

RE: 140.1445.C 20 MASON POINTE - ROOF Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Lot/Block:	Project Name:	140.1445.C
Address:		
City:		

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.3 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 37 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	137903319	ae	7/24/2019	21	137903339	p	7/24/2019
2	137903320	ag	7/24/2019	22	137903340	p1	7/24/2019
3	137903321	cĨ	7/24/2019	23	137903341	p2	7/24/2019
4	137903322	c2	7/24/2019	24	137903342	p3	7/24/2019
5	137903323	c3	7/24/2019	25	137903343	pj1	7/24/2019
6	137903324	h1	7/24/2019	26	137903344	pj2	7/24/2019
7	137903325	h2	7/24/2019	27	137903345	pj3	7/24/2019
8	137903326	h3	7/24/2019	28	137903346	t5	7/24/2019
9	137903327	h4	7/24/2019	29	137903347	t6	7/24/2019
10	137903328	h5	7/24/2019	30	137903348	t7	7/24/2019
11	137903329	h8	7/24/2019	31	137903349	t7a	7/24/2019
12	137903330	h9	7/24/2019	32	137903350	v1	7/24/2019
13	137903331	h10	7/24/2019	33	137903351	v2	7/24/2019
14	137903332	h11	7/24/2019	34	137903352	v3	7/24/2019
15	137903333	h12	7/24/2019	35	137903353	vp1	7/24/2019
16	137903334	hg1	7/24/2019	36	137903354	vp2	7/24/2019
17	137903335	hg2	7/24/2019	37	137903355	vp3	7/24/2019
18	137903336	j1	7/24/2019				
19	137903337	j2	7/24/2019				
20	137903338	j3	7/24/2019				

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision

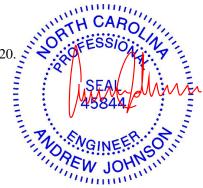
based on the parameters provided by 84 Components - #2383.

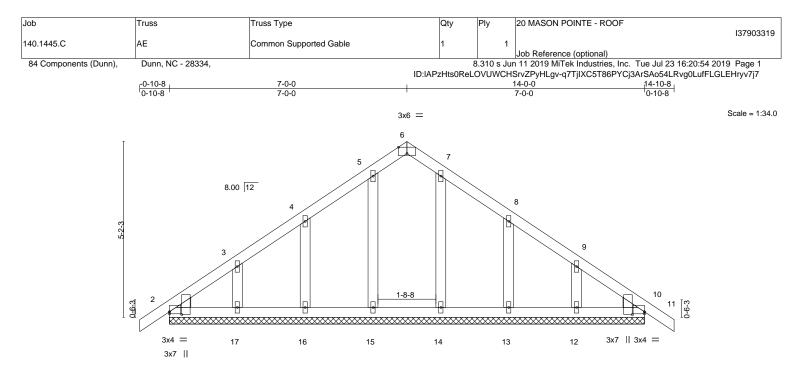
Truss Design Engineer's Name: Johnson, Andrew

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





			<u>14-0-0</u> 14-0-0						
Plate Offsets (X,Y) [2:0-0-15,0-4-5], [2:0-0-0,0-0-12], [6:0-3-0,Edge], [10:Edge,0-0-12], [10:0-0-15,0-4-5]									
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.05	- () -	in (loc) .00 10	l/defl L/ n/r 12	0 MT20 244/190			
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	BC 0.03 WB 0.04 Matrix-S	- (-) -	.00 10 .00 10	n/r 12 n/a n/	-			
LUMBER- TOP CHORD 2x4 SP	No.2		BRACING- TOP CHORD	Structu	ral wood shea	thing directly applied or 6-0-0 oc purlins.			

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

Max Horz 2=130(LC 11) Max Uplift All uplift 100 lb or less at joint(s) 2, 12, 13, 14, 17, 16, 15 Max Grav All reactions 250 lb or less at joint(s) 2, 10, 12, 13, 14, 17, 16, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-0-0, Exterior(2) 2-0-0 to 7-0-0, Corner(3) 7-0-0 to 10-0-0, Exterior(2) 10-0-0 to 14-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

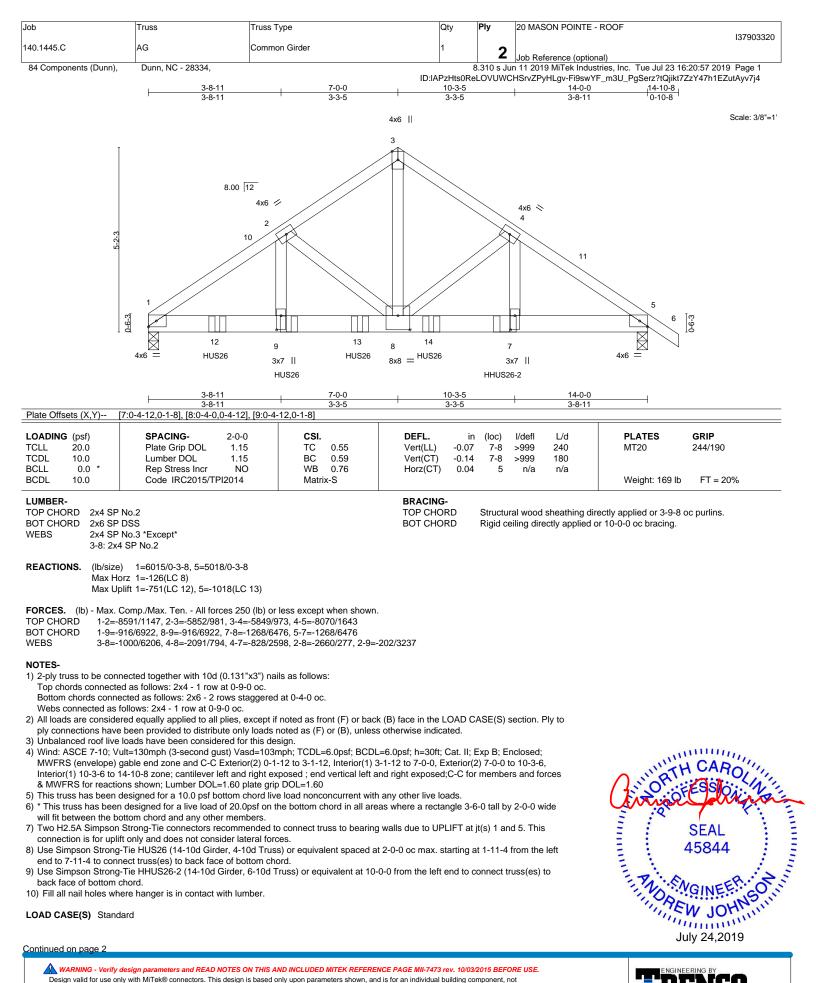
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.





818 Soundside Road Edenton, NC 27932

REACTIONS. All bearings 14-0-0. (lb) -



Design valid for use only with with exe connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss we band/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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Job	Truss	Truss Type	Qty	Ply	20 MASON POINTE - ROOF
					137903320
140.1445.C	AG	Common Girder	1	2	
				_	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			8.310 s Jur	11 2019 MiTek Industries, Inc. Tue Jul 23 16:20:58 2019 Page 2

ID:IAPzHts0ReLOVUWCHSrvZPyHLgv-jujE8uGcXNcr1q1rPhW6zxFvdXvCHXNqGuJRQdyv7j3

LOAD CASE(S) Standard

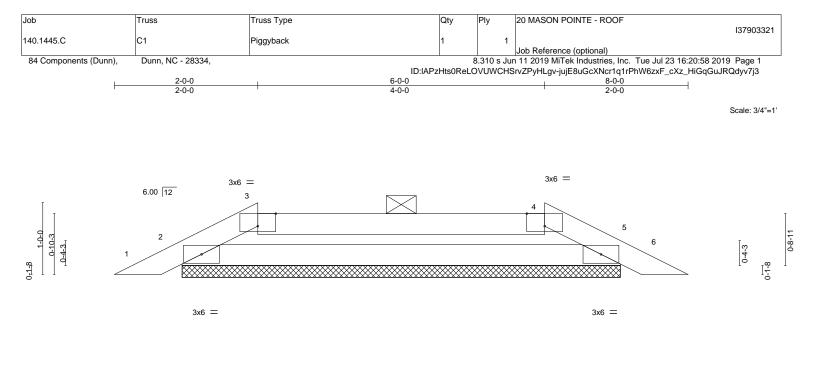
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-6=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 7=-2520(B) 9=-1837(B) 12=-1837(B) 13=-1837(B) 14=-1843(B)





	L		8-0-0		1		
			8-0-0		1		
Plate Offsets (X,Y)	[3:0-3-0,Edge], [4:0-3-0,Edge]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.35 WB 0.00 Matrix-R	DEFL. ii Vert(LL) 0.00 Vert(CT) 0.07 Horz(CT) 0.00	6 n/r 90	PLATES GRIP MT20 244/190 Weight: 22 lb FT = 20%		
SCDL 10.0	Code IRC2013/1FI2014	IVIAUIX-R			Weight. 22 lb FT = 20%		
	P No.2 P No.2		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins 2-0-0 oc purlins (6-0-0 max.): 3-4.				
REACTIONS. (Ib/siz			BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.				

Max Horz 2=13(LC 12) Max Uplift 2=-31(LC 9), 5=-31(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-476/265, 3-4=-439/249, 4-5=-476/265

BOT CHORD 2-5=-207/439

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

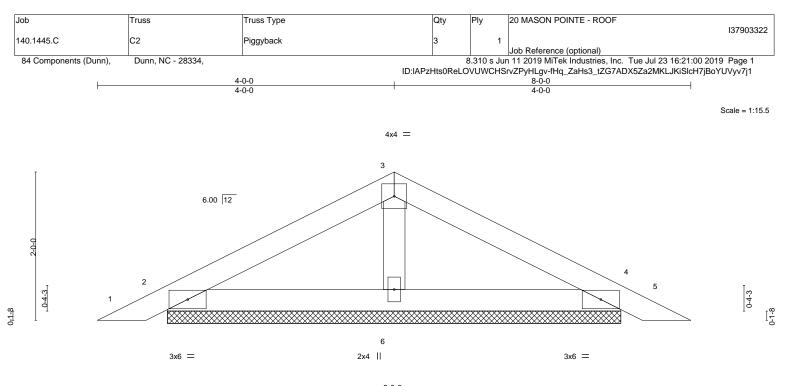
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) n/a

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







			8-0-0 8-0-0						—
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.16	Vert(LL)	0.00	5	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT)	0.01	5	n/r	90		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT)	0.00	4	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 24 lb	FT = 20%

TOP CHORD

BOT CHORD

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LUMBER-
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TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 2=164/6-1-6, 4=164/6-1-6, 6=234/6-1-6 Max Horz 2=31(LC 12) Max Uplift 2=-44(LC 12), 4=-50(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

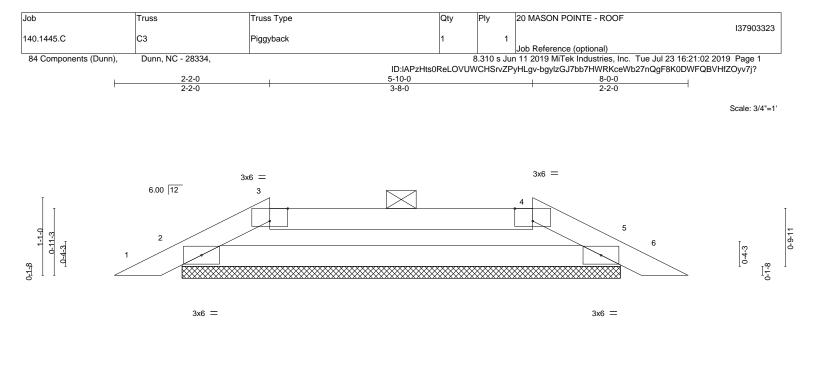
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) n/a
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



			8-0-0						-
			8-0-0						
Plate Offsets (X,Y)	[3:0-3-0,Edge], [4:0-3-0,Edge]								
LOADING (psf) FCLL 20.0 FCDL 10.0 SCLL 0.0 * SCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.19 BC 0.34 WB 0.00 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.01 0.00	(loc) 6 5	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 22 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x4 S REACTIONS. (Ib/si Max	SP No.2 SP No.2 ze) 2=282/6-1-6, 5=282/6-1-6 Horz 2=-14(LC 13) Uplift 2=-28(LC 9), 5=-28(LC 8)		BRACING- TOP CHORI BOT CHORI		2-0-0 o	c purlins	(6-0-0 max.):	ectly applied or 6-0-0 3-4. or 10-0-0 oc bracing.) oc purlins, except
FOP CHORD 2-3	k. Comp./Max. Ten All forces 250 (lb) oi =-441/249, 3-4=-401/234, 4-5=-441/249 =-187/401	less except when shown.							
NOTES-	ve loads have been considered for this de	sign.							

