317, 11-12=-754, =-619/1227, 19-21=-292/802, 17-19=- =-318/223, 5-21=-401/476, 5-19=-586 '=-573/264, 8-17=-1010/678, 8-16=-43 4=-310/218	41, 6-7=-425/875, 7-8=-424 355 I01/448, 16-17=-376/458, 1 455, 6-19=-526/665, 6-17=-	/872, 2-14=-227/640 1451/958,				
<ul> <li>2) Wind: ASCE 7-10; MWFRS (envelope for reactions shown</li> <li>3) Provide adequate c</li> <li>4) This truss has beer</li> <li>5) * This truss has beer</li> <li>will fit between the</li> <li>6) All bearings are ass</li> <li>7) H10A Simpson Street</li> </ul>	re loads have been considered for this Vult=130mph (3-second gust) Vasd=10 ) gable end zone and C-C Exterior(2) z n; Lumber DOL=1.60 plate grip DOL=1 trainage to prevent water ponding. In designed for a 10.0 psf bottom chord en designed for a live load of 20.0psf o bottom chord and any other members, sumed to be User Defined crushing cap ong-Tie connectors recommended to c	Big TCDL=6.0psf; BCDL one; porch left and right exp 60 live load nonconcurrent with n the bottom chord in all are with BCDL = 10.0psf. pacity of 425 psi.	oosed;C-C for me any other live lo as where a recta	mbers and force ads. ngle 3-6-0 tall by	9 2-0-0 wide	2 col	SEAL

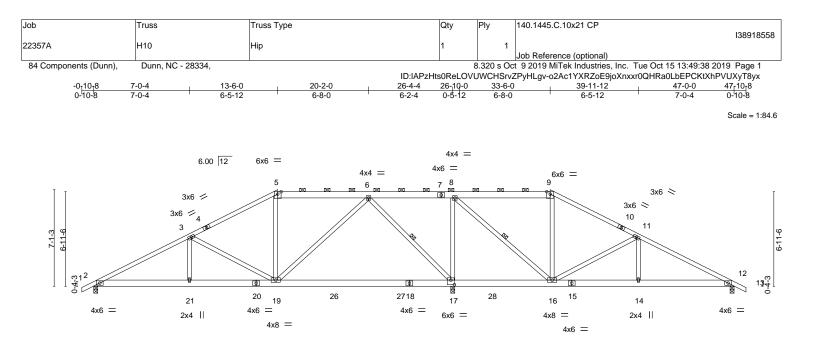
for uplift only and does not consider lateral forces.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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	7-0-4 7-0-4	13-6-0 6-5-12	20-2-0 6-8-0	26-4-4 6-2-4	33-6-0 7-1-12	<u>39-11-12</u> 6-5-12	47-0-0
Plate Offsets (X,Y)	[17:0-3-0,0-4-0]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/7	2-0-0 1.15 1.15 YES IPI2014	<b>CSI.</b> TC 0.52 BC 0.56 WB 0.94 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.20 17-19 >999 -0.36 17-19 >883 0.02 12 n/a	240 M 180 n/a	LATES GRIP T20 244/190 /eight: 302 lb FT = 20%
5-7,7- BOT CHORD 2x6 S	P No.2 *Except* 9: 2x6 SP No.2 P No.2 P No.3			BRACING- TOP CHOR BOT CHOR WEBS	D Structural wood except 2-0-0 oc purlins	sheathing directly app (6-0-0 max.): 5-9. ectly applied or 6-0-0 o 6-17, 8-16	0
Max l	te) 17=2391/(0-3-8 + H Horz 2=119(LC 16) Jplift 17=-735(LC 9), 12= Grav 17=2391(LC 1), 12=	-146(LC 13), 2=		12=592/0-3-8, 2=8		,	

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1489/893, 3-5=-890/577, 5-6=-718/560, 6-8=-398/826, 11-12=-889/491
- BOT CHORD 2-21=-683/1269, 19-21=-683/1269, 16-17=-826/702, 14-16=-330/733, 12-14=-330/733
- WEBS 3-19=-636/489, 6-19=-559/785, 6-17=-1304/790, 8-17=-1198/687, 8-16=-729/1142,
  - 9-16=-352/129, 11-16=-696/551, 11-14=-210/329

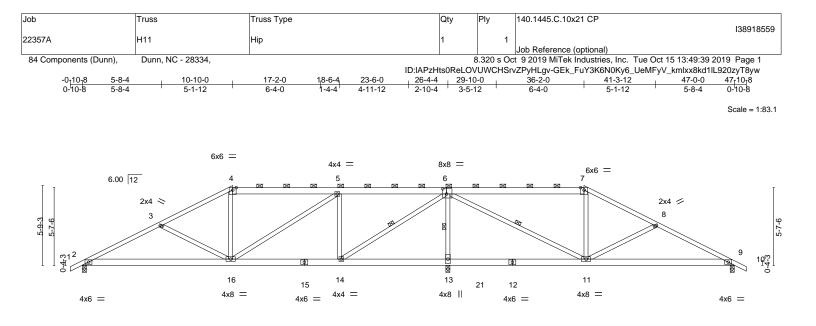
#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17. This connection is for uplift only and does not consider lateral forces.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 2. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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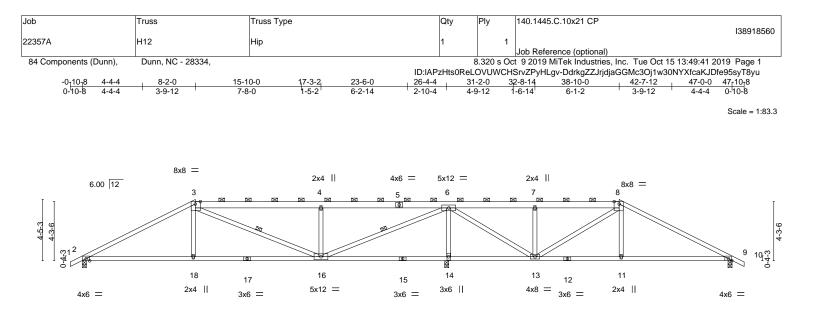
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	-8-4 10-10-0 -8-4 5-1-12	18-6-4 7-8-4	26-4-4 7-10-0	<u>36-2-0</u> 9-9-12	41-3-12	47-0-0 5-8-4
Plate Offsets (X,Y)	[6:0-3-4,0-4-8]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.58 BC 0.53 WB 0.61 Matrix-MS		in (loc) l/defl L/d 18 16-18 >999 240 24 16-18 >999 180 02 9 n/a n/a	PLATES MT20 Weight: 296	<b>GRIP</b> 244/190 lb FT = 20%
			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathir 2-0-0 oc purlins (6-0-0 m Rigid ceiling directly app 1 Row at midpt	nax.): 4-7.	0 oc purlins, except
Max H Max U	e) 2=962/0-3-8, 13=2210/0-3-8, 9=6 lorz 2=-96(LC 13) Jplift 2=-266(LC 9), 13=-762(LC 9), 9= Grav 2=978(LC 23), 13=2210(LC 1), 9	-180(LC 8)				
TOP CHORD         2-3=- 7-8=-           BOT CHORD         2-16=           WEBS         3-16=	Comp./Max. Ten All forces 250 (lb) -1599/979, 3-4=-1264/845, 4-5=-1069, -680/464, 8-9=-1015/598 =-789/1404, 14-16=-285/713, 13-14=- =-379/275, 4-16=-209/294, 5-16=-307, =-2013/1178, 6-11=-812/1252, 8-11=-	/790, 5-6=-713/531, 6-7=-55 578/497, 11-13=-619/521, 9 /450, 5-14=-711/383, 6-14≕	55/453, I-11=-453/882			
<ol> <li>Wind: ASCE 7-10; V MWFRS (envelope) for reactions shown;</li> <li>Provide adequate di 4) This truss has been 5) * This truss has bee will fit between the b</li> <li>All bearings are assi 7) One H2.5A Simpsor connection is for upl</li> <li>Two H2.5A Simpsor</li> </ol>	e loads have been considered for this Vult=130mph (3-second gust) Vasd=11 gable end zone and C-C Exterior(2) z ; Lumber DOL=1.60 plate grip DOL=1 rainage to prevent water ponding. I designed for a 10.0 psf bottom chord en designed for a live load of 20.0psf o bottom chord and any other members, sumed to be User Defined crushing caj n Strong-Tie connectors recommende lift only and does not consider lateral f or sentation does not depict the size of	D3mph; TCDL=6.0psf; BCDL cone; porch left and right exp .60 live load nonconcurrent with n the bottom chord in all are with BCDL = 10.0psf. pacity of 425 psi. d to connect truss to bearing orces. d to connect truss to bearing orces.	oosed;C-C for member of any other live loads. as where a rectangle g walls due to UPLIFT g walls due to UPLIFT	is and forces & MWFRS 3-6-0 tall by 2-0-0 wide at jt(s) 2 and 9. This at jt(s) 13. This	A CONTRACT OF CONTRACT.	H CAROL ESSION SEAL 044925



