

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 140_1582_C 140.1582.C 12x12 CVP

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I40215297 thru I40215339

My license renewal date for the state of North Carolina is December 31, 2020.

North Carolina COA: C-0844



February 11,2020

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	I	7-4-0	I	7-4	4-0	
Plate Offsets (X,Y)	[2:0-0-0,0-1-4], [2:0-0-15,0-4-5], [4:Edge	9,0-1-4], [4:0-0-15,0-4-5]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.88 BC 0.57 WB 0.13 Matrix-S	DEFL. Vert(LL) 0. Vert(CT) -0. Horz(CT) 0.	in (loc) l/defl 07 2-5 >999 14 4-5 >999 01 4 n/a	L/d PLATES 240 MT20 180 n/a Weight:	GRIP 244/190 59 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S	P No.2 P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood s Rigid ceiling dire	sheathing directly applied or ctly applied or 10-0-0 oc bra	2-2-0 oc purlins. cing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (Ib/size) 2=638/0-3-8, 4=573/0-3-8 Max Horz 2=132(LC 11) Max Uplift 2=-83(LC 12), 4=-60(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-699/104, 3-4=-696/105

BOT CHORD 2-5=-8/474, 4-5=-8/474

WEBS 3-5=0/354

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 7-4-0, Exterior(2) 7-4-0 to 10-4-0, Interior(1) 10-4-0 to 14-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be User Defined crushing capacity of 425 psi.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





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Job	Truss	Truss Type	Qty	Ply	140.1582.C 12x12 CVP	
						140215299
140_1582_C	AG	COMMON GIRDER	1	2		
				3	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	330 s Jan	22 2020 MiTek Industries, Inc. Mon Feb 10 17:04:31 2020	Page 2
		ID:RUSz4	4LGuFS2C	1bODNZV	VBaZyX6cZ-IjXJuWuEP?0HIL?S2RwzguQFgjNNfrgugIP8v0;	zmTy

NOTES-

Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss) or equivalent at 4-11-4 from the left end to connect truss(es) to back face of bottom chord.
 Use Simpson Strong-Tie HUS26 (14-10d Girder, 6-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 6-6-12 from the left end to 10-6-12 to

connect truss(es) to back face of bottom chord.

15) Fill all nail holes where hanger is in contact with lumber.

16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

17) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1952 lb down and 187 lb up at 12-6-12, and 1089 lb down and 214 lb up at 16-6-12, and 1091 lb down and 212 lb up at 18-6-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-60, 4-6=-60, 6-8=-60, 2-9=-20 Concentrated Loads (lb)

Vert: 10=-1033(B) 17=-811(B) 18=-1952(B) 19=-1952(B) 20=-1952(B) 21=-1952(B) 24=-1035(B)

