



**Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP**

**PASSED**

BC CALC® Member Report

**RB01 (Roof Beam)**

November 12, 2019 09:23:16

Build 7295

Dry | 1 span | No cant.

Job name:

File name:

Address:

Description:

City, State, Zip:

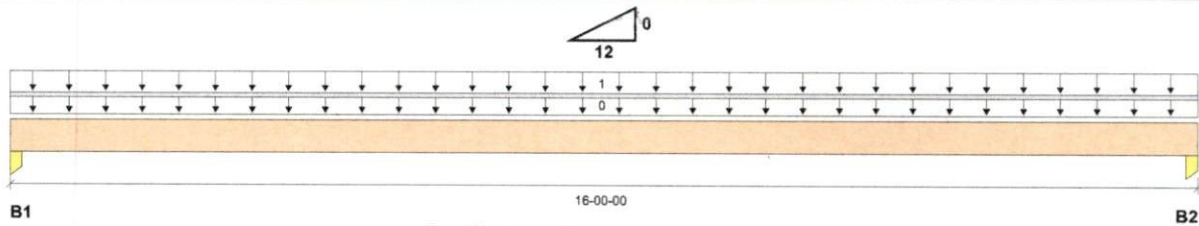
Specifier:

Customer:

Designer: TOM WALKER

Code reports: ESR-1040

Company: LONGLEAF TRUSS COMPANY



**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 5-1/2"		2595 / 0	4800 / 0		4800 / 0
B2, 5-1/2"		2595 / 0	4800 / 0		4800 / 0

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 100%	Dead 90%	Snow 115%	Wind 160%	Roof Live 125%	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	16-00-00	Top		24				00-00-00
1		Unf. Area (lb/ft <sup>2</sup> )	L	00-00-00	16-00-00	Top		10	20		20	30-00-00

**Controls Summary**

	Value	% Allowable	Duration	Case	Location
Pos. Moment	26724 ft-lbs	41.5%	115%	5	08-00-00
End Shear	5739 lbs	31.3%	115%	5	01-09-08
Total Load Deflection	L/588 (0.31")	30.6%	n/a	4	08-00-00
Live Load Deflection	L/906 (0.202")	26.5%	n/a	6	08-00-00
Max Defl.	0.31"	62.1%	n/a	4	08-00-00
Span / Depth	11.4				

**Bearing Supports**

	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column 5-1/2" x 5-1/4"	7395 lbs	31.0%	34.1%	Southern Pine
B2	Column 5-1/2" x 5-1/4"	7395 lbs	31.0%	34.1%	Southern Pine

**Cautions**

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

**Notes**

- Design meets Code minimum (L/180) Total load deflection criteria.
- Design meets Code minimum (L/240) Live load deflection criteria.
- Design meets arbitrary (0.5") Maximum Total load deflection criteria.
- Calculations assume member is fully braced.
- BC CALC® analysis is based on IBC 2009.
- Design based on Dry Service Condition.



# Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP

**PASSED**

BC CALC® Member Report  
Build 7295

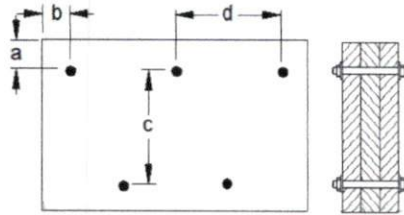
**RB01 (Roof Beam)**  
Dry | 1 span | No cant.

November 12, 2019 09:23:16

Job name:  
Address:  
City, State, Zip:  
Customer:  
Code reports: ESR-1040

File name:  
Description:  
Specifier:  
Designer: TOM WALKER  
Company: LONGLEAF TRUSS COMPANY

## Connection Diagram: Full Length of Member



a minimum = 2"      c = 12"  
b minimum = 2-1/2"      d = 24"

Bolts are assumed to be Grade A307 or Grade 2 or higher.  
Connectors are: 1/2 in. Staggered Through Bolt

### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BC®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



**Triple 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2650 SP**

**PASSED**

BC CALC® Member Report

**RB02 (Roof Beam)**

November 12, 2019 09:25:37

Build 7295

Dry | 1 span | No cant.

Job name:

File name:

Address:

Description:

City, State, Zip:

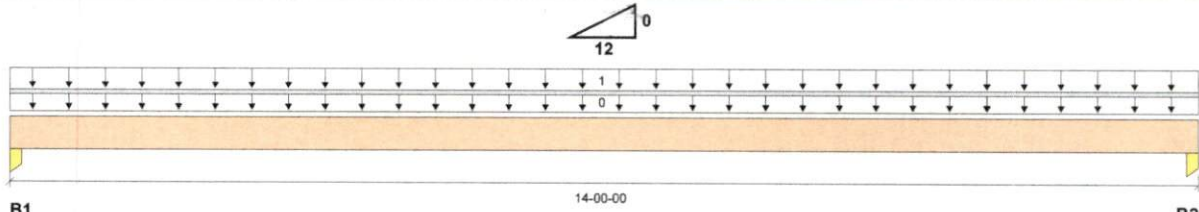
Specifier:

Customer:

Designer: TOM WALKER

Code reports: ESR-1040

Company: LONGLEAF TRUSS COMPANY



Total Horizontal Product Length = 14-00-00

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 5-1/2"		1106 / 0	1960 / 0		1960 / 0
B2, 5-1/2"		1106 / 0	1960 / 0		1960 / 0

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 100%	Dead 90%	Snow 115%	Wind 160%	Roof Live 125%	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-00-00	Top		18				00-00-00
1		Unf. Area (lb/ft <sup>2</sup> )	L	00-00-00	14-00-00	Top		10	20		20	14-00-00

**Controls Summary**

	Value	% Allowable	Duration	Case	Location
Pos. Moment	9553 ft-lbs	30.5%	115%	5	07-00-00
End Shear	2432 lbs	17.9%	115%	5	01-05-06
Total Load Deflection	L/658 (0.241")	27.4%	n/a	4	07-00-00
Live Load Deflection	L/1029 (0.154")	23.3%	n/a	6	07-00-00
Max Defl.	0.241"	48.2%	n/a	4	07-00-00
Span / Depth	13.3				

**Bearing Supports**

	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column 5-1/2" x 5-1/4"	3066 lbs	12.9%	14.2%	Southern Pine
B2	Column 5-1/2" x 5-1/4"	3066 lbs	12.9%	14.2%	Southern Pine

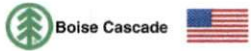
**Cautions**

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

**Notes**

- Design meets Code minimum (L/180) Total load deflection criteria.
- Design meets Code minimum (L/240) Live load deflection criteria.
- Design meets arbitrary (0.5") Maximum Total load deflection criteria.
- Calculations assume member is fully braced.
- BC CALC® analysis is based on IBC 2009.
- Design based on Dry Service Condition.



### Triple 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2650 SP

PASSED

BC CALC® Member Report

RB02 (Roof Beam)

November 12, 2019 09:25:37

Build 7295

Dry | 1 span | No cant.

Job name:

File name:

Address:

Description:

City, State, Zip:

Specifier:

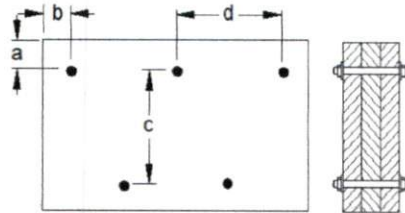
Customer:

Designer: TOM WALKER

Code reports: ESR-1040

Company: LONGLEAF TRUSS COMPANY

#### Connection Diagram: Full Length of Member



a minimum = 2"      c = 7-7/8"  
 b minimum = 2-1/2"      d = 24"

Bolts are assumed to be Grade A307 or Grade 2 or higher.  
 Connectors are: 1/2 in. Staggered Through Bolt

#### Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BC®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®

RE: P19-10021 - PATTERSON

**Trenco**  
 818 Soundside Rd  
 Edenton, NC 27932

**Site Information:**

Project Customer:      Project Name:  
 Lot/Block:                      Subdivision:  
 Model:  
 Address:  
 City:                              State:

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: IRC2018/TPI2014      Design Program: MiTek 20/20 8.3  
 Wind Code: N/A      Wind Speed: 130 mph      Design Method: User defined  
 Roof Load: 40.0 psf      Floor Load: 55.0 psf  
 Mean Roof Height (feet): 12      Exposure Category: B

No.	Seal#	Truss Name	Date
1	E13780449	F01	11/19/19
2	E13780450	F02	11/19/19
3	E13780451	F03	11/19/19
4	E13780452	F04	11/19/19
5	E13780453	F05	11/19/19
6	E13780454	F06	11/19/19
7		F07	11/19/19
8	E13780456	M01	11/19/19
9	E13780457	M02	11/19/19
10	E13780458	PB01	11/19/19
11	E13780459	PB02	11/19/19
12	E13780460	T01	11/19/19
13	E13780461	T01GE	11/19/19
14	E13780462	T02	11/19/19
15	E13780463	T03	11/19/19
16		T04	11/19/19

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Longleaf Truss Company.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2019.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSITPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSITPI 1, Chapter 2.



Job P19-10021	Truss F01	Truss Type Floor Supported Gable	City 1	Qty 1	PATTERSON	E13780449
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Longleaf Truss Company, West End, NC - 27376, 8320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:35 2019 Page 1  
 ID: X0bmUq2xt7MgVT77UDFAyQALw4kDW0chM4OX7DvPyc\_7nvsEwPlu2W0X7BwWEC0Yf00U

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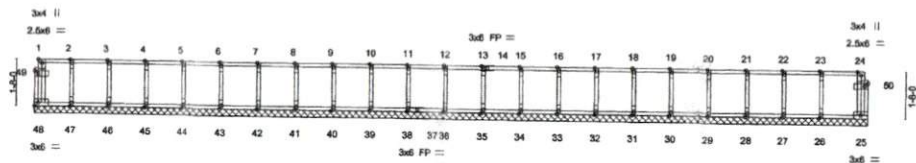


Plate Offsets (X,Y)	(1 Edge, 0-1-6)	(49, 0-1-6, 0-1-4)	(50, 0-1-6, 0-1-4)
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LOADING (psf)	SPACING-	2-0-0	CSL	DEFL.	in (loc)	ldell	Lid	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.07	Vert(LL)	n/a	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.02	Vert(CT)	n/a	n/a	999		
BCLL 0.0	Rap Shrink Insr	YES	WB 0.03	Horz(CT)	0.00	25	n/a		
BCDL 5.0	Code IRC2018/TP2014		Matrix-R					Weight: 144 lb	FT = 8%F, 4%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No. 1(flat)	TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No. 1(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No. 3(flat)	
OTHERS 2x4 SP No. 3(flat)	

REACTIONS. All bearings 20-0-0.  
 (b) - Max Grav. All reactions 250 lb or less at joint(s) 48, 25, 47, 46, 45, 44, 43, 42, 41, 40, 39, 38, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26

FORCES. (b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-
- 1) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
  - 2) All plates are 1.5x4 MT20 unless otherwise indicated.
  - 3) Plates checked for a plus or minus 0 degree rotation about its center.
  - 4) Gable requires continuous bottom chord bearing.
  - 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
  - 6) Gable studs spaced at 1-4-0 oc.
  - 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI-1.
  - 8) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M0-7473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MTEK connections. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI-1 Quality Criteria, DSD-01 and BCD Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**ENGINEERED BY**  
**TRENCO**  
 A MAIR AFFILIATE  
 816 Soundside Road  
 Eden, NC 27532

Job P19-10021	Truss F02	Truss Type Floor	Qty 4	Ply 1	PATTERSON	E13780450
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Longleaf Truss Company, West End, NC - 27376, 8.320 & Oct 29 2019 Mitek Industries, Inc. Tue Nov 19 12:41:36 2019 Page 1  
 Job Reference (optional): ID:XBmLq2d7MgV177UDFAyGALw-CuruEyl\_rCXD-YUCVVVETePllqprvnrGGRFokqfioot