8.0.0

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

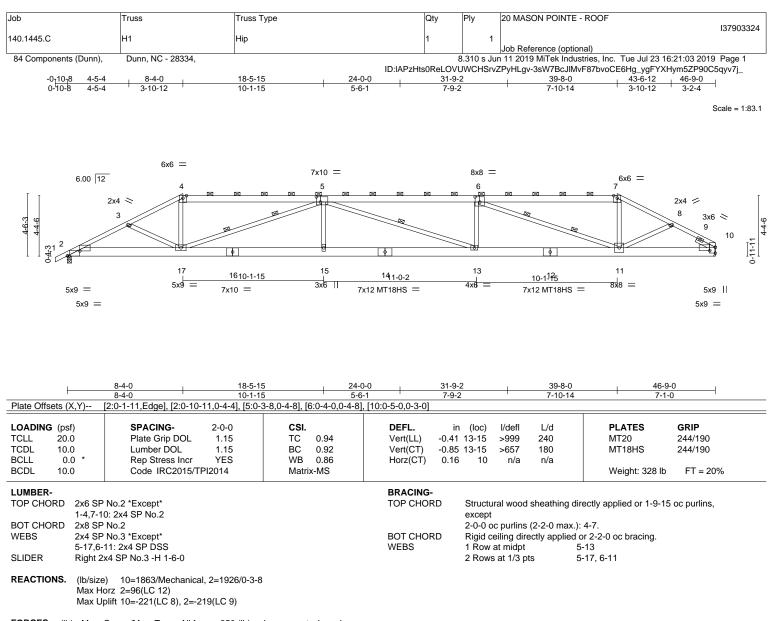
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) n/a

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-3794/820, 3-4=-3624/757, 4-5=-3201/711, 5-6=-5432/1157, 6-7=-2806/631, 7-8=-3162/664, 8-10=-3114/684

 BOT CHORD
 2-17=-695/3362, 15-17=-1052/5573, 13-15=-1052/5573, 11-13=-1022/5414, 10-11=-550/2671

WEBS 4-17=-147/1193, 5-15=0/420, 5-17=-2615/528, 6-13=0/483, 6-11=-2847/575, 7-11=-101/962, 8-11=-62/319

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) All plates are MT20 plates unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) All bearings are assumed to be User Defined crushing capacity of 425 psi.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=221.

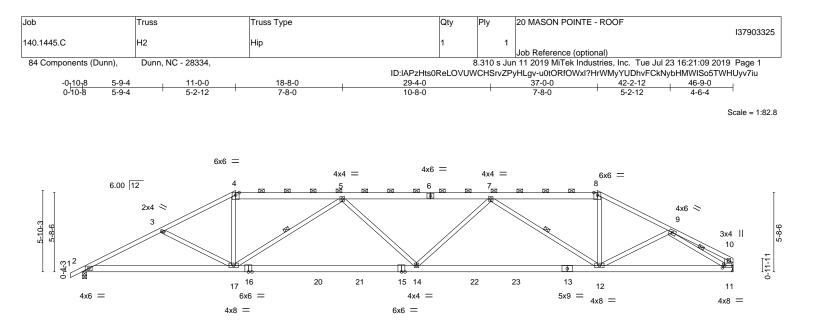
One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

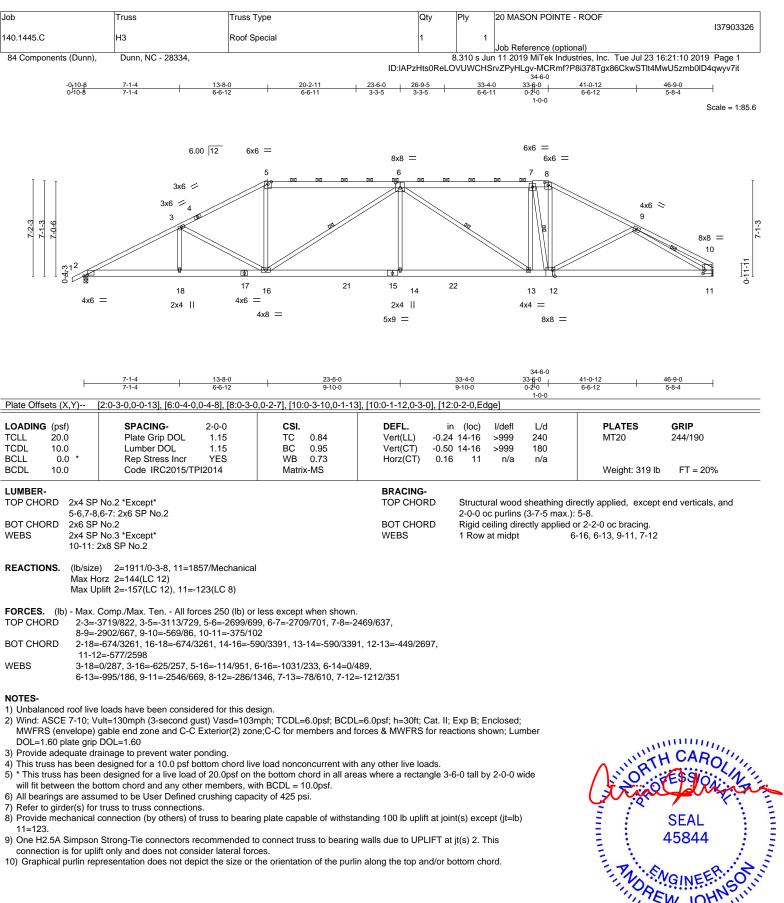
818 Soundside Road Edenton, NC 27932



5	-9-4 11-0-0 -9-4 5-2-12	24-0-0 13-0-0	<u> </u>	46-9-0 9-9-0
Plate Offsets (X,Y)	[11:0-2-0,0-2-0]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.71 BC 0.88 WB 0.75 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) -0.29 14-17 >999 240 Vert(CT) -0.62 14-17 >900 180 Horz(CT) 0.14 11 n/a n/a	MT20 244/190
4-6,6-8 BOT CHORD 2x6 SF 15-16: WEBS 2x4 SF	P No.2 *Except* 5: 2x6 SP No.2 P DSS *Except* 2x6 SP No.2 P No.3 *Except* 2x8 SP No.2	<u> </u>	except end verticals, a	hing directly applied or 2-7-11 oc purlins, Ind 2-0-0 oc purlins (2-8-12 max.): 4-8. pplied or 8-2-15 oc bracing. 5-17, 7-12, 9-11
Max H	e) 2=1911/0-3-8, 11=1857/Mechanica lorz 2=122(LC 12) plift 2=-170(LC 9), 11=-174(LC 8)	I		
TOP CHORD 2-3=- 8-9=- 8-9=- BOT CHORD 2-17= WEBS 3-17=	Comp./Max. Ten All forces 250 (lb) or 3697/838, 3-4=-3383/726, 4-5=-2949/64 -3017/652, 9-10=-458/101, 10-11=-302/5 -706/3275, 14-17=-776/4019, 12-14=-7 341/240, 4-17=-164/1169, 5-17=-1391 1591/390, 8-12=-130/996, 9-12=-37/3	37, 5-7=-4089/872, 7-8=-26 33 49/3886, 11-12=-541/2521 /376, 5-14=0/313, 7-14=0/		
 Wind: ASCE 7-10; WMWFRS (envelope) DOL=1.60 plate grip Provide adequate di This truss has been This truss has been * This truss has bee All bearings are ass Refer to girder(s) for Provide mechanical 11=174. One H2.5A Simpsor connection is for upl 	gable end zone and C-Č Exterior(2) zon p DOL=1.60 rainage to prevent water ponding. designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on pottom chord and any other members, w umed to be User Defined crushing capa r truss to truss connections.	mph; TCDL=6.0psf; BCDL e;C-C for members and for e load nonconcurrent with the bottom chord in all area ith BCDL = 10.0psf. city of 425 psi. Ing plate capable of withstar o connect truss to bearing ces.	as where a rectangle 3-6-0 tall by 2-0-0 wide nding 100 lb uplift at joint(s) except (jt=lb) walls due to UPLIFT at jt(s) 2. This	SEAL 45844

- s) of truss to bearing plate capable of withstanding 100 lb uplift at jease in the second se 8) 11=174.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

JOHN JOHN July 24,2019 818 Soundside Road Edenton, NC 27932

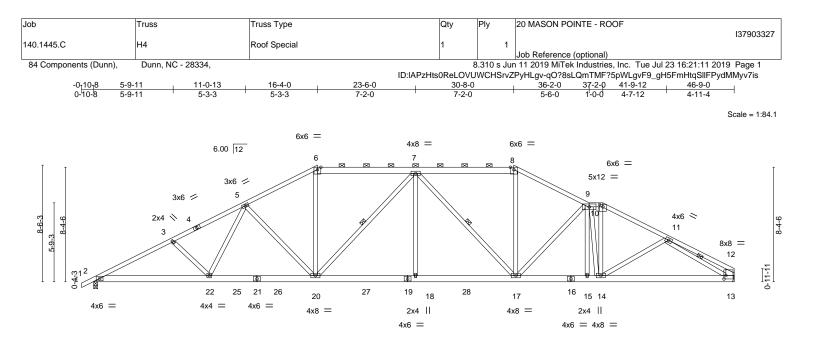


10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

EW JOHN "Innin July 24,2019



Edenton, NC 27932



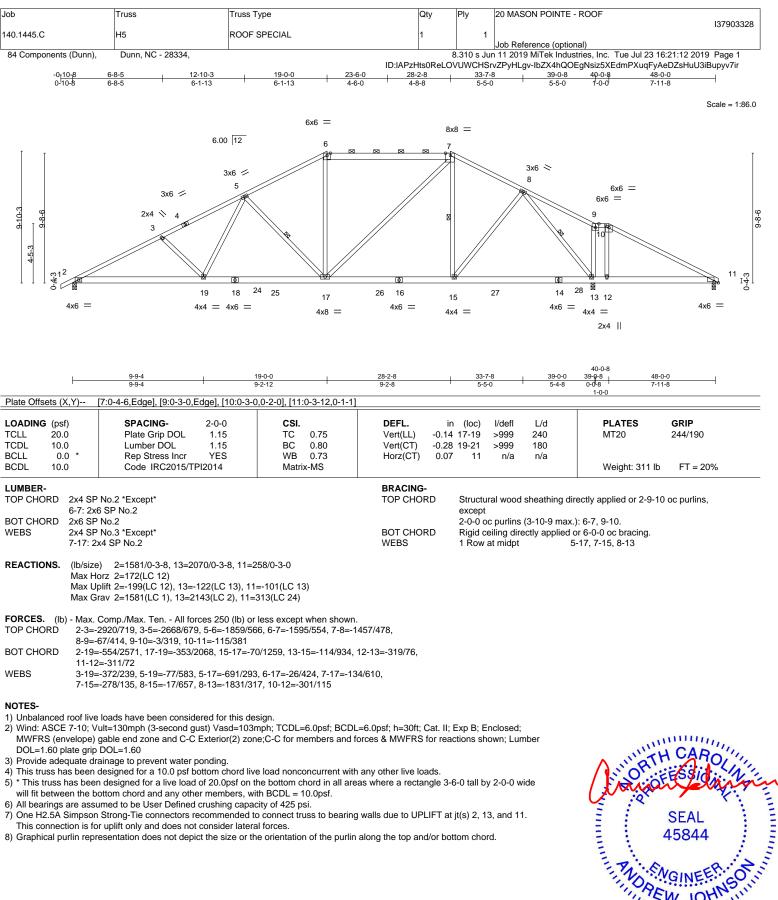
L	8-5-4	16-4-0		23-6-0		30-8-0				37-2-0	46-9-0		
Plate Offsets (X,Y)	8-5-4 [10:0-3-0,0-2-0], [12:0-3	<u>7-10-12</u> 3-10.0-1-13]. [12:0)-1-12.0-3-(7-2-0		7-2-0		5-6	6-0	1 ¹ -0-0	9-7-0	•	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/	2-0-0 1.15 1.15 YES	CSI. TC BC WB Matrix	0.69 0.87 0.62	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.21 1 -0.42 1 0.14	8-20	l/defl >999 >999 n/a	L/d 240 180 n/a		PLATES MT20 Weight: 338 lb	GRIP 244/190 FT = 20%	
BOT CHORD 2x6 SP WEBS 2x4 SP	6 SP No.2				BRACING TOP CHC BOT CHC WEBS	RD S e RD F	except e	end vertica	als, and	2-0-0 oc ed or 8-	/ applied or 2-6-0 ; purlins (3-5-9 ma 7-1 oc bracing. 7-17, 11-13		
Max He	e) 2=1911/0-3-8, 13= orz 2=167(LC 12) plift 2=-184(LC 12), 13=												
8-9=- BOT CHORD 2-22= 14-19 WEBS 3-22= 7-18=	Comp./Max. Ten All f 3723/861, 3-5=-3510/8: 2775/717, 9-10=-2619// -725/3295, 20-22=-555 5=-510/2757, 13-14=-5/ -314/203, 5-22=-65/50/ -0/378, 7-17=-708/148, 3=-2566/639, 9-14=-884	27, 5-6=-2824/732 661, 10-11=-2963 5/2870, 18-20=-45 67/2534 0, 5-20=-583/250, 8-17=-142/888, 9	2, 6-7=-246 /692, 11-12 1/2775, 17- 6-20=-168	3/692, 7-8=-24 2=-503/102, 12 -18=-451/2775 /955, 7-20=-61	-13=-339/104 5, 15-17=-511/2 8/156,	753,							
DOL=1.60 plate grip 3) Provide adequate dr. 4) This truss has been 5) * This truss has been will fit between the b 6) All bearings are assu 7) Refer to girder(s) for 8) Provide mechanical 13=160. 9) One H2.5A Simpson	fult=130mph (3-second gable end zone and C- DOL=1.60 ainage to prevent wate designed for a 10.0 psf n designed for a live loa ottom chord and any ot umed to be User Define truss to truss connection connection (by others) a Strong-Tie connectors ift only and does not co	gust) Vasd=103m C Exterior(2) zone r ponding. bottom chord live d of 20.0psf on th her members, witi d crushing capacions. of truss to bearing recommended to nsider lateral force	ph; TCDL= e;C-C for m load noncc e bottom cl h BCDL = 1 ity of 425 ps g plate capa connect trues.	embers and fo poncurrent with hord in all area 0.0psf. si. ble of withstar uss to bearing	rces & MWFRS any other live lo is where a recta nding 100 lb upl walls due to UF	ads. angle 3-6-0 ift at joint(s PLIFT at jt(s) tall by () excep (s) 2. Th	wn; Lumł 2-0-0 wic ot (jt=lb) is		ī	The	SEAL 45844	In Summannin

