

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 22358A 148.1869.C.8x26'8cp

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I38940013 thru I38940040

My license renewal date for the state of North Carolina is December 31, 2019.

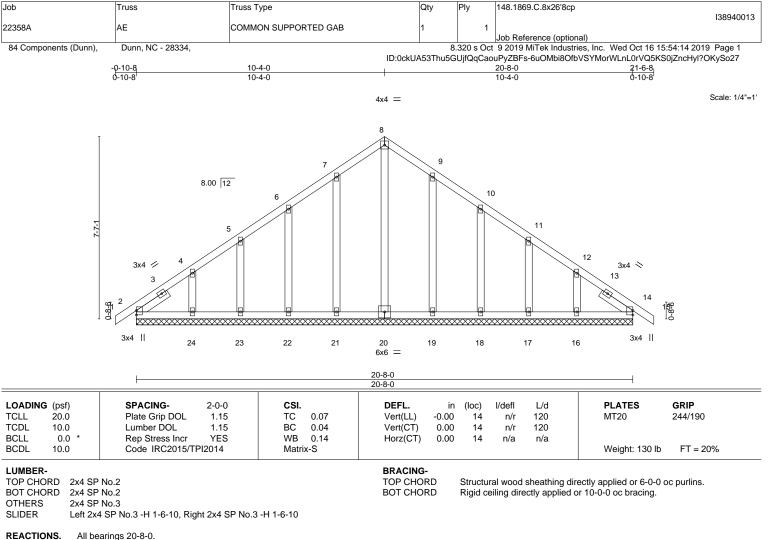
North Carolina COA: C-0844



October 17,2019

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



All bearings 20-8-0.

(lb) -Max Horz 2=185(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 21, 22, 23, 19, 18, 17, 16 except 24=-107(LC 12) Max Grav All reactions 250 lb or less at joint(s) 2, 20, 21, 22, 23, 24, 19, 18, 17, 16, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-4-0, Exterior(2) 2-4-0 to 10-4-0, Corner(3) 10-4-0 to 13-4-0, Exterior(2) 13-4-0 to 21-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

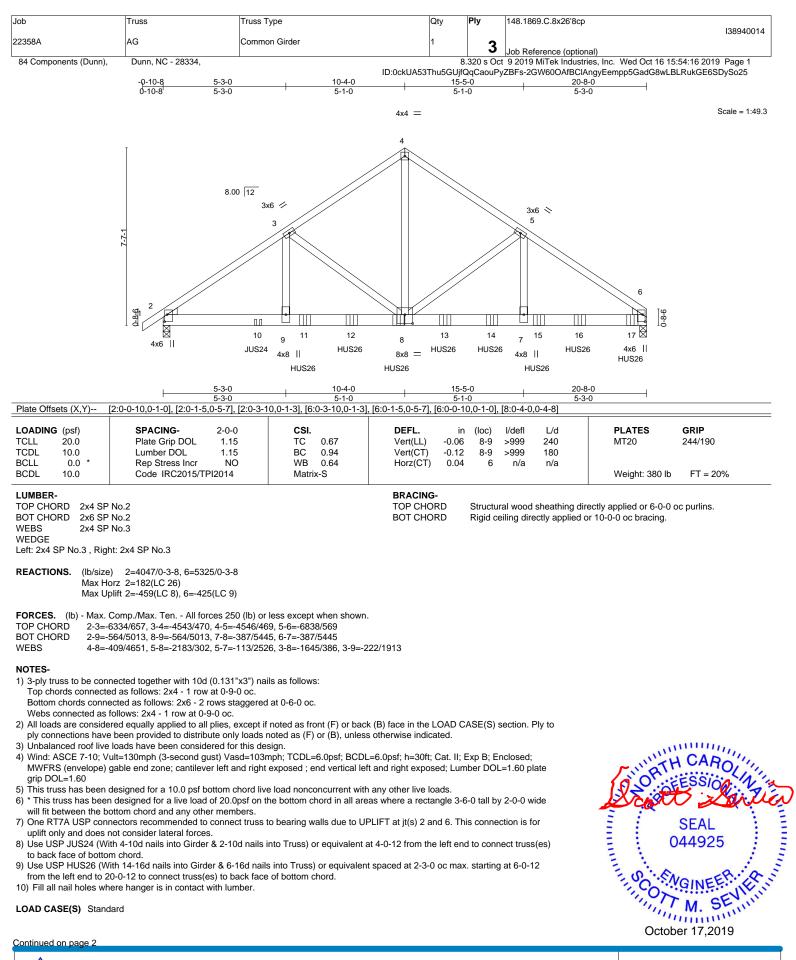
6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	148.1869.C.8x26'8cp			
					138940014			
22358A	AG	Common Girder	1	2				
				3	Job Reference (optional)			
84 Components (Dunn),	Dunn, NC - 28334,		8	320 s Oct	9 2019 MiTek Industries, Inc. Wed Oct 16 15:54:16 2019 Page 2			
		ID:0ckUA53Thu5GUjfQqCaouPyZBFs-2GW60OAfBCIAngyEempp5GadG8wLBLRukGE6SDySo25						

LOAD CASE(S) Standard

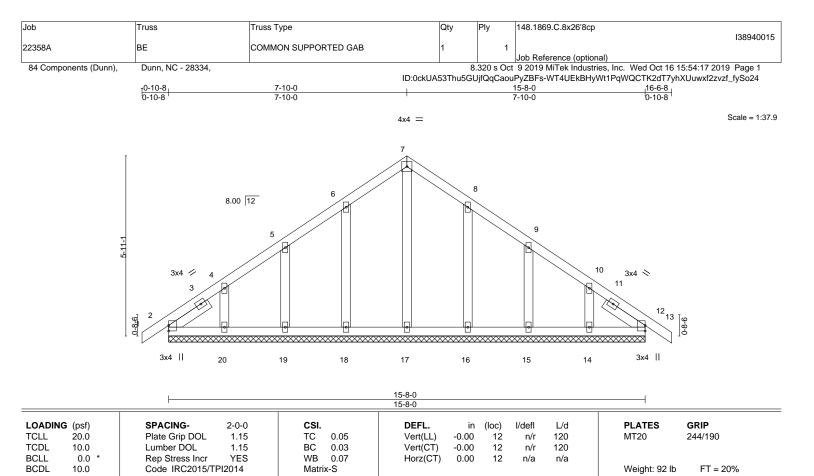
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 2-6=-20

Concentrated Loads (lb)

Vert: 8=-857(B) 10=-817(B) 11=-857(B) 12=-857(B) 13=-857(B) 14=-857(B) 15=-857(B) 16=-857(B) 17=-863(B)





BRACING-

BOT CHORD

U	М	в	E	R-

- TOP CHORD 2x4 SP No.2
- BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 -H 1-6-11, Right 2x4 SP No.3 -H 1-6-11

- REACTIONS. All bearings 15-8-0.
 - (lb) Max Horz 2=144(LC 11)

 Max Uplift
 All uplift 100 lb or less at joint(s) 2, 18, 19, 20, 16, 15, 14

 Max Grav
 All reactions 250 lb or less at joint(s) 2, 12, 17, 18, 19, 20, 16, 15, 14

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 1-10-0, Exterior(2) 1-10-0 to 7-10-0, Corner(3) 7-10-0 to 10-10-0, Exterior(2) 10-10-0 to 16-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