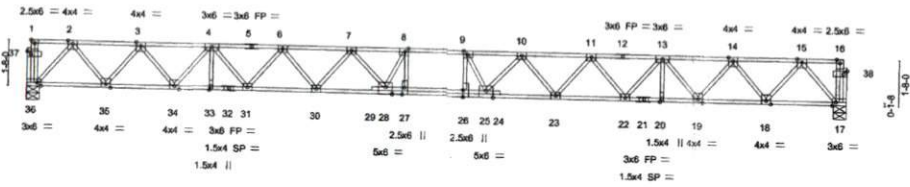
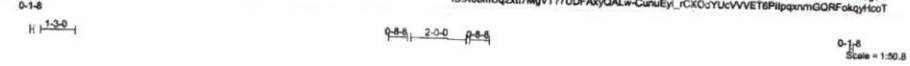


Plate Offsets (X,Y)	[1-Edge,0-1-8], [0-1-8,Edge], [0-1-8,Edge], [17-0-1-8,Edge], [25-0-3-0,Edge], [28-0-3-0,0-0-0], [27-0-3-0,Edge], [28-0-3-0,Edge], [36-0-1-8,Edge], [37-0-1-8,0-1-4], [38-0-1-8,0-1-4]
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LOADING (psf)	SPACING	1-4-0	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCCL 40.0	Plate Grip DCL	1.00	TC 0.54	Vert(LL)	-0.61	26-27	>578	MT20	244/190
TCDL 10.0	Lumber DCL	1.00	BC 0.99	Vert(CT)	-0.84	26-27	>421		360
BCLL 0.0	Rep Stress Incr	YES	WB 0.55	Horz(CT)	0.13	17	n/a		n/a
BCDL 5.0	Code	IRC2018/TP12014	Matrix-S						
								Weight: 175 lb	FT = 8%F, 4%E

**LUMBER:**  
 TOP CHORD 2x4 SP No.1(flat)  
 BOT CHORD 2x4 SP No.1(flat)  
 WEBS 2x4 SP No.3(flat)

**BRACING:**  
 TOP CHORD Sheathed or 5-3-8 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except 2-3-0 oc bracing: 28-30,23-25.

**REACTIONS:** (lb/size) 17=1070/0-5-8, 36=1070/0-5-8

**FORCES:** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-1686/0, 3-4=-2957/0, 4-6=-3975/0, 6-7=-4625/0, 7-8=-5053/0, 8-9=-5288/0, 9-10=-5053/0, 10-11=-4625/0, 11-13=-3975/0, 13-14=-2957/0, 14-15=-1686/0  
 BOT CHORD 35-36=0/965, 34-35=0/2392, 33-34=0/3538, 31-33=0/3538, 30-31=0/4370, 29-30=0/4897, 27-28=0/5288, 26-27=0/5288, 25-26=0/5268, 23-25=0/4897, 22-23=0/4370, 20-22=0/3538, 19-20=0/3538, 18-19=0/2392, 17-18=0/965  
 WEBS 8-27=-127556, 9-28=-127556, 2-36=-1413/0, 2-35=0/1146, 3-35=-1121/0, 3-34=0/897, 4-34=-895/0, 4-31=0/875, 6-31=-627/0, 6-30=0/405, 7-30=-431/0, 7-28=-1/404, 8-28=-766/25, 15-17=-1413/0, 15-18=0/1146, 14-18=-1121/0, 14-19=0/897, 13-19=-895/0, 13-22=0/675, 11-22=-627/0, 11-23=0/405, 10-23=-431/0, 10-25=-1/404, 9-25=-766/25

- NOTES:**
- 1) Unbalanced floor live loads have been considered for this design.
  - 2) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
  - 3) All plates are 3x4 MT20 unless otherwise indicated.
  - 4) The Fabrication Tolerance at joint 32 = 4%, joint 21 = 4%
  - 5) Plates checked for a plus or minus 0 degree rotation about its center.
  - 6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
  - 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



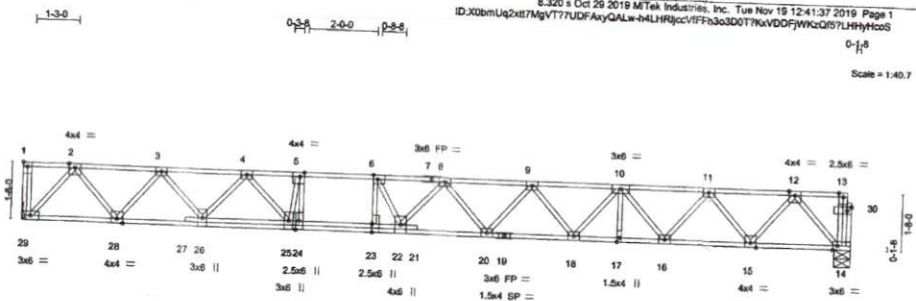
**WARNING -** Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 rev. 10/30/2015 BEFORE USE.  
 Design valid for use only with MITEK connections. This design is based only upon parameters shown, and is for an individual building component, not building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCS Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**ENGINEERED BY**  
**TRENCO**  
 A MEMBER OF  
 818 Soundside Road  
 Eden, NC 27632

Job P19-10021	Truss F03	Truss Type Floor	City 3	Ply 1	PATTERSON	E13780451
Longleaf Truss Company, West End, NC - 27376		Job Reference (optional)				

8-320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:37 2019 Page 1  
ID:XBmUqzXt7MgV77UDFAyGALw-nLHRjccVFh3o3D07NoVDFJWkzGf57LH4HyHccS

0-1/8  
Scale = 1:40.7



LOADING (psf)	SPACING	CSL	DEFL.	PLATES	GRP
TCLL 40.0	Plate Grip DOL 1-4-0	TC 0.40	in (loc) 0.29 22 >983 480	MT20	244/190
TCCL 10.0	Lumber DOL 1.00	BC 0.70	Vert(LL) -0.40 22 >715 360		
BCLL 0.0	Rep Stress Inter YES	WB 0.42	Horz(CT) 0.07 14 n/a n/a		
BCCL 5.0	Code IRC2018/TPI2014	Matrix-S			

Weight: 146 lb FT = 8%, 4%

**LUMBER:**  
 TOP CHORD 2x4 SP No.1(Flat)  
 BOT CHORD 2x4 SP No.1(Flat)  
 WEBS 2x4 SP No.3(Flat)

**BRACING:**  
 TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS:** (lb/size) 29=868/Mechanical, 14=864/0-5-8

**FORCES:** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**TOP CHORD** 2-3=-1298/0, 3-4=-2281/0, 4-5=-3089/0, 5-6=-3254/0, 6-8=-3324/0, 8-9=-3257/0,  
 9-10=-2935/0, 10-11=-2267/0, 11-12=-1329/0  
**BOT CHORD** 28-29=0/740, 26-28=0/1854, 25-26=0/2727, 24-25=0/3229, 23-24=0/3254, 22-23=0/3254,  
 20-22=0/3360, 18-20=0/3164, 17-18=0/2673, 16-17=0/2673, 15-16=0/1868, 14-15=0/774  
**WEBS** 5-24=-51/834, 6-23=-413/117, 2-29=-1112/0, 2-28=0/886, 3-28=-883/0, 3-26=-0/682,  
 4-28=-691/0, 4-25=0/612, 5-25=-1008/0, 12-14=-1133/0, 12-15=0/882, 11-15=-0/570,  
 11-16=0/833, 10-16=-627/0, 10-18=0/403, 9-18=-364/0, 6-22=-236/373

**NOTES:**

- 1) Unbalanced floor live loads have been considered for this design.
- 2) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 3) All plates are 3x4 MT20 unless otherwise indicated.
- 4) The Fabrication Tolerance at joint 19 = 4%.
- 5) Plates checked for a plus or minus 0 degree rotation about its center.
- 6) Refer to girder(s) for truss to truss connections.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R602.11.1 and R602.10.2 and referenced standard ANSI/TPI 1.
- 8) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 9) CAUTION: Do not erect truss backwards.



November 19, 2019

ENGINEERING BY  
**TRENCO**  
 A 50 Year Tradition

818 Stonedale Road  
 Edenton, NC 27832

**WARNING -** Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M1-7473 rev. 10/03/2015 BEFORE USE.  
 Design valid for use only with MTEK's connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPP Quality Criteria, DSB-29 and BCSI Building Component Safety Information available from Truss Plate Institute, 210 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job P19-10021	Truss F04	Truss Type Floor	City 8	Ply 1	PATTERSON	E13780452
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Longleaf Truss Company, West End, NC - 27376, Job Reference (optional) 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:40 2019 Page 1  
ID: X0bmUq2xd7MgV7T7UDFAyQALw-Sf1P3KUVQ1q69cNkLZdyZRRQghWslZD70cyMcoP

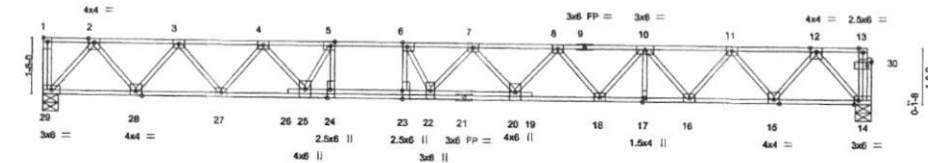


Plate Offsets (X,Y)	8-7-8 1-Edge, 0-1-8, [5-0-1-8, Edge], [6-0-1-8, Edge], [14-0-1-8, Edge], [23-0-3-0, 0-0-0], [24-0-3-0, Edge], [30-0-1-8, 0-1-4]
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LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in (loc)	Wdell	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DCL	1.00	TC 0.40	Vert(LL)	-0.30 20-22	>963	480	MT20	244/190
TCDL 10.0	Lumber DCL	1.00	BC 0.73	Vert(CT)	-0.41 20-22	>700	360		
BCLL 0.0	Rep Stress Insr	YES	WB 0.43	Horz(CT)	0.07 14	n/a	n/a		
BCDL 5.0	Code IRC2018/TPI2014		Matrix-S						
								Weight 148 lb	FT = 8%F, 4%E

**LUMBER-**  
TOP CHORD 2x4 SP No. 1(Rat)  
BOT CHORD 2x4 SP No. 1(Rat)  
WEBS 2x4 SP No. 3(Rat)

**BRACING-**  
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

**REACTIONS.** (lb/size) 29-885/0-5-8, 14-681/0-5-8

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
TOP CHORD 2-3=-1332/0, 3-4=-2268/0, 4-5=-3025/0, 5-6=-3421/0, 6-7=-3602/0, 7-8=-3424/0, 8-10=-3005/0, 10-11=-2323/0, 11-12=-1358/0  
BOT CHORD 28-29=0/798, 27-28=0/1887, 25-27=0/2709, 24-25=0/3421, 23-24=0/3421, 22-23=0/3421, 20-22=0/3585, 18-20=0/3279, 17-18=0/2748, 16-17=0/2748, 15-16=0/1911, 14-15=0/789  
WEBS 5-24=0/664, 6-23=-616/27, 2-29=-1139/0, 2-28=0/911, 3-28=-882/0, 3-27=0/652, 4-27=-653/0, 4-25=0/533, 5-25=-620/0, 12-14=-1156/0, 12-15=0/903, 11-15=-877/0, 11-16=0/654, 10-16=-656/0, 10-18=0/396, 8-18=-436/0, 6-22=-1326/0

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
  - 2) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
  - 3) All plates are 3/4 MT20 unless otherwise indicated.
  - 4) Plates checked for a plus or minus 0 degree rotation about its center.
  - 5) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSITPI 1.
  - 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
  - 7) CAUTION, Do not erect truss backwards.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MR-7473 rev. 10/03/2015 BEFORE USE.**  
Design valid for use only with MTEK's connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For critical guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA, 22314.