- ig þ ar ngʻ up 11(S) 13=160
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



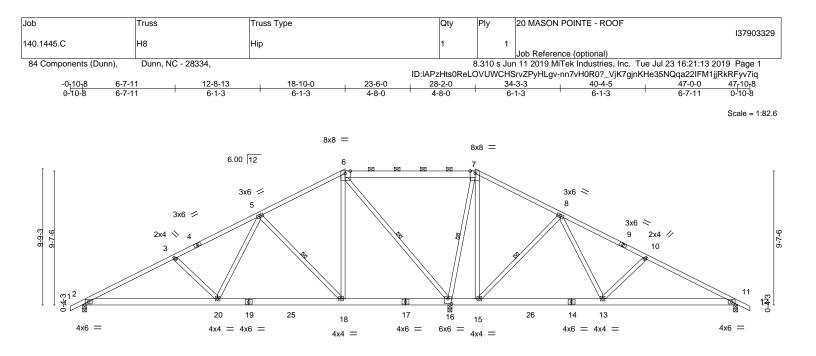
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July 24,2019





818 Soundside Road Edenton, NC 27932



—	9-8-4 9-8-4	<u>18-1</u> 9-1-		26-4-4 7-6-4	28-2-0	<u>37-3-12</u> 9-1-12	47-0-0			
Plate Offsets (X,Y)	9-8-4 [6:0-4-6,Edge], [7:0-4-6,E			7-0-4	1-9-12	9-1-12	9-0-4			
			• · ·]							
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d		GRIP		
TCLL 20.0	Plate Grip DOL	1.15	TC 0.75	Vert(LL)	0.14 20-22	>999 240		244/190		
TCDL 10.0	Lumber DOL	1.15	BC 0.51	Vert(CT)	-0.17 20-22	>999 180				
BCLL 0.0 *	Rep Stress Incr	YES	WB 1.00	Horz(CT	0.02 16	n/a n/a				
BCDL 10.0	Code IRC2015/TF	PI2014	Matrix-MS				Weight: 312 lb	FT = 20%		
LUMBER-				BRACIN	G-					
TOP CHORD 2x4 SF	No.2 *Except*			TOP CH	ORD Struct	ural wood sheat	thing directly applied or 4-7-3	oc purlins, except		
	6 SP No.2					oc purlins (10-0				
BOT CHORD 2x6 SF				BOT CHO	- J.		applied or 6-0-0 oc bracing.			
WEBS 2x4 SF	P No.3			WEBS	1 Row	at midpt	5-18, 6-16, 7-16, 8-15			
Max H Max U Max G FORCES. (Ib) - Max. TOP CHORD 2-3= 8-10:	8-10=-440/214, 10-11=-690/260 BOT CHORD 2-20=-534/1141, 18-20=-167/636, 16-18=-88/293, 15-16=-632/667, 13-15=-285/276,									
	=-369/258, 5-20=-488/591 =-1283/811, 7-15=-434/52	,	, ,	,						
 Wind: ASCE 7-10; MWFRS (envelope) for reactions shown Provide adequate d This truss has been * This truss has been 	a loads have been consid /ult=130mph (3-second g gable end zone and C-C ; Lumber DOL=1.60 plate rainage to prevent water p designed for a 10.0 pst n designed for a live load pottom chord and any othe	ust) Vasd=103m Exterior(2) zone grip DOL=1.60 ponding. ottom chord live of 20.0psf on the	b); TCDL=6.0psf; B ; porch left and right load nonconcurrent e bottom chord in all	exposed;C-C for m	embers and forco	ces & MWFRS	Anna Print	H CAROLINI		

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

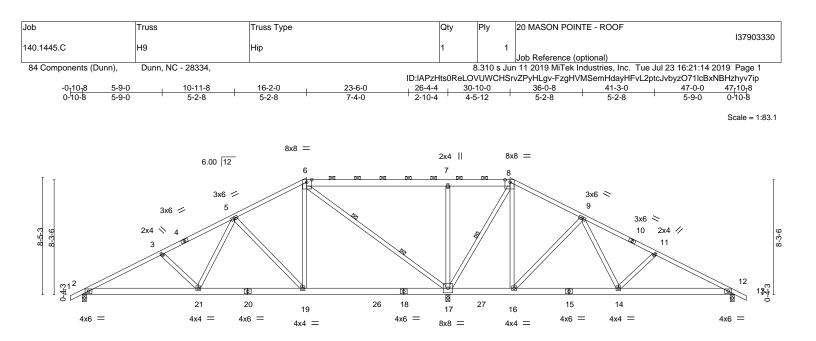
7) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16. This connection is for uplift only and does not consider lateral forces.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







 	8-4-4 16-2-0 8-4-4 7-9-12	<u>21-3-2</u> 5-1-2	<u>26-4-4</u> <u>30-10-0</u> <u>38-7-12</u> 5-1-2 <u>4-5-12</u> <u>7-9-12</u>	47-0-0
Plate Offsets (X,Y)	[6:0-4-6,Edge], [8:0-4-6,Edge]	5-1-2	<u> </u>	0-4-4
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.45 WB 0.81 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) 0.10 21-23 >999 240 Vert(CT) -0.15 17-19 >999 180 Horz(CT) 0.02 12 n/a n/a	PLATES GRIP MT20 244/190 Weight: 315 lb FT = 20%
6-8: 25 BOT CHORD 2x6 SF WEBS 2x4 SF	P No.2 *Except* x6 SP No.2 P No.2 P No.3 *Except* 2x4 SP No.2		BRACING- TOP CHORD Structural wood sheathin 2-0-0 oc purlins (10-0-0 BOT CHORD Rigid ceiling directly appl WEBS 1 Row at midpt 2 Rows at 1/3 pts	
Max H Max L	e) 2=820/0-3-8, 17=2533/(0-3-8 + H10 lorz 2=-141(LC 13) Jplift 2=-200(LC 9), 17=-677(LC 9), 12=- Grav 2=879(LC 23), 17=2533(LC 1), 12=	131(LC 13)	req. 0-4-0), 12=512/0-3-8	
TOP CHORD 2-3=	. Comp./Max. Ten All forces 250 (lb) ol -1409/799, 3-5=-1196/759, 5-6=-523/34 -159/466, 9-11=-544/317, 11-12=-754/3	I, 6-7=-425/875, 7-8=-424		
BOT CHORD 2-21 WEBS 3-21 7-17	=-619/1227, 19-21=-292/802, 17-19=-10 =-619/1223, 5-21=-401/476, 5-19=-586/4 =-573/264, 8-17=-1010/678, 8-16=-433/- 4=-310/218	1/448, 16-17=-376/458, 1 55, 6-19=-526/665, 6-17≕	-1451/958,	
 Wind: ASCE 7-10; MWFRS (envelope) for reactions shown Provide adequate d This truss has been * This truss has been) gable end zone and C-C Exterior(2) zon ; Lumber DOL=1.60 plate grip DOL=1.60 rainage to prevent water ponding. designed for a 10.0 psf bottom chord liv	mph; TCDL=6.0psf; BCDI he; porch left and right exp) e load nonconcurrent with he bottom chord in all are	=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; bosed;C-C for members and forces & MWFRS in any other live loads. has where a rectangle 3-6-0 tall by 2-0-0 wide	ALLOP TH CAROL

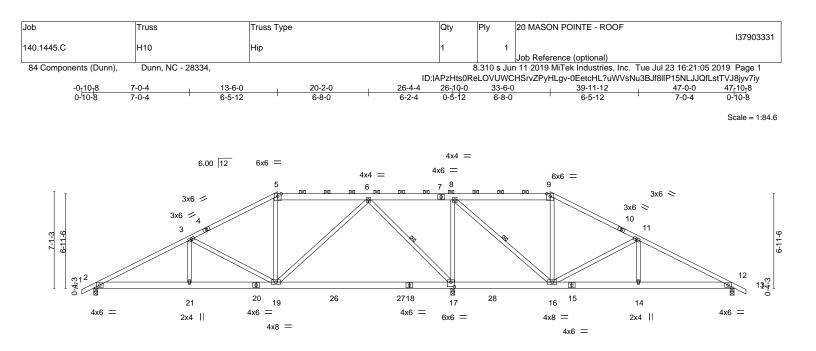
 All bearings are assumed to be User Defined crushing capacity of 425 psi.
 H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17. This connection is for uplift only and does not consider lateral forces.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

The manual of the second Younner Press SEAL 45844 EW JOY "HILLING July 24,2019





<u> </u>	7-0-4 13-6-0	20-2-0	26-4-4	33-6-0	39-11-12	47-0-0				
Plate Offsets (X,Y)	7-0-4 6-5-12 [17:0-3-0,0-4-0]	6-8-0	6-2-4	7-1-12	6-5-12	7-0-4				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.52 BC 0.56 WB 0.94	Vert(CT) -0.	in (loc) l/defl 20 17-19 >999 36 17-19 >883 02 12 n/a		PLATES GRIP MT20 244/190				
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MS			1	Weight: 302 lb FT = 20%				
BOT CHORD 2x6 SF WEBS 2x4 SF			BOT CHORD WEBS		(6-0-0 max.): 5-9. ctly applied or 6-0-0 6-17. 8-1					
Max U	e) 17=2391/(0-3-8 + H10A Simpson S lorz 2=119(LC 16) plift 17=-735(LC 9), 12=-146(LC 13), 2= irav 17=2391(LC 1), 12=643(LC 24), 2=	=-227(LC 9)								

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1489/893, 3-5=-890/577, 5-6=-718/560, 6-8=-398/826, 11-12=-889/491
- BOT CHORD 2-21=-683/1269, 19-21=-683/1269, 16-17=-826/702, 14-16=-330/733, 12-14=-330/733
- WEBS 3-19=-636/489, 6-19=-559/785, 6-17=-1304/790, 8-17=-1198/687, 8-16=-729/1142,
 - 9-16=-352/129, 11-16=-696/551, 11-14=-210/329