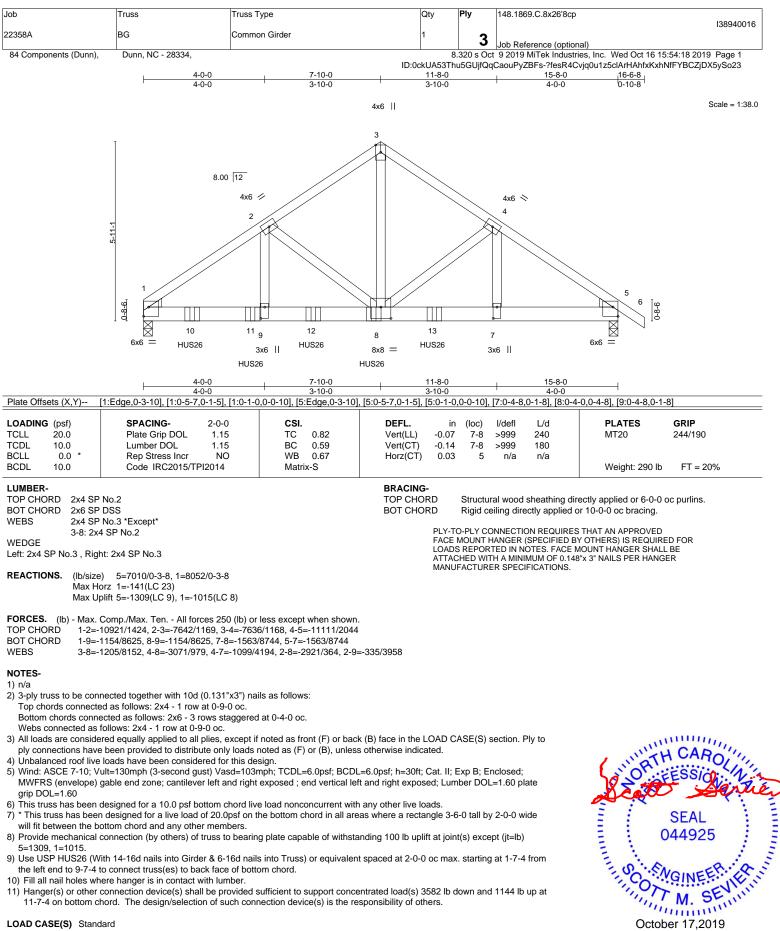
- will fit between the bottom chord and any other members.
- 9) n/a
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing





Continued on page 2

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	148.1869.C.8x26'8cp
					I38940016
22358A	BG	Common Girder	1	2	
				3	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,		8	.320 s Oct	9 2019 MiTek Industries, Inc. Wed Oct 16 15:54:18 2019 Page 2

ID:0ckUA53Thu5GUjfQqCaouPyZBFs-?fesR4Cvjq0u1z5clArHAhfxKxhNfFYBCZjDX5ySo23

LOAD CASE(S) Standard

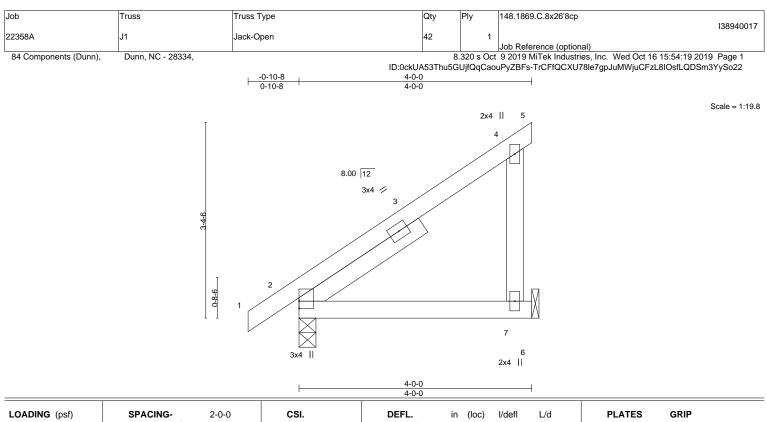
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-6=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 8=-2038(B) 7=-3582(B) 10=-2038(B) 11=-2038(B) 12=-2038(B) 13=-2038(B)





LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.26 BC 0.16 WB 0.03	DEFL. in Vert(LL) -0.01 Vert(CT) -0.02 Horz(CT) 0.00	2-7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 23 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
	Left Out CD Ne

Left 2x4 SP No.3 -H 2-6-0 SLIDER

REACTIONS. (lb/size) 2=207/0-3-8, 7=154/Mechanical Max Horz 2=123(LC 12)

Max Uplift 7=-75(LC 12) Max Grav 2=207(LC 1), 7=170(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate

grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

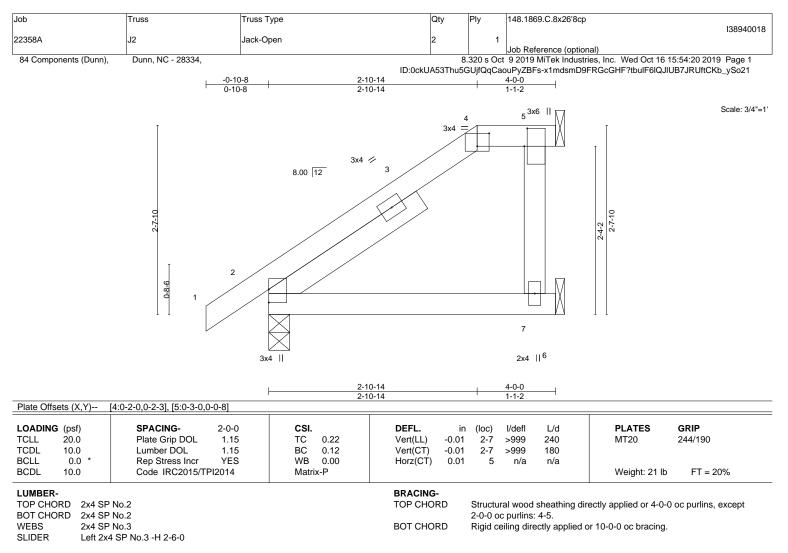
5) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 4-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





REACTIONS. (lb/size) 2=207/0-3-8, 5=103/Mechanical, 7=45/Mechanical Max Horz 2=97(LC 12) Max Uplift 2=-18(LC 12), 5=-52(LC 12) Max Grav 2=207(LC 1), 5=103(LC 1), 7=76(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

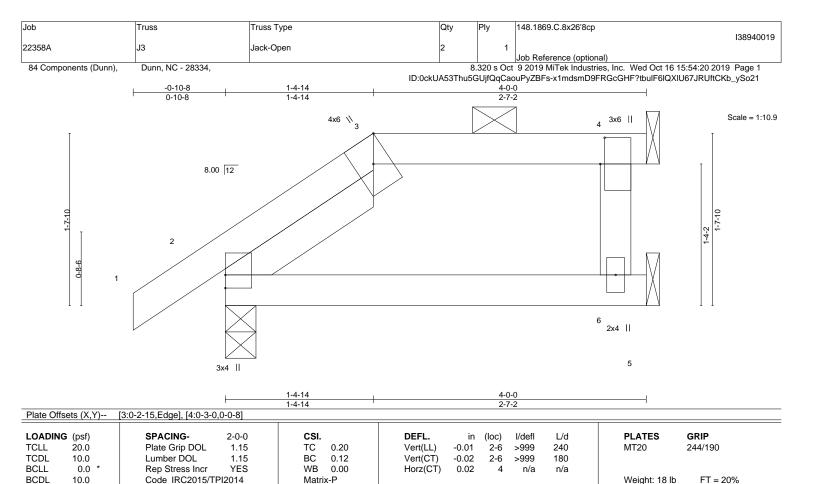
NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-10-14, Exterior(2) 2-10-14 to 3-8-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 10) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.







BRACING-

TOP CHORD

BOT CHORD

2-0-0 oc purlins: 3-4.

NOTES-

LUMBER-

WEBS

SLIDER REACTIONS.

TOP CHORD

BOT CHORD

2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

Left 2x4 SP No.3 -H 1-8-11

Max Horz 2=59(LC 12)

1) Unbalanced roof live loads have been considered for this design.