**DESIGNED BY**  
**TRENCO**  
A TRUSS AUTHORITY

818 Soundside Road  
Eden, NC 27932

Job P19-10021	Truss F05	Truss Type Floor	Qty 26	Ply 1	PATTERSON	E13780453
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Longleaf Truss Company, West End, NC - 27376, Job Reference (optional)  
 8.320 s Oct 29 2019 M/Tak Industries, Inc. Tue Nov 19 12:41:41 2019 Page 1  
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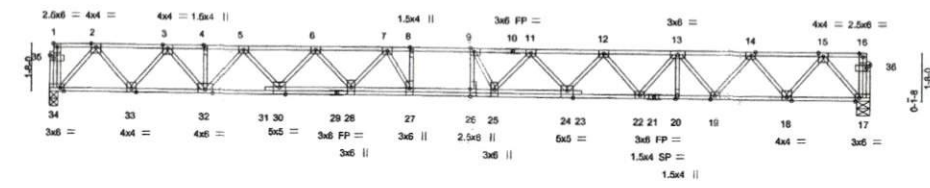
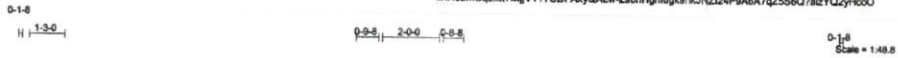


Plate Offsets (X,Y)	[1-Edge,0-1-8], [0-1-8,Edge], [14-0-1-12,Edge], [17-0-1-8,Edge], [19-0-1-12,Edge], [24-0-2-8,Edge], [26-0-3-0-0-0], [30-0-2-8,Edge], [34-0-1-8,Edge], [35-0-1-8,0-1-4], [36-0-1-8,0-1-4]
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LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.47	in (loc) Vdefl 480	MT20	244/190
TCCL 10.0	Lumber DOL 1.00	BC 0.92	Vert(CT) -0.67 26 >501 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.52	Horz(CT) 0.10 17 n/a n/a		
BCDL 5.0	Code IRC2018/TP12014	Matrix-S			
				Weight 178 lb	FT = 8%F, 4%E

**LUMBER-**  
 TOP CHORD 2x4 SP No.1(flat)  
 BOT CHORD 2x4 SP No.1(flat)  
 WEBS 2x4 SP No.3(flat)

**BRACING-**  
 TOP CHORD Sheathed or 5-8-7 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except 2-2-0 oc bracing; 22-24.

**REACTIONS.** (lb/size) 34=1027/0-3-8, 17=1027/0-5-8

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD  
 2-3=1613/0, 3-4=2842/0, 4-5=2842/0, 5-6=3815/0, 6-7=4534/0, 7-8=4850/0,  
 8-9=4850/0, 9-11=4868/0, 11-12=4414/0, 12-13=3739/0, 13-14=2813/0,  
 14-15=1612/0  
 BOT CHORD  
 33-34=0/928, 32-33=0/2274, 30-32=0/3369, 28-30=0/4251, 27-28=0/4756, 26-27=0/4850,  
 25-26=0/4850, 24-25=0/4708, 22-24=0/4140, 20-22=0/3363, 19-20=0/3363, 16-19=0/2283,  
 17-18=0/928  
 WEBS  
 9-26=458/253, 2-34=1350/0, 2-33=0/1087, 3-33=1050/0, 3-32=0/876, 5-32=844/0,  
 5-30=0/660, 6-30=474/0, 6-29=0/439, 7-29=487/0, 15-17=1355/0, 15-18=0/1091,  
 14-18=1065/0, 14-19=0/641, 13-19=848/0, 13-22=0/951, 12-22=637/0, 12-24=0/425,  
 11-24=454/0, 11-25=0/388, 9-25=428/421, 7-27=156/489

- NOTES-**
- Unbalanced floor live loads have been considered for this design.
  - As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
  - All plates are 3x4 MT20 unless otherwise indicated.
  - The Fabrication Tolerance at joint Z1 = 4%.
  - Plates checked for a plus or minus 0 degree rotation about its center.
  - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
  - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

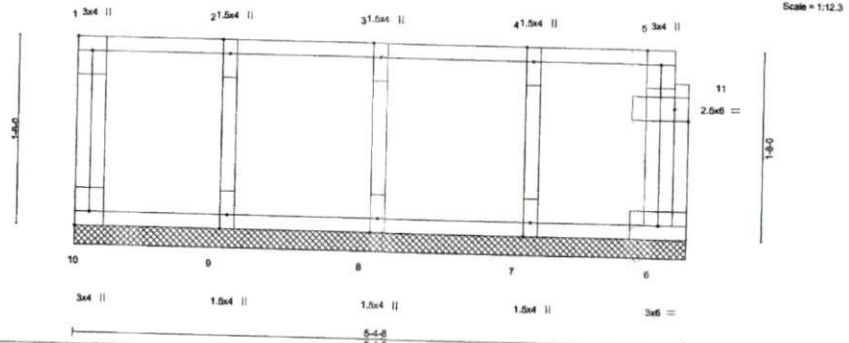


**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MB-7473 rev. 10/3/2015 BEFORE USE.**  
 Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, Q38-09 and BCSF Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**TRENCO**  
 A MITEK COMPANY  
 818 Soundside Road  
 Eden, NC 27032

Job P19-10021	Truss F06	Truss Type Floor Supported Gable	Qty 1	Ply 1	PATTERSON	E13780454
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Longleaf Truss Company, West End, NC - 27376, Job Reference (optional)  
 8.320 + Oct 29 2019 MITek Industries, Inc. Tue Nov 19 12:41:41 2019 Page 1  
 ID:X2bmUqZxt7MgVT77UDFAxyQALw-2abHgm6g9i9k-NZDz4Pm6HVqnnSE27akzYQzYHcoO



LOADING (psf)	SPACING-	2-0-0	CSL	DEFL.	in (loc)	Udefl	Ltd	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.06	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDD 10.0	Lumber DOL	1.00	BC 0.01	Vert(CT)	n/a	-	n/a		
BCLL 0.0	Rep Streas Incr	YES	WB 0.03	Horz(CT)	0.00	6	n/a		
BCDD 5.0	Code IRC2018/TPI2014		Matrix-R						
								Weight: 32 lb	FT = 8%F, 4%E

**LUMBER-**  
 TOP CHORD 2x4 SP No.1(fat)  
 BOT CHORD 2x4 SP No.1(fat)  
 WEBS 2x4 SP No.3(fat)  
 OTHERS 2x4 SP No.3(fat)

**BRACING-**  
 TOP CHORD Sheathed or 5-4-8 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 5-4-8.  
 (b) - Max Grav All reactions 250 lb or less at joint(s) 10, 6, 9, 8, 7

**FORCES.** (b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
  - 2) Plates checked for a plus or minus 0 degree rotation about its center.
  - 3) Gable requires continuous bottom chord bearing.
  - 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
  - 5) Gable studs spaced at 1-4-0 oc.
  - 6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSITPI 1.
  - 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
  - 8) CAUTION, Do not erect truss backwards.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M0-7473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MITek products. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-49 and BCS Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**ENGINEERED BY**  
**TRENCO**  
 A MIRA GROUP  
 818 Soundside Road  
 Eden, NC 27032

Job P19-10021	Truss F07	Truss Type Floor Supported Gable	Qty 1	Ply 1	PATTERSON	E13780455
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Longleaf Truss Company, West End, NC - 27376,

8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:42 2019 Page 1  
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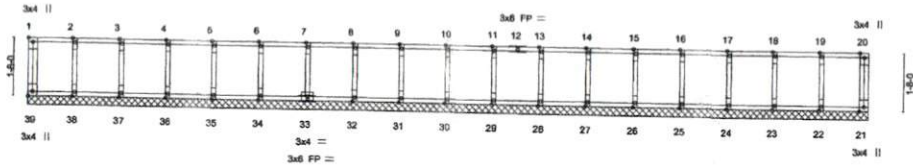


Plate Offsets (X,Y) -		[1 Edge, 0-1-8] [39 Edge, 0-1-8]		24-0-0 13-10-0	
LOADING (psf)	SPACING -	2-0-0	CSL	DEFL.	PLATES
TCLL 40.0	Plate Grip DOL	1.00	TC 0.06	in (loc)	GRIP
TCDL 10.0	Lumber DOL	1.00	BC 0.01	Vert(LL) n/a - n/a	MT20 244/190
BCLL 0.0	Rep Stress Incr	YES	WB 0.03	Vert(CT) n/a - n/a	
BCDL 5.0	Code	IRC2018/TP12014	Matrix-R	Horz(CT) 0.00 21 n/a n/a	
				Weight: 116 lb	FT = 8%F, 4%E

LUMBER -  
TOP CHORD 2x4 SP No. 1(flat)  
BOT CHORD 2x4 SP No. 1(flat)  
WEBS 2x4 SP No. 3(flat)  
OTHERS 2x4 SP No. 3(flat)

BRACING -  
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 24-0-0.  
(lb) - Max Grav All reactions 250 lb or less at joint(s) 39, 21, 38, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26, 25, 24, 23, 22

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES -
- 1) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
  - 2) All plates are 1.5x4 MT20 unless otherwise indicated.
  - 3) Plates checked for a plus or minus 0 degree rotation about its center.
  - 4) Gable requires continuous bottom chord bearing.
  - 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
  - 6) Gable studs spaced at 1-4-0 oc.
  - 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R602.10.2 and referenced standard ANSITPI 1.
  - 8) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



November 19, 2019

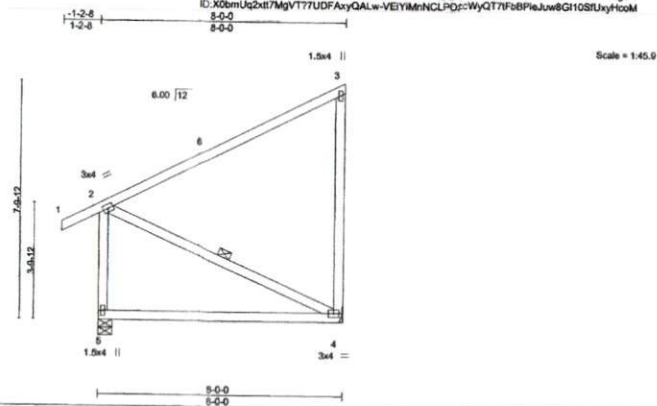
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGES 60-7473 Rev. 10/30/2018 BEFORE USE.**  
Design valid for use only with MTEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is for preventing twisting of individual trusses into end-of-chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, OSB-89 and BCS Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Albemarle, VA 22014.

**TRENCO**  
A MATEK COMPANY

818 Soundside Road  
Edenton, NC 27932

Job P19-10021	Truss M01	Truss Type Monopitch	Qty 14	Ply 1	PATTERSON	E13780456
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Longleaf Truss Company, West End, NC - 27376, 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:43 2019 Page 1  
 ID: X02mUq2d7MgV77UDFAxyQALw-VEYIMhNCLPp:WYQ77F8BPeJuw8H1G0SUxyHoom



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.91	in (loc) 499	MT20	244/190
Snow (P/Pg) 11.6/15.0	Plate Grip DOL 1.15	BC 0.58	Vert(LL) -0.19 4-5 >499 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.05	Vert(CT) -0.37 4-5 >250 180		
BCLL 0.0	Rep Stress Incr YES	Matrix-P	Horz(CT) -0.00 4 n/a n/a		
BCDL 10.0	Code IRC2018/TP12014			Weight: 85 lb	FT = 20%

**LUMBER:**  
 TOP CHORD 2x4 SP No.1  
 BOT CHORD 2x4 SP No.1  
 WEBS 2x4 SP No.3

**BRACING:**  
 TOP CHORD Sheathed or 2-2-0 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpt 2-4

**REACTIONS:** (lb/size) 4=238/Mechanical, 5=307/0-5-8  
 Max Horz 5=225(LC 9)  
 Max Uplift 4=85(LC 9), 5=-14(LC 12)  
 Max Grav 4=359(LC 24), 5=307(LC 2)

**FORCES:** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-5=320/60

- NOTES:**
- 1) Wind: ASCE 7-16, Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf, BCDL=6.0psf, h=12ft; B=45ft, L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional), cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.80 plate grip DOL=1.80
  - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf, Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
  - 3) Unbalanced snow loads have been considered for this design.
  - 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
  - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
  - 8) Refer to girder(s) for truss to truss connections.
  - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 4 and 14 lb uplift at joint 5.
  - 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSITPI 1.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M0-7473 rev. 10/3/2018 BEFORE USE.**  
 Design valid for use only with MTEK's connection. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSS-89 and BCSI Building Component Safety Information available from Truss Place Inc./Buck, 215 N. Lee Street, Suite 312, Alexandria, VA 22314.