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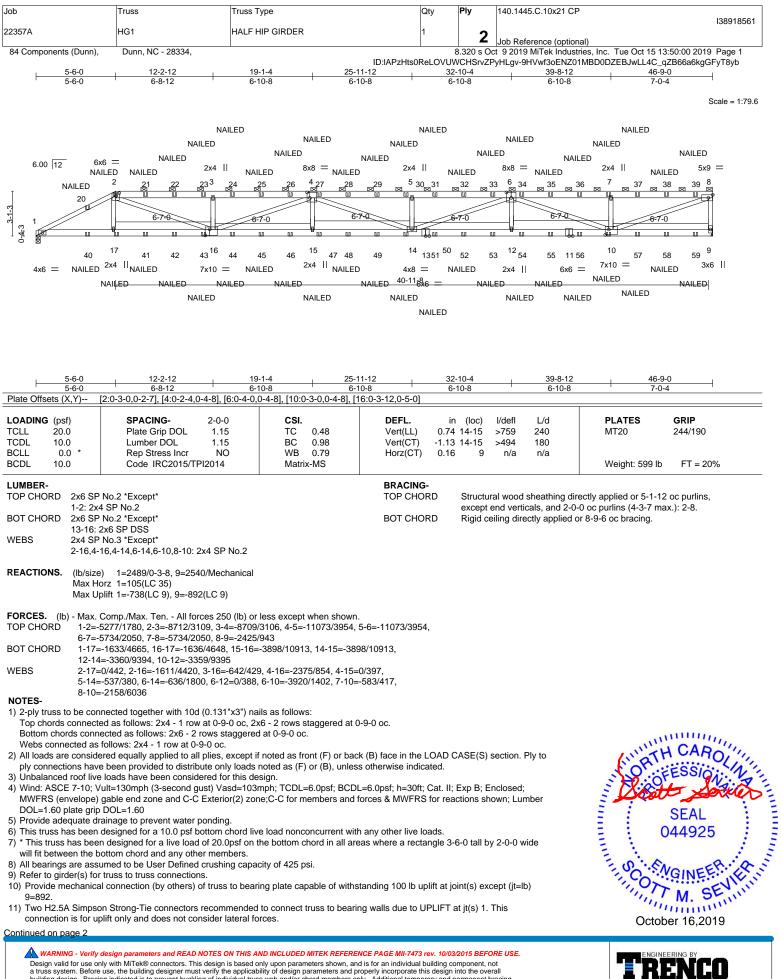
	8-2-0 17-3-2	26-4-4	32-8-14	38-10-0	47-0-0	
	8-2-0 9-1-2	9-1-2	6-4-10	6-1-2	8-2-0	
Plate Offsets (X,Y)	3:0-4-6,Edge], [8:0-4-6,Edge]					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI.         DEFL.           TC         0.83         Vert(L)           BC         0.84         Vert(C)           WB         0.86         Horz(C)           Matrix-MS         Horz (C)         Matrix-MS	T) -0.28 16-18 >999	240 180		<b>GRIP</b> 244/190 FT = 20%
					110.g.m. 2 10 10	
3-5,5-8 BOT CHORD 2x4 SP WEBS 2x4 SP	No.1 *Except* 2x6 SP No.2 No.2 No.3 *Except* (4 SP No.2	BRAC TOP C BOT C WEBS	HORD Structural woo 2-0-0 oc purlir	ns (5-10-6 max.): 3- directly applied or 6-		e purlins, except
Max H Max U	) 2=994/0-3-8, 14=2152/0-3-8, 9=719 prz 2=-74(LC 13) plift 2=-294(LC 9), 14=-812(LC 9), 9=-19 rav 2=999(LC 23), 14=2152(LC 1), 9=7	4(LC 8)				
TOP CHORD 2-3=-	Comp./Max. Ten All forces 250 (lb) or 1595/1005, 3-4=-1270/892, 4-6=-1265/8 374/614					
BOT CHORD 2-18=	-776/1348, 16-18=-768/1340, 14-16=-6 -431/791	5/476, 13-14=-665/476, 11-13=-423/7	83,			
	-227/371, 4-16=-611/276, 6-16=-1272/2 -209/329, 7-13=-342/159, 6-13=-750/12		,			
<ol> <li>Wind: ASCE 7-10; V MWFRS (envelope) for reactions shown;</li> <li>Provide adequate dr 4) This truss has been</li> <li>* This truss has been will fit between the b</li> <li>All bearings are assu</li> <li>One H2.5A Simpson connection is for upli</li> <li>Two H2.5A Simpson</li> </ol>	loads have been considered for this de ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Exterior(2) zor Lumber DOL=1.60 plate grip DOL=1.60 ainage to prevent water ponding. designed for a 10.0 psf bottom chord liv designed for a live load of 20.0psf on to totom chord and any other members. med to be User Defined crushing capac Strong-Tie connectors recommended to ft only and does not consider lateral forc ft only and does not consider lateral force	ph; TCDL=6.0psf; BCDL=6.0psf; h=3 e; porch left and right exposed;C-C for e load nonconcurrent with any other liv he bottom chord in all areas where a re- ity of 425 psi. connect truss to bearing walls due to es. connect truss to bearing walls due to	members and forces & M e loads. ectangle 3-6-0 tall by 2-0-0 UPLIFT at jt(s) 2 and 9. Th	WFRS ) wide	- •	CAROL SSIO

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

SEAL 044925 MGINEER M. SEVIER

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	140.1445.C.10x21 CP
					138918561
22357A	HG1	HALF HIP GIRDER	1	2	
				<b>∠</b>	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.320 s Oc	t 9 2019 MiTek Industries, Inc. Tue Oct 15 13:50:00 2019 Page 2
		ID:IAPzHts0	ReLOVUV	VCHSrvZP <sup>1</sup>	yHLgv-9HVwf3oENZ01MBD0DZEBJwLL4C_qZB66a6kgGFyT8yb

## NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

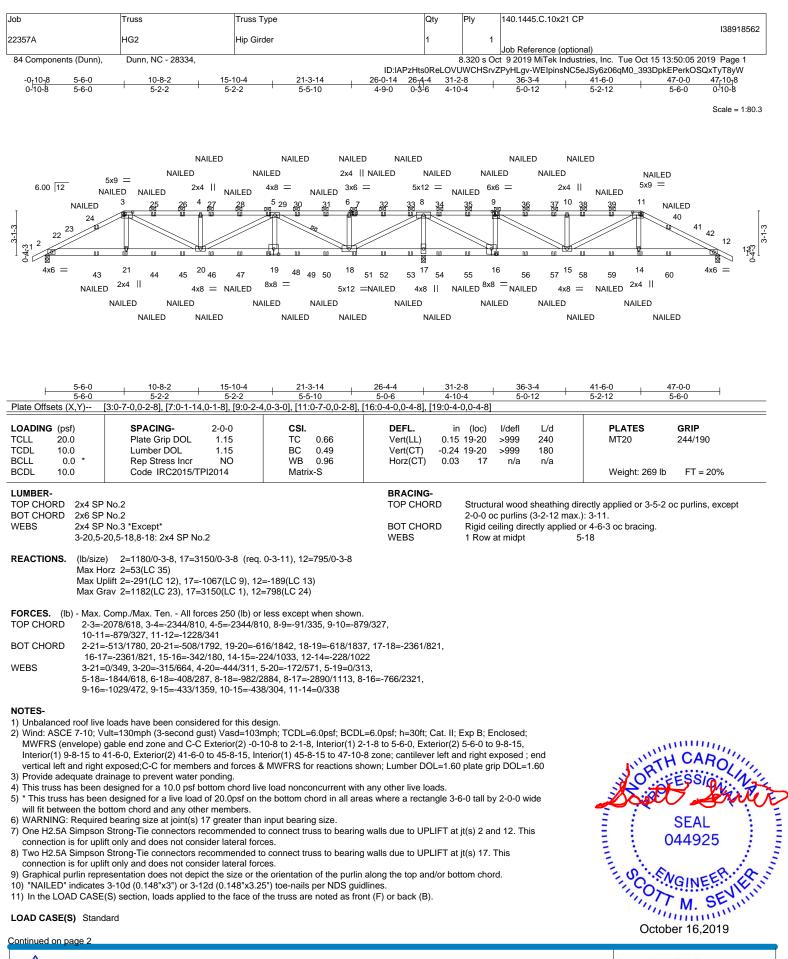
Uniform Loads (plf) Vert: 1-2=-60, 2-8=-60, 1-9=-20

Concentrated Loads (lb)

Vert: 2=-41(B) 17=-17(B) 7=-41(B) 10=-17(B) 20=-38(B) 21=-41(B) 22=-41(B) 23=-41(B) 24=-41(B) 25=-41(B) 26=-41(B) 27=-41(B) 28=-41(B) 29=-41(B) 30=-41(B) 31=-41(B) 32=-41(B) 32

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#### 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	140.1445.C.10x21 CP
					138918562
22357A	HG2	Hip Girder	1	1	
					Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			3.320 s Oc	t 9 2019 MiTek Industries, Inc. Tue Oct 15 13:50:05 2019 Page 2

