

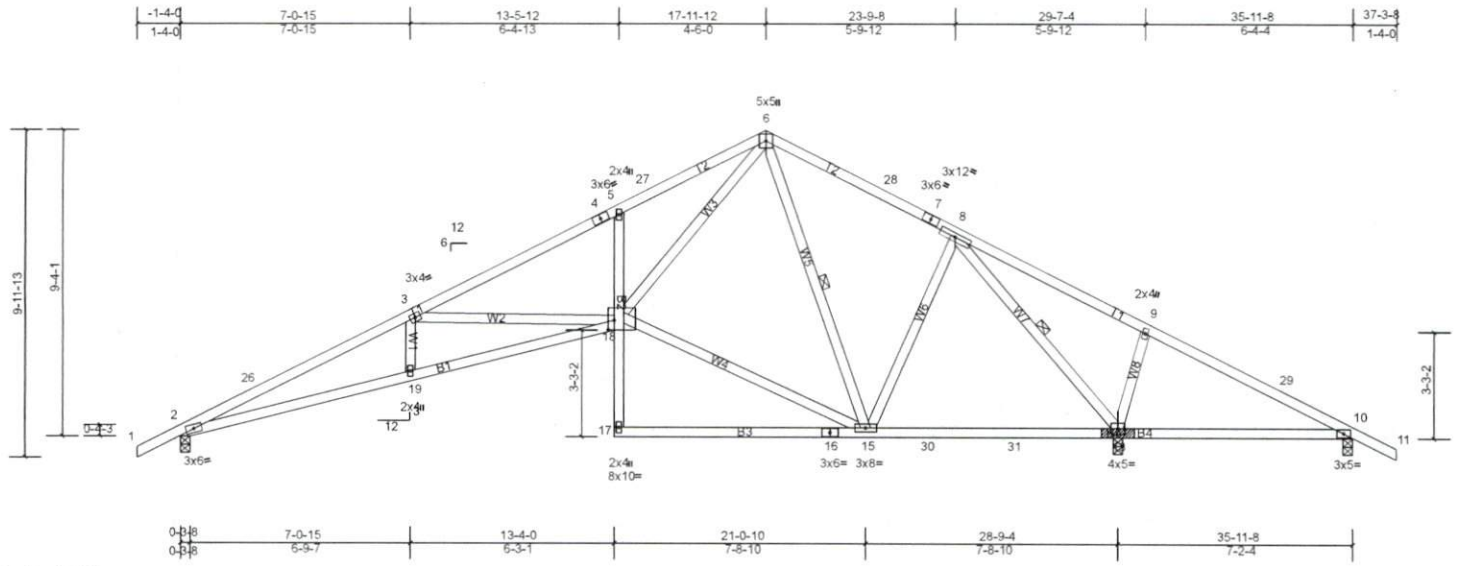
Job	Truss	Truss Type	Qty	Ply	Mike Sherer-Roof
Q-1901119-1	T1	Roof Special	1	1	Job Reference (optional)

Peak Truss Builders LLC, New Hill, user

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Scale = 1:67.7

Plate Offsets (X, Y): [18:0-2-4,0-3-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.18	18-19	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	Vert(CT)	-0.40	18-19	>862	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	Horz(CT)	0.22	13	n/a	n/a		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS							
										Weight: 203 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1 *Except* B2:2x4 SP No.3
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-1-7 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 5-5-15 oc bracing.
 WEBS 1 Row at midpt 6-15, 8-13

REACTIONS (lb/size) 2=1064/0-3-8, (min. 0-1-11), 10=-302/0-3-8, (min. 0-1-8), 13=2275/(0-3-8 + bearing block), (req. 0-3-9)
 Max Horiz 2=180 (LC 10)
 Max Uplift 2=-258 (LC 11), 10=-430 (LC 20), 13=-410 (LC 11)
 Max Grav 2=1064 (LC 1), 10=-12 (LC 11), 13=2275 (LC 1)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-26=-3130/523, 3-26=-3050/549, 3-4=-2042/340, 4-5=-1872/343, 5-27=-1975/432, 6-27=-1910/454, 6-28=-444/273, 7-28=-466/253, 7-8=-515/245, 8-9=-131/1492, 9-29=-188/1407, 10-29=-202/1298
 BOT CHORD 2-19=-384/2829, 18-19=-385/2823, 5-18=-328/207, 12-13=-1172/269, 10-12=-1172/269
 WEBS 3-18=-988/314, 15-18=0/646, 6-18=-246/1788, 6-15=-688/8, 8-15=0/660, 8-13=-2234/371, 9-13=-368/206

NOTES

- 2x4 SP No.1 bearing block 12" long at jt. 13 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SPF No.2.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=30ft; B=20ft; L=36ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior (2) -1-4-0 to 2-3-2, Interior (1) 2-3-2 to 17-11-12, Exterior (2) 17-11-12 to 21-6-14, Interior (1) 21-6-14 to 37-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 410 lb uplift at joint 13, 430 lb uplift at joint 10 and 258 lb uplift at joint 2.
- This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