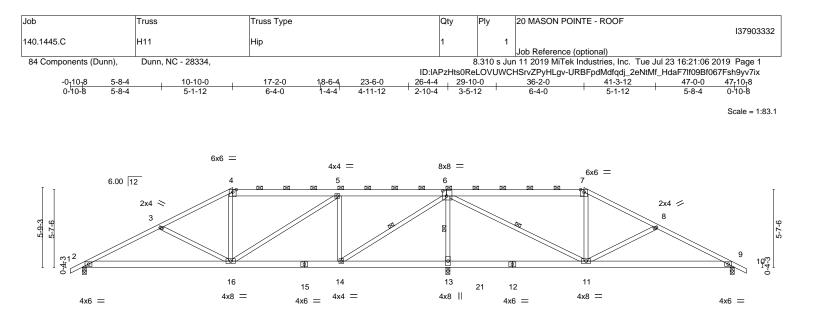
NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17. This connection is for uplift only and does not consider lateral forces.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12 and 2. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







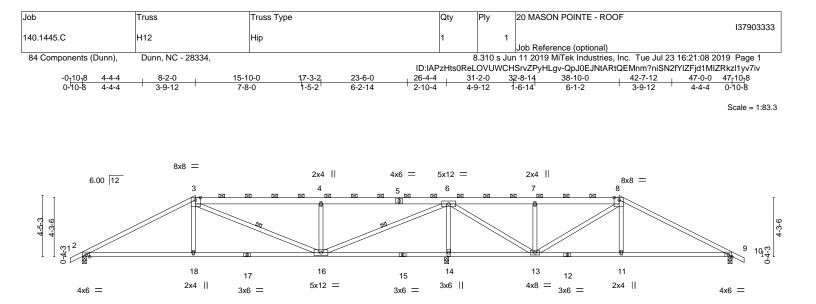
	3-6-4 26-4-4	36-2-0	41-3-12 47-0-0
	-8-4 7-10-0	9-9-12	5-1-12 5-8-4
LOADING (psf) SPACING- 2-0-0 TCLL 20.0 Plate Grip DOL 1.15 TCDL 10.0 Lumber DOL 1.15 BCLL 0.0 * Rep Stress Incr YES BCDL 10.0 Code IRC2015/TPI2014	BC 0.53 Ver	FL. in (loc) l/defl L/d t(LL) 0.18 16-18 >999 240 t(CT) -0.24 16-18 >999 180 z(CT) 0.02 9 n/a n/a	PLATES GRIP MT20 244/190 Weight: 296 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 *Except* 4-6,6-7: 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 REACTIONS. (lb/size) 2=962/0-3-8, 13=2210/0-3-8, 9=693 Max Horz 2=-96(LC 13) Max Uplift 2=-266(LC 9), 13=-762(LC 9), 9=-18 Max Grav 2=978(LC 23), 13=2210(LC 1), 9=72 FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or TOP CHORD 2-3=-1599/979, 3-4=-1264/845, 4-5=-1069/79 7.9 = 690/464, 8.9 = 1045/699	TOF BOT WEI 20(LC 8) 20(LC 24) less except when shown.	2-0-0 oc purlins (6-0-0 maz CHORD Rigid ceiling directly applie	
7-8=-680/464, 8-9=-1015/598 BOT CHORD 2-16=-789/1404, 14-16=-285/713, 13-14=-57 WEBS 3-16=-379/275, 4-16=-209/294, 5-16=-307/45 6-13=-2013/1178, 6-11=-812/1252, 8-11=-38	0, 5-14=-711/383, 6-14=-933/1483,		
 NOTES- Uhbalanced roof live loads have been considered for this details and the second gust) Vasd=103r MWFRS (envelope) gable end zone and C-C Exterior(2) zon for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Provide adequate drainage to prevent water ponding. This truss has been designed for a 10.0 psf bottom chord live 5) * This truss has been designed for a live load of 20.0psf on the will fit between the bottom chord and any other members, wiil All bearings are assumed to be User Defined crushing capacity One H2.5A Simpson Strong-Tie connectors recommended to connection is for uplift only and does not consider lateral ford 8) Two H2.5A Simpson Strong-Tie connectors recommended to connection is for uplift only and does not consider lateral ford 9) Graphical purlin representation does not depict the size or the si	sign. nph; TCDL=6.0psf; BCDL=6.0psf; h e; porch left and right exposed;C-C e load nonconcurrent with any other ne bottom chord in all areas where a th BCDL = 10.0psf. ity of 425 psi. o connect truss to bearing walls due tes. o connect truss to bearing walls due tes.	for members and forces & MWFRS live loads. a rectangle 3-6-0 tall by 2-0-0 wide to UPLIFT at jt(s) 2 and 9. This to UPLIFT at jt(s) 13. This	SEAL 45844

- connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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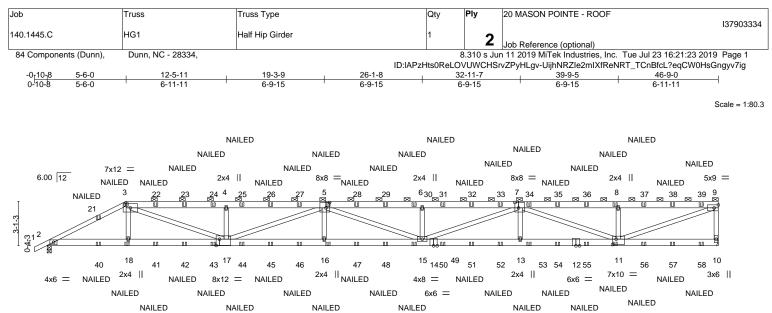
July 24,2019



L	8-2-0	17-3-2		26-4-4	32-8-14	38-10-0	47-0-0	
	8-2-0	9-1-2	· · ·	9-1-2	6-4-10	6-1-2	8-2-0	
Plate Offsets (X,	,Y) [3:0-4-6,Edge], [8:0	-4-6,Edgej		1				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	Plate Grip D Lumber DOL * Rep Stress I	. 1.15 ncr YES	CSI. TC 0.83 BC 0.84 WB 0.86 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/d 0.22 11-24 >99 -0.28 16-18 >99 0.03 9 r	99 240	PLATES MT20 Weight: 248 lb	GRIP 244/190 FT = 20%
BOT CHORD	2x4 SP No.1 *Except* 3-5,5-8: 2x6 SP No.2 2x4 SP No.2 2x4 SP No.3 *Except* 8-13: 2x4 SP No.2			BRACING- TOP CHOF BOT CHOF WEBS	D Structural w 2-0-0 oc pu	vood sheathing direct rlins (5-10-6 max.): 3 g directly applied or 6 idpt 3-16	-8.	oc purlins, except
	(lb/size) 2=994/0-3-8, 14 Max Horz 2=-74(LC 13) Max Uplift 2=-294(LC 9), 7 Max Grav 2=999(LC 23),	14=-812(LC 9), 9=-19	4(LC 8)					
FORCES. (Ib) TOP CHORD BOT CHORD	- Max. Comp./Max. Ten 2-3=-1595/1005, 3-4=-12 8-9=-974/614 2-18=-776/1348, 16-18=- 9-11=-431/791	270/892, 4-6=-1265/8	88, 6-7=-401/351, 7-8=-	405/354,				
WEBS	3-18=-227/371, 4-16=-61 8-11=-209/329, 7-13=-34			8-13=-473/312,				
 Wind: ASCE MWFRS (env for reactions s Provide adeq This truss has * This truss has will fit betwee All bearings a One H2.5A Si connection is Two H2.5A Si 	oof live loads have been c 7-10; Vult=130mph (3-sect relope) gable end zone and shown; Lumber DOL=1.60 uate drainage to prevent w s been designed for a 10.0 as been designed for a live n the bottom chord and an are assumed to be User De impson Strong-Tie connec for uplift only and does no impson Strong-Tie connec for uplift only and does no	and gust) Vasd=103n d C-C Exterior(2) zon- plate grip DOL=1.60 vater ponding. psf bottom chord live e load of 20.0psf on th y other members. fined crushing capac tors recommended to t consider lateral forc tors recommended to	ph; TCDL=6.0psf; BCD e; porch left and right ex e load nonconcurrent wit he bottom chord in all ar ity of 425 psi. o connect truss to bearin es.	posed;C-C for mer h any other live loa eas where a rectar g walls due to UPL	nbers and forces & ids. igle 3-6-0 tall by 2-0 IFT at jt(s) 2 and 9.	MWFRS 0-0 wide This	•	SEAL 45844

connection is for uplift only and does not consider lateral forces. 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. SEAL 45844





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5-6-0	<u> </u>		19-3-9 6-9-15	26-1-8 6-9-15	32-11-7 6-9-15	39-9-5 6-9-15	46-9- 6-11-	
Plate Offsets (X,Y)	[5:0-3-0,0-4-8], [7:0-3-0	,0-4-8], [11:0-3	3-0,0-4-8], [17:0-2-12	,0-5-4]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/ ⁷	2-0-0 1.15 1.15 NO FPI2014	CSI. TC 0.47 BC 0.98 WB 0.78 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT	in (loc) l/defl 0.74 15-16 >758 -1.13 15-16 >494 0.16 10 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 601 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x6 14-1 WEBS 2x4	2x4 SP No.2	2x4 SP No.2		BRACIN TOP CHO BOT CHO	ORD Structural woo except end ve	rticals, and 2-0-0	ctly applied or 5-1-15 oc purlins (4-3-11 m 8-9-6 oc bracing.	
Max	ize) 10=2540/Mechanic Horz 2=119(LC 12) Uplift 10=-892(LC 9), 2=-		-8					
TOP CHORD 2-3 7-8 7-8 BOT CHORD 2-1 13 3-1 WEBS 3-1 6-1 6-1	x. Comp./Max. Ten All fi i=-5257/1779, 3-4=-8817/3 i=-5702/2039, 8-9=-5702/2 8=-1632/4647, 17-18=-16 :15=-3357/9380, 11-13=-3 8=0/452, 3-17=-1653/449 5=-521/368, 7-15=-627/17 1=-2148/6009	3148, 4-5=-88 2039, 9-10=-24 29/4665, 16-1 353/9389 2, 4-17=-658/4	15/3145, 5-6=-11047/ 426/943 7=-3909/10962, 15-1 442, 5-17=-2311/823,	/3941, 6-7=-11047/394 6=-3914/10952, 5-16=0/396,	:1,			
Top chords conne Bottom chords co Webs connected : 2) All loads are cons ply connections h: 3) Unbalanced roof I 4) Wind: ASCE 7-10 MWFRS (envelop DOL=1.60 plate g 5) Provide adequate 6) This truss has be will fit between the 8) All bearings are a 9) Refer to grider(s) 10) Provide mechan 10=892. 11) Two H2.5A Simp	onnected together with 10 icted as follows: 2x4 - 1 ro nnected as follows: 2x6 - 2 as follows: 2x4 - 1 row at 0 idered equally applied to a ave been provided to distri- ive loads have been consis (> Vult=130mph (3-second e) gable end zone and C-1 rip DOL=1.60 drainage to prevent water en designed for a 10.0 psf seen designed for a live loa a bottom chord and any ot ssumed to be User Define for truss to truss connectio ical connection (by others) oson Strong-Tie connector uplift only and does not con	w at 0-9-0 oc, Prows stagger -9-0 oc. Ill plies, excep bute only load dered for this gust) Vasd=10 C Exterior(2) z ponding. bottom chord d of 20.0psf on her members. d crushing cap ins. of truss to be s recommende	2x6 - 2 rows staggere ed at 0-9-0 oc. t if noted as front (F) s noted as (F) or (B), design. 13mph; TCDL=6.0psf; one;C-C for members live load nonconcurrent in the bottom chord in pacity of 425 psi. aring plate capable of ed to connect truss to	or back (B) face in the unless otherwise indi ; BCDL=6.0psf; h=30ff s and forces & MWFR ent with any other live all areas where a rect f withstanding 100 lb u	cated. ; Cat. II; Exp B; Enclose S for reactions shown; L oads. angle 3-6-0 tall by 2-0-0 plift at joint(s) except (jt	d; umber wide	THE REAL	SEAL 45844 GINEER JOHNSON
Design valid for use a truss system. Be	rify design parameters and REAI e only with MiTek® connectors. T fore use, the building designer mu acing indicated is to prevent buck	his design is base ist verify the appli	d only upon parameters sh cability of design parameter	own, and is for an individual rs and properly incorporate	building component, not his design into the overall	2		

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	20 MASON POINTE - ROOF
					137903334
140.1445.C	HG1	Half Hip Girder	1	2	
				_	Job Reference (optional)
84 Components (Dunn),	Dunn, NC - 28334,			3.310 s Jur	11 2019 MiTek Industries, Inc. Tue Jul 23 16:21:23 2019 Page 2
		ID:IAPzH	Its0ReLO	VUWCHSr	vZPyHLgv-UijhNRZIe2mIXfReNRT_TCnBfcL?eqCW0HsGngyv7ig

NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

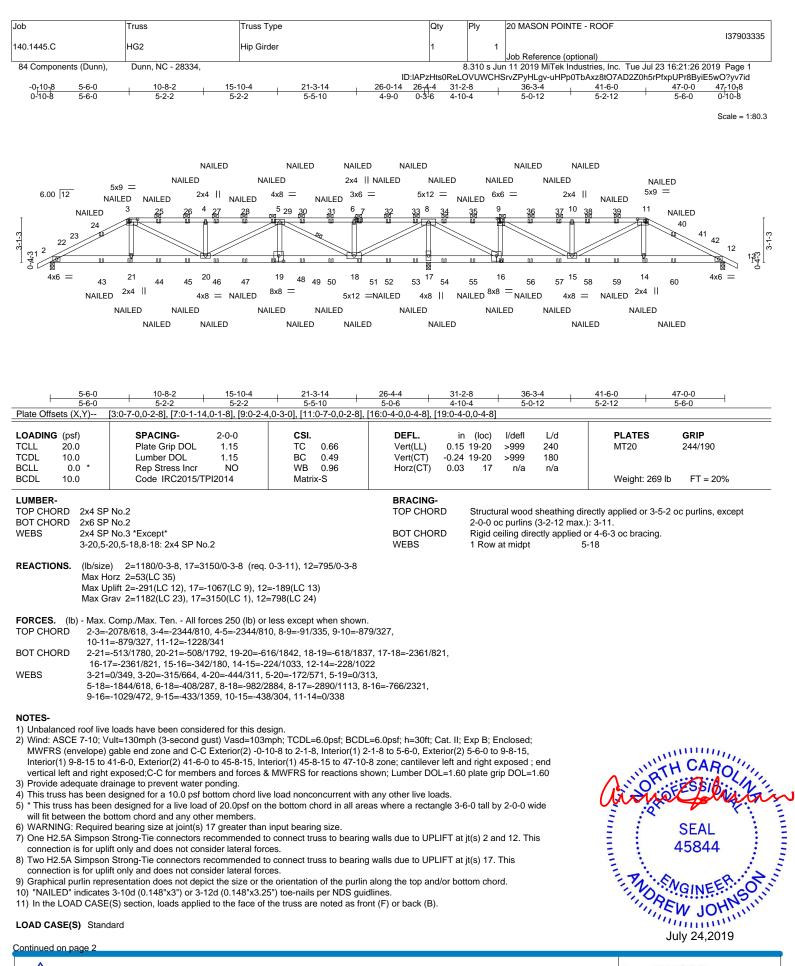
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-9=-60, 2-10=-20

Concentrated Loads (lb)

Vert: 3=-41(B) 18=-17(B) 5=-41(B) 16=-17(B) 8=-41(B) 11=-17(B) 21=-38(B) 22=-41(B) 23=-41(B) 24=-41(B) 25=-41(B) 26=-41(B) 27=-41(B) 28=-41(B) 29=-41(B) 30=-41(B) 31=-41(B) 31=-41(B) 32=-41(B) 32=





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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J	ob	Truss	Truss Type	Qty	Ply	20 MASON POINTE - ROOF
						137903335
1	40.1445.C	HG2	Hip Girder	1	1	
						Job Reference (optional)
	84 Components (Dunn),	Dunn, NC - 28334,			3.310 s Jur	11 2019 MiTek Industries, Inc. Tue Jul 23 16:21:26 2019 Page 2

8.310 s Jun 11 2019 MiTek Industries, Inc. Tue Jul 23 16:21:26 2019 Page 2 ID:IAPzHts0ReLOVUWCHSrvZPyHLgv-uHPp0TbAxz8t07AD2Z0h5rPfxpUPr8ByiE5wO?yv7id

LOAD CASE(S) Standard

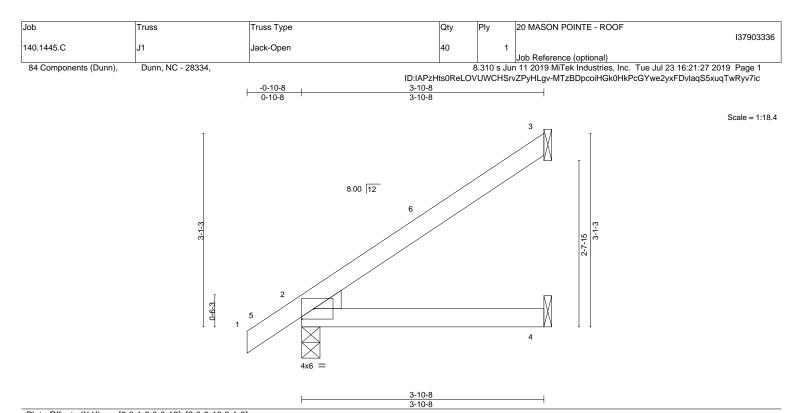
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-11=-60, 11-13=-60, 2-12=-20

Concentrated Loads (lb)

Vert: 3=-41(F) 7=-41(F) 11=-41(F) 21=-17(F) 16=-17(F) 9=-41(F) 14=-17(F) 24=-38(F) 25=-41(F) 26=-41(F) 27=-41(F) 28=-41(F) 29=-41(F) 30=-41(F) 31=-41(F) 32=-41(F) 32=-41(F) 33=-41(F) 35=-41(F) 35=





LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.24	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.16	Vert(CT)	-0.02	2-4	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P						Weight: 15 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEDGE Left: 2x4 SP No.3 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-10-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 3=101/Mechanical, 2=216/0-3-8, 4=37/Mechanical Max Horz 2=119(LC 12)

Max Uplift 3=-85(LC 12), 2=-13(LC 12) Max Grav 3=113(LC 19), 2=216(LC 1), 4=73(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

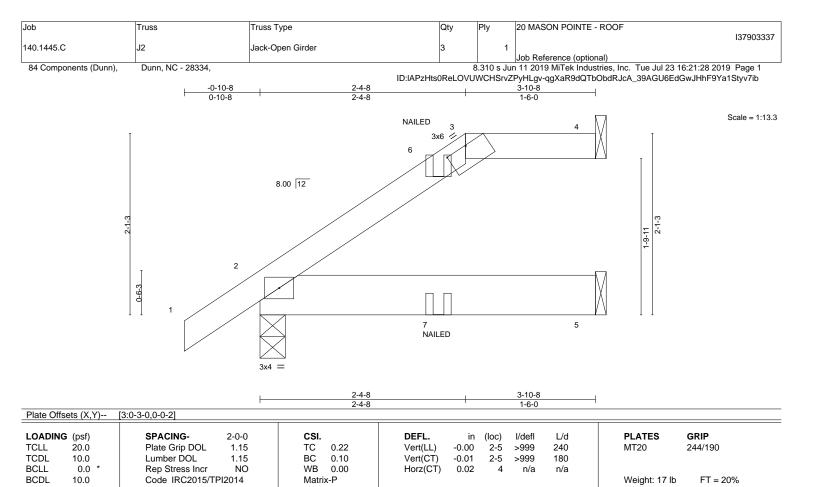
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-9-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.







TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 3-10-8 oc purlins, except 2-0-0 oc purlins: 3-4.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (lb/size) 4=98/Mechanical, 2=237/0-3-8, 5=62/Mechanical Max Horz 2=82(LC 12) Max Uplift 4=-45(LC 9), 2=-37(LC 12)

Max Grav 4=98(LC 1), 2=237(LC 1), 5=88(LC 3)

NOTES-

LUMBER-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
connection is for uplift only and does not consider lateral forces.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 2-5=-20 Concentrated Loads (lb)

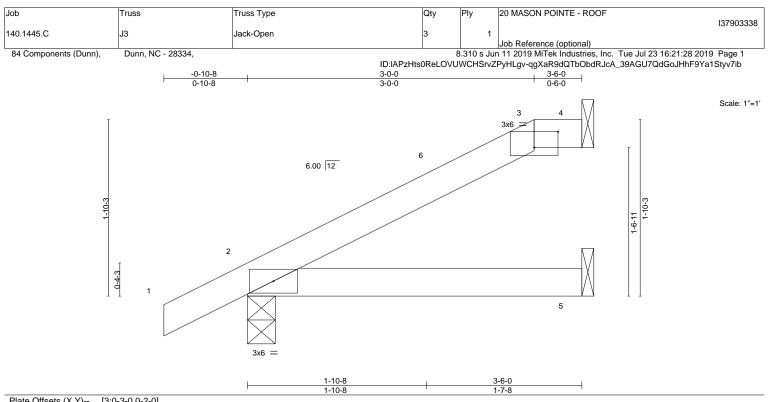
Vert: 6=-25(F) 7=-17(F)





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.15	Vert(LL) -0.01 2-5 >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.11	Vert(CT) -0.01 2-5 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 4 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 13 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

BRACING-TOP CHORD

 TOP CHORD
 Structural wood sheathing directly applied or 3-6-0 oc purlins, except 2-0-0 oc purlins: 3-4.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=85/Mechanical, 2=202/0-3-8, 5=37/Mechanical Max Horz 2=73(LC 12) Max Uplift 4=-39(LC 12), 2=-35(LC 12) Max Grav 4=85(LC 1), 2=202(LC 1), 5=61(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-0-0, Exterior(2) 3-0-0 to 3-5-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

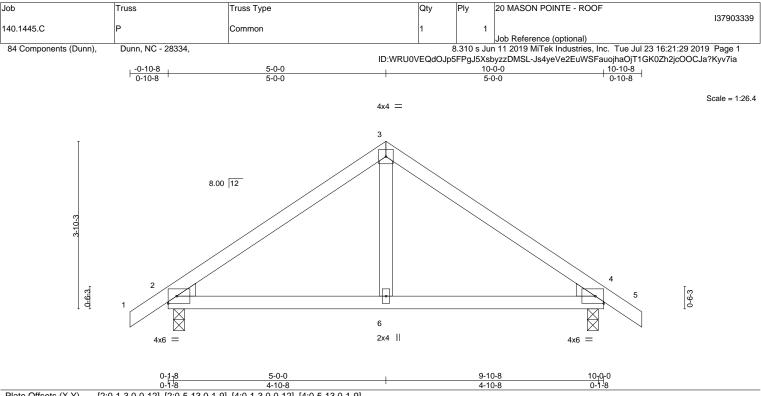
One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
connection is for uplift only and does not consider lateral forces.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.Ó	Plate Grip DOL	1.15	тс	0.26	Vert(LL)	-0.02	`6-9	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.26	Vert(CT)	-0.04	6-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.01	2	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matrix	x-MS						Weight: 43 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (Ib/size) 2=452/0-3-0, 4=452/0-3-0 Max Horz 2=-87(LC 10) Max Uplift 2=-75(LC 12), 4=-75(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-456/110, 3-4=-456/110

BOT CHORD 2-6=0/322, 4-6=0/322

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

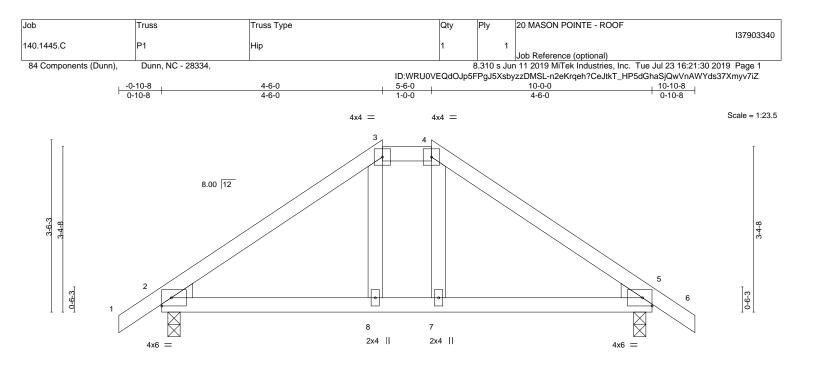
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.