ID:IAPzHts0ReLOVUWCHSrvZPyHLgv-WEIpinsNC5eJSy6z06qM0\_393DpkEPerkOSQxTyT8yW

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

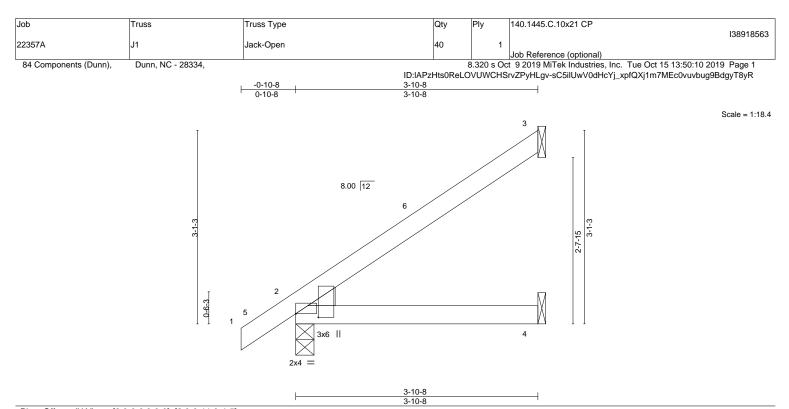
Uniform Loads (plf)

Vert: 1-3=-60, 3-11=-60, 11-13=-60, 2-12=-20 Concentrated Loads (lb)

Vert: 3=-41(F) 7=-41(F) 11=-41(F) 21=-17(F) 16=-17(F) 9=-41(F) 14=-17(F) 24=-38(F) 25=-41(F) 26=-41(F) 27=-41(F) 28=-41(F) 29=-41(F) 30=-41(F) 31=-41(F) 30=-41(F) 30= 32=-41(F) 33=-41(F) 34=-41(F) 35=-41(F) 36=-41(F) 37=-41(F) 38=-41(F) 39=-41(F) 40=-38(F) 43=-42(F) 44=-17(F) 46=-17(F) 46=-17

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LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.24	Vert(LL) -0.01 2-4 >999 240 MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.16	Vert(CT) -0.02 2-4 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00 3 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Weight: 15 lb FT = 20 <sup>4</sup>	FT = 20%

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEDGE BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-10-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

## Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=101/Mechanical, 2=216/0-3-8, 4=37/Mechanical Max Horz 2=119(LC 12) Max Uplift 3=-85(LC 12), 2=-13(LC 12) Max Grav 3=113(LC 19), 2=216(LC 1), 4=73(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

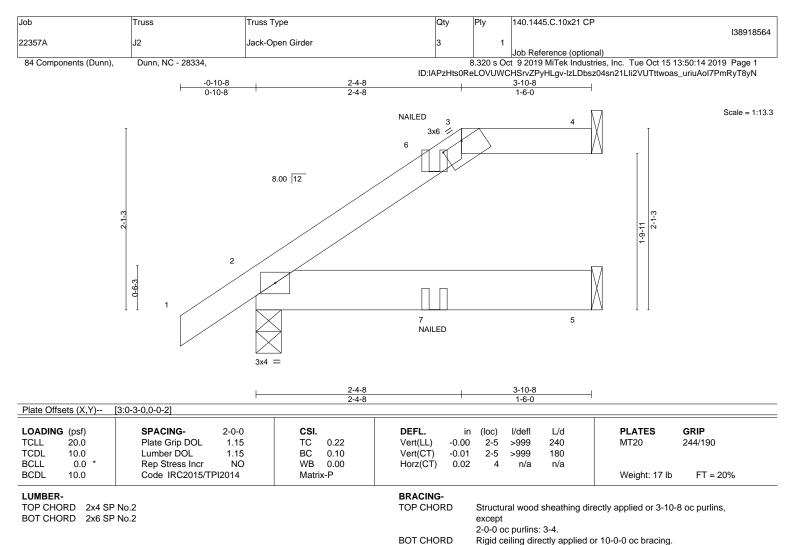
3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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REACTIONS. (Ib/size) 4=98/Mechanical, 2=237/0-3-8, 5=62/Mechanical Max Horz 2=82(LC 12) Max Uplift 4=-45(LC 9), 2=-37(LC 12)

Max Grav 4=98(LC 1), 2=237(LC 1), 5=88(LC 3)

#### NOTES-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
connection is for uplift only and does not consider lateral forces.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 2-5=-20 Concentrated Loads (lb)

Vert: 6=-25(F) 7=-17(F)

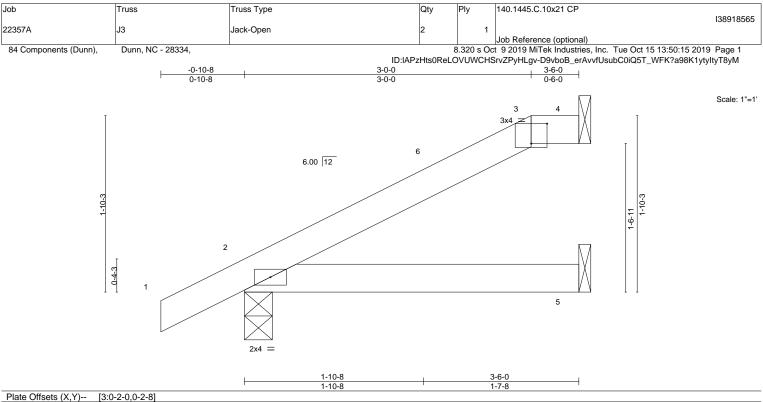


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TRENCO AMITEK ATIMATE 818 Soundside Road Edenton, NC 27932

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) -0.01 2-5 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.11	Vert(CT) -0.01 2-5 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 4 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 13 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 3-6-0 oc purlins, except 2-0-0 oc purlins: 3-4.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=85/Mechanical, 2=202/0-3-8, 5=37/Mechanical Max Horz 2=73(LC 12) Max Uplift 4=-39(LC 12), 2=-35(LC 12) Max Grav 4=85(LC 1), 2=202(LC 1), 5=61(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-0-0, Exterior(2) 3-0-0 to 3-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
connection is for uplift only and does not consider lateral forces.

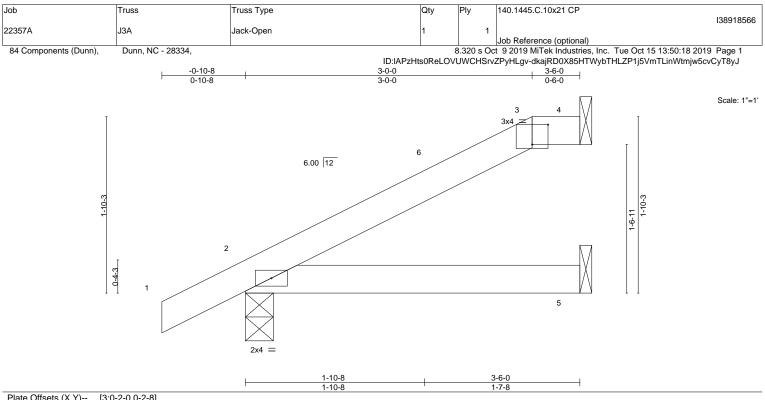
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) -0.01 2-5 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.11	Vert(CT) -0.01 2-5 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 4 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 13 lb FT = 20%

#### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 3-6-0 oc purlins, except 2-0-0 oc purlins: 3-4.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=85/Mechanical, 2=202/0-3-8, 5=37/Mechanical Max Horz 2=73(LC 12) Max Uplift 4=-39(LC 12), 2=-35(LC 12) Max Grav 4=85(LC 1), 2=202(LC 1), 5=61(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-0-0, Exterior(2) 3-0-0 to 3-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
connection is for uplift only and does not consider lateral forces.

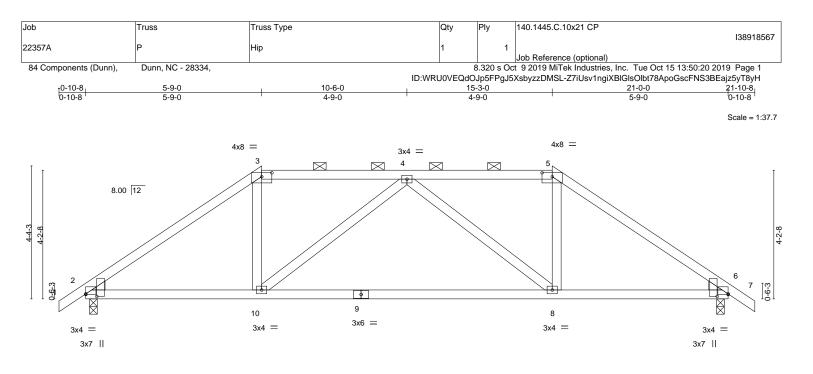
One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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0-1 <u>-8</u> 0-1-8	5-9-0 5-7-8		<u>15-3-0</u> 9-6-0				<u>20-10-8</u> 5-7-8	<u>21-0</u> -0 0-1-8
Plate Offsets (X,Y)	[2:0-0-15,0-4-5], [2:0-0-0,0-0-4], [3:0-4							
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014	CSI. TC 0.30 BC 0.78 WB 0.25 Matrix-MS	DEFL. ii Vert(LL) -0.21 Vert(CT) -0.42 Horz(CT) 0.03	2 8-10	l/defl >999 >595 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 100 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP WEDGE	BRACING- TOP CHORD	except 2-0-0 c	oc purlins	(5-10-7 max.)	ectly applied or 4-11-1 ): 3-5. or 10-0-0 oc bracing.	oc purlins,		