Max Uplift 2=-27(LC 12), 4=-40(LC 9) Max Grav 2=207(LC 1), 4=101(LC 1), 6=77(LC 3) FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

(lb/size) 2=207/0-3-8, 4=101/Mechanical, 6=47/Mechanical

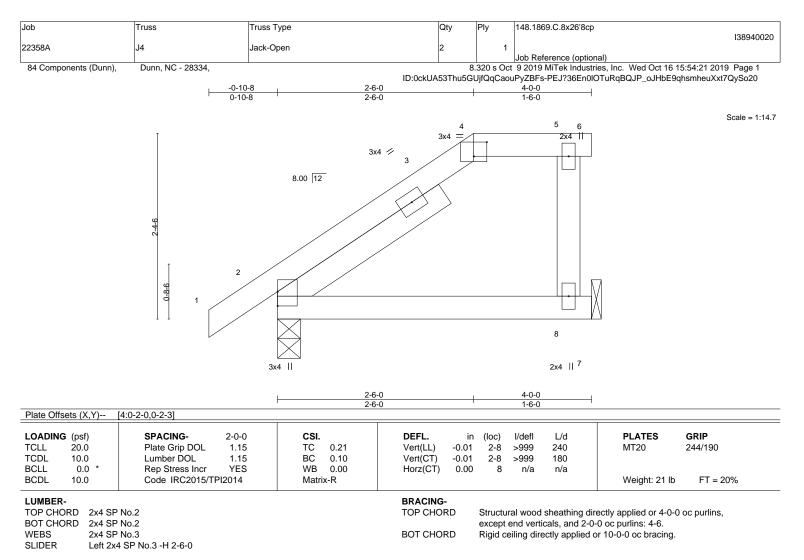
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



Structural wood sheathing directly applied or 4-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing





REACTIONS. (lb/size) 8=154/Mechanical, 2=207/0-3-8 Max Horz 2=82(LC 9)

Max Uplift 8=-37(LC 9), 2=-33(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-6-0, Exterior(2) 2-6-0 to 4-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

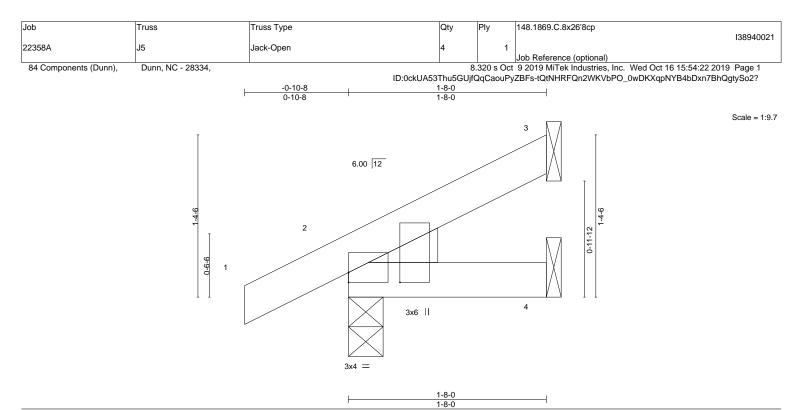
 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 2. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





¹⁾ Unbalanced roof live loads have been considered for this design.



PACING- 2-0-0	CSI.	DEFL. in (lee) l/defi		
			loc) l/defl	L/d	PLATES GRIP
late Grip DOL 1.15	TC 0.05	Vert(LL) -0.00	2 >999	240	MT20 244/190
umber DOL 1.15	BC 0.03	Vert(CT) -0.00	2 >999	180	
ep Stress Incr YES	WB 0.00	Horz(CT) -0.00	3 n/a	n/a	
ode IRC2015/TPI2014	Matrix-P				Weight: 8 lb FT = 20%
u le	imber DOL 1.15 ep Stress Incr YES	Imber DOL1.15BC0.03op Stress IncrYESWB0.00	Imber DOL 1.15 BC 0.03 Vert(CT) -0.00 ap Stress Incr YES WB 0.00 Horz(CT) -0.00	Imber DOL 1.15 BC 0.03 Vert(CT) -0.00 2 >999 ap Stress Incr YES WB 0.00 Horz(CT) -0.00 3 n/a	Imber DOL 1.15 BC 0.03 Vert(CT) -0.00 2 >999 180 ap Stress Incr YES WB 0.00 Horz(CT) -0.00 3 n/a n/a

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEDGE

Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=34/Mechanical, 2=134/0-3-8, 4=16/Mechanical Max Horz 2=48(LC 12) Max Uplift 3=-29(LC 12), 2=-24(LC 12) Max Grav 3=34(LC 1), 2=134(LC 1), 4=33(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

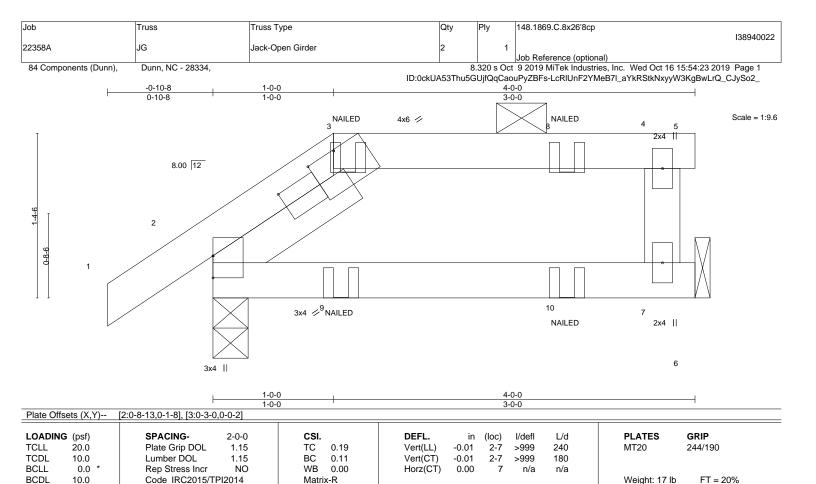


Structural wood sheathing directly applied or 1-8-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED MITER REFERENCE PAGE MIT-14's rev. Towards BEFORE OSE. Design valid for use only with MiTeR's connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BRACING-

TOP CHORD

BOT CHORD

NOTES-1) Unbalanced roof live loads have been considered for this design.

Max Uplift 7=-35(LC 5), 2=-36(LC 8)

(lb/size) 7=154/Mechanical, 2=207/0-3-8

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 2. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

- 9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

LUMBER-

WEBS

SLIDER REACTIONS.

BOT CHORD

TOP CHORD 2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

Left 2x4 SP No.3 -H 1-4-0

Max Horz 2=44(LC 5)

 Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-5=-20, 2-6=-20

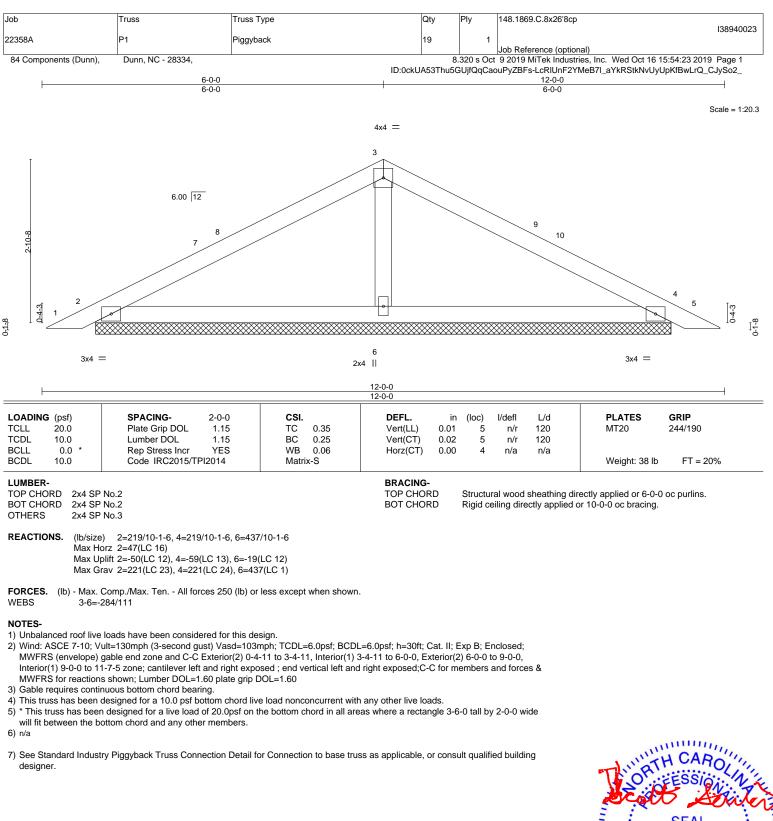


Structural wood sheathing directly applied or 4-0-0 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-5.