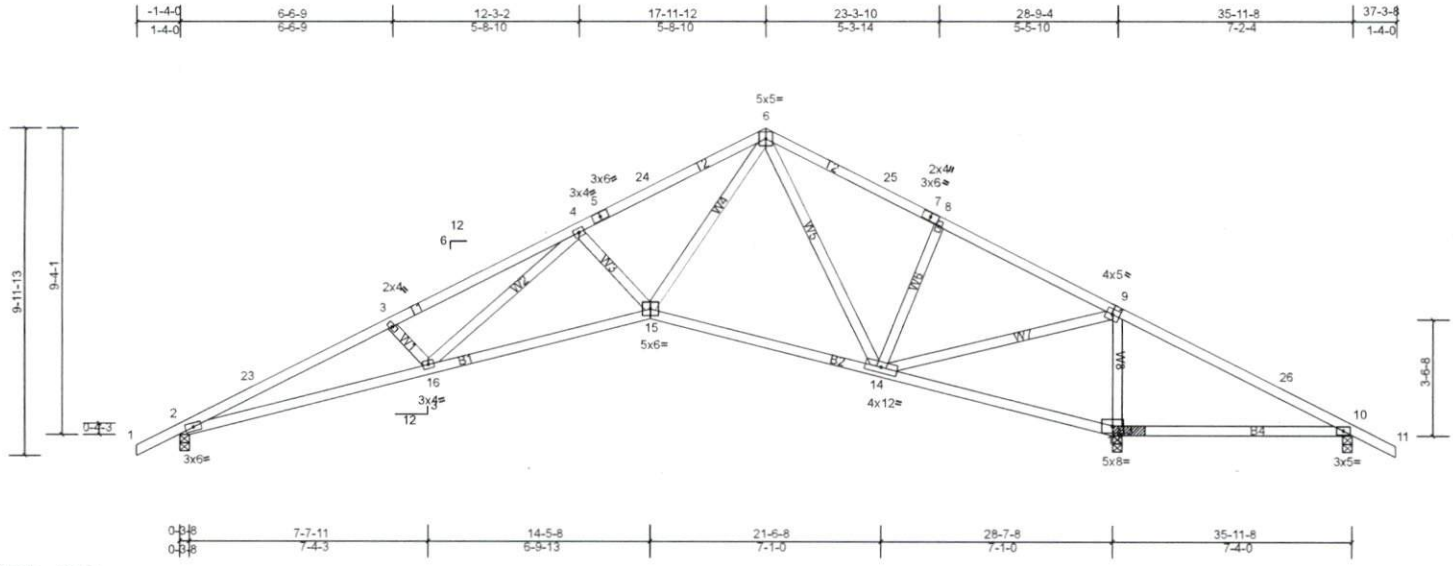
Job	Truss	Truss Type	Qty	Ply	Mike Sherer-Roof
Q-1901119-1	T1A	Roof Special	7	1	Job Reference (optional)

Peak Truss Builders LLC, New Hill, user

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Scale = 1:67.7

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.69	Vert(LL)	-0.18	15-16	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.58	Vert(CT)	-0.38	15-16	>895	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.86	Horz(CT)	0.19	13	n/a	n/a		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS								

Weight: 181 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 WEBS 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 3-2-8 oc purlins.
 Rigid ceiling directly applied or 4-8-8 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1040/0-3-8, (min. 0-1-10), 10=-349/0-3-8, (min. 0-1-8),
 13=2346/(0-3-8 + bearing block), (req. 0-3-11)
 Max Horiz 2=180 (LC 10)
 Max Uplift 2=-258 (LC 11), 10=-472 (LC 20), 13=-403 (LC 11)
 Max Grav 2=1040 (LC 1), 10=-20 (LC 11), 13=2346 (LC 1)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-23=-3051/544, 3-23=-2977/571, 3-4=-2826/555, 4-5=-1712/318, 5-24=-1655/326, 6-24=-1628/345, 6-25=-429/237,
 7-25=-496/218, 7-8=-504/208, 8-9=-630/195, 9-26=-182/1598, 10-26=-199/1466
 BOT CHORD 2-16=-411/2762, 15-16=-141/1910, 14-15=0/713, 13-14=-1468/310, 12-13=-1333/273, 10-12=-1333/273
 WEBS 3-16=-312/196, 4-16=-186/867, 4-15=-603/268, 6-15=-158/1442, 6-14=-700/28, 8-14=-279/180, 9-14=-221/1952,
 9-13=-1823/394

NOTES

- 2x4 SP No.1 bearing block 12" long at jt. 13 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SPF No.2.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCCL=6.0psf; h=30ft; B=20ft; L=36ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior (2) -1-4-0 to 2-3-2, Interior (1) 2-3-2 to 17-11-12, Exterior (2) 17-11-12 to 21-6-14, Interior (1) 21-6-14 to 37-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 2, 403 lb uplift at joint 13 and 472 lb uplift at joint 10.
- This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