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

#### REACTIONS. (Ib/size) 2=893/0-3-0, 6=892/0-3-0 Max Horz 2=108(LC 11) Max Uplift 2=-74(LC 12), 6=-74(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1207/209, 3-4=-924/223, 4-5=-924/223, 5-6=-1207/209

- BOT CHORD 2-10=-87/935, 8-10=-152/1126, 6-8=-67/935
- WEBS 3-10=0/406, 4-10=-327/176, 4-8=-327/176, 5-8=0/406

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

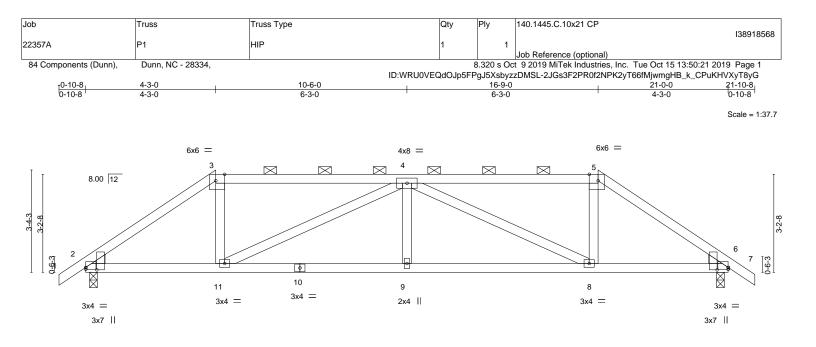
3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





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0-1 <u>-8</u> 0-1-8	4-3-0 4-1-8		<u>10-6-0</u> 6-3-0		16-9-0 6-3-0		20-10-8 4-1-8	<u>21-0</u> -0 0-1-8
Plate Offsets (X,Y)	[2:0-0-15,0-4-5], [2:0-0-0	0,0-0-12], [3:0-3-	8,Edge], [5:0-3-8,Edge], [6	5:Edge,0-0-12], [6:0-0-	15,0-4-5]		1.	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 YES PI2014	<b>CSI.</b> TC 0.54 BC 0.51 WB 0.62 Matrix-MS	DEFL.iiVert(LL)0.1'Vert(CT)-0.1'Horz(CT)0.04	1 9-11 >999 3 9-11 >999	L/d 240 180 n/a	PLATES MT20 Weight: 101 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S WEDGE	BRACING- TOP CHORD BOT CHORD	2-0-0 oc purlin	s (5-2-3 max.):	ectly applied or 5-2-7 3-5. or 5-4-9 oc bracing.	oc purlins, except			

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

#### REACTIONS. (Ib/size) 2=893/0-3-0, 6=893/0-3-0 Max Horz 2=-83(LC 10) Max Uplift 2=-264(LC 9), 6=-264(LC 8)

### FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1239/931, 3-4=-1009/832, 4-5=-1009/832, 5-6=-1239/931
- BOT CHORD 2-11=-692/989, 9-11=-1153/1607, 8-9=-1153/1607, 6-8=-695/989
- WEBS 3-11=-387/406, 4-11=-711/490, 4-9=-216/259, 4-8=-711/490, 5-8=-387/406

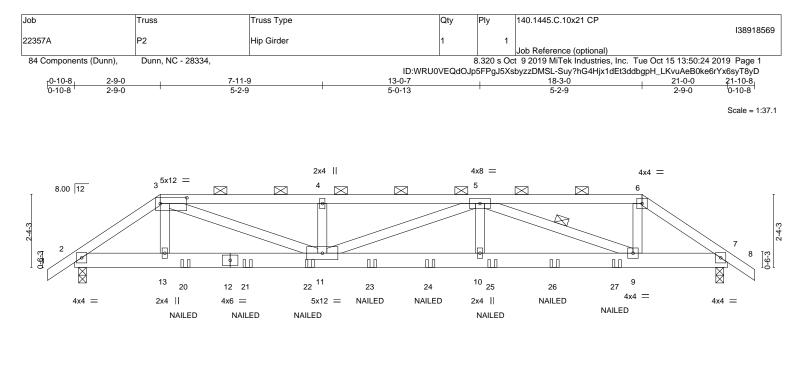
#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



0-1 <u>-8 2-9-</u> 0-1-8 2-7-			<u>13-0-7</u> 5-0-13	18-3-0	20-10	
	3:0-10-4,0-2-4]		0010	020		
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	<b>CSI.</b> TC 1.00 BC 0.97		n (loc) I/defl L/d 3 10-11 >999 240 5 10-11 >713 180	PLATES MT20	<b>GRIP</b> 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NO Code IRC2015/TPI2014	WB 0.87 Matrix-MS	Horz(CT) 0.06	6 7 n/a n/a	Weight: 118 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP	No.2		BRACING- TOP CHORD	Structural wood sheathing dire	<i>y</i> 11	oc purlins,
WEBS 2x4 SP	No.3		BOT CHORD WEBS	2-0-0 oc purlins (2-0-13 max.) Rigid ceiling directly applied o 1 Row at midpt 5-	r 5-5-8 oc bracing.	

- REACTIONS. (lb/size) 2=1429/0-3-0, 7=1429/0-3-0 Max Horz 2=-60(LC 33) Max Uplift 2=-465(LC 9), 7=-465(LC 8)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-2279/1101, 3-4=-3828/1839, 4-5=-3828/1839, 5-6=-1904/957, 6-7=-2246/1083
- BOT CHORD 2-13=-843/1855, 11-13=-856/1877, 10-11=-1781/3882, 9-10=-1781/3882, 7-9=-834/1828
- WEBS 3-13=-179/310, 3-11=-958/2108, 4-11=-313/147, 5-10=-217/465, 5-9=-2143/981, 6-9=-509/1027

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-6=-60, 6-8=-60, 14-17=-20 Concentrated Loads (lb)

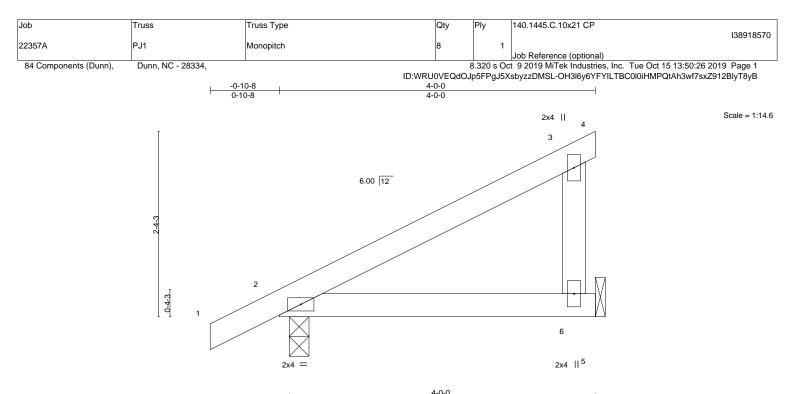
Vert: 20=-134(B) 21=-134(B) 22=-134(B) 23=-134(B) 24=-134(B) 25=-134(B) 26=-134(B) 27=-134(B)





Edenton, NC 27932

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		3-10-8										
	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	0.02	6-9	>999	240	MT20	244/190
FCDL	10.0	Lumber DOL	1.15	BC	0.15	Vert(CT)	-0.02	6-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MP						Weight: 17 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 6=154/Mechanical, 2=207/0-3-0 Max Horz 2=91(LC 12) Max Uplift 6=-48(LC 12), 2=-27(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.

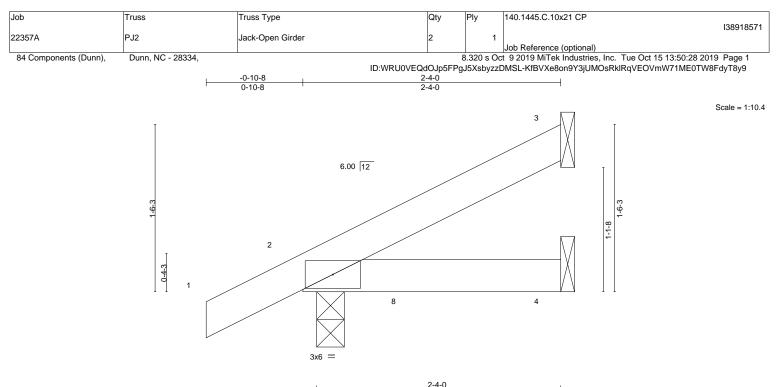
5) Refer to girder(s) for truss to truss connections.

- 6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6. This connection is for uplift only and does not consider lateral forces.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
  connection is for uplift only and does not consider lateral forces.