**TRENCO**  
 ENGINEERING BY  
 618 Scenicdale Road  
 Eden, NC 27932

Job P19-10021	Truss M02	Truss Type Monopitch Supported Gable	Qty 1	Ply 1	PATTERSON	E13780457
Longleaf Truss Company, West End, NC - 27376,		Job Reference (optional) 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:43 2019 Page 1				

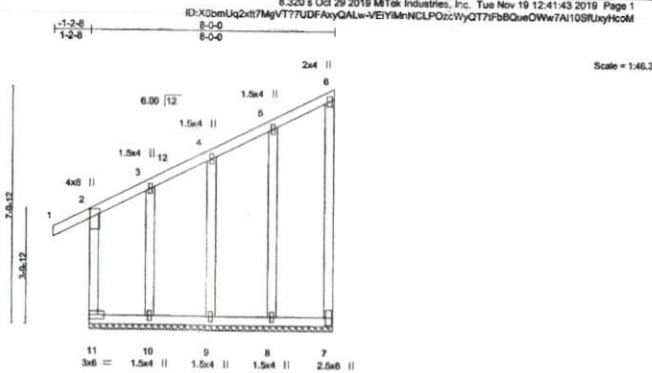


Plate Offsets (X,Y)	[2.0-3.10.0-0.0], [7.0-3.0-0.0-12]			
LOADING (psf)				
TCLL (roof)	20.0			
Snow (P/Pg)	11.6/15.0			
TCDL	10.0			
BCLL	0.0 *			
BCDL	10.0			
SPACING-	2-0-0			
Plate Grip DOL	1.15			
Lumber DOL	1.15			
Rep Stress Incr	YES			
Code	IRC2018/TPI2014			
CSL				
TC	0.83			
BC	0.29			
WB	0.12			
Matrix-R				
DEFL				
in (loc)	l'def	L/d		
Vert(LL)	-0.00	2	n/r	120
Vert(CT)	-0.00	2	n/r	120
Horz(CT)	-0.00	7	n/a	n/a
PLATES	GRIP			
MT20	244/190			
Weight: 67 lb	FT = 20%			

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1	TOP CHORD	Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.1	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2x4 SP No.3		
OTHERS	2x4 SP No.3		

**REACTIONS.** All bearings 8-0-0.  
 (lb) - Max Horiz 11=225(LC 11)  
 Max Uplift All uplift 100 lb or less at joint(s) 7 except 11=147(LC 10), 10=375(LC 9)  
 Max Grav All reactions 250 lb or less at joint(s) 7, 8, 9 except 11=370(LC 25), 10=344(LC 10)

**FORCES.** (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-11=250/53

- NOTES-**
- 1) Wind: ASCE 7-16, Vu=130mph (3-second gust) Vwd=103mph; TCCL=6.0psf; BCDL=8.0psf; h=12ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed, MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
  - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15; Plate DOL=1.15; Pg=15.0 psf; Pp=11.6 psf (Lum DOL=1.15; Plate DOL=1.15); I=1.0; Rough Cat. B; Partially Exp.; Ce=1.0; Cp=1.00; Ct=1.10
  - 4) Unbalanced snow loads have been considered for this design.
  - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
  - 6) Gable requires continuous bottom chord bearing.
  - 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
  - 8) Gable studs spaced at 2-0-0 oc.
  - 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - 11) All bearings are assumed to be User Defined crushing capacity of 425 psi.
  - 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (I=ft) 11=147, 10=375.
  - 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



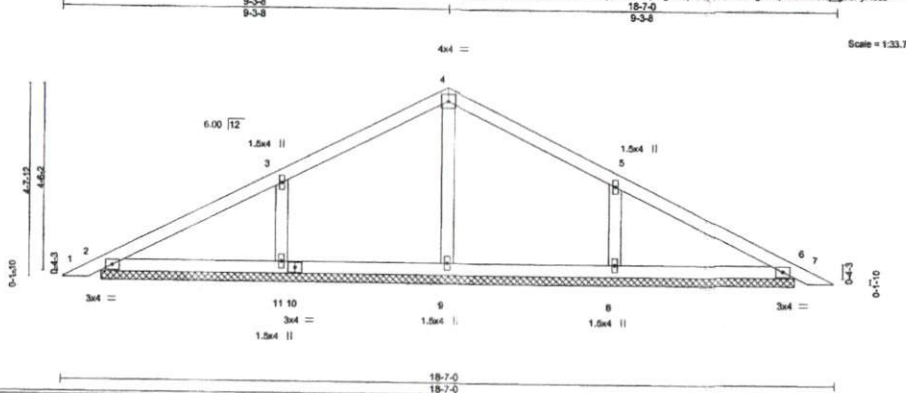
November 19, 2019

**WARNING -** Verify design parameters and READ NOTES on THIS and INCLUDED MTEK REFERENCE PAGE M0-7473 rev. 10/3/2015 BEFORE USE. Design valid for use only with MTEK connections. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, R508-01 and R509 Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**TRENCO**  
 A MTEK AFFILIATE  
 818 Soundside Road  
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	PATTERSON	E13780458
P19-10021	PB01	Piggyback	1	1		

Longleaf Truss Company, West End, NC - 27376, Job Reference (optional) ID: X0bmlUqzxt7MgVT77UDFAyQALw-wpOgKqFVGrzq4FX5ogasDp5rT07UkI\_gJ5FyHccJ 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:46 2019 Page 1



LOADING (psf)	SPACING-	CSL	DEFL	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.18	in (loc) l/dell L/d	MT20	244/160
Snow (P/F)g 11.6/15.0	Plate Grip DOL 1.15	BC 0.11	Vert(LL) 0.00 7 n/r 120		
TCDL 10.0	Lumber DOL 1.15	WB 0.07	Vert(CT) 0.01 7 n/r 120		
BCLL 0.0 *	Rep Stress liner YES	Matrix-S	Horz(CT) 0.00 6 n/a n/a		
BCDL 10.0	Code IRC2018/TP12014			Weight: 67 lb	FT = 20%

**LUMBER-**  
TOP CHORD 2x4 SP No.1  
BOT CHORD 2x4 SP No.1  
OTHERS 2x4 SP No.3

**BRACING-**  
TOP CHORD Sheathed or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

**REACTIONS.** All bearings 16-7-14.  
(lb) - Max Horz 2=77(LC 11)  
Max Uplift All uplift 100 lb or less at joint(s) 2, 11, 8, 6  
Max Grav All reactions 250 lb or less at joint(s) 2, 9, 6 except 11=397(LC 30), 6=397(LC 31)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**WEBS** 3-11=293/82, 5-8=293/62

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; Vult=130mph (3-second gust); Vasd=103mph; TCCL=6.0psf; BCCL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
  - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
  - Unbalanced snow loads have been considered for this design.
  - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
  - Gable requires continuous bottom chord bearing.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - All bearings are assumed to be User Defined crushing capacity of 425 psi.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11, 8, 6.
  - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSIT/PI 1.
  - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

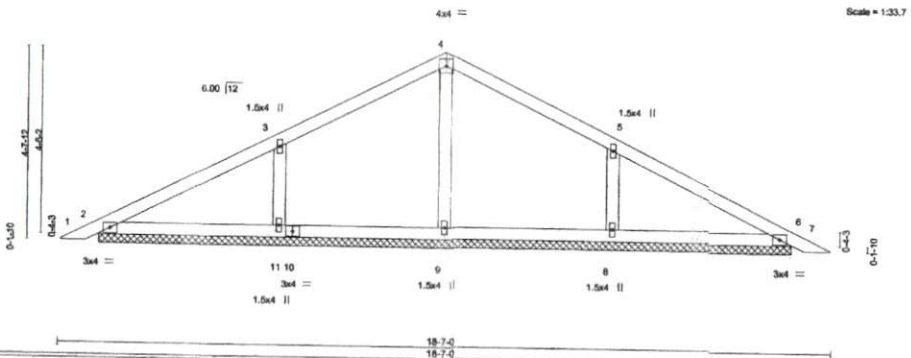


**WARNING -** Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M17473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTEK connections. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Drawing indicated is to prevent building of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIT/PI Quality Criteria, D59-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 219 N. Lee Street, Suite 312, Alexandria, VA 22314.

**LANGRADING BY**  
**TRENCO**  
A MEMBER OF  
618 Southside Road  
Edenton, NC 27932

Job P19-10021	Truss PB02	Truss Type Piggyback	Qty 23	Ply 1	PATTERSON	E13780458
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Longleaf Truss Company, West End, NC - 27376, Job Reference (optional) 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:47 2019 Page 1  
ID: X0bmUg2xt7MgVT77UDFAxyQALw-0y3XrtFavSgEqUBpPRMG1FpFaw\_uyeQtdlyHcol



LOADING (psf)									
TCLL (roof)	20.0	SPACING- 2-0-0	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
Snow (Pf/Pg)	11.6/15.0	Plate Grip DOL 1.15	TC 0.18	Vert(LL)	0.00	7	n/r	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.11	Vert(CT)	0.01	7	n/r		
BCDL	0.0	Rep Stress Iner YES	WB 0.07	Horz(CT)	0.00	6	n/a		
BCDL	10.0	Code IRC2018/TP12014	Matrix-S					Weight: 67 lb	FT = 20%

**LUMBER-**  
TOP CHORD 2x4 SP No.1  
BOT CHORD 2x4 SP No.1  
OTHERS 2x4 SP No.3

**BRACING-**  
TOP CHORD Sheathed or 6-0-0 cc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 cc bracing.

**REACTIONS.** All bearings 16-7-14.  
(lb) - Max Horz 2=77(LC 11)  
Max Uplift All uplift 100 lb or less at joint(s) 2, 11, 8, 6  
Max Grav All reactions 250 lb or less at joint(s) 2, 9, 6 except 11=397(LC 30), 8=397(LC 31)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**WEBS** 3-11=293/82, 5-8=293/82

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; VuL=130mph (3-second gust) Vbase=100mph; TC DL=6.0psf, BC DL=6.0psf, h=12ft; B=45ft; L=24ft; eave=4ft; Cat II, Exp B, Enclosed, MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.80 plate grip DOL=1.80
  - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pp=15.0 psf; Pf=11.8 psf (Lum DOL=1.15 Plate DOL=1.15); Iq=1.0; Rough Cat B, Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
  - Unbalanced snow loads have been considered for this design.
  - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.8 psf on overhangs non-concurrent with other live loads.
  - Gable requires continuous bottom chord bearing.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - All bearings are assumed to be User Defined crushing capacity of 425 psf.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11, 8, 6.
  - This truss is designed in accordance with the 2018 International Residential Code sections R302.11.1 and R802.10.2 and referenced standard ANSITP1.
  - See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE 1017473 rev. 10/3/2019 BEFORE USE.**  
Design valid for use only with MTEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the bracing, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, 05B-89 and BCS Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Job P19-10021	Truss T01	Truss Type Piggyback Base Supported Gable	Qty 1	Prj 1	PATTERSON	E13780460
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Longleaf Truss Company, West End, NC - 27376, Job Reference (optional)  
 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:52 2019 Page 1  
 ID: X02mUq2x7MgV77UDFAxyQALw-kzy6Ru046Y7YgRan\_6U384GWmXAbd5w7JvJyHcoD