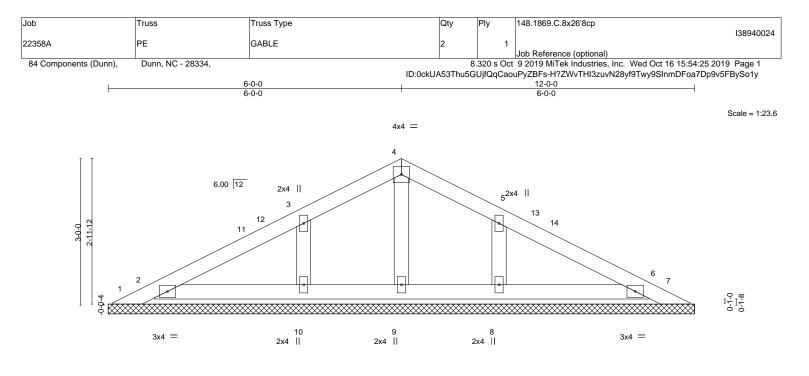
Rigid ceiling directly applied or 10-0-0 oc bracing











<u>12-0-0</u> 12-0-0											
OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	MT20	244/190
CDL 10.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 42 lb	FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2OTHERS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-0-0. (lb) - Max Horz 1=-47(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 2, 6, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-11 to 3-4-11, Interior(1) 3-4-11 to 6-0-0, Exterior(2) 6-0-0 to 9-0-0, Interior(1) 9-0-0 to 11-7-5 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

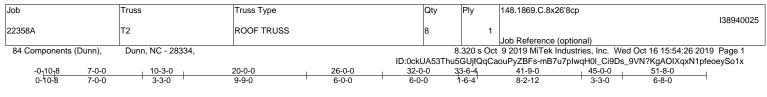
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.
 9) n/a

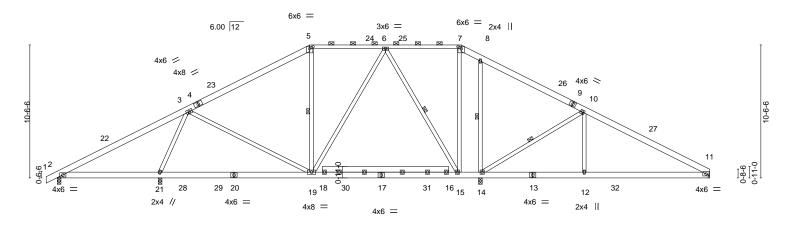
10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.







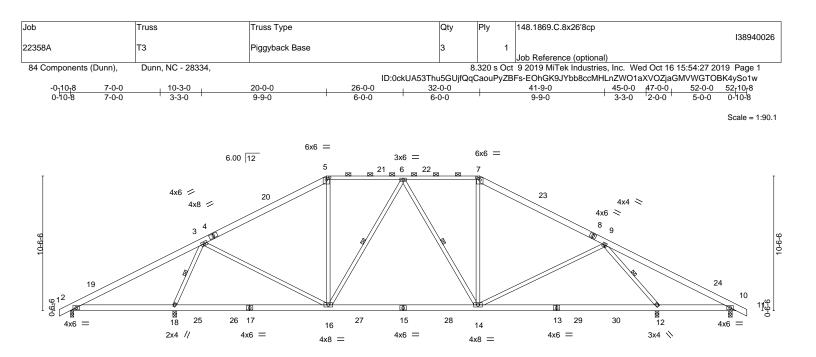
Scale = 1:91.3



	-12 20-0-0 -12 11-10-4		32-0-0 12-0-0	<u>33-6-4</u> 1-6-4	43-10-4	51-8-0	
	[5:0-3-0,0-2-0], [7:0-3-0,0-2-0]		1200	104	10 4 0	1012	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.67 BC 0.65 WB 0.90 Matrix-S	Vert(CT) -0	in (loc) l/defi 0.10 15-19 >999 0.19 19-21 >999 0.03 11 n/a	9 240 9 180	PLATES MT20 Weight: 390 lb	GRIP 244/190 FT = 20%
		-	BRACING- TOP CHORD BOT CHORD	except 2-0-0 oc purli Rigid ceiling o 6-0-0 oc brac	ns (6-0-0 max.): 5-7 directly applied or 1 ing: 2-21.	0-0-0 oc bracing, E	•
(lb) - Max H Max U Max G FORCES. (lb) - Max. TOP CHORD 2-3=- 8-10= BOT CHORD 19-21	arings 0-3-8 except (jt=length) 11=Mech orz 2=182(LC 16) plift All uplift 100 lb or less at joint(s) 2, rav All reactions 250 lb or less at joint(Comp./Max. Ten All forces 250 (lb) or 131/434, 3-5=-1057/236, 5-6=-831/278, -633/151, 10-11=-1304/147 =-120/425, 15-19=-92/829, 14-15=-33/6	11 except 21=-195(LC 1 (s) 2 except 21=1669(LC less except when shown 6-7=-546/193, 7-8=-700/ 512, 12-14=-10/1088, 11-	26), 14=1433(LC 1), 219, 12=-10/1088	1 Row at mid 11=877(LC 1)	pt 5-19	, 6-15, 8-14, 10-14	
10-12 NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) Interior(1) 27-3-11 to vertical left and right 3) Provide adequate dr 4) All plates are 4x4 M 5) This truss has been will fit between the b 7) Refer to girder(s) for 8) Provide mechanical 9) One RT7A USP con for uplift only and do 10) Graphical purlin rep	-1528/357, 3-19=0/534, 6-15=-591/176, =0/384, 10-14=-793/289 loads have been considered for this de ult=130mph (3-second gust) Vasd=103; gable end zone and C-C Exterior(2) -0- 9 32-0-0, Exterior(2) 32-0-0 to 39-3-11, In exposed;C-C for members and forces & ainage to prevent water ponding. I20 unless otherwise indicated. designed for a 10.0 psf bottom chord liv n designed for a 10.0 psf bottom chord liv n designed for a 10.0 psf bottom chord liv n designed for a 10.0 psf bottom chord liv n totom chord and any other members, wi truss to truss connections. connection (by others) of truss to bearin nectors recommended to connect truss es not consider lateral forces. presentation does not depict the size or DWN IS DESIGNED AS UNINHABITAB	sign. mph; TCDL=6.0psf; BCDI 10-8 to 4-3-8, Interior(1) 4 nterior(1) 39-3-11 to 51-7 & MWFRS for reactions sl e load nonconcurrent with he bottom chord in all are th BCDL = 10.0psf. g plate capable of withsta to bearing walls due to U the orientation of the purl	L=6.0psf; h=30ft; Cat I-3-8 to 20-0-0, Exteri -4 zone; cantilever le hown; Lumber DOL= h any other live loads eas where a rectangle anding 100 lb uplift at PLIFT at jt(s) 2, 21, a	or(2) 20-0-0 to 27- ft and right expose 1.60 plate grip DOI a 3-6-0 tall by 2-0-0 joint(s) 11. nd 14. This connect	-3-11, kd ; end L=1.60) wide		CAROL ESSIA SEAL 44925 GINEER, HUMAN

October 17,2019

A MITEK Affiliate B18 Soundside Road Edenton, NC 27932



	1-12 20-0-0 1-12 11-10-4		32-0-0 12-0-0	<u>41-9-0</u> 9-9-0	<u>45-10-4</u> <u>51-8-12</u> <u>52-0</u> -0 <u>4-1-4</u> <u>5-10-8</u> <u>0-3-4</u>
Plate Offsets (X,Y)	[5:0-3-0,0-2-0], [7:0-3-0,0-2-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.68 BC 0.71 WB 0.56 Matrix-S	Vert(LL) -0.2	n (loc) l/defl L/d l 14-16 >999 240 l 12-14 >999 180 4 12 n/a n/a	PLATES GRIP MT20 244/190 Weight: 362 lb FT = 20%
			BRACING- TOP CHORD BOT CHORD WEBS	2-0-0 oc purlins (4-9-4 max.) Rigid ceiling directly applied	
(lb) - Max H Max U	earings 0-3-8 except (jt=length) 2=0-3-0 lorz 2=175(LC 12) Iplift All uplift 100 lb or less at joint(s) 2 Grav All reactions 250 lb or less at joint	2, 10 except 18=-223(LC 1			
TOP CHORD 2-3=	Comp./Max. Ten All forces 250 (lb) c -140/421, 3-5=-1465/305, 5-6=-1195/32 =-54/324				