	0- <u>1-8</u> 0-1-8	<u>4-6-0</u> 4-4-8		5-6-0 1-0-0			9-10- 4-4-8		<u>10₁0</u> 0 0-1-8	
Plate Offsets (X,Y)	[2:0-1-3,0-0-12], [2:0-5-13	3,0-1-9], [5:0-1-3,0-0-	12], [5:0-5-13,0-1-9]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	1.15 1.15 YES	CSI. TC 0.22 BC 0.22 WB 0.04 Matrix-MS	Vert(CT) -	in -0.02 -0.03 0.00	(loc) 7-14 7-14 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 46 lb	GRIP 244/190 FT = 20%
BOT CHORD 2x4 S	SP No.2 SP No.2 SP No.3 ight: 2v4 SP No.3			BRACING- TOP CHORD BOT CHORD		2-0-0 o	c purlins	(6-0-0 max.):	rectly applied or 6-0-0 3-4. or 10-0-0 oc bracing.	oc purlins, except

REACTIONS. (lb/size) 2=452/0-3-0, 5=452/0-3-0 Max Horz 2=-78(LC 10) Max Uplift 2=-75(LC 12), 5=-75(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-467/118, 3-4=-351/138, 4-5=-467/118

BOT CHORD 2-8=-2/340, 7-8=-2/336, 5-7=-2/340

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSIVTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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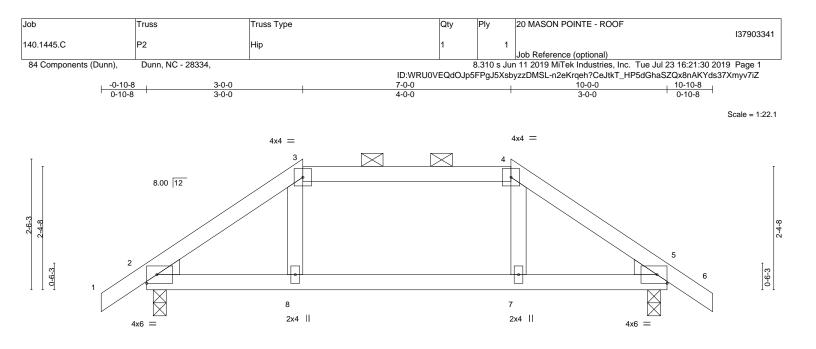


Plate Offsets (X,Y)	0- <u>1-8 3-0-0</u> 0-1-8 2-10-8 [2:0-1-3,0-0-12], [2:0-5-13,0-1-9], [5:0-1	-3,0-0-12], [5:0-5-13,0-1-9]	7-0-0 4-0-0		9-10-8 2-10-8	<u>10₁0</u> 0 0-1-8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.18 WB 0.06 Matrix-MS	DEFL. in Vert(LL) -0.02 Vert(CT) -0.03 Horz(CT) 0.01	(loc) l/defl 8 >999 7-8 >999 5 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 42 lb	GRIP 244/190 FT = 20%
Max H	No.2 No.3		BRACING- TOP CHORD BOT CHORD	2-0-0 oc purlins	(6-0-0 max.): 3-		oc purlins, except
TOP CHORD 2-3=-	Comp./Max. Ten All forces 250 (lb) o 525/140, 3-4=-408/147, 4-5=-525/140 48/413, 7-8=-51/408, 5-7=-48/413	r less except when shown.					
2) Wind: ASCE 7-10; V	loads have been considered for this de ult=130mph (3-second gust) Vasd=103 MWFRS (directional) and C-C Exterior(mph; TCDL=6.0psf; BCDL=					

Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

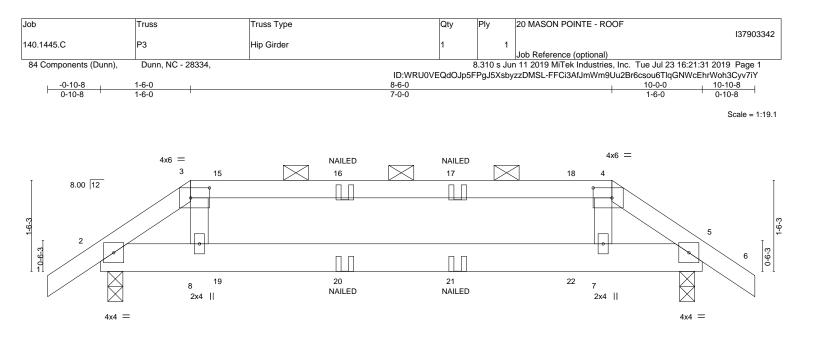
6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







0- <u>1-8</u> 0-1-8 Plate Offsets (X,Y)	1-6-0 1-4-8 [3:0-3-12,0-2-0], [4:0-3-12,0-2-0]		8-6-0 7-0-0		9-10-8 10 ₀ 0 1-4-8 0-1-8
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.84 BC 0.24 WB 0.08 Matrix-MS	DEFL. ir Vert(LL) -0.02 Vert(CT) -0.04 Horz(CT) 0.01	7-8 >999 180	PLATES GRIP MT20 244/190 Weight: 45 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP	No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing di 2-0-0 oc purlins (4-8-11 max Rigid ceiling directly applied	,
REACTIONS. (Ib/size Max Ho	e) 2=488/0-3-0, 5=488/0-3-0 orz 2=-36(LC 10)				

Max Uplift 2=-100(LC 12), 5=-100(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-713/186, 3-4=-646/198, 4-5=-713/186

BOT CHORD 2-8=-152/640, 7-8=-138/646, 5-7=-152/640

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 69 lb down and 58 lb up at 2-0-12, and 69 lb down and 58 lb up at 7-11-4 on top chord, and 25 lb down and 23 lb up at 2-0-12, and 25 lb down and 23 lb up at 7-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 4-6=-60, 9-12=-20

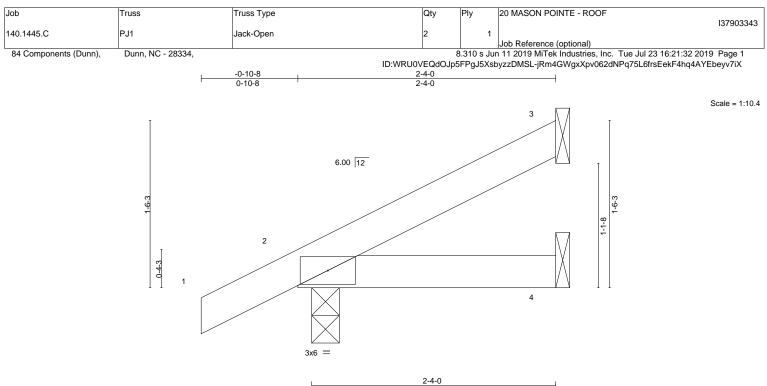
Concentrated Loads (lb)

Vert: 15=-3(B) 16=-1(B) 17=-1(B) 18=-3(B) 19=-25(B) 20=-8(B) 21=-8(B) 22=-25(B)



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Edenton, NC 27932



					r	2-:	2-8			1		
LOADING	· · · ·	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	-0.00	7	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	-0.00	7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-MP						Weight: 9 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (Ib/size) 3=53/Mechanical, 4=28/Mechanical, 2=153/0-3-0 Max Horz 2=58(LC 12) Max Uplift 3=-20(LC 12), 2=-39(LC 12) Max Grav 3=53(LC 1), 4=40(LC 3), 2=153(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

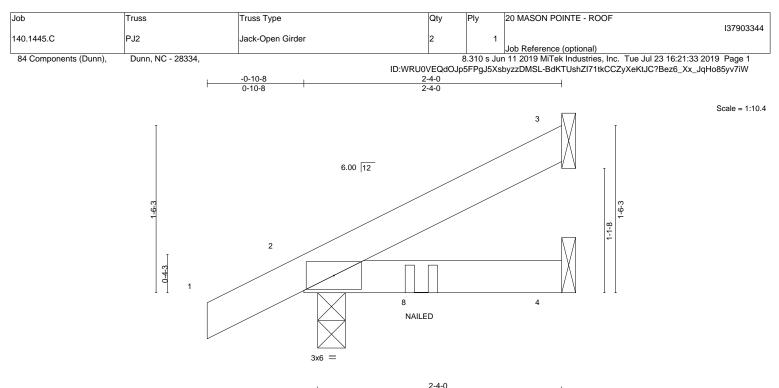
- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown;
- Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-4-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing





			2-2-8	
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.08	DEFL. in (loc) I/defl L/d Vert(LL) -0.00 7 >999 240	PLATES GRIP MT20 244/190
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr NO	BC 0.10 WB 0.00	Ver(LL) -0.00 7 >333 240 Vert(CT) -0.00 4-7 >999 180 Horz(CT) -0.00 3 n/a n/a	10120 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP		Weight: 9 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (Ib/size) 3=61/Mechanical, 4=45/Mechanical, 2=182/0-3-0 Max Horz 2=58(LC 12) Max Uplift 3=-27(LC 12), 4=-7(LC 12), 2=-65(LC 12) Max Grav 3=61(LC 1), 4=45(LC 3), 2=182(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown;
- Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
- connection is for uplift only and does not consider lateral forces.
- 8) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 4-5=-20 Concentrated Loads (lb)

Vert: 8=-54(B)



Structural wood sheathing directly applied or 2-4-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



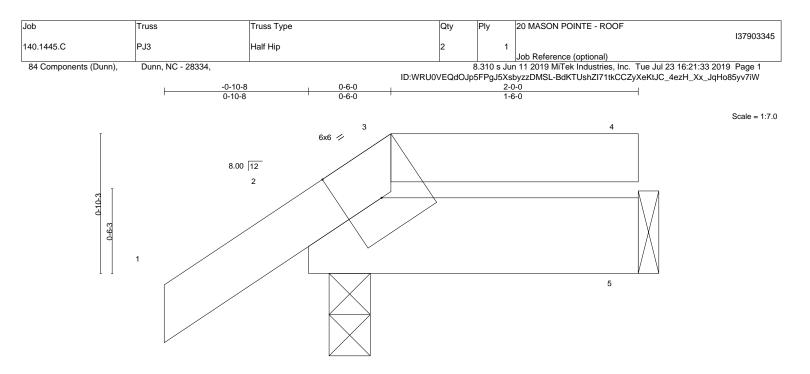


Plate Offsets (X,Y)	[2:0-3-3,0-0-0], [3:0-2-13,Edge], [3:0	0-1-8' -1-0,0-1-7]	0-9-15	1	1-(0-9		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.15 BC 0.09 WB 0.00 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.00 8	l/defl 3 >999 3 >999 2 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 10 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP	No.2		BRACING TOP CHOI		tural wood s	heathing dir	rectly applied or 2-0-0) oc purlins, except

BOT CHORD

2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x6 SP No.2

REACTIONS. 5=69/Mechanical, 2=144/0-3-0 (lb/size) Max Horz 2=40(LC 12) Max Uplift 5=-36(LC 12), 2=-68(LC 12) Max Grav 5=74(LC 22), 2=144(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=45ft; L=45ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members. 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) Refer to girder(s) for truss to truss connections.

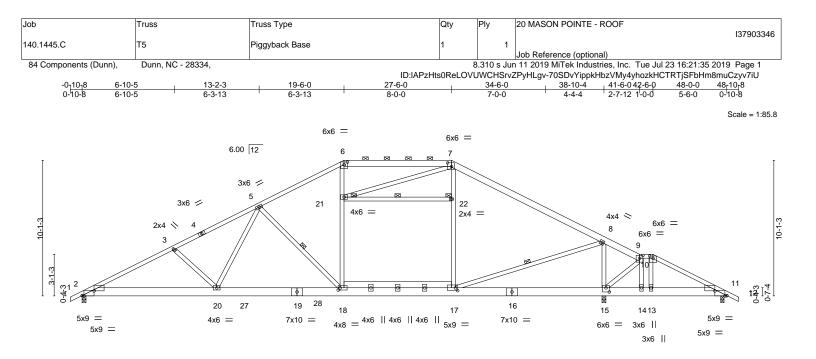
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 2. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



L	10-0-4	19-6-0	27-6-	0	34-0-0 3	8-10-4 41-6-042-6-0	47-9-8 48-0-0
	10-0-4	9-5-12	8-0-0	0 '	7-0-0	4-4-4 2-7-12 1-0-0	5-3-8 0-2-8
Plate Offsets (X,	Y) [2:0-10-11,0-4-4], [2:0-1	-11,Edge], [6:0-3-0,0-2-7], [7:0-3-8,0-3-4], [9:0-3	3-0,0-2-8], [10:0-3	-0,0-2-0], [11:0-10-11,0)-4-4], [11:0-1-11,Edge], [15	:0-3-0,0-3-12]
	, [17:0-2-0,0-2-8], [18:0-	1-12,0-2-0]					-
LOADING (psf)	SPACING-	2-0-0 CSI.		DEFL. ir	n (loc) l/defl L/	d PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15 TC	0.67	Vert(LL) 0.34	18-20 >999 24	0 MT20	244/190
TCDL 10.0	Lumber DOL	1.15 BC	0.86	Vert(CT) -0.56	6 18-20 >837 18	0	
BCLL 0.0	* Rep Stress Incr	YES WB	0.82	Horz(CT) 0.06	6 11 n/a n/	a	
BCDL 10.0	Code IRC2015/T	PI2014 Matr	ix-MS	~ /		Weight: 382	lb FT = 20%
LUMBER-			I	BRACING-			
	2x4 SP No.2 *Except*			TOP CHORD	Structural wood shea	thing directly applied or 2-1	1-6 oc purlins.
	6-7,7-9: 2x6 SP No.2				except		
	2x8 SP No.2 *Except*				2-0-0 oc purlins (5-0-	6 max.): 6-7, 9-10.	
	17-18: 2x6 SP No.2			BOT CHORD		applied or 10-0-0 oc bracing	a. Except:
WEBS 2	2x4 SP No.3				6-3-1 oc bracing: 2-2		
					7-0-1 oc bracing: 18-		
				WEBS	1 Row at midpt	5-18, 8-17, 21-22	
				JOINTS	1 Brace at Jt(s): 21, 2		