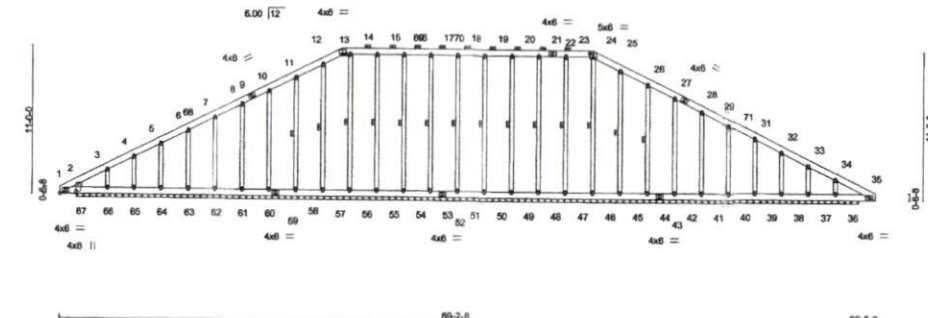


Plate Offsets (X,Y) - [2-0-0-14,0-1-12] [2-1-0-3-0-3-4] [67-0-1-6,1-2-4] [67-0-0-0,0-1-12]  
 69-2-8 69-2-8 69-5-0 7-2-8

LOADING (psf)	SPACING-	2-0-0	CSL	DEFL	in (loc)	l'defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15	TC 0.10	TC 0.10	Vert(LL) n/a	-	n/a	999	MT20	244/190
Snow (P/Pg) 16.5/15.0	Lumber DOL 1.15	BC 0.09	BC 0.09	Vert(CT) n/a	-	n/a	999		
TCDL 10.0	Rep Stress Incr YES	WB 0.17	WB 0.17	Horz(CT) -0.01	36	n/a	n/a		
BCLL 0.0	Code IRC2018/TPI2014	Matrix-S	Matrix-S						
BCDL 10.0								Weight: 586 lb	FT = 20%

LUMBER-  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 OTHERS 2x4 SP No.3

BRACING-  
 TOP CHORD Sheathed or 10-0-0 oc purlins, except  
 2-0-0 oc purlins (10-0-0 max.); 13-24.  
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.  
 WEBS 1 Row at midpt  
 24-46, 23-47, 21-48, 20-49, 19-50, 18-51,  
 17-53, 16-54, 15-55, 14-56, 12-57, 11-58,  
 25-45, 26-44

REACTIONS. All bearings 58-0-0.  
 (b) - Max Horz 67=20(LC 10)  
 Max Uplift All uplift 100 lb or less at joint(s) 48, 49, 54, 56, 60, 61, 62, 63, 64, 65, 66, 67, 44, 42, 41, 40, 39, 38, 37  
 Max Grav All reactions 250 lb or less at joint(s) 46, 47, 48, 49, 50, 51, 53, 54, 55, 56, 57, 58, 60, 61, 62, 63, 64, 65, 66, 67, 45, 44, 42, 41, 40, 39, 38, 37 except 36=40(LC 50)

FORCES. (b) - Max. Comp./Max. Ten. - All forces 250 (b) or less except when shown.

- NOTES-
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vase=103mph; TCCL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=60ft; eave=2ft; Cat. II, Exp B, Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - TCLL ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pp=15.0 psf; Pp=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Ipr=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.0; Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IRC 1608.3.4.
  - Unbalanced snow loads have been considered for this design.
  - WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSP"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MTEK assumes no responsibility for truss manufacture, handling, erection, or bracing.
  - Provide adequate drainage to prevent water ponding.
  - All plates are 2x4 MT20 unless otherwise indicated.
  - Gable studs spaced at 2-0-0 oc.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - All bearings are assumed to be User Defined crushing capacity of 425 psf.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 48, 49, 54, 58, 60, 61, 62, 63, 64, 65, 66, 67, 44, 42, 41, 40, 39, 38, 37.



November 19, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGES BEFORE USE. Design valid for use only with MTEK connectors. This design is based upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 210 N. Lee Street, Suite 312, Alexandria, VA 22314.

DESIGNED BY  
**TRENCO**  
 A NRSI AFFILIATE  
 818 Southside Road  
 Edenboro, NC 27932

Job P19-10021	Truss T01	Truss Type Piggyback Base Supported Gable	Qty 1	Ply 1	PATTERSON	E13780460
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Longleaf Truss Company, West End, NC - 27376,

8.320 s Oct 29 2019 MTEK Industriell, Inc. Tue Nov 19 12:41:53 2019 Page 2  
 ID:X0bmUqZdt7MgVT77UDFAxyQALw-D9JKomvetQz\_A9H7aIDfCJgs7GdmKatlElyHocC

- NOTES-**
- 14) Non Standard bearing condition. Review required.
  - 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSITP1 1.
  - 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

<p><b>WARNING</b> - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1743 rev. 10/03/2015 BEFORE USE.</p> <p>Design valid for use only with MTEK connection. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, OSB-09 and ICSI Building Component Safety Information available from Truss Plate Institute, 210 N. Lee Street, Suite 312, Alexandria, VA 22314.</p>	<p>ENGINEERED BY  <b>TRENCO</b>  <small>A MITEK COMPANY</small></p> <p>819 Soundside Road        Eden, NC 27522</p>
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Job P19-10021	Truss T01GE	Truss Type Common Supported Gable	Qty 1	Ply 1	PATTERSON	E13780481
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Longleaf Truss Company, West End, NC - 27376, 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:54 2019 Page 1  
 ID:X0cmUqzxt7MgVt77UDFvYQALw-HM76wGokemJs3ZhpSBwS74BQ72wZEcNoyHcoB  
 15-2-8 15-2-8 30-5-0 30-5-0 15-2-8  
 6.00 [12] 4x4 =

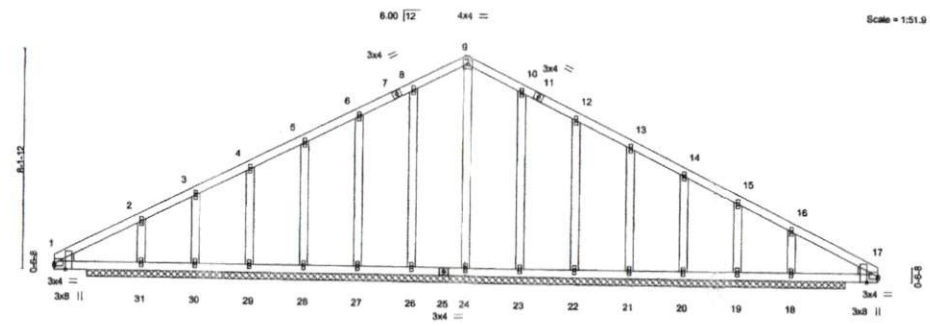


Plate Offsets (X,Y)	[1.0-0.0-1-1], [1.0-2-9,Edge], [17.0-2-9,Edge]
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LOADING (psf)	SPACING	2-0-0	CSL	DEFL.	in (loc)	l'defl	L/d	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL) n/a	-	n/a	999	MT20	244/190	
Snow (PIPg) 11.6/15.0	Lumber DOL 1.15	BC 0.14	Vert(CT) n/a	-	n/a	999			
TCDL 10.0	Rep Stress Incr YES	WB 0.31	Horz(CT) -0.01	18	n/a	n/a			
BCLL 0.0	Code IRC2018/TP1014	Matrix-S							
BCDL 10.0							Weight: 182 lb	FT = 20%	

**LUMBER:**  
 TOP CHORD 2x4 SP No.1  
 BOT CHORD 2x4 SP No.1  
 OTHERS 2x4 SP No.3  
**WEDGE**  
 Left: 2x4 SP No.3, Right: 2x4 SP No.3

**BRACING:**  
 TOP CHORD Sheathed or 10-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 5-0-0 oc bracing.

**REACTIONS.** All bearings 28-0-0  
 (lb) - Max Horz 31=142(LC 11)  
 Max Uplift All uplift 100 lb or less at joint(s) 26, 27, 28, 29, 30, 31, 23, 22, 21, 20, 19, 18  
 Max Grav All reactions 250 lb or less at joint(s) 26, 27, 28, 29, 30, 23, 22, 21, 20, 19 except 24=304(LC 2), 31=373(LC 29), 18=373(LC 30)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 1-2=14/268, 16-17=13/265  
 WEBS 9-24=264/0

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-16; Valt=130mph (3-second gust) Vast=103mph; TCCL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=30ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - 4) TCLL: ASCE 7-16; P=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pp=15.0 psf; P1=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Ie=1.0, Rough Cat B, Partially Exp.; Ce=1.0, Cp=1.00; Ct=1.10
  - 5) Unbalanced snow loads have been considered for this design.
  - 6) All plates are 1.5x4 MT20 unless otherwise indicated.
  - 7) Gable studs spaced at 2-0-0 oc.
  - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 9) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - 10) All bearings are assumed to be Used Defined crushing capacity of 425 psi.
  - 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26, 27, 28, 29, 30, 31, 23, 22, 21, 20, 19, 18.
  - 12) Non Standard bearing condition. Review required.
  - 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M0-7473 on 10/30/2019 BEFORE USE.**  
 Design valid for use only with MTEK connectors. This design is based only upon parameters shown, and is for individual building component, not building design. Building designer must verify the applicability of design parameters and properly interpret the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personnel injury and property damage. For general guidelines regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSS-89 and BCSJ Building Component Safety Information available from Truss Fabricators, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.

**ENGINEERING BY**  
**TRENCO**  
 A MESA Alliance  
 818 Southside Road  
 Eden, NC 27622

Job P19-10021	Truss T02	Truss Type Piggyback Base	Qty 7	Ply 1	PATTERSON	E13780462
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Longleaf Truss Company, West End, NC - 27376, Job Reference (optional)  
 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:56 2019 Page 1  
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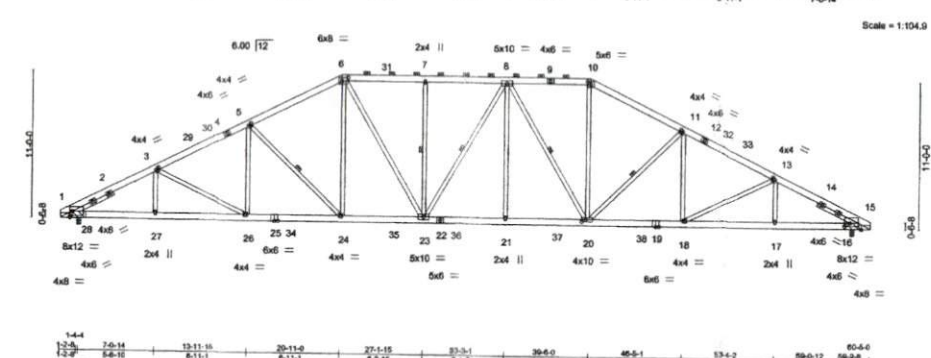


Plate Offsets (X, Y)	[1.0-2.1-0.4-0], [1.0-2.2-Edge], [3.0-1.8-0.2-0], [6.0-1.8-0.2-0], [8.0-5.4-0.3-0], [10.0-3.0-0.2-12], [11.0-1.8-0.2-0], [13.0-1.8-0.2-0], [15.0-10.0-Edge], [15.0-4.13.0-2-14], [20.0-3.8-0.2-0]
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<b>LOADING (psf)</b>	SPACING-	CSL	DEFL	PLATES	GRIP
TCLL (roof) 20.0	2-0-0	TC 0.20	in (loc) / l/d	MT20	244/190
Snow (PIPg) 16.5/15.0	Plate Grip DOL 1.15	BC 0.85	Vert(LL) -0.31 21-23 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.68	Vert(CT) -0.56 21-23 >999 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-S	Horz(CT) 0.23 15 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014				
				Weight: 501 lb	FT = 20%

<b>LUMBER-</b>	<b>BRACING-</b>
TOP CHORD 2x6 SP No.1	TOP CHORD Sheathed or 3-4-5 oc purlins, except
BOT CHORD 2x6 SP No.1	2-0-0 oc purlins (3-10-0 max.); 8-10-
WEBS 2x4 SP No.3	Rigid coating directly applied or 10-0-0 oc bracing.
SLIDER Left 2x4 SP No.3-H 3-4-11, Right 2x4 SP No.3-H 3-4-11	WEBS 1 Row at midpt 6-24, 7-23, 8-23, 8-20, 11-20