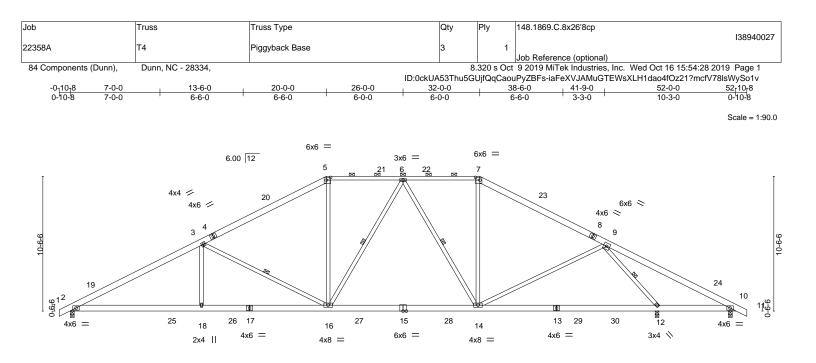
- BOT CHORD 16-18=-120/539, 14-16=-54/1340, 12-14=-145/1105
- WEBS 3-18=-1903/380, 3-16=0/806, 5-16=0/306, 6-16=-428/145, 7-14=0/341, 9-14=0/434, 9-12=-1966/387

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 20-0-0, Exterior(2) 20-0-0 to 24-2-15, Interior(1) 24-2-15 to 32-0-0, Exterior(2) 32-0-0 to 36-2-15, Interior(1) 36-2-15 to 52-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 18, 12, and 10. This connection is for uplift only and does not consider lateral forces.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







	<u>-1-12 10-3-0</u> -1-12 4-1-4	<u>+ 13-6-0</u> 3-3-0	20-0-0	<u>32-0-0</u> 12-0-0		38-6-0		45-10-4	<u>51-8-12 52-0</u> -0 5-10-8 0-3-4
Plate Offsets (X,Y)	[5:0-4-0,0-2-8], [7	0-4-0,0-2-8]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip Lumber DC Rep Stress Code IRC	DOL 1.15 DL 1.15	BC	DEFL. 0.72 Vert(LL) 0.82 Vert(CT) 0.76 Horz(CT) ·S ·S	in -0.28 -0.44 0.10	14-16 >99	99 240	PLATES MT20 Weight: :	244/190
	6 SP No.2 *Except* 7: 2x4 SP No.2			BRACING- TOP CHOF	D		ood sheathing d	2 11	3-2-7 oc purlins, except
BOT CHORD 2x	6 SP No.2 4 SP No.3		BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 10-12.						
REACTIONS. (It	o/size) 2=1856/0-3-8	, 12=2440/0-3-8, 10)=-36/0-3-8	WEBS		1 Row at mi	αρτ	3-16, 6-16, 6-14,	9-12

Max Horz 2=-175(LC 13) Max Uplift 2=-216(LC 12), 12=-142(LC 13), 10=-212(LC 25) Max Grav 2=1856(LC 1), 12=2440(LC 1), 10=40(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3332/422, 3-5=-2388/407, 5-6=-2029/426, 6-7=-1717/383, 7-9=-2044/362,

9-10=-115/834

 BOT CHORD
 2-18=-351/2887, 16-18=-351/2887, 14-16=-89/1951, 12-14=-141/1101, 10-12=-614/171

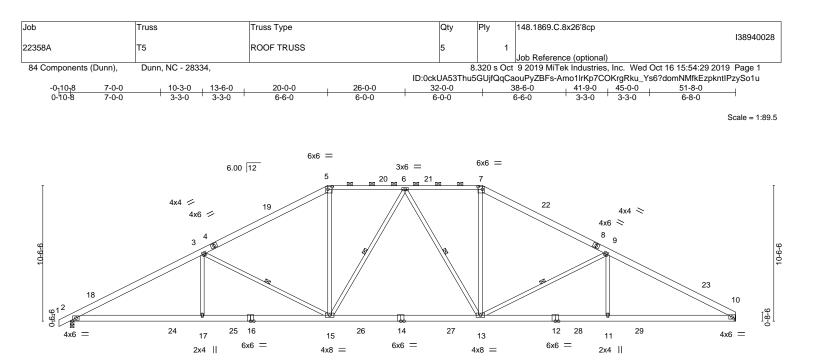
 WEBS
 3-16=-993/334, 5-16=-14/654, 6-14=-615/168, 7-14=0/523, 9-14=-7/844, 9-12=-2731/473, 3-18=0/430

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 20-0-0, Exterior(2) 20-0-0 to 24-2-15, Interior(1) 24-2-15 to 32-0-0, Exterior(2) 32-0-0 to 36-2-15, Interior(1) 36-2-15 to 52-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 12, and 10. This connection is for uplift only and does not consider lateral forces.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







1	10-3-0	20-0-0	1	32-0-0	41-9-0	51-8-0
	10-3-0	9-9-0	I	12-0-0	9-9-0	9-11-0
Plate Offsets (X,Y)	[5:0-4-0,0-2-8], [7:0-4	-0,0-2-8]				
_OADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL	. 1.15	TC 0.88		37 13-15 >999 240	MT20 244/190
CDL 10.0	Lumber DOL	1.15	BC 0.93		5 13-15 >943 180	
3CLL 0.0 *	Rep Stress Inc	r YES	WB 0.62	Horz(CT) 0.1	7 10 n/a n/a	
BCDL 10.0	Code IRC2015	5/TPI2014	Matrix-S			Weight: 354 lb FT = 20%
UMBER-				BRACING-		
TOP CHORD 2x6 S	SP No.2 *Except*			TOP CHORD	Structural wood sheathing	directly applied or 2-2-0 oc purlins, except
	2x4 SP No.2				2-0-0 oc purlins (3-3-3 max	
BOT CHORD 2x6 S	SP No.2			BOT CHORD	Rigid ceiling directly applie	d or 10-0-0 oc bracing, Except:
VEBS 2x4 S	SP No.3				2-2-0 oc bracing: 13-15.	0, 1
				WEBS	1 Row at midpt	3-15, 6-15, 6-13, 9-13
FACTIONS (Ib/s	ize) 2-2120/0-3-8 10	-2058/Mechanica	1			

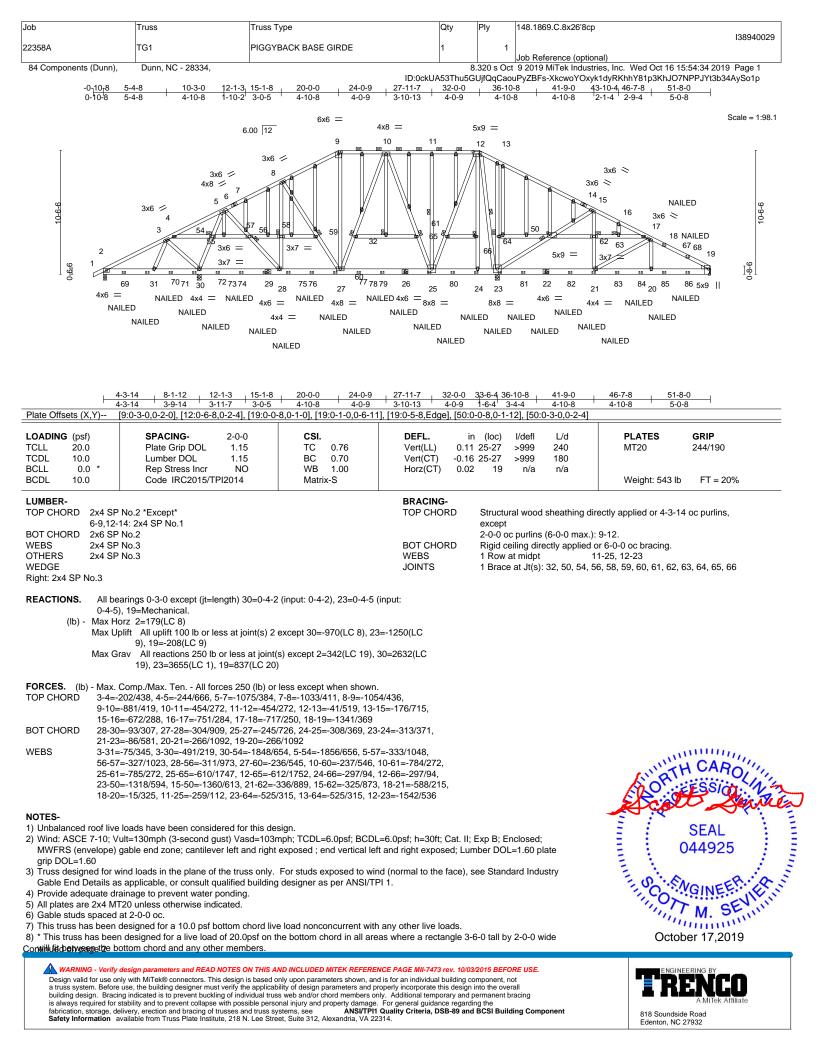
- REACTIONS. (lb/size) 2=2120/0-3-8, 10=2058/Mechanical Max Horz 2=182(LC 16) Max Uplift 2=-220(LC 12), 10=-196(LC 13)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-3912/480, 3-5=-2995/461, 5-6=-2574/476, 6-7=-2564/472, 7-9=-2983/463,
- 9-10=-3861/472
- BOT CHORD 2-17=-367/3417, 15-17=-367/3417, 13-15=-177/2643, 11-13=-324/3356, 10-11=-324/3356
- WEBS 3-15=-983/337, 5-15=-39/921, 6-15=-372/172, 6-13=-389/171, 7-13=-42/915,
 - 9-13=-931/334, 3-17=0/419, 9-11=0/415