27-6-0

34-6-0

38-10-4

41-6-042-6-0

47-9-8

48-0-0

REACTIONS. (lb/size) 2=1675/0-3-8, 15=1575/0-3-8, 11=692/0-3-0 Max Horz 2=170(LC 16) Max Uplift 2=-398(LC 9), 15=-460(LC 8), 11=-173(LC 9) Max Grav 2=1675(LC 1), 15=1661(LC 24), 11=692(LC 1)

10-0-4

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3133/1993, 3-5=-2874/1951, 5-6=-1924/1355, 6-7=-1647/1254, 7-8=-1960/1285, 8-9=-1018/529, 9-10=-1026/594, 10-11=-1144/619

BOT CHORD 2-20=-1665/2741, 18-20=-1251/2201, 17-18=-810/1621, 15-17=-376/944,

 14-15=-453/1039, 13-14=-455/1026, 11-13=-449/1005

 WEBS
 3-20=-343/240, 5-20=-561/719, 5-18=-840/628, 18-21=-462/539, 6-21=-444/516,

17-22=-238/364, 7-22=-240/366, 8-17=-448/903, 8-15=-1451/875, 10-13=-130/385

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

19-6-0

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

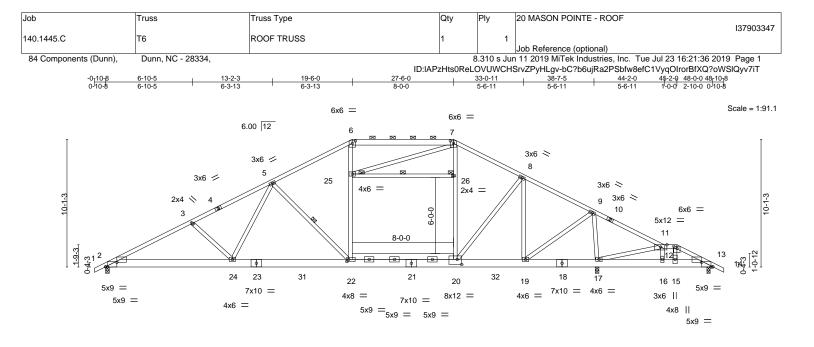
6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 15, and 11. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







	,0-2-0]						
Plate Offsets (X,Y)	[2:0-10-11,0-4-4], [2:0-1-12	1,Edge], [6:0-3-0,0-2-7], [7:0-3	3-0,0-2-7], [12:0-3-0,0-2-0],	[13:0-10-11,0-4-4],	[13:0-1-11,Edge]	, [20:0-4-4,0-4-	-12], [22:0-1-12
F	10-0-4	9-5-12	8-0-0	5-6-11	5-9-9	5-3-12	1-0-0 2-7-8 0-2-8
L	10-0-4	19-6-0	27-6-0	33-0-11	38-10-4	44-2-0	45-2-0 47-9-8

LOADING (psf) SPACING- 2-0-0 TCLL 20.0 Plate Grip DOL 1.15 TCDL 10.0 Lumber DOL 1.15 BCLL 0.0 * Rep Stress Incr YES BCDL 10.0 Code IRC2015/TPI2014	CSI.DEFTC0.60VertBC0.93VertWB0.97HorzMatrix-MSAttice	L) 0.34 22-24 >999 CT) -0.63 22-24 >734	240 180 a n/a	PLATES MT20 Weight: 383 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 *Except* 6-7: 2x6 SP No.2 BOT CHORD 2x8 SP DSS *Except* 21-23: 2x8 SP No.2, 20-22: 2x6 SP No.2 WEBS 2x4 SP No.3	ТОР	2-0-0 oc purli CHORD Rigid ceiling of S 1 Row at mid IS 1 Brace at Jt(ns (4-3-14 max. directly applied of pt 5 s): 25, 26 juires both edge	rectly applied or 2-9-2 d .): 6-7, 11-12. or 2-2-0 oc bracing. 5-22, 25-26 es of the bottom chord l	

REACTIONS. (lb/size) 2=1766/0-3-8, 17=1916/0-3-8, 13=564/0-3-0 Max Horz 2=-170(LC 13) Max Uplift 2=-228(LC 9), 17=-262(LC 8), 13=-116(LC 9) Max Grav 2=1848(LC 2), 17=2039(LC 27), 13=564(LC 1)

- TOP CHORD
 2-3=-3622/1732, 3-5=-3409/1691, 5-6=-2464/1091, 6-7=-2133/1027, 7-8=-2401/1071, 8-9=-1868/837, 9-11=-785/261, 11-12=-885/496, 12-13=-948/511

 BOT CHORD
 2-24=-1430/3186, 22-24=-1016/2674, 20-22=-575/2114, 19-20=-472/1611, 17-19=-130/761, 16-17=-397/846, 15-16=-414/885, 13-15=-389/841
- WEBS 3-24=-338/237, 5-24=-56/734, 5-22=-849/629, 22-25=-401/744, 6-25=-294/746, 20-26=-336/634, 7-26=-250/679, 8-20=-152/812, 8-19=-1112/389, 9-19=-425/1279, 9-17=-1638/698, 11-17=-404/324, 11-16=-368/161, 12-15=-233/402

NOTES-

1) Unbalanced roof live loads have been considered for this design.

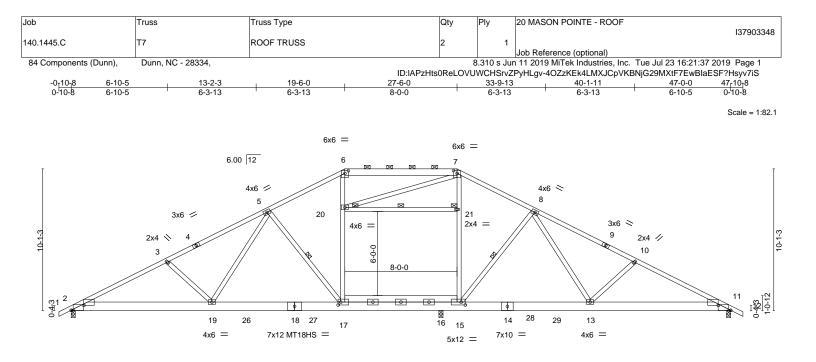
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Ceiling dead load (5.0 psf) on member(s). 25-26; Wall dead load (5.0 psf) on member(s).22-25, 20-26
- 7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 20-22
- 8) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 9) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 17. This connection is for uplift only and does not consider lateral forces.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

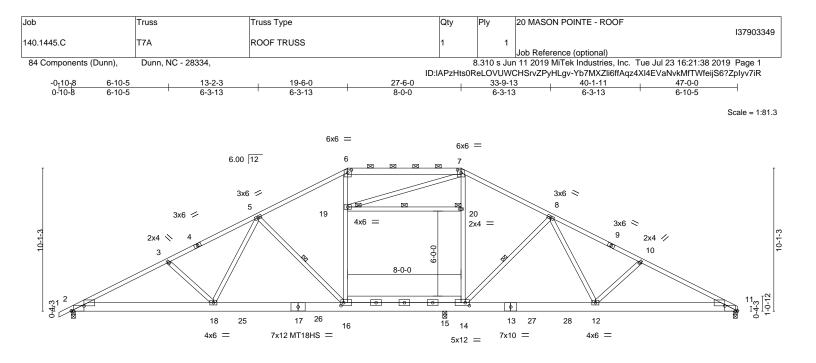


	L	10-0-4	19-6			6-4-4	27-6-0	3	36-11-12	1	47-0-0	1	
		10-0-4	9-5-			-10-4	1-1-12		9-5-12		10-0-4	1	
Plate Offse	ets (X,Y)	[2:0-10-11,0-4-4], [2:0-1-1	1,Edge], [6:0-3	5-0,0-2-7], [7:0-3	3-0,0-2-7], [[*]	11:0-10-11,0)-4-4], [11	0-1-11,E	:dge], [15	5:0-3-8,0-2-8],	[17:0-2-4,0-2-8]		
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TP	2-0-0 1.15 1.15 YES 12014	CSI. TC 0.6 BC 0.9 WB 0.6 Matrix-MS	98 58	DEFL. Vert(LL Vert(CT Horz(C Attic) -0.37) -0.71 T) 0.06	(loc) 17-19 17-19 17-19 11 16-17	l/defl >853 >440 n/a 538	L/d 240 180 n/a 360	PLATES MT20 MT18HS Weight: 362 lb	GRIP 244/190 244/190 FT = 20%	
	10.0		2011			71110	0.01	10 17			1101gm. 002 10	11 - 2070	
LUMBER- TOP CHO BOT CHO WEBS	RD 2x4 SP 6-7: 2x6 RD 2x8 SP 14-18: 2 2x4 SP	No.2 *Except* 6 SP No.2 No.2 *Except* 2x8 SP DSS, 15-17: 2x6 S No.3 *Except* 15,20-21: 2x4 SP No.2	SP No.2			BRACII TOP CH BOT CH WEBS JOINTS	iord Iord	2-0-0 c Rigid c 1 Row 1 Brace This tru	oc purlins eiling dir at midpt e at Jt(s)	(4-11-11 max ectly applied o 5 : 20, 21	rectly applied or 2-8-6 o x.): 6-7. or 2-2-0 oc bracing. i-17, 8-15, 20-21 as of the bottom chord b		pt
REACTIO	Max He Max U	e) 2=1496/0-3-8, 11=133 orz 2=-170(LC 13) plift 2=-169(LC 12), 11=-3 rav 2=1583(LC 26), 11=13	9(LC 13), 16=-	99(LC 13)									
TOP CHO	RD 2-3=- 8-10=	Comp./Max. Ten All forc 3062/463, 3-5=-2837/418, -2181/354, 10-11=-2402/2	, 5-6=-1794/27! 400	5, 6-7=-1573/29	3, 7-8=-167	,							
BOT CHO WEBS	11-13	:-338/2689, 17-19=-133/2()=-251/2117 :-380/262, 5-19=-127/866,											
WEDS		=-187/340, 7-21=-81/385,		,	,	,							
 Wind: A MWFRS DOL=1. Provide All plate All plate All plate This tru This tru This tru This tru Bottom All beat Caraph 	SCE 7-10; V S (envelope) .60 plate grip a adequate dri as are MT20 p as are SX9 MT ss has been rruss has been etween the bidead load (5, chord live loa arings are ass 12.5A Simpso connection is f ical purlin rep	loads have been conside ult=130mph (3-second gu gable end zone and C-C E DOL=1.60 ainage to prevent water pr plates unless otherwise indic designed for a 10.0 psf bo n designed for a 10.0 psf bo n designed for a live load of ottom chord and any other 0 psf) on member(s). 20-2 ad (40.0 psf) and additiona sumed to be User Defined on Strong-Tie connectors r for uplift only and does not opresentation does not depi DWN IS DESIGNED AS U	st) Vasd=103m Exterior(2) zone onding. dicated. cated. totom chord live of 20.0psf on th r members, wit 21; Wall dead l al bottom chord crushing capa recommended t consider later ict the size or th	Ph; TCDL=6.0p e;C-C for memb e bottom chord h BCDL = 10.0p load (5.0psf) on l dead load (5.0 city of 425 psi. to connect truss al forces. he orientation of	ers and for irrent with a in all areas osf. member(s) psf) applied to bearing	ces & MWF any other live s where a re).17-20, 15-7 d only to roc walls due to	RS for rea loads. ctangle 3- 21 m. 16-17, UPLIFT a	ctions sh 6-0 tall b 15-16 at jt(s) 2,	nown; Lur y 2-0-0 w 11, and	mber vide	THE AND RE	SEAL 45844	A A MANUMAN