**REACTIONS.** (lb/size) 1=1990(0-3-8 + bearing block) (req. 0-4-6), 15=1990(0-3-8 + bearing block) (req. 0-4-6)  
 Max Horz 1=204(LC 10)  
 Max Grav 1=2799(LC 23), 15=2797(LC 24)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**TOP CHORD** 1-3=-3469/23, 3-5=-4872/69, 5-8=-4150/114, 6-7=-3954/125, 7-8=-3954/125, 8-10=-3689/123, 10-11=-4148/113, 11-13=-4866/70, 13-15=-5466/23  
**BOT CHORD** 1-27=0/4942, 26-27=0/4942, 24-26=0/4028, 23-24=0/3726, 21-23=0/3984, 20-21=0/3984, 18-20=0/4249, 17-18=0/4787, 15-17=0/4787  
**WEBS** 3-27=0/274, 3-26=601/49, 5-26=0/570, 5-24=-1032/86, 6-24=0/1014, 6-23=-2/828, 7-23=-481/73, 8-21=0/332, 8-20=-835/5, 10-20=0/1489, 11-20=-1026/7, 11-16=0/568, 13-18=-606/49, 13-17=0/275

- NOTES-**
- 2x6 SP No.1 bearing block 12" long at j. 1 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity=425psf.
  - 2x6 SP No.1 bearing block 12" long at j. 15 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. User Defined Bearing crushing capacity=425psf.
  - Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; Vult=130mph (3-second gust); Vase=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=60ft; eave=7ft; Cat. II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
  - TCLL ASCE 7-16; P=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); h=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1808.3.4.
  - Unbalanced snow loads have been considered for this design.
  - WARNING:** This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses (BCSIF), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MTEK assumes no responsibility for truss manufacture, handling, erection, or bracing.
  - Provide adequate drainage to prevent water ponding.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

**WARNING** Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE 1867473 rev. 19832915 BEFORE USE. Design valid for use only with MTEK's connectors. This design is based only upon parameters shown, and is for individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss webs and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personnel injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIF77H Quality Criteria, DSD-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**TRENCO**  
 A SBCA AFFILIATE  
 818 Soundside Road  
 Eden, NC 27632



Job	Truss	Truss Type	Qty	Ply	PATTERSON	E13780462
P19-10021	T02	Piggyback Base	7	1		

Longleaf Truss Company, West End, NC - 27376.

8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:56 2019 Page 2  
 ID: X0bmUqzdt7MgVT77UDFAyQALw-dk7SQuX8L2Y1ccSglawGKE8kzTadDOYsSgyHcc9

**NOTES-**

- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R602.10.2 and referenced standard ANSITPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**WARNING:** Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE 88-7473 rev. 10032019 BEFORE USE  
 Design valid for use only with METAB connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plus Institute, 215 N. Lee Street, Suite 312, Alexandria, VA 22314.

DESIGNED BY  
**TRENCO**  
 A MESA GROUP

818 Soundside Road  
 Eden, NC 27632

Job P19-10021	Truss T03	Truss Type Piggyback Base	Qty 16	Ply 1	PATTERSON	E13780463
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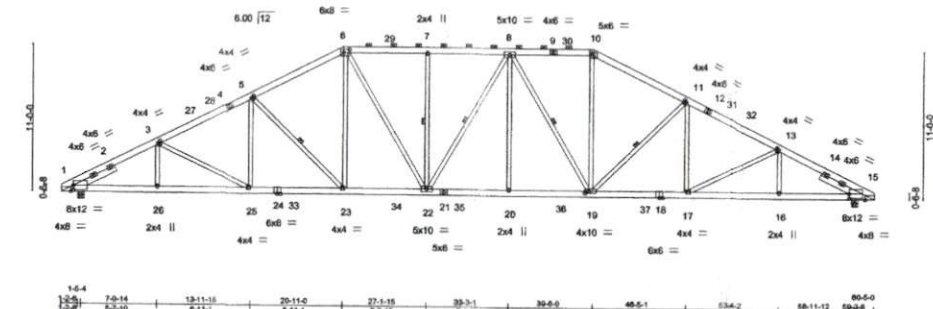
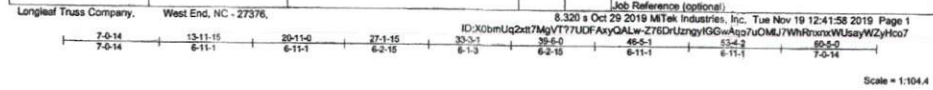


Plate Offsets (X,Y) - [1-0-0-2-0-4-0], [1-0-2-2-Edge], [3-0-1-8-0-2-0], [5-0-1-8-0-2-0], [6-0-6-4-0-3-0], [10-0-3-0-0-2-12], [11-0-1-8-0-2-0], [13-0-1-8-0-2-0], [15-1-2-7-Edge], [15-0-0-2-0-4-0], [19-0-3-8-0-2-0]

LOADING (psf)	SPACING-	2-0-0	CSL	DEFL.	In (loc)	Vdefl	L/d	PLATES	GRIP
TCLL (roof)	Plate Grip DOL	1.15	TC	0.30	-0.31	20-22	>999	240	244/190
Snow (P/Pg)	Lumber DOL	1.15	BC	0.62	Vert(L)	-0.55	20-22	>999	180
TCDL	Rep Stress Incr	YES	WB	0.63	Horz(CT)	0.23	15	n/a	n/a
BCLL	Code	IRC2018/TPI2014	Matrix-S						
BCDL									
								Weight: 502 lb	FT = 20%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Sheathed or 3-4-14 oc purlins, except
BOT CHORD 2x6 SP No.1	2-0 oc purlins (3-10-11 max.); 6-10,
WEBS 2x4 SP No.3	Rigid casing directly applied or 18-0-0 oc bracing.
SLIDER Left 2x6 SP No.1 -H 3-4-11, Right 2x6 SP No.1 -H 3-4-11	WEBS 1 Row at midpt 5-23, 7-22, 8-22, 8-19, 11-19

REACTIONS. (lb/size) 1=19850-5-8, 15=19850-5-8  
Max Horz 1=204(LC 11)  
Max Grav 1=2792(LC 23), 15=2790(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
TOP CHORD 1-3=5400/23, 3-5=4619/69, 5-6=4127/114, 6-7=3033/125, 7-8=3932/125, 8-10=3647/23, 10-11=4124/113, 11-13=4813/70, 13-15=5397/23  
BOT CHORD 1-26=94889, 25-26=0/369, 23-25=0/4377, 22-23=0/3704, 20-22=0/3963, 19-20=0/3963, 17-19=0/4218, 16-17=0/4714, 15-16=0/4714  
WEBS 3-26=0/268, 3-25=554/49, 5-25=0/550, 5-23=1020/66, 6-23=0/1005, 6-22=2/828, 7-22=481/73, 8-20=0/333, 8-19=835/5, 10-19=0/1459, 11-19=1013/67, 11-17=0/547, 13-17=559/48, 13-16=0/269

- NOTES-
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16, Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=60ft; eave=7ft; Cat II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed; e and vertical left and right exposed; Lumber DOL=1.80 plate grip DOL=1.80
  - TCLL ASCE 7-16, P=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; P=16.5 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0, Cs=1.00, Ch=1.0, Lu=50-0-0; Min: flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IRC 1608.3.4.
  - Unbalanced snow loads have been considered for this design.
  - WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses (BCSP), jointly produced by SBCEA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MTEK assumes no responsibility for truss manufacture, handling, erection, or bracing.
  - Provide adequate drainage to prevent water ponding.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
  - All bearings are assumed to be User Defined crushing capacity of 425 psi.
  - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Conditio...



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M0-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTEK connection. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-29 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERS BY  
**TRENCO**  
A MTEK AFFILIATE  
818 Scenic Road  
Eden, NC 27932

Job PH-10021	Truss T03	Truss Type Piggyback Base	Qty 16	Ply 1	PATTERSON	E13780463
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Longleaf Truss Company, West End, NC - 27376, 8.320 s Oct 29 2019 MTEK Industries, Inc. Tue Nov 19 12:41:58 2019 Page 2  
 ID:XX0bmUq2xt7MgVT77UDFAxyQALw-276DrUzngyIGGwAqo7uQMj7WhRrnxWUsayWZyHco7

**NOTES:**  
 1) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

**WARNING** - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE No. 7473 rev. 1803/2015 BEFORE USE.  
 Design valid for use only with MTEK's connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate the design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For precise guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIPPT Quality Criteria, DSB-85 and DCS Building Component Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.

**ENGINEERED BY**  
**TRENCO**  
 A MTEK COMPANY  
 818 Soundside Road  
 Eden, NC 27622

Job P19-10021	Truss T04	Truss Type Common	City 14	Ply 1	PATTERSON	E13780464
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Longleaf Truss Company, West End, NC - 27376, Job Reference (optional) 8.320 s Oct 29 2019 M Tek Industries, Inc. Tue Nov 19 12:41:59 2019 Page 1  
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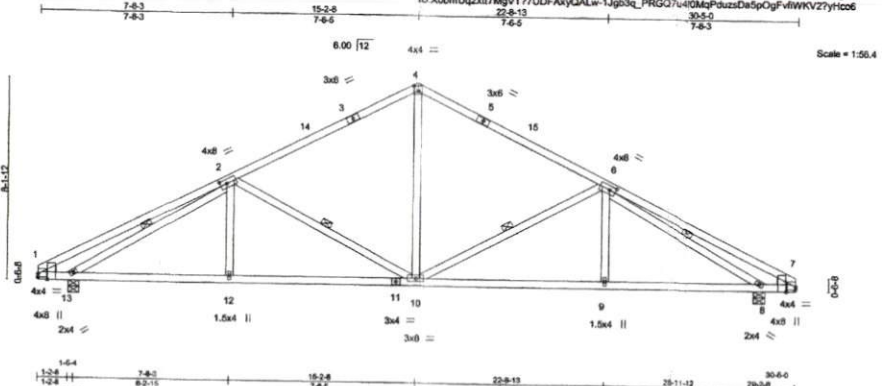


Plate Offsets (X, Y) [1-0-0-0-1-9], [1-0-2-9, Edge], [2-0-3-1-0-2-0], [4-0-2-0-0-2-8], [6-0-3-1-0-2-0], [7-Edge, 0-1-9], [7-0-2-9, Edge]

LOADING (psf)	SPACING-	2-0-0	CSL	DEFL.	in (loc)	l'def	Lid	PLATES	GRIP
TCLL (roof) 20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.07	10-12	>999	240
Snow (PIPg) 11.6/15.0	Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.17	10-12	>999	180
TCDL 10.0	Rep Stress Incr	YES	WB	0.52	Horz(CT)	0.06	8	n/a	n/a
BCLL 0.0	Code IRC2018/TP12014		Matrix-S						
BCDL 10.0									
								Weight: 167 lb	FT = 20%

**LUMBER-**  
 TOP CHORD 2x4 SP No.1  
 BOT CHORD 2x4 SP No.1  
 WEBS 2x4 SP No.3  
 WEDGE  
 Left: 2x4 SP No.3 ; Right: 2x4 SP No.3

**BRACING-**  
 TOP CHORD Sheathed or 4-10-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing  
 WEBS 1 Row at midpt 6-10; 2-10, 2-13, 6-8

**REACTIONS.** (lb/size) 13=960/D-5-8, 8=960/D-5-8  
 Max Horz 13=142(LC 11)  
 Max Uplift 13=20(LC 12)  
 Max Grav 13=1217(LC 2), 8=1217(LC 2)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 1-2=251/28, 2-4=1285/75, 4-6=1285/75, 6-7=251/29  
 BOT CHORD 12-13=0/1455, 10-12=0/1455, 9-10=0/1455, 8-9=0/1455  
 WEBS 4-10=0/845, 6-10=528/70, 6-9=0/268, 2-10=528/68, 2-12=0/268, 2-13=1592/62,  
 6-8=1592/47