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	2-2-8											
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	-0.00	7	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	-0.00	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MP						Weight: 9 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

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LUMBER-
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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 3=59/Mechanical, 4=45/Mechanical, 2=172/0-3-0 Max Horz 2=58(LC 12) Max Uplift 3=-37(LC 12), 4=-19(LC 12), 2=-65(LC 12) Max Grav 3=60(LC 33), 4=52(LC 3), 2=172(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 39 lb down and 35 lb up at 2-3-4 on top chord, and 33 lb down and 48 lb up at 0-11-12, and 11 lb down and 18 lb up at 2-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 4-5=-20

Concentrated Loads (lb) Vert: 3=-1(B) 4=-8(B) 8=-33(B)

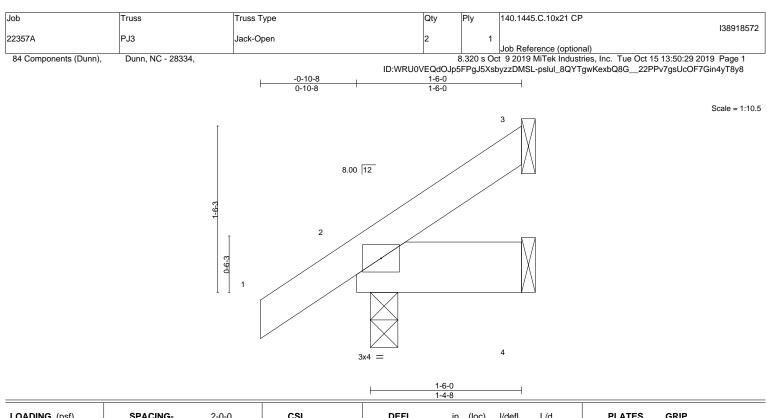


Structural wood sheathing directly applied or 2-4-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





RIP
44/190
FT = 20%
14

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2

REACTIONS. (lb/size) 3=27/Mechanical, 4=17/Mechanical, 2=127/0-3-0 Max Horz 2=58(LC 12) Max Uplift 3=-20(LC 12), 4=-9(LC 9), 2=-22(LC 12) Max Grav 3=29(LC 19), 4=28(LC 3), 2=127(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.

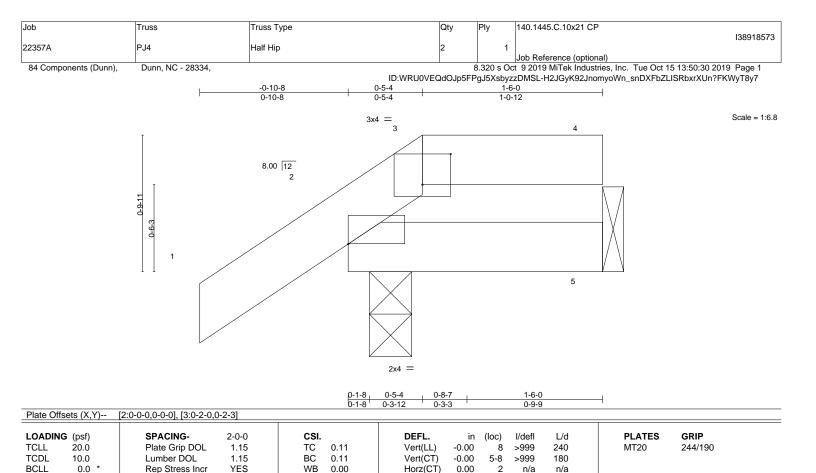


Structural wood sheathing directly applied or 1-6-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-	
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BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

10.0

BRACING-TOP CHORD 2

n/a

Horz(CT)

Structural wood sheathing directly applied or 1-6-0 oc purlins, except 2-0-0 oc purlins: 3-4. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

Weight: 6 lb

n/a

REACTIONS. 2=127/0-3-0, 5=45/Mechanical (lb/size) Max Horz 2=32(LC 12) Max Uplift 2=-27(LC 9), 5=-33(LC 9) Max Grav 2=127(LC 1), 5=53(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code IRC2015/TPI2014

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MP

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) Refer to girder(s) for truss to truss connections.

8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

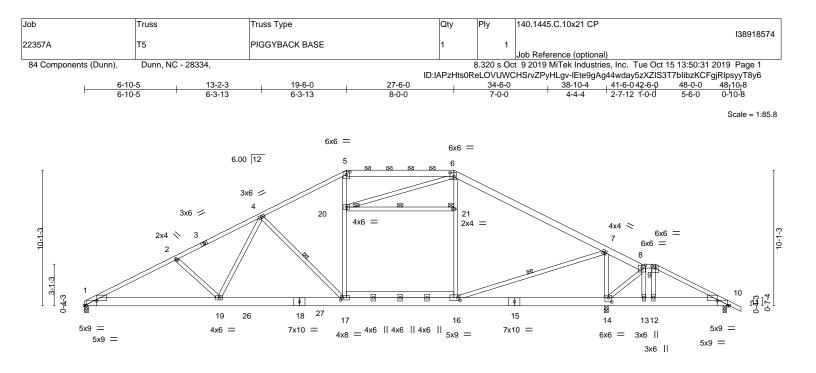
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



FT = 20%

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 100322010 SECORE 052. Design valid for use only with MITEK @ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





		10-0-4	1	9-6-0		27-6-0	_	34-6-	0	38-10-4	41-6-042-6-0	47-9-8 48-0-0
	I	10-0-4	9.	-5-12	1	8-0-0	I	7-0-0	)	4-4-4	2-7-12 1-0-0	5-3-8 0-2-8
ate Offsets (	X,Y)	[1:0-10-11,0-4-4], [1:0-1-	11,Edge], [5:0-	-3-0,0-2-7], [6	6:0-3-8,0-3-4	4], [8:0-3-0,0-2-8], [	9:0-3-0	,0-2-0],	[10:0-10-	11,0-4-4], [10	:0-1-11,Edge], [14:0	)-3-0,0-3-12],
		[16:0-2-0,0-2-8], [17:0-1-	12,0-2-0]					_	-			
.OADING (ps	sf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.	.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	0.34	17-19	>999	240	MT20	244/190
CDL 10.	.0	Lumber DOL	1.15	BC	0.86	Vert(CT)	-0.56	17-19	>836	180		
BCLL 0.	.0 *	Rep Stress Incr	YES	WB	0.82	Horz(CT)	0.06	10	n/a	n/a		
3CDL 10.	.0	Code IRC2015/TI	PI2014	Matri	k-MS						Weight: 381	lb FT = 20%
LUMBER-	5-6,6-8	PNo.2 *Except* 3: 2x6 SP No.2				BRACING TOP CHOP		except		Ū	ectly applied or 2-1	1-4 oc purlins,
BOT CHORD		PNo.2 *Except* 2x6 SP No.2				BOT CHO	RD			(5-0-6 max.): ectly applied c	5-6, 8-9. or 10-0-0 oc bracing	, Except:
WEBS	2x4 SP	9 No.3							oc bracing 5 oc brac	g: 1-19 bing: 17-19.		
						WEBS JOINTS			at midpt e at Jt(s)		-17, 7-16, 20-21	
EACTIONS	(lb/oitre	a) 1-1612/0 2 8 14-16	76/0 2 9 10	602/0.2.0		001110		i Diac	0 01 01(3)	. 20, 21		

27.6.0

24 6 0

29-10-/

11 6 0 12 6 0

19.0.0

REACTIONS. (lb/size) 1=1613/0-3-8, 14=1576/0-3-8, 10=692/0-3-0 Max Horz 1=-178(LC 13) Max Uplift 1=-392(LC 9), 14=-461(LC 8), 10=-173(LC 9) Max Grav 1=1613(LC 1), 14=1661(LC 24), 10=692(LC 1)

10.0.4

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3141/1998, 2-4=-2881/1956, 4-5=-1926/1356, 5-6=-1649/1255, 6-7=-1961/1286, 7-8=-1019/529, 8-9=-1027/595, 9-10=-1144/619

BOT CHORD 1-19=-1673/2748, 17-19=-1255/2205, 16-17=-811/1622, 14-16=-377/944,

 13-14=-454/1040, 12-13=-456/1027, 10-12=-450/1006

 WEBS
 2-19=-345/240, 4-19=-565/724, 4-17=-844/631, 17-20=-463/539, 5-20=-444/516, 16-21=-238/364, 6-21=-240/366, 7-16=-449/903, 7-14=-1451/876, 9-12=-130/386