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 20-0, Exterior(2) 20-0-0 to 24-2-15, Interior(1) 24-2-15 to 32-0-0, Exterior(2) 32-0-0 to 36-2-15, Interior(1) 36-2-15 to 51-7-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=196.
- One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

MILLIN ORT MULLIUM III SEAL 044925 M. M. S October 17,2019





Job	Truss	Truss Type	Qty	Ply	148.1869.C.8x26'8cp	
					138940029	
22358A	TG1	PIGGYBACK BASE GIRDE	1	1		
					Job Reference (optional)	
84 Components (Dunn),	Components (Dunn), Dunn, NC - 28334,				9 2019 MiTek Industries, Inc. Wed Oct 16 15:54:34 2019 Page 2	
		ID:0ckU/	ID:0ckUA53Thu5GUjfQqCaouPyZBFs-XkcwoYOxyk1dyRKhhY81p3KhJO7NPPJYt3b34A			

NOTES-

9) Bearings are assumed to be: , Joint 30 User Defined crushing capacity of 425 psi, Joint 19 User Defined crushing capacity of 425 psi.

- 10) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 30=970, 23=1250, 19=208.
 One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces. 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

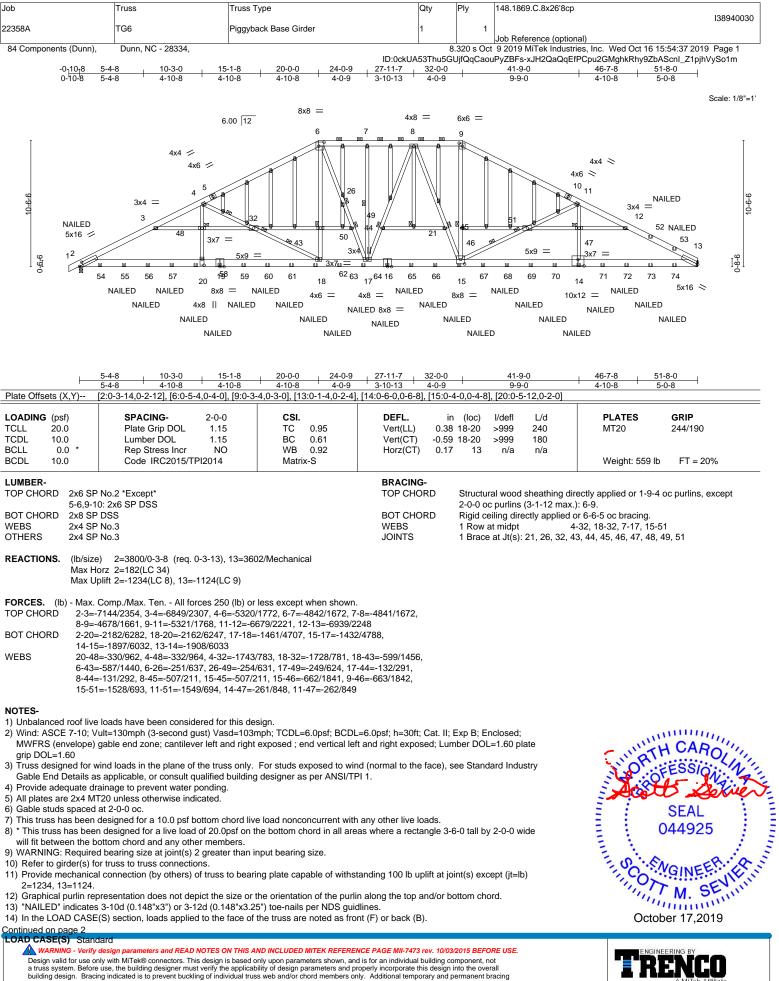
Uniform Loads (plf)

Vert: 1-9=-60, 9-12=-60, 12-19=-60, 2-19=-20

Concentrated Loads (lb)

Vert: 29=-134(F) 25=-134(F) 24=-134(F) 23=-134(F) 21=-134(F) 22=-134(F) 26=-134(F) 67=-43(F) 68=-41(F) 69=-134(F) 70=-134(F) 71=-134(F) 72=-134(F) 72=-134 73=-134(F) 74=-134(F) 75=-134(F) 76=-134(F) 77=-134(F) 78=-134(F) 80=-134(F) 81=-134(F) 82=-134(F) 83=-134(F) 84=-134(F) 85=-25(F) 86=-27(F)





a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	148.1869.C.8x26'8cp				
					138940030				
22358A	TG6	Piggyback Base Girder	1	1					
					Job Reference (optional)				
84 Components (Dunn),	Dunn, NC - 28334,		8	.320 s Oct	9 2019 MiTek Industries, Inc. Wed Oct 16 15:54:38 2019 Page 2				
		ID:0ckUA53Thu5GUjfQqCaouPyZBFs-PVrQewRS?zX2Q2dSwNCz_vUKJ?WhLEY8ohZHDxySo11							

LOAD CASE(S) Standard

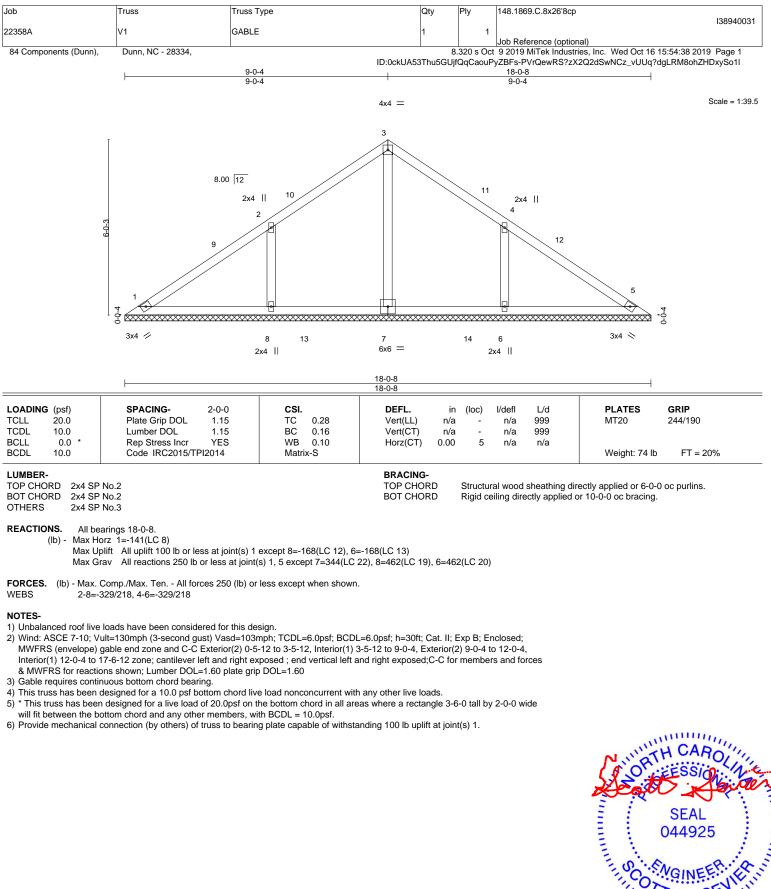
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-60, 6-9=-60, 9-13=-60, 2-13=-20