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf, BCDL=6.0psf, h=12ft; B=45ft; L=30ft; eave=4ft; Cat II; Exp B; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
  - TCLL: ASCE 7-16; Pn=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pp=15.0 psf; Pn=11.6 psf; Cn=1.00; Ct=1.10 DOL=1.15; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Cw=1.00
  - Unbalanced snow loads have been considered for this design.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
  - All bearings are assumed to be User Defined crushing capacity of 425 psi.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13.
  - This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R602.10.2 and referenced standard ANSITPI 1.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED METER REFERENCE PAGE 80-7473 Rev. 10032015 BEFORE USE.**  
 Design valid for use only with MET web connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidelines regarding the fabrication, storage, shipping, erection and bracing of trusses and truss systems, see ANSITPI Quality Criteria, DSB-89 and IBCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**TRENCO**  
 ENGINEERING BY  
 A M TEK INDUSTRY  
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 Selma, NC 27582

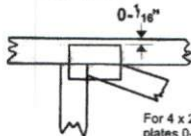


## Symbols

### PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

\*Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

### BEARING

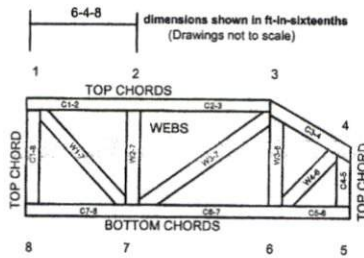


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

### Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.  
DSB-89: Design Standard for Bracing.  
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

## Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

### PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1966  
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

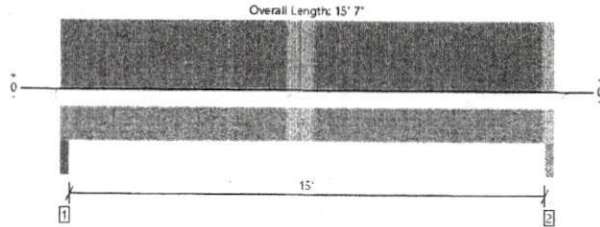


## General Safety Notes

### Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

Level, Beam over storage area TS2  
**3 piece(s) 1 3/4" x 16" 2.0E Microllam® LVL**



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load Combination (Pattern)
Member Reaction (lbs)	7203 @ 2"	13322 (3.50")	Passed (54%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	5701 @ 1' 7 1/2"	19950	Passed (29%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	26876 @ 7' 9 1/2"	58339	Passed (46%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Def. (in)	0.228 @ 7' 9 1/2"	0.508	Passed (L/804)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Def. (in)	0.351 @ 7' 9 1/2"	0.762	Passed (L/522)	--	1.0 D + 1.0 Lr (All Spans)

System : Floor  
 Member Type : Drop Beam  
 Building Use : Residential  
 Building Code : IBC 2015  
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Top Edge Bracing (Lu): Top compression edge must be braced at 13' 5" o/c unless detailed otherwise.
- Bottom Edge Bracing (Lb): Bottom compression edge must be braced at 15' 7" o/c unless detailed otherwise.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Column - SPF	3.50"	3.50"	1.89"	2528	4675	3857	11060	None
2 - Column - SPF	3.50"	3.50"	1.89"	2528	4675	3857	11060	None

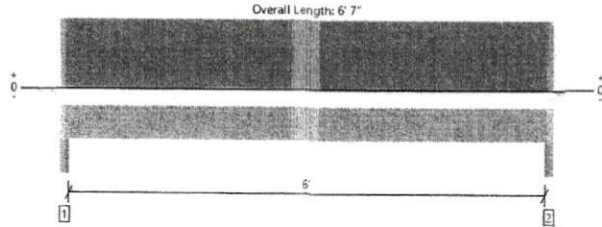
Vertical Loads	Location (side)	Tributary Width	Dead (D.90)	Roof Live (non-snow, 1.25)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 15' 7"	N/A	24.5	--	--	
1 - Uniform (PSF)	0 to 15' 7" (Top)	30'	10.0	20.0	16.5	TS 2 Truss load

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ForteWEB Software Operator	Job Notes
Jay Johnson Weyerhaeuser (910) 495-5383 jay.johnson@weyerhaeuser.com	

Level, Front Porch beam/side FO2  
**3 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL**



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load Combination (Pattern)
Member Reaction (lbs)	3369 @ 2"	13322 (3.50")	Passed (25%)	--	1.0 D + 0.75 L + 0.75 Lr (All Spans)
Shear (lbs)	1907 @ 1' 3 3/8"	11845	Passed (16%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	4631 @ 3' 3 1/2"	26772	Passed (17%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.022 @ 3' 3 1/2"	0.313	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 Lr (All Spans)
Total Load Defl. (in)	0.033 @ 3' 3 1/2"	0.417	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 Lr (All Spans)

System : Roof  
 Member Type : Drop Beam  
 Building Use : Residential  
 Building Code : IRC 2015  
 Design Methodology : ASD  
 Member P/Nch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Top Edge Bracing (Lu): Top compression edge must be braced at 6' 7" o/c unless detailed otherwise.
- Bottom Edge Bracing (Lb): Bottom compression edge must be braced at 6' 7" o/c unless detailed otherwise.

Supports	Bearing Length			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Column - SPF	3.50"	3.50"	1.50"	1147	1975	988	741	4851	None
2 - Column - SPF	3.50"	3.50"	1.50"	1147	1975	988	741	4851	None

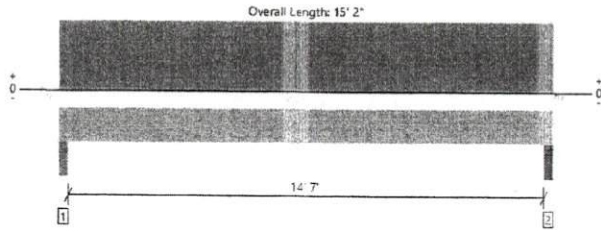
Vertical Loads	Location (Side)	Tributary Width	Dead (0.80)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 6' 7"	N/A	18.2	--	--	--	
1 - Uniform (PSF)	0 to 6' 7" (Top)	15'	12.0	-	20.0	15.0	Roof Load
2 - Uniform (PSF)	0 to 6' 7" (Top)	15'	10.0	40.0	-	-	Floor load

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ForteWEB Software Operator	Job Notes
Jay Johnson Weyerhaeuser (910) 495-5383 Jay.Johnson@weyerhaeuser.com	

Level, Front Porch beam/front F01  
**3 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL**



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1174 @ 15'	7809 (3.50*)	Passed (15%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	975 @ 1' 3 3/8"	14807	Passed (7%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	4255 @ 7' 7"	33465	Passed (13%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.064 @ 7' 7"	0.742	Passed (L/999+)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.123 @ 7' 7"	0.989	Passed (L/999+)	--	1.0 D + 1.0 Lr (All Spans)

System : Roof  
 Member Type : Drop Beam  
 Building Use : Residential  
 Building Code : IBC 2015  
 Design Methodology : ASD  
 Member Pkch : Q/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Top Edge Bracing (Lu): Top compression edge must be braced at 15' 2" o/c unless detailed otherwise.
- Bottom Edge Bracing (Lb): Bottom compression edge must be braced at 15' 2" o/c unless detailed otherwise.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Column - SPF	3.50"	3.50"	1.50"	567	607	455	1629	None
2 - Stud wall - SPF	3.50"	3.50"	1.50"	567	607	455	1629	None

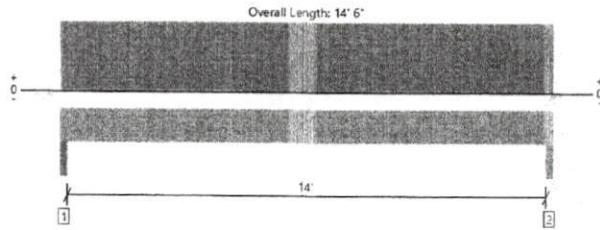
Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
0 - Self Weight (HLF)	0 to 15' 2"	N/A	18.2	--	--	
1 - Uniform (PSF)	0 to 15' 2" (Top)	2'	14.1	20.0	15.0	Roof gable end
2 - Uniform (PSF)	0 to 15' 2" (Top)	2'	14.1	20.0	15.0	Floor

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ForteWEB Software Operator	Job Notes
Jay Johnson Weyerhaeuser (910) 495-5383 jay.johnson@weyerhaeuser.com	

Level, Garage Beam  
**3 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL**



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load Combination (Pattern)
Member Reaction (lbs)	4739 @ 1 1/2"	11813 (3.00")	Passed (40%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	3928 @ 1' 2 7/8"	14807	Passed (27%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	16590 @ 7' 3"	33465	Passed (50%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.204 @ 7' 3"	0.712	Passed (L/B38)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.445 @ 7' 3"	0.950	Passed (L/385)	--	1.0 D + 1.0 Lr (All Spans)

System : Roof  
 Member Type : Drop Beam  
 Building Use : Residential  
 Building Code : IRC 2015  
 Design Methodology : ASD  
 Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Top Edge Bracing (Lu): Top compression edge must be braced at 14' 6" o/c unless detailed otherwise.
- Bottom Edge Bracing (Lb): Bottom compression edge must be braced at 14' 6" o/c unless detailed otherwise.

Supports	Bearing Length			Loads to Supports (lb)			Accessories	
	Total	Available	Required	Dead	Roof Live	Snow		Total
1 - Column - SYP	3.00"	3.00"	1.50"	2564	2175	1631	6370	None
2 - Column - SYP	3.00"	3.00"	1.50"	2564	2175	1631	6370	None

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow 1.25)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 14' 0"	N/A	18.2	--	--	
1 - Uniform (PSF)	0 to 14' 6" (Top)	15'	22.4	20.0	15.0	roof

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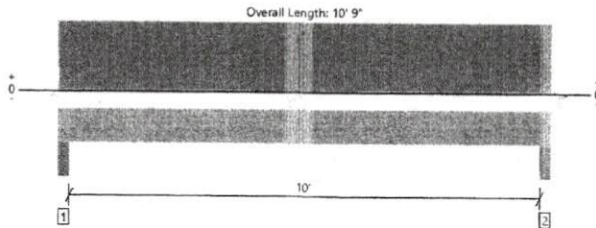


ForteWEB Software Operator	Job Notes
Jay Johnson Weyerhaeuser (910) 495-5383 j.johnson@weyerhaeuser.com	

**MEMBER REPORT**

**PASSED**

Level, Garage Door header  
 3 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load Combination (Pattern)
Member Reaction (lbs)	5261 @ 3"	17128 (4.50")	Passed (31%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	3925 @ 1' 4 3/8"	14807	Passed (27%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	12854 @ 5' 4 1/2"	33465	Passed (38%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.116 @ 5' 4 1/2"	0.342	Passed (L/999+)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.190 @ 5' 4 1/2"	0.512	Passed (L/640)	--	1.0 D + 1.0 Lr (All Spans)

System : Wall  
 Member Type : Header  
 Building Use : Residential  
 Building Code : IRC 2015  
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Top Edge Bracing (Lu): Top compression edge must be braced at 10' 9" o/c unless detailed otherwise.
- Bottom Edge Bracing (Lu): Bottom compression edge must be braced at 10' 9" o/c unless detailed otherwise.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	4.50"	4.50"	1.50"	2036	3225	2419	7680	None
2 - Trimmer - SPF	4.50"	4.50"	1.50"	2036	3225	2419	7680	None

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 2.25)	Snow (1.15)	Comments
0 - Self Weight (PLP)	0 to 10' 9"	N/A	18.2	--	--	
1 - Uniform (PSF)	0 to 10' 9"	30'	12.0	20.0	15.0	Roof Load

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 The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator



ForteWEB Software Operator	Job Notes
Jay Johnson Weyerhaeuser (910) 495-5383 j.johnson@weyerhaeuser.com	



**Triple 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2650 SP**

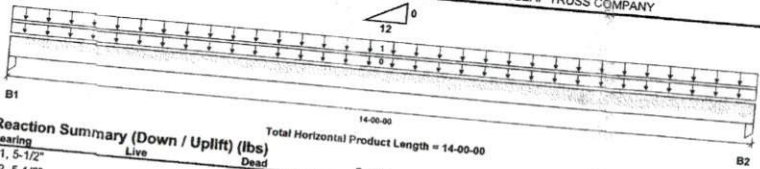
BC CALC® Member Report  
 Build 7295  
 Job name:  
 Address:  
 City, State, Zip:  
 Customer:  
 Code reports: ESR-1040

RB02 (Roof Beam)  
 Dry | 1 span | No cant.