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

10.6.0

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

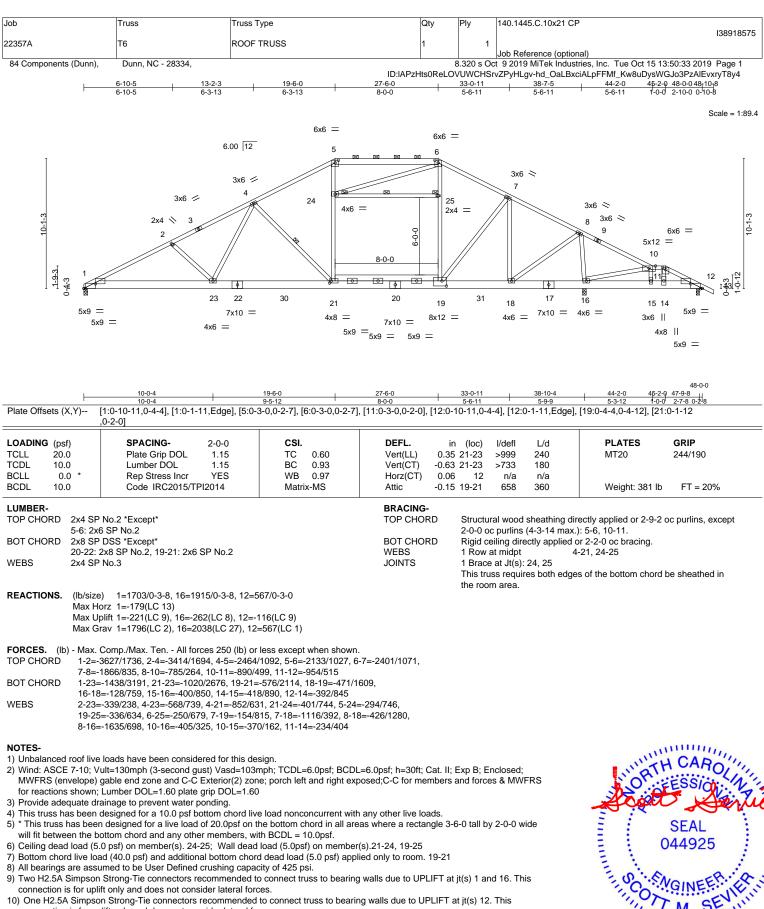
7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 14, and 10. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 19-21
- 8) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 9) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 16. This connection is for uplift only and does not consider lateral forces.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

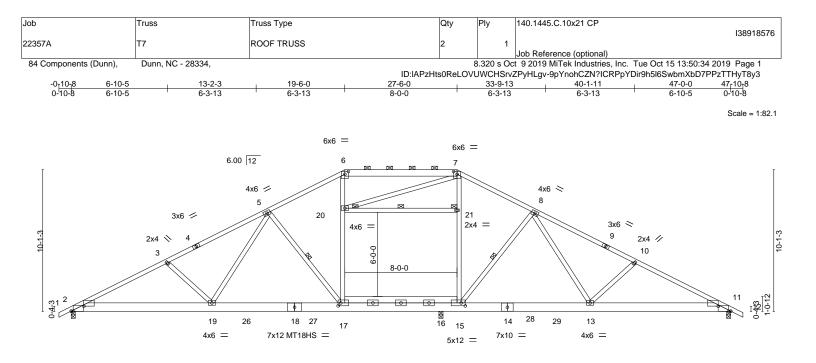
🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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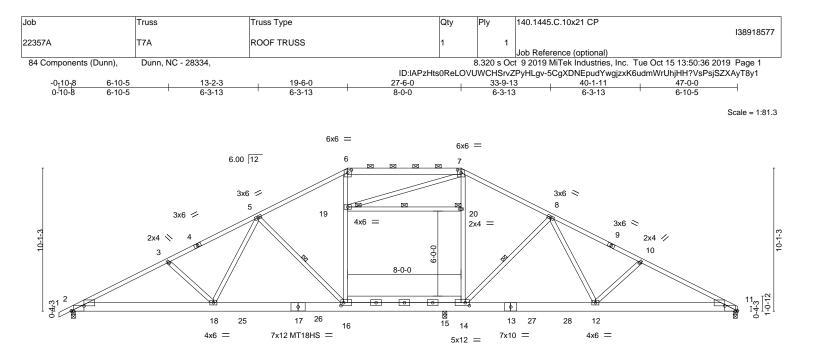


10-0-4	19-6-0 9-5-12	26-4-4 6-10-4	27-6-0	:	<u>36-11-12</u> 9-5-12		47-0-0		
	9-5-12 11,Edge], [6:0-3-0,0-2-7], [7:0-3-0,0-		=	:0-1-11,E		5:0-3-8,0-2-8],			
LOADING         (psf)         SPACING-           TCLL         20.0         Plate Grip DOL           TCDL         10.0         Lumber DOL           BCLL         0.0 *         Rep Stress Incr           BCDL         10.0         Code IRC2015/TF	2-0-0 <b>CSI.</b> 1.15 TC 0.67 1.15 BC 0.98 YES WB 0.68 Pl2014 Matrix-MS		(LL) -0.37 (CT) -0.71 2(CT) 0.06	n (loc) 7 17-19 17-19 17-19 11 10-17	l/defl >853 >440 n/a 538	L/d 240 180 n/a 360	PLATES MT20 MT18HS Weight: 362 lb	<b>GRIP</b> 244/190 244/190 FT = 20%	
LUMBER- TOP CHORD 2x4 SP No.2 *Except* 6-7: 2x6 SP No.2 BOT CHORD 2x8 SP No.2 *Except* 14-18: 2x8 SP DSS, 15-17: 2x6 WEBS 2x4 SP No.3 *Except* 6-17,7-15,20-21: 2x4 SP No.2 REACTIONS. (lb/size) 2=1496/0-3-8, 11=13 Max Horz 2=-170(LC 13) Max Uplift 2=-169(LC 12), 11=- Max Grav 2=1583(LC 26), 11=	337/0-3-8, 16=1330/0-3-8 39(LC 13), 16=-99(LC 13)	TOP		2-0-0 c Rigid c 1 Row 1 Brac This tru	oc purlins ceiling dir at midpt ce at Jt(s)	(4-11-11 max ectly applied o 5 : 20, 21	rectly applied or 2-8-6 ( ,.): 6-7. or 2-2-0 oc bracing. -17, 8-15, 20-21 es of the bottom chord l		
FORCES.         (lb)         Max. Comp./Max. Ten All for           TOP CHORD         2-3=-3062/463, 3-5=-2837/418           8-10=-2181/354, 10-11=-2402           BOT CHORD         2-19=-338/2689, 17-19=-133/2           11-13=-251/2117           WEBS         3-19=-380/262, 5-19=-127/866	cces 250 (lb) or less except when shi 3, 5-6=-1794/275, 6-7=-1573/293, 7- /400 2038, 16-17=0/1542, 15-16=0/1570, 3, 5-17=-863/333, 17-20=-64/508, 6- 5, 8-15=-638/297, 8-13=-106/505, 10 ered for this design. ust) Vasd=103mph; TCDL=6.0psf; B Exterior(2) zone;C-C for members a yonding. ndicated. cated. ottom chord live load nonconcurrent of 20.0psf on the bottom chord in al er members, with BCDL = 10.0psf. -21; Wall dead load (5.0psf) on al bottom chord dead load (5.0 psf) d crushing capacity of 425 psi.	8=-1671/266, 13-15=-26/168 20=0/489, 0-13=-404/265 CDL=6.0psf; h- and forces & MV with any other I areas where a nber(s).17-20, 1 applied only to	=30ft; Cat. II; VFRS for rea live loads. rectangle 3- 5-21 room. 16-17,	6-0 tall b 15-16	nown; Lu	mber	A LING RTH	SEAL	A States Manuality

INFERING

818 Soundside Road Edenton, NC 27932

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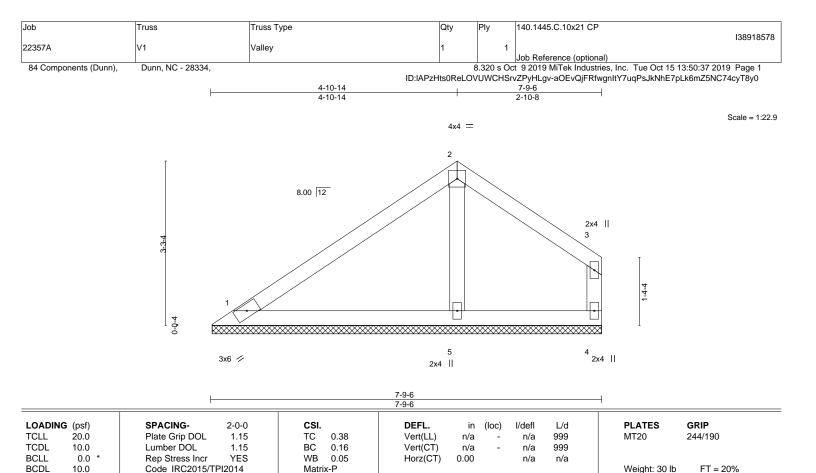