Concentrated Loads (lb)

Vert: 19=-134(B) 15=-134(B) 14=-134(B) 16=-134(B) 52=-43(B) 53=-41(B) 54=-134(B) 55=-134(B) 56=-134(B) 57=-134(B) 58=-134(B) 59=-134(B) 59=-134

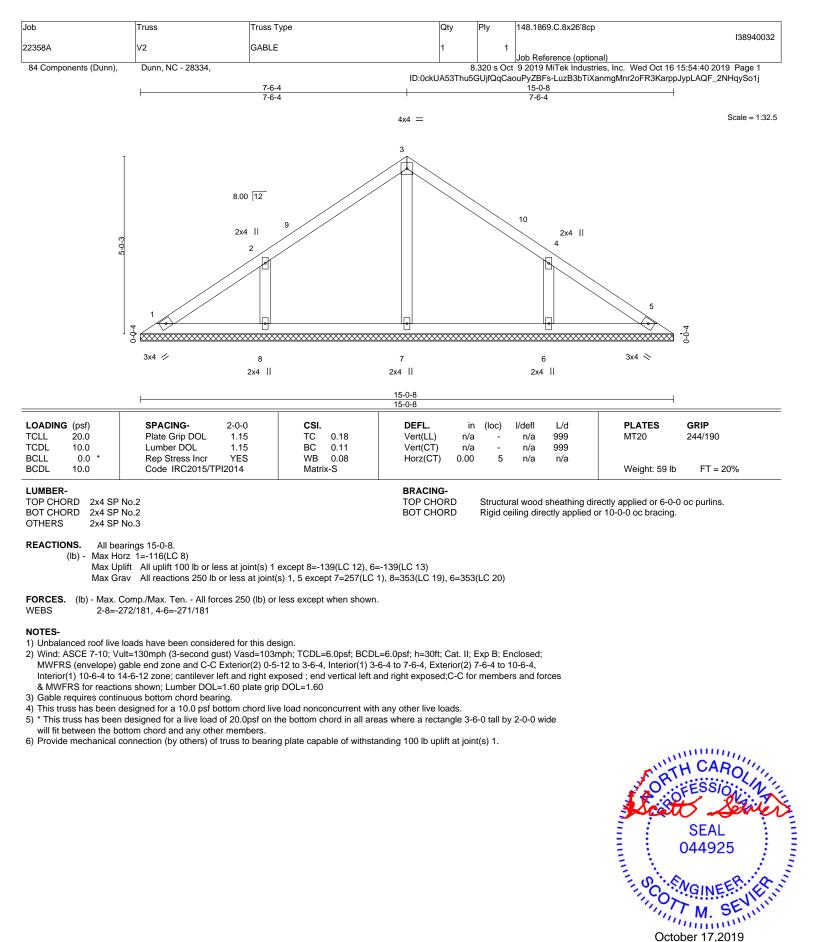




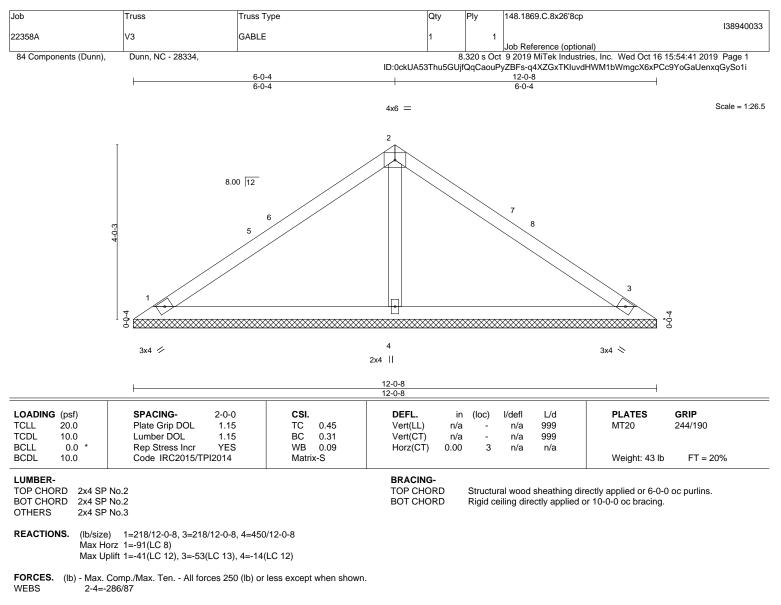


🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPERVICE PAGE MIT-14/3 refer to 1000 SEC. Design valid for use only with MITER deconnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.









NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-0-4, Exterior(2) 6-0-4 to 9-0-4, Interior(1) 9-0-4 to 11-6-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

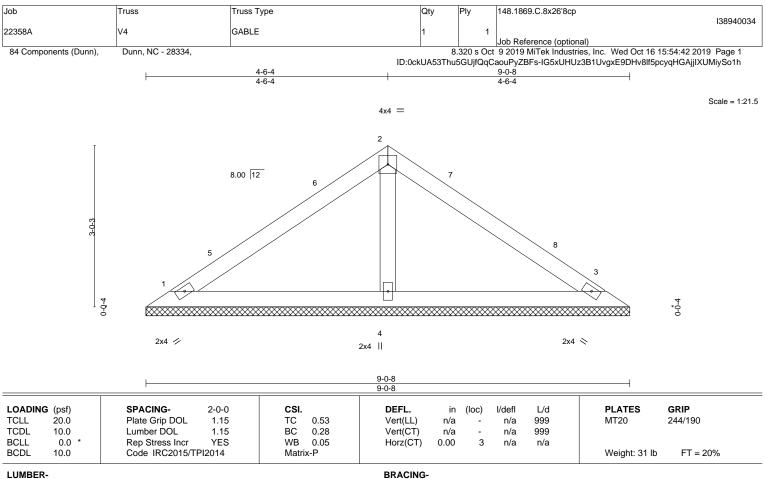
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

TRENCO A MiTek Affiliate 818 Soundside Road Edenton, NC 27932



TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

2x4 SP No.3 OTHERS

REACTIONS. 1=173/9-0-8, 3=173/9-0-8, 4=300/9-0-8 (lb/size) Max Horz 1=-67(LC 8) Max Uplift 1=-38(LC 12), 3=-47(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-6-4, Exterior(2) 4-6-4 to 7-6-4, Interior(1) 7-6-4 to 8-6-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Gable requires continuous bottom chord bearing.

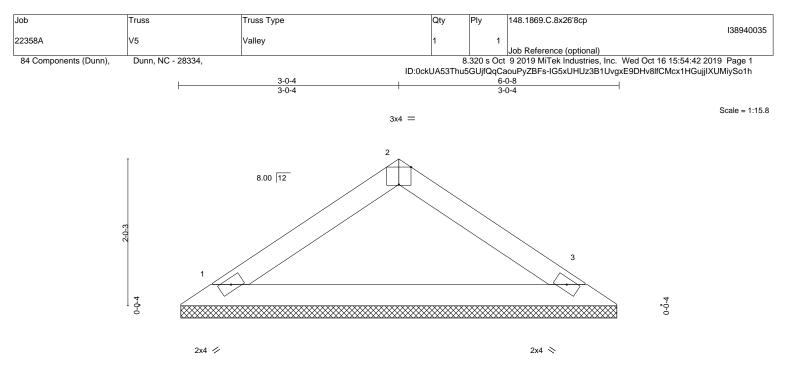
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5)

will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







0-9 <u>r6</u> 0-0-6			<u>6-0-8</u> 6-0-2								
Plate Offsets (X,Y) [2:0-2-0,Edge]										
OADING (psf)	SPACING- 2-	2-0-0 CSI .		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
CLL 20.0	Plate Grip DOL 1	1.15 TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
CDL 10.0	Lumber DOL 1	1.15 BC	0.33	Vert(CT)	n/a	-	n/a	999			
BCLL 0.0 *	Rep Stress Incr Y	YES WB	0.00	Horz(CT)	0.00	3	n/a	n/a			
3CDL 10.0	Code IRC2015/TPI20	014 Matrix	x-P						Weight: 18 lb	FT = 20%	