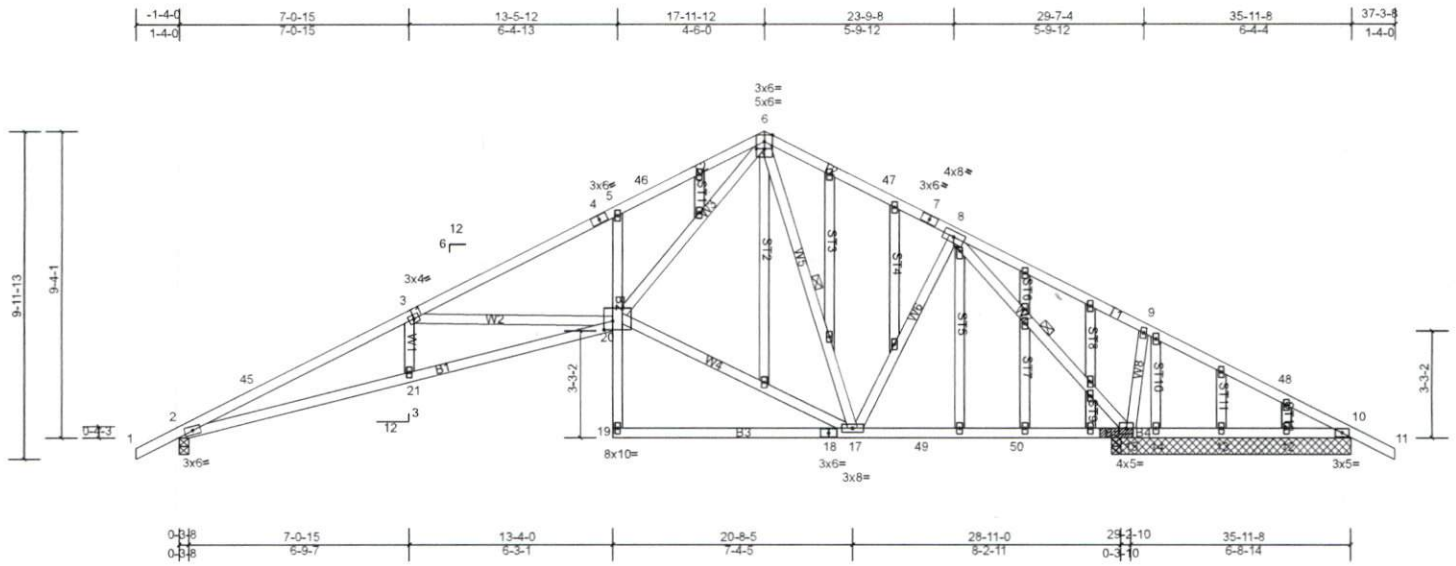
Job	Truss	Truss Type	Qty	Ply	Mike Sherer-Roof
Q-1901119-1	T1BSE	Roof Special	1	1	Job Reference (optional)

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Scale = 1:67.7

Plate Offsets (X, Y): [6:0-3-0,0-0-3], [8:0-1-4,0-1-0], [20:0-3-4,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.20	20-21	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	Vert(CT)	-0.42	20-21	>830	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	Horz(CT)	0.23	15	n/a	n/a		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS							
										Weight: 259 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1 *Except* B2:2x4 SP No.3
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-6 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 5-4-10 oc bracing.
 WEBS 1 Row at midpt 6-17, 8-15

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 7-4-0, except 2=0-3-8
 (lb) - Max Uplift All uplift 100 (lb) or less at joint(s) 12 except 14=-361 (LC 15),
 15=-349 (LC 11), 10=-502 (LC 20)
 Max Grav All reactions 250 (lb) or less at joint(s) 12, 13, 14, 10 except
 15=2367 (LC 1)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-45=-3253/547, 3-45=-3172/572, 3-4=-2170/365, 4-5=-2000/367, 5-46=-2103/456, 6-46=-2038/478, 6-47=-546/290,
 7-47=-572/271, 7-8=-632/262, 8-9=-118/1475, 9-48=-202/1433, 10-48=-217/1323
 BOT CHORD 2-21=-406/2941, 20-21=-406/2935, 5-20=-328/207, 17-49=0/291, 49-50=0/291, 16-50=0/291, 15-16=0/291,
 14-15=-1206/284, 13-14=-1206/284, 12-13=-1206/284, 10-12=-1206/284
 WEBS 3-20=-984/313, 17-20=0/721, 6-20=-260/1868, 6-17=-632/0, 8-17=0/587, 8-15=-2272/372, 9-15=-350/210

NOTES

- 2x4 SP No.1 bearing block 12" long at jt. 15 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SPF No.2.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=30ft; B=20ft; L=36ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior (2) 1-4-0 to 2-3-2, Interior (1) 2-3-2 to 17-11-12, Exterior (2) 17-11-12 to 21-6-14, Interior (1) 21-6-14 to 37-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 15=348, 10=501, 14=361, 2=264, 10=501.
- This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

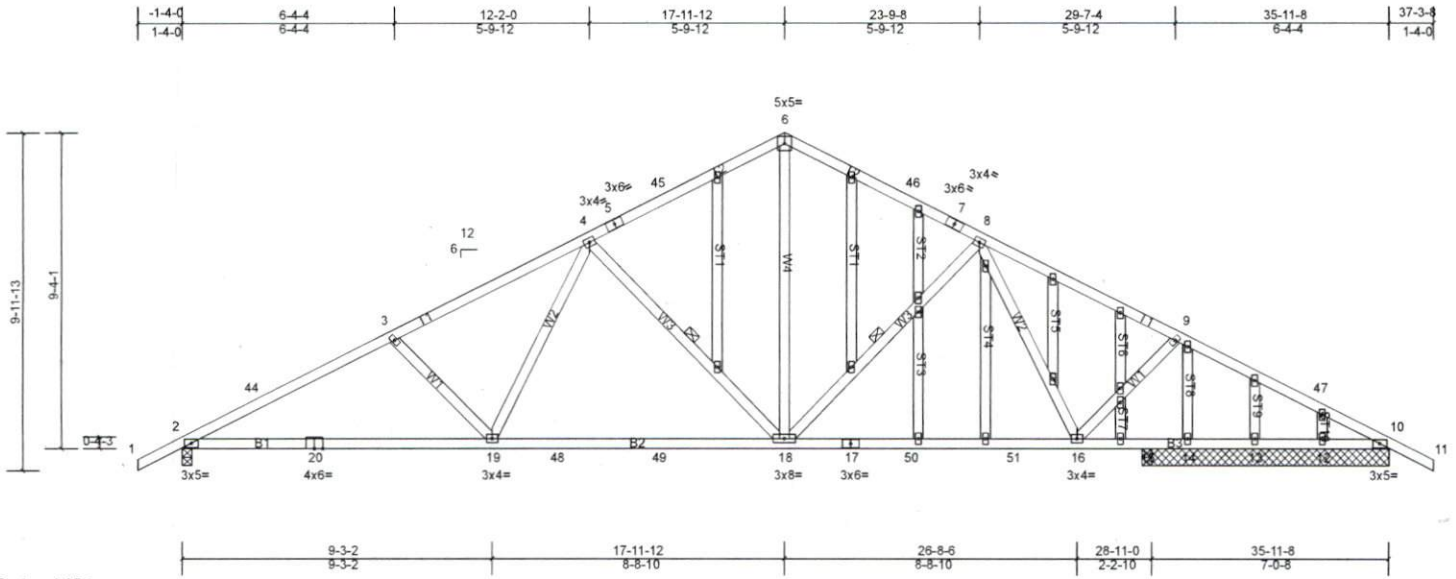
Job	Truss	Truss Type	Qty	Ply	Mike Sherer-Roof
Q-1901119-1	T1SE	Common	1	1	Job Reference (optional)