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

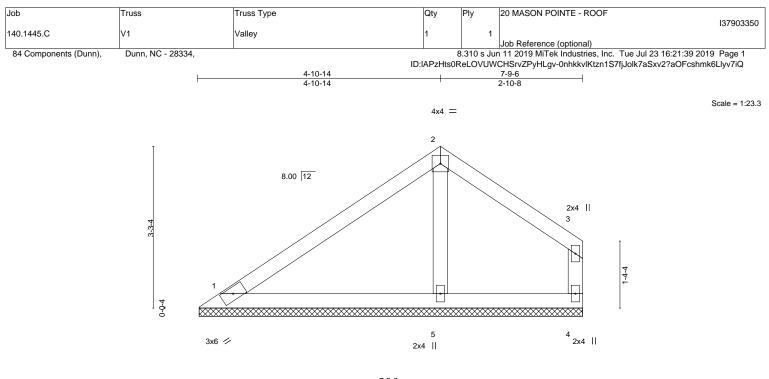


July 24,2019



<u> </u>		6-0 -12	<u>26-4-4</u> <u>27-6</u> 6-10-4 1-1-	5-0 : 12	36-11-12 9-5-12	47-0-0 10-0-4	
Plate Offsets (X,Y)	[2:0-10-11,0-4-4], [2:0-1-11,Edge], [6:0-	3-0,0-2-7], [7:0-3-0,0-2-7]	, [11:0-10-11,0-4-4]	, [11:0-1-11,Ec	dge], [14:0-3-8,0-2-8],	[16:0-2-4,0-2-8]	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.98 WB 0.67 Matrix-MS			l/defl L/d >866 240 >444 180 n/a n/a 538 360	PLATES MT20 MT18HS Weight: 359 lb	GRIP 244/190 244/190 FT = 20%
6-7: 2xi BOT CHORD 2x8 SP 14-16∷ WEBS 2x4 SP 6-16,7-	P No.2 *Except* 6 SP No.2 P No.2 *Except* 2x6 SP No.2, 13-17: 2x8 SP DSS P No.3 *Except* 14: 2x4 SP No.2		BRACING- TOP CHORI BOT CHORI WEBS JOINTS	2-0-0 oc Rigid ce 1 Row a 1 Brace	c purlins (5-0-7 max.): eiling directly applied of at midpt 5 e at Jt(s): 19, 20 ss requires both edge		
Max H Max U	e) 2=1500/0-3-8, 11=1279/0-3-8, 15=1 orz 2=178(LC 12) plift 2=-170(LC 12), 11=-15(LC 13), 15= rav 2=1580(LC 26), 11=1299(LC 2), 15	-103(LC 13)					
TOP CHORD 2-3=-	Comp./Max. Ten All forces 250 (lb) or 3047/453, 3-5=-2833/412, 5-6=-1793/26 =-2184/354, 10-11=-2407/396						
11-12	338/2671, 16-18=-164/2118, 15-16=0/ 2=-263/2107 340/235, 5-18=-106/816, 5-16=-894/33	, , ,	,				
14-20 NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) DOL=1.60 plate grip 3) Provide adequate dr 4) All plates are MT20 J 5) All plates are MT20 J 5) All plates are 5x9 M 6) This truss has been will fit between the b 8) Ceiling dead load (5 9) Bottom chord live load 10) All bearings are assist 11) One H2.5A Simpson This connection is f 12) Graphical purlin reg)=-215/337, 7-20=-110/382, 8-14=-661/2 e loads have been considered for this de (ult=130mph (3-second gust) Vasd=103 gable end zone and C-C Exterior(2) zor	96, 8-12=-90/470, 10-12= sign. nph; TCDL=6.0psf; BCDL e;C-C for members and f e bottom chord in all are th BCDL = 10.0psf. load (5.0psf) on member d dead load (5.0 psf) appl acity of 425 psi. to connect truss to bearir ral forces.	=-367/239 ==6.0psf; h=30ft; Ca orces & MWFRS for a any other live load as where a rectang (s).16-19, 14-20 ied only to room. 1 ng walls due to UPL	r reactions sho ls. le 3-6-0 tall by 5-16, 14-15 IFT at jt(s) 2, 1	own; Lumber v 2-0-0 wide 11, and 15.	AND RE	SEAL 45844 W JOHNSON





	ŀ				7-9-6 7-9-6					-	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	BC	0.38 0.16 0.05	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - -	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/T		Matrix		1012(01)	0.00		n/a	n/a	Weight: 30 lb	FT = 20%
LUMBER-					BRACING-						

TOP CHORD

2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS OTHERS 2x4 SP No.3

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 7-9-6 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=169/7-9-0, 4=95/7-9-0, 5=308/7-9-0 Max Horz 1=74(LC 12) Max Uplift 1=-26(LC 12), 4=-52(LC 13), 5=-14(LC 12) Max Grav 1=169(LC 1), 4=102(LC 20), 5=310(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

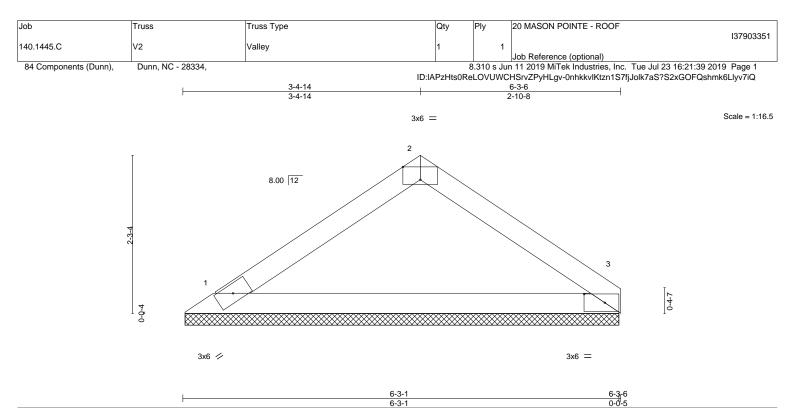
6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) n/a

8) n/a







DEFL. Vert(LL) Vert(CT)	in n/a n/a	(loc) - -	l/defl n/a n/a	L/d 999 999	PLATES MT20	GRIP 244/190
					MT20	244/190
Vert(CT)	n/a	-	n/a	999		
Horz(CT)	0.00	3	n/a	n/a		
					Weight: 20 lb	FT = 20%
BRACING-						
	BRACING-	BRACING-	BRACING-	BRACING-	BRACING-	Weight: 20 lb

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

REACTIONS. (Ib/size) 1=231/6-2-11, 3=231/6-2-11 Max Horz 1=48(LC 9) Max Uplift 1=-25(LC 12), 3=-24(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

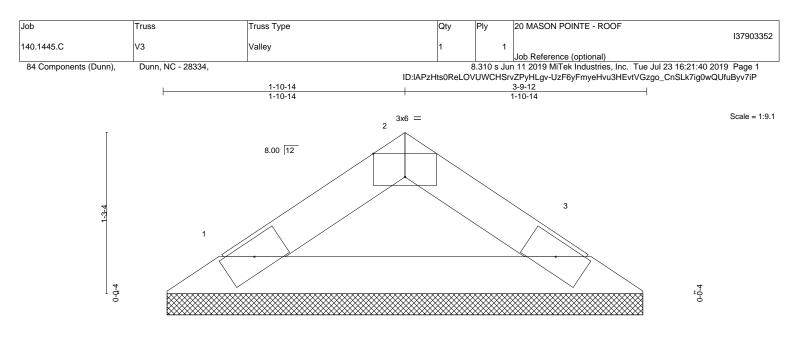
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) n/a

8) n/a







3x6 🥢

3x6 📎

Structural wood sheathing directly applied or 3-9-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Plate Offsets (X,Y)	0-0 <u>+6</u> 0-0-6 [2:0-3-0,Edge]				3-9-12 3-9-6						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES	CSI. TC BC WB Matri	0.06 0.16 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (Ib/size) 1=114/3-9-0, 3=114/3-9-0 Max Horz 1=-23(LC 8) Max Uplift 1=-12(LC 12), 3=-12(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

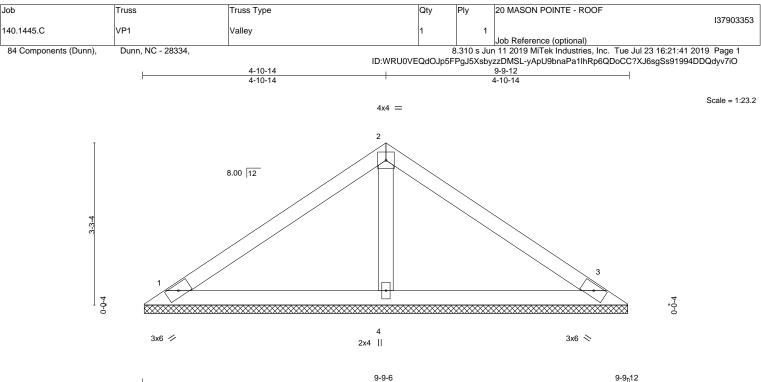
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) n/a







		9-9-6		0-0-6
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. DEFL. TC 0.28 Vert(LL) BC 0.20 Vert(CT)	in (loc) l/defl L/d n/a - n/a 999 n/a - n/a 999	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.06 Horz(CT) Matrix-S	0.00 3 n/a n/a	Weight: 34 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 1=174/9-9-0, 3=174/9-9-0, 4=359/9-9-0 Max Horz 1=-73(LC 8) Max Uplift 1=-32(LC 12), 3=-42(LC 13), 4=-12(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

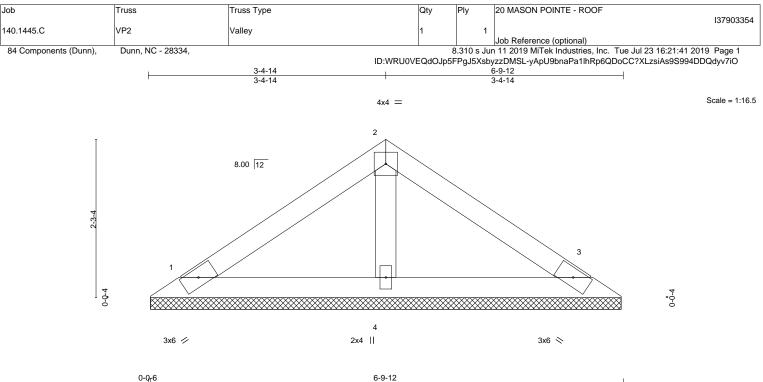
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) n/a

8) n/a







	0-0-6		6-9-6	T	
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/de	efl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.16	Vert(LL) n/a - n	/a 999	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) n/a - n	/a 999	
CLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT) 0.00 3 n	/a n/a	
CDL 10.0	Code IRC2015/TPI2014	Matrix-P			Weight: 23 lb FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 1=126/6-9-0, 3=126/6-9-0, 4=215/6-9-0 Max Horz 1=48(LC 9)

Max Uplift 1=-28(LC 12), 3=-34(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-Č Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

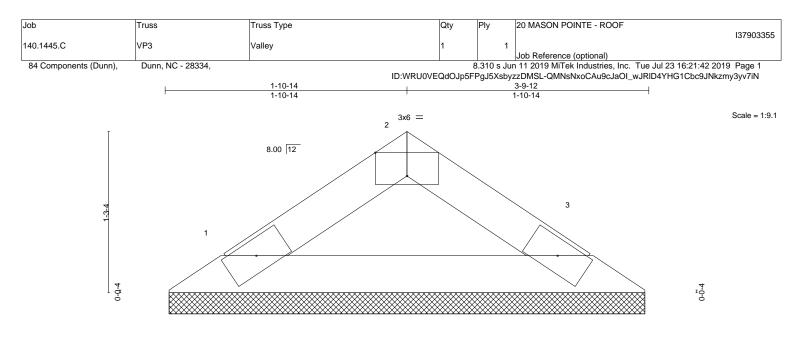
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) n/a







3x6 🥢

3x6 📎

Structural wood sheathing directly applied or 3-9-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

0-0 <u>16</u> 0-0 ¹ 6			<u>3-9-12</u> 3-9-6								
Plate Offsets (X,Y)	[2:0-3-0,Edge]				-					1	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.16	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
3CDL 10.0	Code IRC2015/TF	PI2014	Matri	x-P						Weight: 11 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (lb/size) 1=114/3-9-0, 3=114/3-9-0 Max Horz 1=23(LC 11) Max Uplift 1=-12(LC 12), 3=-12(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

7) n/a