<b> </b>	<u> </u>	<u>19-6-0</u> 9-5-12	<u>26-4-4</u> 27-6			<u>47-0-0</u> 10-0-4	
Plate Offsets (X,Y)							
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.98 WB 0.67 Matrix-MS	Vert(CT) · Horz(CT)	in (loc) l/defl 0.36 16-18 >866 0.71 16-18 >444 0.06 11 n/a 0.31 15-16 538	L/d 240 180 n/a 360	PLATES MT20 MT18HS Weight: 359 lb	<b>GRIP</b> 244/190 244/190 FT = 20%
6-7 BOT CHORD 2x8 14- WEBS 2x4	4 SP No.2 *Except* 7: 2x6 SP No.2 3 SP No.2 *Except* -16: 2x6 SP No.2, 13-17: 2x8 SP DS 4 SP No.3 *Except* 6,7-14: 2x4 SP No.2		BRACING- TOP CHORE BOT CHORE WEBS JOINTS	2-0-0 oc purlins Rigid ceiling di 1 Row at midpt 1 Brace at Jt(s)	s (5-0-7 max.): rectly applied of t 5- ): 19, 20 ires both edges	ectly applied or 3-1-1 c 6-7. r 2-2-0 oc bracing. ·16, 8-14, 19-20 s of the bottom chord t	
Ma Ma	/size) 2=1500/0-3-8, 11=1279/0-3- ax Horz 2=178(LC 12) ax Uplift 2=-170(LC 12), 11=-15(LC 1 ax Grav 2=1580(LC 26), 11=1299(LC	8), 15=-103(LC 13)					
TOP CHORD 2 8	Ax. Comp./Max. Ten All forces 250 -3=-3047/453, 3-5=-2833/412, 5-6=-7 -10=-2184/354, 10-11=-2407/396 -18=-338/2671, 16-18=-164/2118, 15	793/264, 6-7=-1570/294, 7-8=	-1671/256,				
1 WEBS 3	1-12=-263/2107 -18=-340/235, 5-18=-106/816, 5-16= 4-20=-215/337, 7-20=-110/382, 8-14	894/333, 16-19=-44/471, 6-19	=0/466,				
<ol> <li>Wind: ASCE 7-1 MWFRS (envelo DOL=1.60 plate</li> <li>Provide adequat</li> <li>Provide adequat</li> <li>All plates are MI</li> <li>All plates are MI</li> <li>All plates are Sx</li> <li>This truss has b</li> <li>This truss has b</li> <li>This truss has b</li> <li>This truss has built for the second secon</li></ol>	f live loads have been considered for 0; Vult=130mph (3-second gust) Vas ope) gable end zone and C-C Exterior grip DOL=1.60 te drainage to prevent water ponding T20 plates unless otherwise indicated. 9 MT20 unless otherwise indicated. een designed for a 10.0 psf bottom cl been designed for a 10.0 psf bottom cl been designed for a live load of 20.0 he bottom chord and any other memit d (5.0 psf) on member(s). 19-20; Wa e load (40.0 psf) and additional botto e assumed to be User Defined crushi npson Strong-Tie connectors recomm n is for uplift only and does not consii n representation does not depict the SHOWN IS DESIGNED AS UNINHA	d=103mph; TCDL=6.0psf; BCI (2) zone;C-C for members and sf on the bottom chord in all a ers, with BCDL = 10.0psf. I dead load (5.0psf) on member n chord dead load (5.0 psf) ap ig capacity of 425 psi. ended to connect truss to beal er lateral forces. ize or the orientation of the pu	t forces & MWFRS fo ith any other live load reas where a rectang er(s).16-19, 14-20 plied only to room. 15 ring walls due to UPL	r reactions shown; Lu s. le 3-6-0 tall by 2-0-0 5-16, 14-15 IFT at jt(s) 2, 11, and	imber wide	SCOT	CARO ESSION SEAL 044925 GINEER M. SEVIET

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ENGINEERING BY ERENCO A MITEK ATHILLE 818 Soundside Road Edenton, NC 27932

October 16,2019



BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 7-9-6 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing

except end verticals.

#### NOTES-

LUMBER-

WEBS

OTHERS

TOP CHORD

BOT CHORD

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

2x4 SP No.3

Max Horz 1=74(LC 12)

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

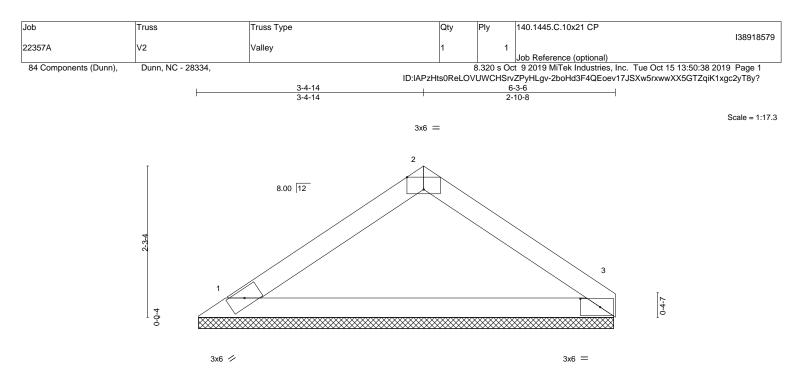
(lb/size) 1=169/7-9-0, 4=95/7-9-0, 5=308/7-9-0

Max Uplift 1=-26(LC 12), 4=-52(LC 13), 5=-14(LC 12) Max Grav 1=169(LC 1), 4=102(LC 20), 5=310(LC 19) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.



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			6-3-1 6-3-1			<u>6-3-</u> 6 0-0-5	
Plate Offsets (X,Y)	[2:0-3-0,Edge], [3:0-3-9,0-1-8]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	<b>CSI.</b> TC 0.16 BC 0.43 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 20 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP	No.2		BRACING- TOP CHOF		Iral wood sheathing o	directly applied or 6-0-0	oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 1=231/6-2-11, 3=231/6-2-11 Max Horz 1=48(LC 9) Max Uplift 1=-25(LC 12), 3=-24(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

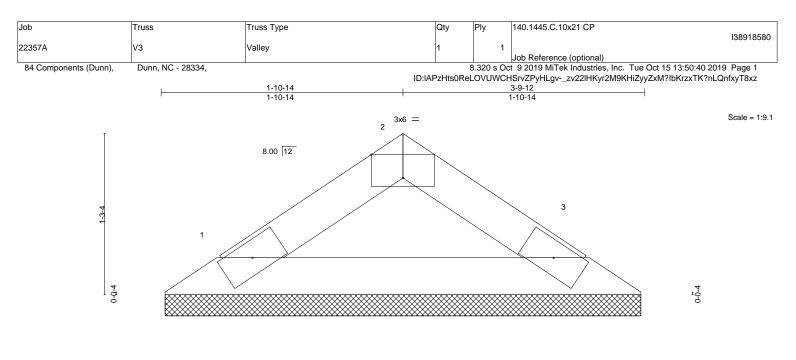
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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3x6 🥢

3x6 📎

Structural wood sheathing directly applied or 3-9-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

0-0 <u>16</u> 0-0-6				3-9-12 3-9-6								
Plate Offsets (X,Y)	[2:0-3-0,Edge]	I										
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP				
TCLL 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL)	n/a -	n/a	999	MT20	244/190				
TCDL 10.0	Lumber DOL 1.15	BC 0.16	Vert(CT)	n/a -	n/a	999						
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00 3	n/a	n/a						
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 11 lb	FT = 20%				

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (Ib/size) 1=114/3-9-0, 3=114/3-9-0 Max Horz 1=-23(LC 8) Max Uplift 1=-12(LC 12), 3=-12(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

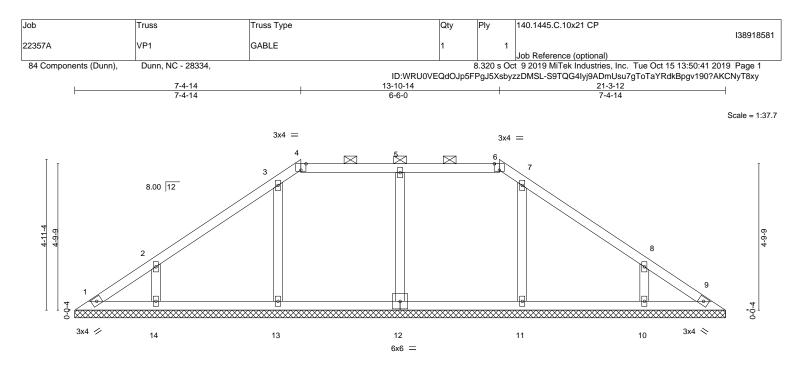
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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<b> </b>			<u>21-3-12</u> 21-3-12		
Plate Offsets (X,Y)	[4:0-2-0,Edge], [6:0-2-0,Edge]		21-0-12		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.17 BC 0.19 WB 0.10 Matrix-S	DEFL. i Vert(LL) n/ Vert(CT) n/ Horz(CT) 0.0	a - n/a 999	PLATES         GRIP           MT20         244/190           Weight: 88 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP OTHERS 2x4 SP	No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di 2-0-0 oc purlins (6-0-0 max.) Rigid ceiling directly applied	

REACTIONS. All bearings 21-3-12.