Peak Truss Builders LLC, New Hill, user

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Scale = 1:65.8

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.21	16-18	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	Vert(CT)	-0.43	16-18	>807	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	Horz(CT)	0.08	41	n/a	n/a		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS							
										Weight: 241 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP DSS *Except* B1:2x4 SP No.1
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-6-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 8-18, 4-18

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 7-4-0. except 2=0-3-8, 15=0-3-8

(lb) - Max Horiz 2=180 (LC 9)
 Max Uplift All uplift 100 (lb) or less at joint(s) 12 except 14=520 (LC 1),
 15=217 (LC 11), 2=324 (LC 11), 10=273 (LC 11)
 Max Grav All reactions 250 (lb) or less at joint(s) 12, 13, 14 except
 15=815 (LC 1), 2=1423 (LC 1), 10=1059 (LC 1)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-44=2480/499, 3-44=2434/526, 3-4=2237/499, 4-5=1500/397, 5-45=1440/406, 6-45=1416/425, 6-46=1416/425,
 7-46=1442/406, 7-8=1501/397, 8-9=1796/422, 9-47=1941/449, 10-47=2017/421
 BOT CHORD 2-20=353/2254, 19-20=353/2254, 19-48=196/1785, 48-49=196/1785, 18-49=196/1785, 17-18=161/1515,
 17-50=161/1515, 50-51=161/1515, 16-51=161/1515, 15-16=283/1731, 14-15=283/1731, 13-14=283/1731,
 12-13=283/1731, 10-12=283/1731
 WEBS 6-18=223/994, 8-18=423/196, 9-16=308/201, 4-18=681/247, 4-19=42/542, 3-19=360/201

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=30ft; B=20ft; L=36ft; eave=5ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior (2) -1-4-0 to 2-3-2, Interior (1) 2-3-2 to 17-11-12, Exterior (2) 17-11-12 to 21-6-14, Interior (1) 21-6-14 to 37-3-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BC DL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=323, 10=272, 14=519, 15=216, 10=272.
- This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

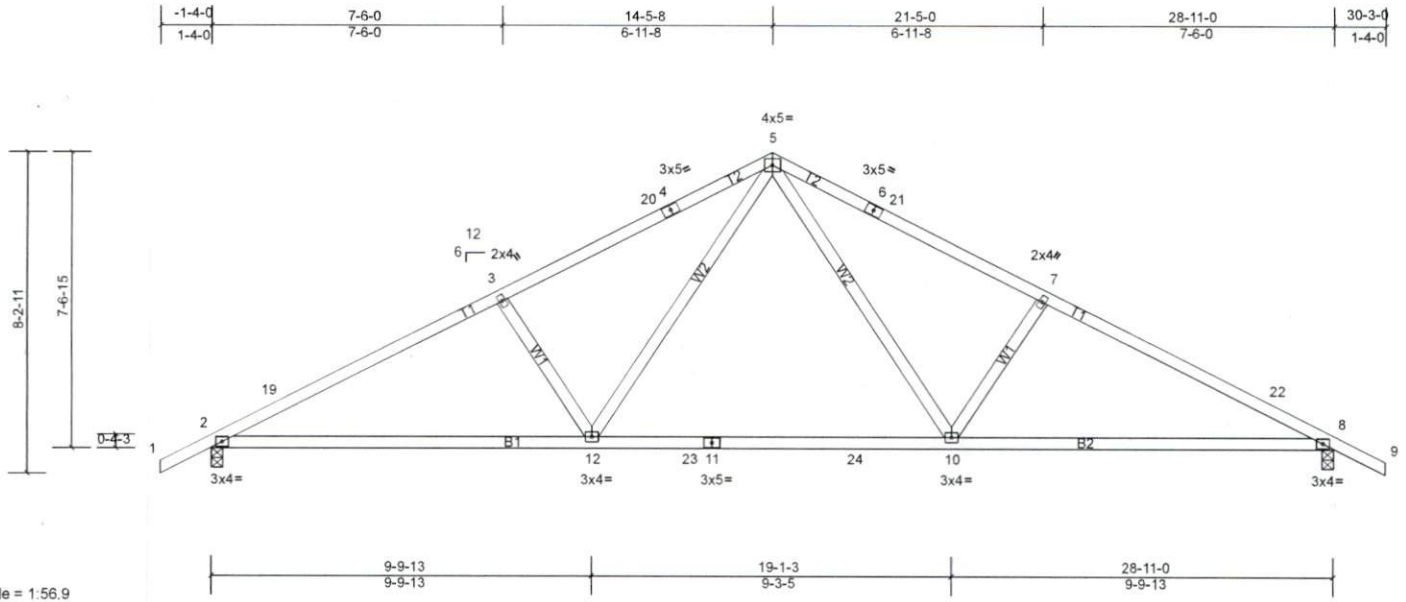
Job	Truss	Truss Type	Qty	Ply	Mike Sherer-Roof
Q-1901119-1	T2	Common	6	1	Job Reference (optional)