**PASSED**

November 12, 2019 09:25:37

File name:  
 Description:  
 Specifier:  
 Designer: TOM WALKER  
 Company: LONGLEAF TRUSS COMPANY



**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 5-1/2"	1106 / 0	1960 / 0	1960 / 0		1960 / 0
B2, 5-1/2"	1106 / 0	1960 / 0	1960 / 0		1960 / 0

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live	Dead	Snow	Wind	Roof Live	Tributary
0	Self-Weight											
1		Unit. Lin. (lb/ft)	L	00-00-00	14-00-00	Top	100%	90%	115%	160%	125%	00-00-00
		Unit. Area (lb/ft²)	L	00-00-00	14-00-00	Top		10	20		20	14-00-00

**Controls Summary**

Pos. Moment	Value	% Allowable	Duration	Case	Location
End Shear	9553 ft-lbs	30.5%	115%	5	07-00-00
Total Load Deflection	2432 lbs	17.9%	115%	5	01-05-06
Live Load Deflection	L/R58 (0.241")	27.4%	nia	4	07-00-00
Max Defl.	L/1029 (0.154")	23.3%	nia	6	07-00-00
Span / Depth	0.241"	48.2%	nia	4	07-00-00

**Bearing Supports**

Bearing	Column	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column	5-1/2" x 5-1/4"	3066 lbs	12.9%	14.2%	Southern Pine
B2	Column	5-1/2" x 5-1/4"	3066 lbs	12.9%	14.2%	Southern Pine

**Cautions**

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.  
 For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge

**Notes**

Design meets Code minimum (L/180) Total load deflection criteria.  
 Design meets Code minimum (L/240) Live load deflection criteria.  
 Design meets arbitrary (0.5") Maximum Total load deflection criteria.  
 Calculations assume member is fully braced.  
 BC CALC® analysis is based on IBC 2009.  
 Design based on Dry Service Condition.



Triple 1-3/4" x 11-7/8" VERSA-LAM® 1.7 2650 SP

PASSED

BC CALC® Member Report

RB02 (Roof Beam)

November 12, 2019 09:25:37

Build 7295

Dry | 1 span | No cant.

Job name:

File name:

Address:

Description:

City, State, Zip:

Specifier:

Customer:

Designer:

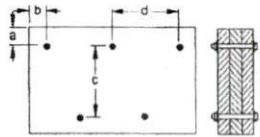
TOM WALKER

Code reports: ESR-1040

Company:

LONGLEAF TRUSS COMPANY

Connection Diagram: Full Length of Member



a minimum = 2"      c = 7-7/8"  
b minimum = 2-1/2"      d = 24"

Bolts are assumed to be Grade A307 or Grade 2 or higher.  
Connectors are: 1/2 in. Staggered Through Bolt

**Disclosure**

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOISTS®, BC RIM BOARD™, BC®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®.



**Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP****PASSED**

BC CALC® Member Report

**RB01 (Roof Beam)**

November 12, 2019 09:23:16

Build 7295

Dry | 1 span | No cant.

Job name:

File name:

Address:

Description:

City, State, Zip:

Specifier:

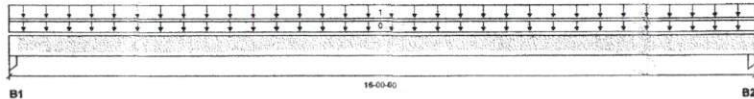
Customer:

Designer:

TOM WALKER

Code reports: ESR-1040

Company: LONGLEAF TRUSS COMPANY

**Reaction Summary (Down / Uplift) (lbs)**

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 5-1/2"		2595 / 0	4800 / 0		4800 / 0
B2, 5-1/2"		2595 / 0	4800 / 0		4800 / 0

**Load Summary**

Tag	Description	Load Type	Ref.	Start	End	Loc.	Live 100%	Dead 90%	Snow 115%	Wind 180%	Roof Live 125%	Tributary
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	16-00-00	Top	24					00-00-00
1		Unf. Area (lb/ft²)	L	00-00-00	16-00-00	Top	10	20		20		30-00-00

**Controls Summary**

	Value	% Allowable	Duration	Case	Location
Pos. Moment	26724 ft-lbs	41.5%	115%	5	08-00-00
End Shear	5739 lbs	31.3%	115%	5	01-09-08
Total Load Deflection	L/588 (0.31")	30.6%	n/a	4	08-00-00
Live Load Deflection	L/906 (0.202")	26.5%	n/a	6	08-00-00
Max Defl.	0.31"	62.1%	n/a	4	08-00-00
Span / Depth	11.4				

**Bearing Supports**

	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column 5-1/2" x 5-1/4"	7395 lbs	31.0%	34.1%	Southern Pine
B2	Column 5-1/2" x 5-1/4"	7395 lbs	31.0%	34.1%	Southern Pine

**Cautions**

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

**Notes**

Design meets Code minimum (L/180) Total load deflection criteria.  
 Design meets Code minimum (L/240) Live load deflection criteria.  
 Design meets arbitrary (0.5") Maximum Total load deflection criteria.  
 Calculations assume member is fully braced.  
 BC CALC® analysis is based on IBC 2009.  
 Design based on Dry Service Condition.



Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP

PASSED

BC CALC® Member Report

RB01 (Roof Beam)

November 12, 2019 09:23:16

Build 7295

Dry | 1 span | No cant.

Job name:

File name:

Address:

Description:

City, State, Zip:

Specifier:

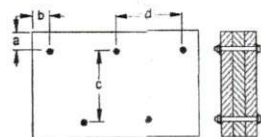
Customer:

Designer: TOM WALKER

Code reports: ESR-1040

Company: LONGLEAF TRUSS COMPANY

Connection Diagram: Full Length of Member



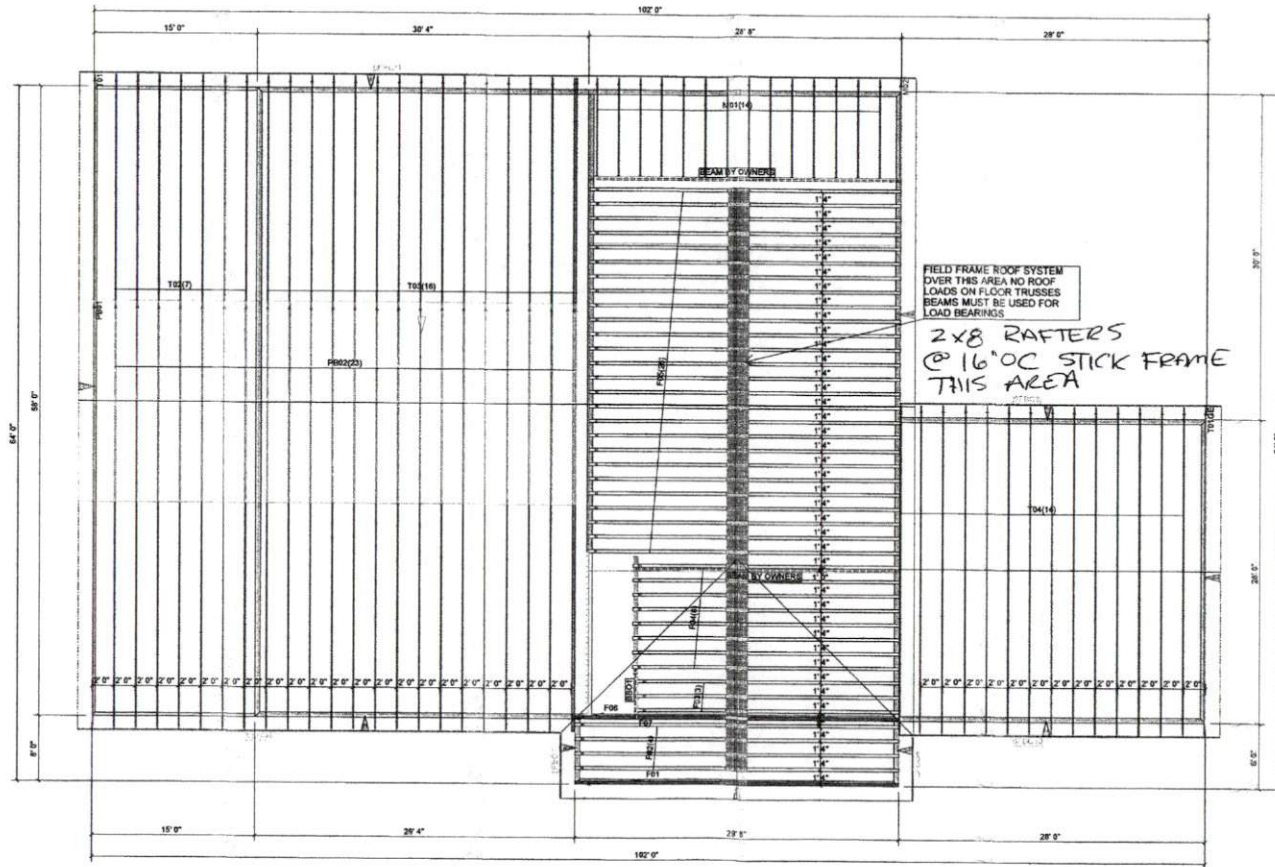
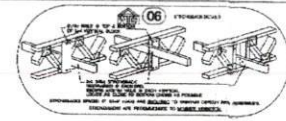
a minimum = 2"  
b minimum = 2-1/2"  
c = 12"  
d = 24"

Bolts are assumed to be Grade A307 or Grade 2 or higher.  
Connectors are: 1/2 in. Staggered Through Bolt

Disclosure

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Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BC®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,



Floor Area: 1603 SF  
 Floor Plywood: 1843.45  
 Roof Area: 6422.73 SF  
 Roof Plywood: 90 sheets  
 Roof Shingles: 90 Squares

FLOOR TRUSS LAYOUT  
 1/4" = 1'-0"

**PRELIMINARY**  
 FOR APPROVAL ONLY  
 THE LAYOUT IS THE SOLE BASIS FOR FABRICATION OF TRUSSES  
 AND YOURS. ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS  
 LAYOUTS, REVISED AND APPROVAL OF THIS LAYOUT MUST BE  
 SECURED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL  
 CONDITIONS TO BEING MADE CHANGES THAT WILL RESULT IN  
 EXTRA CHARGES TO YOU.  
 APPROVED BY: \_\_\_\_\_  
 DATE: \_\_\_\_\_

**NOTE**  
 IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE  
 CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER CONNECTIONS SHOWN ON TRUSS ENGINEERING  
 DRAWINGS. THE TRUSS COMPANY WILL NOT BE RESPONSIBLE FOR THE DESIGN OF THE CONNECTIONS.  
 TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.  
 THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN  
 WTCAL-1199 "DESIGN RESPONSIBILITY" ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY  
 FOR THE DESIGN OF THE TRUSSES INCLUDING THE INSTALLATION AND BRACING OF  
 TRUSSES MANUFACTURED BY THIS COMPANY. SEE NEAREST NECESSARY COMPANY TECHNICALS.

**LONGLEAF TRUSS CO.**  
 4476 Hwy. 21 W  
 West End, NC 27376  
 (910) 673-4711

Client: SERVICE BUILDING SUPPLY SANF

Project: PATTERSON

Market: FLOOR

CSI #: [blank]

Order #: [blank]

Designer: [blank]

Date: / /

Order #: P19-10021F