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 1=203/5-11-12, 3=203/5-11-12 Max Horz 1=-42(LC 8) Max Uplift 1=-22(LC 12), 3=-22(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

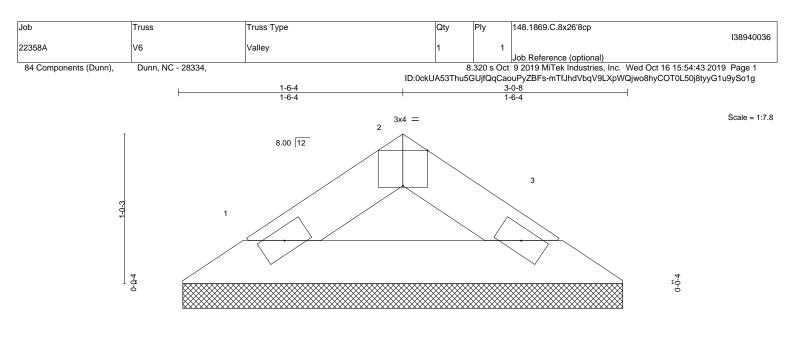
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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2x4 🥢

2x4 📎

Structural wood sheathing directly applied or 3-0-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

	0-0 <u>-</u> 6 0-0 ¹ 6		3-0-8 3-0-2			{		
Plate Offsets (X,Y)	2:0-2-0,Edge]							
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (lo	oc) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.03	Vert(LL)	n/a `	- n/a	999	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT)	n/a	- n/a	999		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	0.00	3 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 8 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (Ib/size) 1=83/2-11-12, 3=83/2-11-12 Max Horz 1=-17(LC 8) Max Uplift 1=-9(LC 12), 3=-9(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

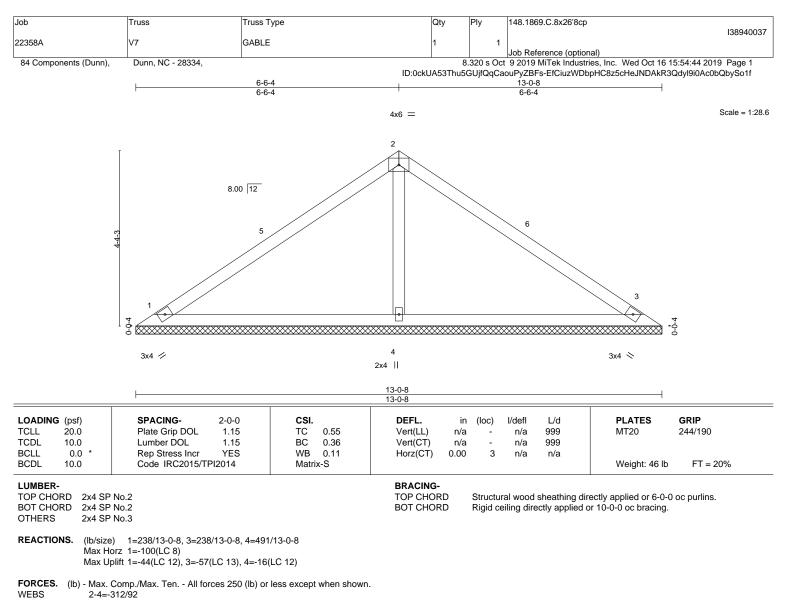
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-6-4, Exterior(2) 6-6-4 to 9-6-4, Interior(1) 9-6-4 to 12-6-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

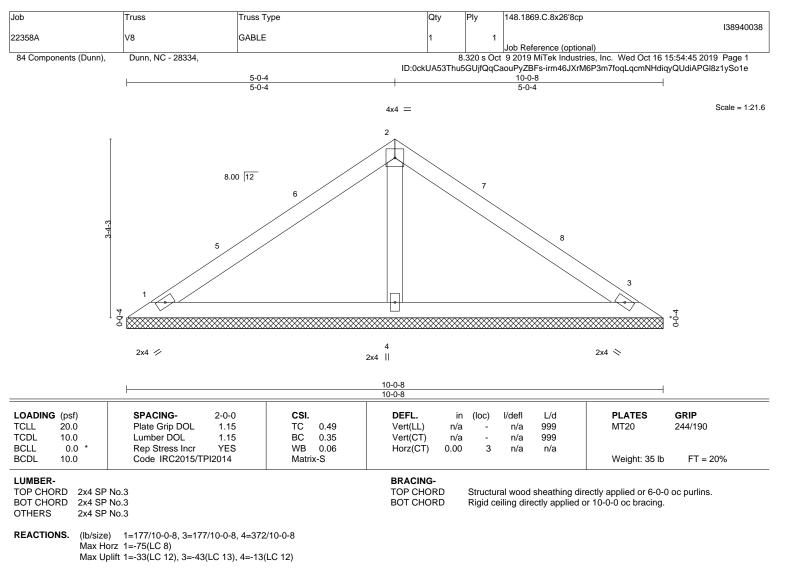
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-0-4, Exterior(2) 5-0-4 to 8-0-4, Interior(1) 8-0-4 to 9-6-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Gable requires continuous bottom chord bearing.

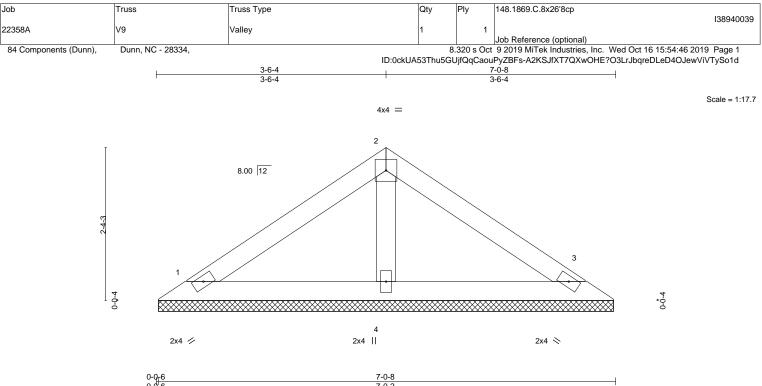
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







0-0 ¹ 6		7-0-2										
TCDL BCLL	20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.29 0.16 0.03	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2015/TF	12014	Matri	x-P						Weight: 24 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 1=130/6-11-12, 3=130/6-11-12, 4=226/6-11-12 Max Horz 1=-50(LC 8) Max Uplift 1=-29(LC 12), 3=-36(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

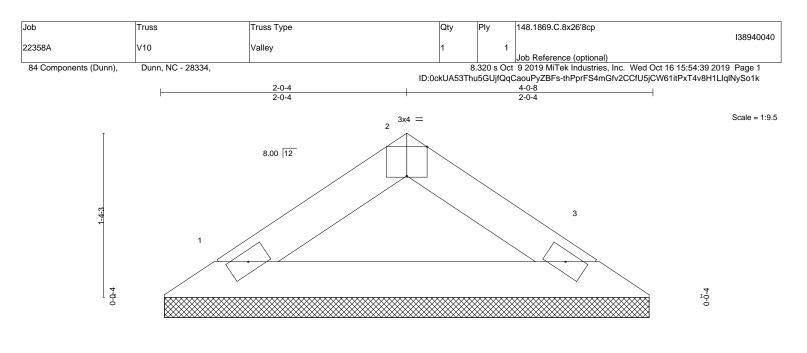


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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2x4 1/

2x4

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y) [2:0-2-0.Edge]	4-0-2 4-0-2			<u>4-0-8</u> 0-0-6			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.07 BC 0.19 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP	No.3	BRACING- TOP CHOR		ural wood	sheathing di	rectly applied or 4-0-8	oc purlins.	

BOT CHORD

TOP CHORD 2x4 SP No.3 BOT CHORD 2x4 SP No.3

REACTIONS. (lb/size) 1=123/3-11-12, 3=123/3-11-12 Max Horz 1=-25(LC 8) Max Uplift 1=-13(LC 12), 3=-13(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



818 Soundside Road Edenton, NC 27932

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