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.54	Vert(LL)	-0.29	10-12	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.67	Vert(CT)	-0.41	10-12	>856	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.29	Horz(CT)	0.06	8	n/a	n/a		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS								Weight: 133 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 WEBS 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 3-9-5 oc purfins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1237/0-3-8, (min. 0-1-15), 8=1237/0-3-8, (min. 0-1-15)
 Max Horiz 2=137 (LC 9)
 Max Uplift 2=285 (LC 11), 8=285 (LC 11)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-19=-2023/403, 3-19=-1969/436, 3-20=-1810/430, 4-20=-1702/431, 4-5=-1691/447, 5-6=-1691/447, 6-21=-1702/431,
 7-21=-1810/430, 7-22=-1969/436, 8-22=-2023/403
 BOT CHORD 2-12=-261/1779, 12-23=-57/1154, 11-23=-57/1154, 11-24=-57/1154, 10-24=-57/1154, 8-10=-261/1762
 WEBS 5-10=-124/752, 7-10=-452/255, 5-12=-124/752, 3-12=-452/255

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; B=20ft; L=29ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior (2) -1-4-0 to 1-8-0, Interior (1) 1-8-0 to 14-5-8, Exterior (2) 14-5-8 to 17-5-8, Interior (1) 17-5-8 to 30-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 285 lb uplift at joint 8.
- This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

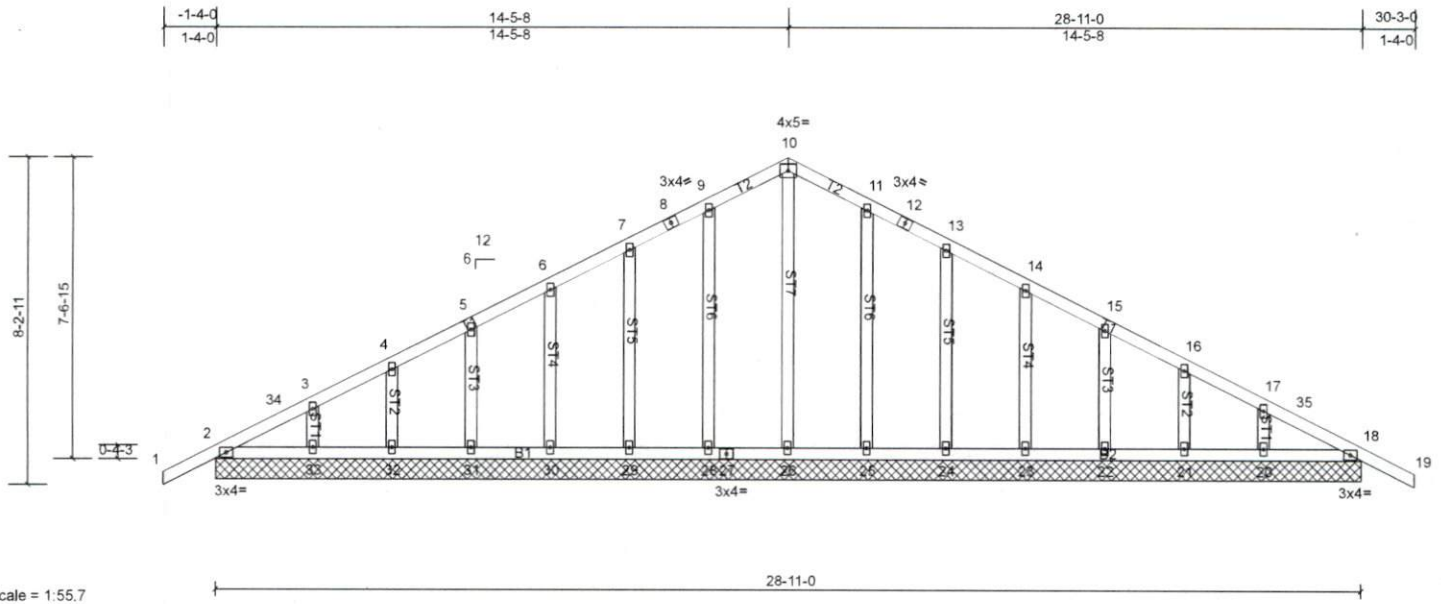
Job Q-1901119-1	Truss T2CGE	Truss Type Common Supported Gable	Qty 2	Ply 1	Mike Sherer-Roof Job Reference (optional)
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Peak Truss Builders LLC, New Hill, user

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.00	18	n/a	n/a		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-S								
											Weight: 169 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.1
 OTHERS 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purfins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

All bearings 28-11-0.

(lb) - Max Horiz 2=137 (LC 9)

Max Uplift All uplift 100 (lb) or less at joint(s) 2, 18, 20, 21, 22, 23, 24, 25, 28, 29, 30, 31, 32, 33

Max Grav All reactions 250 (lb) or less at joint(s) 2, 18, 20, 21, 22, 23, 24, 25, 26, 28, 29, 30, 31, 32, 33

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

10-11=-89/250

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCCL=6.0psf; h=30ft; B=20ft; L=29ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner (3) -1-4-0 to 1-8-0, Exterior (2) 1-8-0 to 14-5-8, Corner (3) 14-5-8 to 17-5-8, Exterior (2) 17-5-8 to 30-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 18, 28, 29, 30, 31, 32, 33, 25, 24, 23, 22, 21, 20.
- This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S)