Max Horz 1=112(LC 9) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 12, 13, 11 except 14=-124(LC 12), 10=-126(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 9 except 12=412(LC 25), 13=364(LC 19), 14=298(LC 19), 11=352(LC 20), 10=300(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 7)
- will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 9) n/a

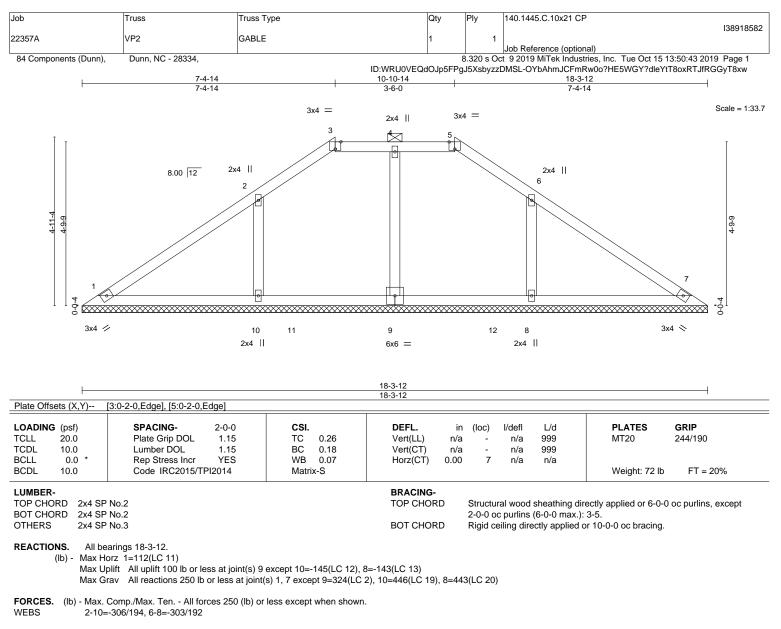
10) n/a

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



818 Soundside Road Edenton, NC 27932

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS ON TIPS REPRETED FACE PAGE MIT-14/3 refer to 100 Sec. Design valid for use only with MTRK exposure connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

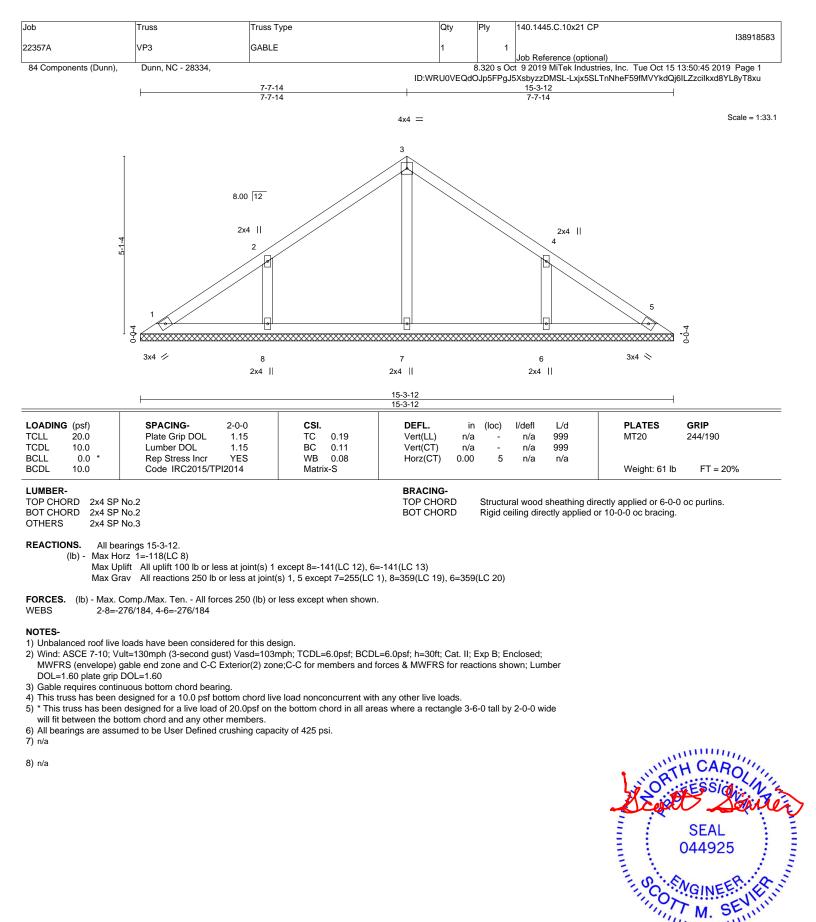
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) n/a

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



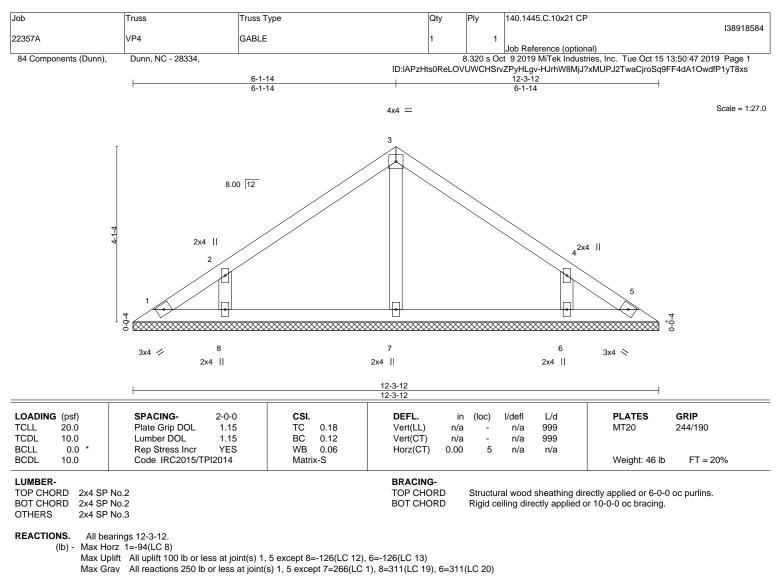


October 16,2019

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A MiTek . 818 Soundside Road

Edenton, NC 27932



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

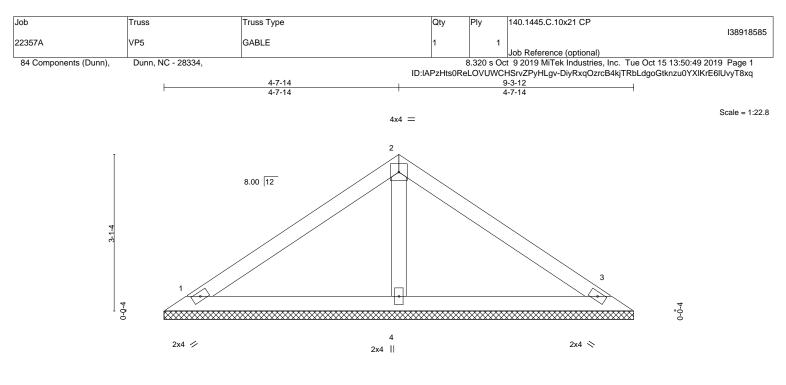
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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			9-3-12 9-3-12						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.41	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.30	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S						Weight: 32 lb	FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD OTHERS

2x4 SP No.3

REACTIONS. (lb/size) 1=163/9-3-12, 3=163/9-3-12, 4=342/9-3-12 Max Horz 1=69(LC 11) Max Uplift 1=-30(LC 12), 3=-39(LC 13), 4=-12(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

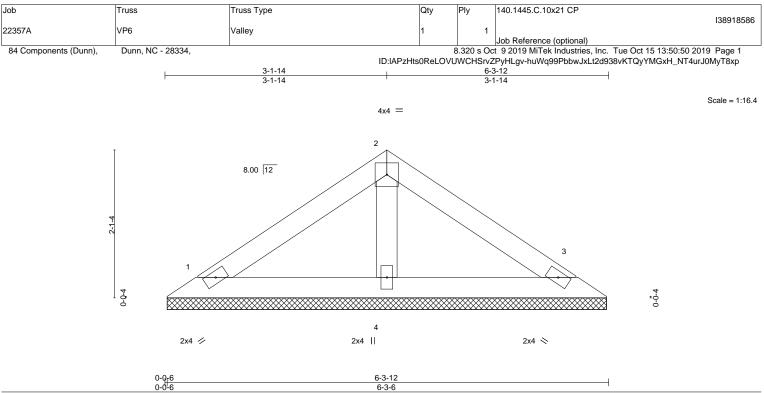
6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REPERENCE PAGE MIL-14's rev. Invozens Derrore USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



LUMBER-



	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-P						Weight: 21 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3OTHERS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=115/6-3-0, 3=115/6-3-0, 4=199/6-3-0 Max Horz 1=44(LC 9) Max Uplift 1=-25(LC 12), 3=-31(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

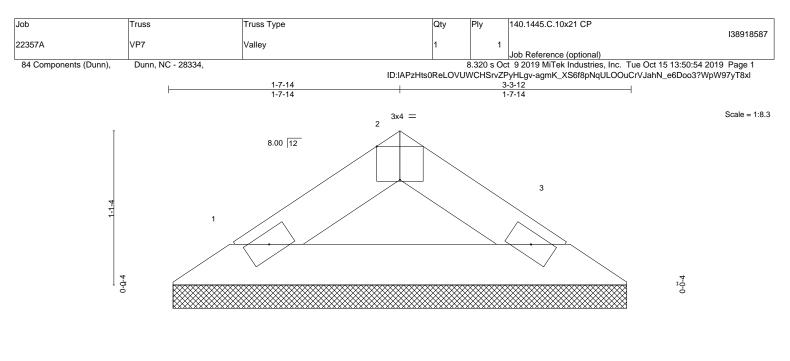
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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2x4 🥢

2x4 📎

Structural wood sheathing directly applied or 3-3-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

		<u>3-3-12</u> 3-3-6									
Plate Offsets (X,Y)	[2:0-2-0,Edge]										
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	тс	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TF	912014	Matrix	κ-Ρ						Weight: 9 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

REACTIONS. (lb/size) 1=94/3-3-0, 3=94/3-3-0 Max Horz 1=19(LC 11) Max Uplift 1=-10(LC 12), 3=-10(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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