



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

July 10, 2019 07:26:27

BM6-2 (Roof Beam)

BC CALC® Member Report

Build 7082

Job name: Address:

Builder:

Ellington

LAMCO

Code reports:

City, State, Zip:

ESR-1040

Dry | 1 span | No cant.

File name:

Description:

Specifier:

Designer:

Company

BFS

17-00-00 **B**1 B2

Total Horizontal Product Length = 17-00-00

Reaction Summary (Down / Uplift) (lbs)

Snow Roof Live B1, 3-1/2" 959 / 0 B2, 3-1/2" 893 / 0

Loa	Load Summary				Live	Dead	Snow	Wind	Roof Live	Tributary		
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	17-00-00	Тор		12				00-00-00
1	Gable	Trapezoidal (lb/ft)	L	00-00-00		Тор		80				n\a
					05-11-04			120				
2	Gable	Trapezoidal (lb/ft)	L	05-11-04		Top		120				n\a
					17-00-00	0.000		70				

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	3954 ft-lbs	20.6 %	90%	0	08-02-05
End Shear	920 lbs	12.9 %	90%	0	01-03-06
Total Load Deflection	L/806 (0.246")	22.3 %	n\a	2	08-06-00
Live Load Deflection	L/999 (0.048")	n\a	n\a	5	08-06-00
Max Defl.	0.246"	24.6 %	n\a	2	08-06-00
Span / Depth	16.7				

Bear	ing Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material	
B1	Wall/Plate	3-1/2" x 3-1/2"	1228 lbs	n\a	13.4 %	Unspecified	
B2	Wall/Plate	3-1/2" x 3-1/2"	1163 lbs	n\a	12.7 %	Unspecified	

Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.

Design meets Code minimum (L/240) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

BC CALC® analysis is based on IBC 2009.

Design based on Dry Service Condition.

Member has no side loads.





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Dry | 1 span | No cant.

File name:

Description:

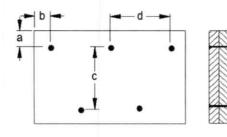
Specifier:

Designer:

Company:

BFS

Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

b minimum = 3"

d = 24"

Member has no side loads. Connectors are: 16d Sinker Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER®, AJS™ ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,





Quadruple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

PASSED

B2

BM7-4 (Roof Beam) Dry | 1 span | No cant.

BC CALC® Member Report

Build 7082

Ellington

July 10, 2019 07:28:30

Job name:

Address:

City, State, Zip: Builder:

Code reports:

LAMCO

ESR-1040

File name:

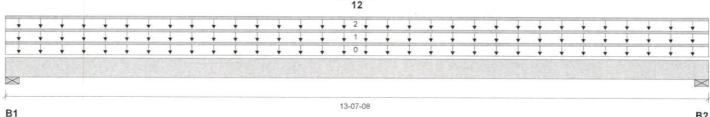
Description:

Specifier:

Designer:

Company **BFS**





Total Horizontal Product Length = 13-07-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 3-1/2"		5474 / 0			5310 / 0
B2, 3-1/2"		5474 / 0			5310 / 0

Lo	ad Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	13-07-08	Тор		24				00-00-00
1	A02 Trusses	Unf. Lin. (lb/ft)	L	00-00-00	13-07-08	Top		734			734	n\a
2	J02 Trusses	Unf. Lin. (lb/ft)	L	00-00-00	13-07-08	Тор		46			46	n\a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	34305 ft-lbs	64.5 %	125%	4	06-09-12
End Shear	8756 lbs	44.4 %	125%	4	01-03-06
Total Load Deflection	L/288 (0.548")	62.4 %	n\a	4	06-09-12
Live Load Deflection	L/586 (0.27")	41.0 %	n\a	5	06-09-12
Max Defl.	0.548"	54.8 %	n\a	4	06-09-12
Span / Depth	13.3				
	Pos. Moment End Shear Total Load Deflection Live Load Deflection Max Defl.	Pos. Moment 34305 ft-lbs End Shear 8756 lbs Total Load Deflection L/288 (0.548") Live Load Deflection L/586 (0.27") Max Defl. 0.548"	Pos. Moment 34305 ft-lbs 64.5 % End Shear 8756 lbs 44.4 % Total Load Deflection L/288 (0.548") 62.4 % Live Load Deflection L/586 (0.27") 41.0 % Max Defl. 0.548" 54.8 %	Pos. Moment 34305 ft-lbs 64.5 % 125% End Shear 8756 lbs 44.4 % 125% Total Load Deflection L/288 (0.548") 62.4 % n\a Live Load Deflection L/586 (0.27") 41.0 % n\a Max Defl. 0.548" 54.8 % n\a	Pos. Moment 34305 ft-lbs 64.5 % 125% 4 End Shear 8756 lbs 44.4 % 125% 4 Total Load Deflection L/288 (0.548") 62.4 % n\a 4 Live Load Deflection L/586 (0.27") 41.0 % n\a 5 Max Defl. 0.548" 54.8 % n\a 4

Bear	ing Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material	
B1	Wall/Plate	3-1/2" x 7"	10785 lbs	n\a	58.7 %	Unspecified	
B2	Wall/Plate	3-1/2" x 7"	10785 lbs	n\a	58.7 %	Unspecified	

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Design meets arbitrary (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

BC CALC® analysis is based on IBC 2009.

Design based on Dry Service Condition.

Beams 7 inches wide will be assumed to be either top-loaded only, or equally loaded from each side.

Bolts are assumed to be Grade A307 or Grade 2 or higher.

Member has no side loads.





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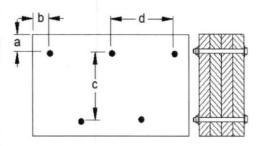
Code reports

ESR-1040

Company

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Connection Diagram: Full Length of Member



a minimum = 2"

c = 7-7/8"

b minimum = 2-1/2"

d = 24"

Beams 7 inches wide will be assumed to be either top-loaded only, or equally loaded from each side. Bolts are assumed to be Grade A307 or Grade 2 or higher. Member has no side loads.

Connectors are: 1/2 in. Staggered Through Bolt

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