Standard

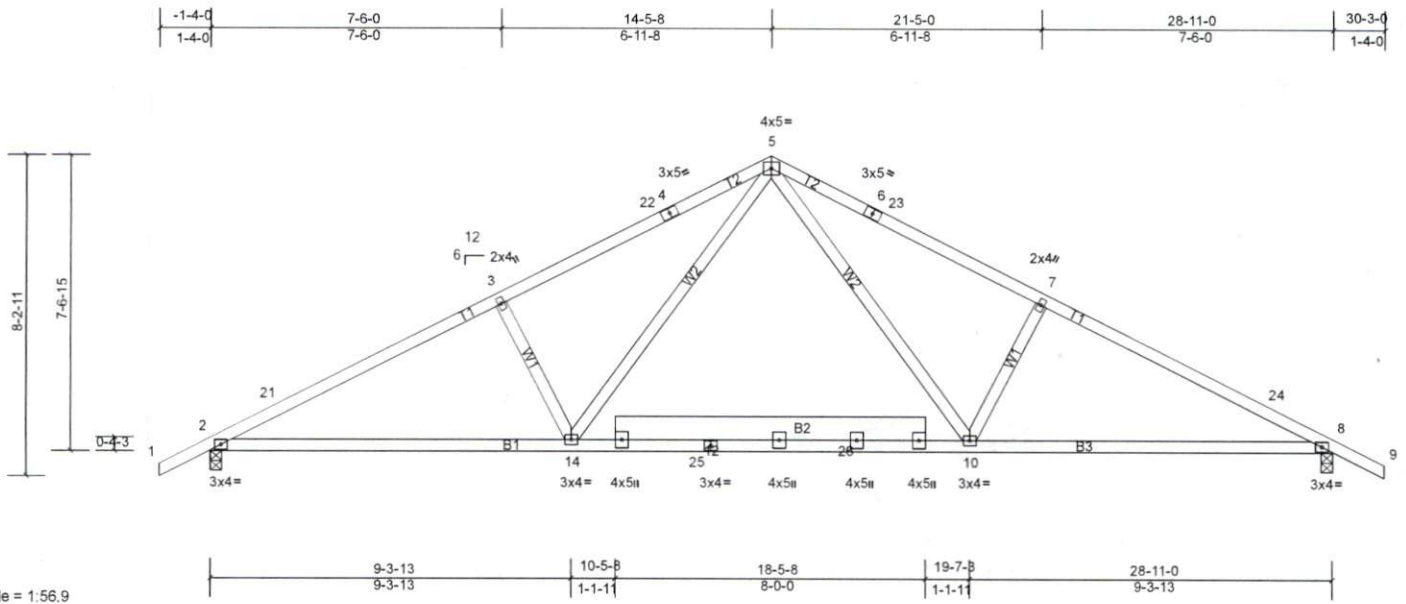
Job Q-1901119-1	Truss T3	Truss Type Common	Qty 5	Ply 1	Mike Sherer-Roof Job Reference (optional)
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Peak Truss Builders LLC, New Hill, user

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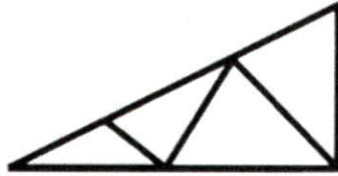
Loading	(psf)	Spacing	2-0-0	CSI	0.53	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.53	Vert(LL)	0.11 10-20	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.58	Vert(CT)	-0.30 10-20	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.05 8	n/a	n/a		
BCDL	10.0	Code	IBC2015/TPI2014	Matrix-MS							Weight: 158 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 3-9-11 oc purlins.
BOT CHORD 2x4 SP No.1 *Except* B2:2x8 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	
REACTIONS (lb/size) 2=1237/0-3-8, (min. 0-1-15), 8=1237/0-3-8, (min. 0-1-15) Max Horiz 2=137 (LC 10) Max Uplift 2=-285 (LC 11), 8=-285 (LC 11)	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-21=-2032/402, 3-21=-1977/435, 3-22=-1865/455, 4-22=-1757/456, 4-5=-1746/472, 5-6=-1746/472, 6-23=-1756/456, 7-23=-1864/455, 7-24=-1976/435, 8-24=-2032/402
BOT CHORD	2-14=-260/1768, 13-14=-64/1146, 13-25=-65/1142, 12-25=-65/1142, 12-26=-64/1147, 11-26=-65/1145, 10-11=-64/1146, 8-10=-260/1768
WEBS	5-10=-142/791, 7-10=-446/253, 5-14=-142/791, 3-14=-445/253

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vuft=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=30ft; B=20ft; L=29ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior (2) -1-4-0 to 1-8-0, Interior (1) 1-8-0 to 14-5-8, Exterior (2) 14-5-8 to 17-5-8, Interior (1) 17-5-8 to 30-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 285 lb uplift at joint 8.
 - This truss is designed in accordance with the 2015 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Peak Truss Builders, LLC

PO Box 340, New Hill, NC 27562

Agreement to Purchase

Job #:

Q-1901119

Customer:

Value Customer

Address:

Description:

Mike Sherer

Contact:

Mike Sherer 919-608-8962

Site Address:

**120 Fletcher
Fuquay Varina NC**

Notes:

Roof Trusses
2' OC, 16" OH
6/12 Pitch

Truss Design Date:

TOTAL AMOUNT OF ORDER

\$3,767.35

Please Review the terms and conditions for the above captioned job

I have examined the attached design package and agree to purchase from PEAK TRUSS BUILDERS, LLC (hereinafter Peak) the articles therein described. I acknowledge that the layouts and truss designs attached hereto have been produced using plans and data provided to Peak by me, and having examined them, do hereby agree that the products represented by these designs are acceptable for use in the structure I intend to build. I understand that orders may not be cancelled once material has been cut for the job.

TERMS: I understand and agree that purchased items shall be invoiced as delivered, and that payment shall be due subject to the terms disclosed at time of order. I agree that a finance charge of 1.5% per month may be assessed on accounts 30 days or more past due. I agree to pay the costs of collection on accounts past due, including but not limited to reasonable attorney's fees and court costs. Verbal Orders shall incorporate all of the terms and conditions contained herein, and Verbal Orders, once accepted by Peak, are binding upon Purchaser.

I acknowledge that it is my responsibility to verify quantities, spans, pitches, overhangs, bearing locations, point load locations, size and location of required openings, and other contractor-verifiable items related to the proper function and appearance of these products, and to notify Peak at least five days prior to the scheduled cutting and/or manufacture of the products described herein of any changes I want made. I acknowledge loads imposed. I acknowledge that Peak is responsible only for the design of the components supplied by Peak, and is not responsible for building design.

DELIVERY: I agree to provide for a reasonably smooth, level and accessible area for delivery of trusses at the job site. I understand that trusses are delivered on a 60' long "roll off" tractor-trailer, and I will insure that the approach path to the desired drop location is straight, level, compacted, and with clear width and height of at least 13 1/2 feet. Should Peak's delivery truck arrive at the jobsite and find that these conditions are not met and trusses cannot be dropped, I will be responsible for re-delivery costs. Should Peak attempt to deliver despite these conditions not being met, I accept responsibility for damage caused by and to unlevel ground or obstacles. Should the delivery vehicle get stuck on my jobsite, I agree to pay reasonable and actual towing costs.

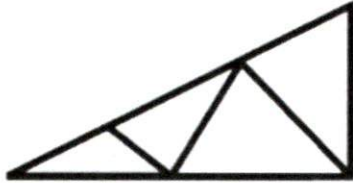
If I am not present at the jobsite at the time of delivery, I authorize Peak to use their reasonable judgement in deciding whether and where to unload the order, and do hereby indemnify Peak from any liability for damages resulting from the exercise thereof. I agree that estimated delivery dates and times are made on a "best effort" basis, and that Peak shall not be liable for costs occasioned by delays in delivery.

INSTALLATION: I understand that it is my responsibility to be knowledgeable of the warnings and recommendations related to the safe handling and erecting of wood trusses as described in WTCA Manual BCSI 1-03 or its equivalent. I understand and agree that I, as the builder/contractor, am solely responsible for the safe and proper installation of these products, and to ensure that the installation is in conformance with engineering and permanent bracing notes included as part of the design package.

BRACING: I understand that Truss Bracing and Building Bracing are the responsibility of the Engineer of Record. Peak will provide guidance on the types and recommended locations for bracing, but it is my responsibility to understand and oversee the overall Bracing Design for the building of which trusses are a part.

Signed: _____

Date: _____



Peak Truss Builders, LLC

PO Box 340, New Hill, NC 27562

Comments and Clarifications

Job #:

Q-1901119

Customer:

Value Customer

Address:

Description:

Mike Sherer

Contact:

Mike Sherer 919-608-8962

Site Address:

**120 Fletcher
Fuquay Varina NC**

Notes:

Roof Trusses
2' OC, 16" OH
6/12 Pitch

Truss Design Date:

-
1. All exterior/bearing walls are 2x4 (3-1/2" wide) unless otherwise noted.
 2. All perimeter dimensions on layout reflect outside to outside of the sheathing. Studs are held in 1/2" to allow sheathing to line up with edge of slab.
 3. Overhang - - horizontal truss dimension is 16". Sub-fascia and fascia are beyond.
 4. No carport per customer.

I have Reviewed and Approved above Clarifications:

Signed: _____

Date: _____

THIS LAYOUT IS TO BE USED AS A TRUSS PLACEMENT GUIDE ONLY.
PLEASE REFER TO BUILDING PLANS FOR BUILDING CONSTRUCTION AND DETAILS,
SUCH AS PLUMBING OR DUCT DROPS.

PROPOSED DESIGN-
NOT FOR
CONSTRUCTION

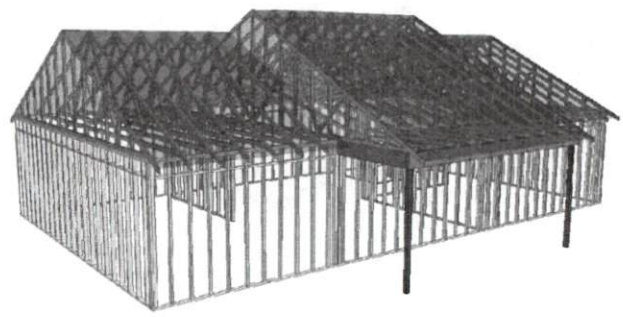
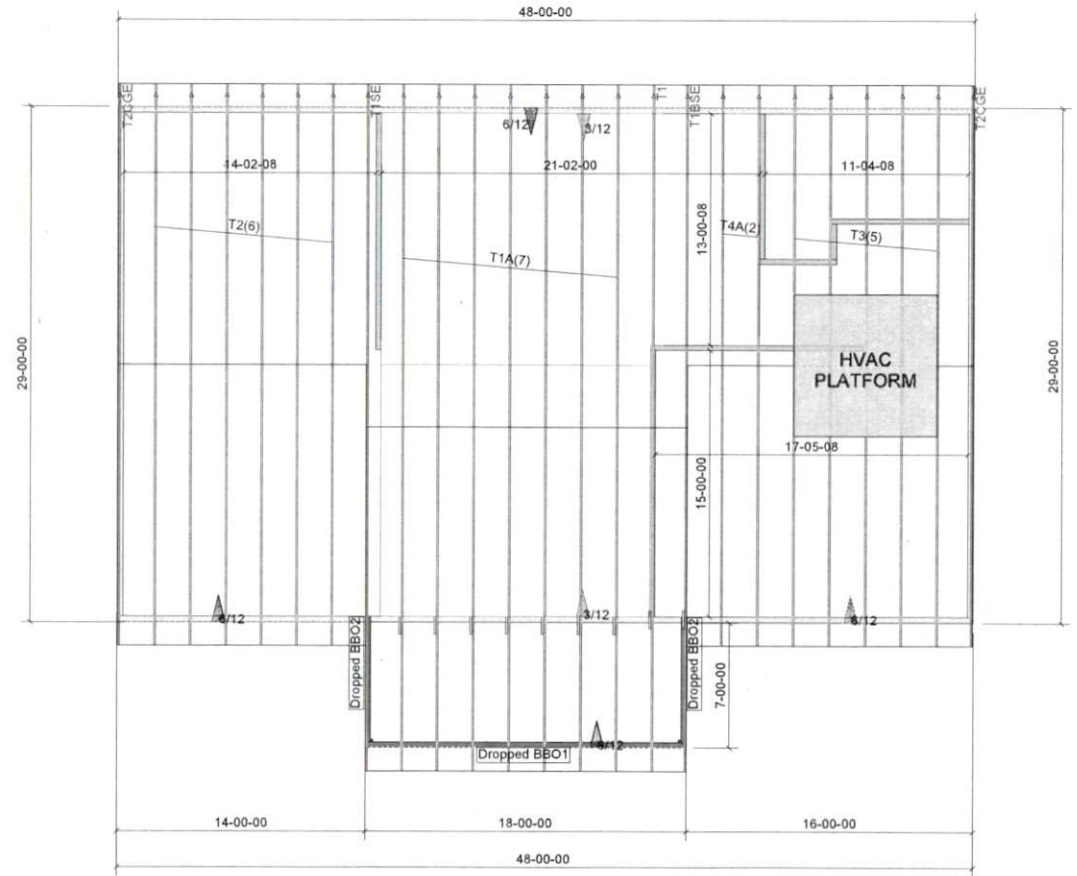
Notes:
1. Exterior dimensions shown are assumed to be:
 Out-to-out of stud
 Out-to-out of sheathing
2. Adjust truss locations as needed for plumbing and mechanical clearance. Unless otherwise noted, trusses may be shifted as long as O.C. spacing shown is not exceeded.
3. Do not cut, drill, or otherwise damage any part of any truss without prior approval from Peak Truss.
4. Do not approve drawings if any information herein is unclear. Once ordered trusses will be fabricated as approved.
5. Please contact Peak Truss Builders with any questions. We are available to help any way we can. We can be reached at 919-545-5555 or sales@peaktruss.com

Roof Truss Loading per 2018 NC Residential Code
Top Chord Live Load 20# PSF
Top Chord Dead Load 10# PSF
Bottom Chord Live Load 0# PSF
Bottom Chord Dead Load 10# PSF

Trusses are designed for additional storage load wherever a 42"x24" box will fit between the webs.
△ - This symbol denotes left end of truss as shown on truss drawings
● - Approximate location of rafter drop. Builder please confirm.

Truss connections by others
(N) - Nailed
(L) - Ledger

MIKE SHERER
ROOF TRUSSES
2' OC, 16" OH



Job # Q-1901119

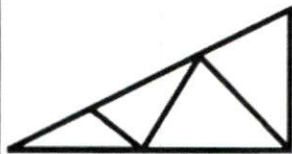
Mike Sherer
120 Fletcher
Fuquay Varina NC

Date Quoted: 7/1/2019 12:00:00 AM

Designer: Torrance Hamilton

Value Customer

Peak Truss Builders, LLC
PO Box 340, New Hill, NC 27562



**Peak Truss
Builders, LLC**

PO Box 340, New Hill, NC 27562

Proposal Detail

<p>Customer: Value Customer</p> <p>Address:</p>	<p>Description: Mike Sherer</p> <p>Contact: Mike Sherer 919-608-8962</p> <p>rmichaelsherer@gmail.com</p> <p>Site Address: 120 Fletcher Fuquay Varina NC</p>	<p>Notes: Roof Trusses 2' OC, 16" OH 6/12 Pitch</p>
<p>Truss Design Date:</p>		

Roof Trusses								
Qty	Label	Ply	Span	Height	L-OH	R-OH	Profile	Unit Price
1	T1	1-ply	35-11-08 Roof Special	9-04-01	1-04-00	1-04-00		\$201.32
7	T1A	1-ply	35-11-08 Roof Special	9-04-01	1-04-00	1-04-00		\$136.68
1	T1BSE	1-ply	35-11-08 Roof Special Structural Gable	9-04-01	1-04-00	1-04-00		\$279.18
1	T1SE	1-ply	35-11-08 Common Structural Gable	9-04-01	1-04-00	1-04-00		\$250.39
6	T2	1-ply	28-11-00 Common	7-06-15	1-04-00	1-04-00		\$98.66
2	T2CGE	1-ply	28-11-00 Common Supported Gable	7-06-15	1-04-00	1-04-00		\$162.78
5	T3	1-ply	28-11-00 Common	7-06-15	1-04-00	1-04-00		\$110.90
2	T4A	1-ply	28-11-00 Roof Special	7-06-15	1-04-00	1-04-00		\$144.61
Roof Truss Total:								\$3,448.89

Material Subtotal:	\$3,448.89
Engineering Fee	\$72.00

PreTax Total:	\$3,520.89
Sales Tax 7%	\$246.46
Grand Total	\$3,767.35