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Scale = 1:87.9



	9-0-0 1	7 11 2	25-0-0	33-0-14	41-0-0	49-7-4	
Plate Offsets (X,Y)	[16:0-4-0,0-5-4]	7-11-2	0-0-14	6-0-14	7-11-2	0-7-4	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0 Plate Grip DOL 1. Lumber DOL 1. Rep Stress Incr YE Code IRC2015/TPI201	0-0 CSI. 15 TC 15 BC ES WB 4 Matrix-	0.48 Vert(L 0.96 Vert(C 0.98 Horz(-S	. in (loc) L) -0.44 16 CT) -0.89 16-17 CT) 0.21 12	l/defl L/d >999 240 >665 180 n/a n/a	PLATES MT20 Weight: 316 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x6 SP 1-4,10- BOT CHORD 2x6 SP 14-16,1 WEBS 2x4 SP	No.2 *Except* 12: 2x4 SP No.2 No.2 *Except* 6-18: 2x6 SP DSS No.3		BRAC TOP (BOT (WEBS	CHORD Struct 2-0-0 CHORD Rigid CHORD Rigid CHORD Row 2 Row	ural wood sheathing d oc purlins (2-10-12 ma ceiling directly applied v at midpt vs at 1/3 pts	irectly applied or 2-7-2 c ax.): 4-10. or 2-2-0 oc bracing. 5-19, 7-17, 7-15 9-13	c purlins, except
REACTIONS. (Ib/size Max H Max U	e) 12=1972/Mechanical, 2=2 orz 2=89(LC 16) plift 12=-221(LC 8), 2=-225(LC	2034/0-3-8 C 9)					
FORCES. (lb) - Max. TOP CHORD 2-3=- 9-10= BOT CHORD 2-19= 12-11 WEBS 4-19=	Comp./Max. Ten All forces 2 3978/660, 3-4=-3759/576, 4-5: 3194/528, 10-11=-3631/562, 549/3495, 17-19=-786/5222, 3=-495/3228 111/1284, 5-19=-2234/450, 5	250 (lb) or less except v =-3308/548, 5-7=-5222 11-12=-3747/621 16-17=-867/5809, 15-1 5-17=0/539, 7-17=-727/	when shown. 2/831, 7-9=-5165/822, 16=-867/5809, 13-15=-73 /141, 7-16=0/332,	37/5165,			
7-15= NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) Interior(1) 16-0-6 to reactions shown; Lu 3) Provide adequate dr 4) This truss has been will fit between the b 6) All bearings are assis 7) Refer to girder(s) for 8) Provide mechanical 12=221.	786/147, 9-15=0/563, 9-13=-: loads have been considered f ult=130mph (3-second gust) V gable end zone and C-C Exter 41-0-0, Exterior(2) 41-0-0 to 48 mber DOL=1.60 plate grip DOI ainage to prevent water pondii designed for a 10.0 psf bottom n designed for a live load of 20 ottom chord and any other me umed to be User Defined crush truss to truss connections. connection (by others) of truss	2293/454, 10-13=-109/ for this design. /asd=103mph; TCDL=6 rior(2) -0-10-8 to 4-1-3, 8-0-6, Interior(1) 48-0-6 L=1.60 ng. the chord live load noncor 0.0psf on the bottom chi mbers. hing capacity of 425 psi to bearing plate capat	/1228 6.0psf; BCDL=6.0psf; h=/ , Interior(1) 4-1-3 to 9-0-0 5 to 49-5-8 zone;C-C for r ncurrent with any other li nord in all areas where a r i. ble of withstanding 100 lb	25ft; Cat. II; Exp B; , Exterior(2) 9-0-0 t nembers and force ve loads. ectangle 3-6-0 tall I uplift at joint(s) exc uplift at joint(s) exc	Enclosed; o 16-0-6, s & MWFRS for by 2-0-0 wide cept (jt=lb)	Control Print	CARO ESSION SEAL 44925

. JI(S) connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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6-	1-4 11-8-0 1-4 5-6-12	20-6-11 25-0-0 8-10-11 4-5-5	29-5-5	38-4-0	<u>43-10-12</u> <u>49-7-4</u> 5-6-12 <u>5-8-8</u>
Plate Offsets (X,Y)	[13:0-4-0,0-4-8]			0.10.11	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.73 BC 0.98 WB 0.90 Matrix-S	DEFL. in Vert(LL) -0.34 Vert(CT) -0.70 Horz(CT) 0.17	(loc) I/defl L/d 13-15 >999 240 13-15 >842 180 10 n/a n/a	PLATES GRIP MT20 244/190 Weight: 303 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP 4-6,6-8 BOT CHORD 2x6 SP 12-13,1 WEBS 2x4 SP	No.2 *Except* : 2x6 SP No.2 No.2 *Except* 3-14: 2x6 SP DSS No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing of 2-0-0 oc purlins (3-2-11 ma Rigid ceiling directly applied 1 Row at midpt	directly applied or 2-2-0 oc purlins, except x.): 4-8. d or 2-2-0 oc bracing. 5-15, 7-11
REACTIONS. (Ib/size Max He Max Up	e) 10=1972/Mechanical, 2=2034/0-3-8 orz 2=111(LC 12) plift 10=-172(LC 8), 2=-177(LC 9)				
FORCES. (lb) Hax. TOP CHORD 2-3=- 8-9=- BOT CHORD 2-15= WEBS 3-15= 7-11=	Comp./Max. Ten All forces 250 (lb) or 3906/661, 3-4=-3559/554, 4-5=-3112/54 3476/539, 9-10=-3724/633 542/3424, 13-15=-578/4279, 11-13=-54 326/243, 4-15=-71/1148, 5-15=-1481/3 1532/375, 8-11=-78/1110	less except when shown. 0, 5-7=-4374/668, 7-8=-3039/ 46/4254, 10-11=-502/3230 73, 5-13=0/334, 7-13=0/365,	526,		
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) Interior(1) 18-8-6 to 3 reactions shown; Lun 3) Provide adequate dr 4) This truss has been will fit between the b 6) All bearings are assu 7) Refer to girder(s) for 8) Provide mechanical 10=172.	loads have been considered for this derult=130mph (3-second gust) Vasd=103r gable end zone and C-C Exterior(2) -0-1 38-4-0, Exterior(2) 38-4-0 to 45-4-6, Inte mber DOL=1.60 plate grip DOL=1.60 ainage to prevent water ponding. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on the ottom chord and any other members, wi umed to be User Defined crushing capace truss to truss connections. connection (by others) of truss to bearing	sign. nph; TCDL=6.0psf; BCDL=6.0 0-8 to 4-1-3, Interior(1) 4-1-3 rior(1) 45-4-6 to 49-5-8 zone;(e load nonconcurrent with any ne bottom chord in all areas w th BCDL = 10.0psf. sity of 425 psi. g plate capable of withstandin	opsf; h=25ft; Cat. II; to 11-8-0, Exterior(2 C-C for members an other live loads. where a rectangle 3-f g 100 lb uplift at joir	Exp B; Enclosed; 2) 11-8-0 to 18-8-6, d forces & MWFRS for 6-0 tall by 2-0-0 wide ht(s) except (jt=lb)	SEAL

- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

SEAL 044925 MGINEER February 11,2020

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⊢	7-5-4	14-4-0	21-4-12	28-7-4	35-8-0	42-6-12	49-7	7-4
	7-5-4	6-10-12	7-0-12	7-2-8	7-0-12	6-10-12	7-0	-8
Plate Offsets (X,Y)-	[3:0-2-8,Edge], [12:0-	3-8,0-4-8], [16:0-2-	4,0-4-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DO Lumber DOL Rep Stress Ind Code IRC201	2-0-0 L 1.15 1.15 sr YES 5/TPI2014	CSI. TC 0.97 BC 0.78 WB 0.69 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT)	in (loc) l/defl 0.26 13-15 >999 0.53 13-15 >999 0.17 10 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 328 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 6BOT CHORD 2x6 WEBS 2x4	4 SP No.1 *Except* ,6-8: 2x6 SP No.2, 1-3: 2 3 SP No.2 4 SP No.3	x4 SP No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural woor 2-0-0 oc purlins Rigid ceiling di 1 Row at midpl	d sheathing direct s (3-9-4 max.): 4-8 rectly applied or 11 5-16	y applied, except , 0-0-0 oc bracing. , 5-13, 7-12	
REACTIONS. (Ib) Ma Ma	/size) 10=1972/Mechar ax Horz 2=133(LC 12) ax Uplift 10=-139(LC 13),	nical, 2=2034/0-3-8 2=-166(LC 12)						
FORCES. (Ib) - M TOP CHORD 2- 8 BOT CHORD 2- 1	lax. Comp./Max. Ten A -3=-3916/594, 3-4=-3299 -9=-3240/549, 9-10=-378 -17=-474/3417, 16-17=-4 1-12=-436/3263, 10-11=	l forces 250 (lb) or /562, 4-5=-2860/55 1/568 71/3422, 15-16=-38 -436/3263	less except when shown. 5, 5-7=-3480/622, 7-8=-28 32/3495, 13-15=-382/3495	321/543, , 12-13=-381/3480	,			
WEBS 3- 7-	-17=0/295, 3-16=-620/22 -12=-1073/220, 8-12=-97	2, 4-16=-88/1079, 5 /1053, 9-12=-506/2	5-16=-1057/220, 5-15=0/34 17, 9-11=0/287	47, 7-13=0/327,				
NOTES- 1) Unbalanced roof 2) Wind: ASCE 7-1 MWFRS (envelo Interior(1) 21-4-1 for reactions sho 3) Provide adequat 4) This truss has be 5) * This truss has la will fit between th 6) All bearings are at 2) Befor to girdgreg	f live loads have been cor 0; Vult=130mph (3-secor pe) gable end zone and (12 to 35-8-0, Exterior(2) 3 wm; Lumber DOL=1.60 p te drainage to prevent wa sen designed for a 10.0 p been designed for a 10.0 p been designed for a live I ne bottom chord and any assumed to be User Defi	asidered for this des d gust) Vasd=103n C-C Exterior(2) -0-1 5-8-0 to 42-6-12, Ir late grip DOL=1.60 ter ponding. sf bottom chord live oad of 20.0psf on the other members, with the crushing capace tions	sign. hph; TCDL=6.0psf; BCDL= 0-8 to 4-1-3, Interior(1) 4-7 terior(1) 42-6-12 to 49-5-8 bload nonconcurrent with a he bottom chord in all area h BCDL = 10.0psf. ity of 425 psi.	=6.0psf; h=25ft; Ca 1-3 to 14-4-0, Exter 3 zone;C-C for men any other live loads is where a rectangl	t. II; Exp B; Enclosed rior(2) 14-4-0 to 21-4 nbers and forces & M s. e 3-6-0 tall by 2-0-0 to	; -12, WFRS wide	A CARL	CAROLINA SOLAR

Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=139.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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F		8-9-4 8-9-4	17-0-0 8-2-12		<u>25-0-0</u> 8-0-0		<u>33-0-0</u> 8-0-0			41-2-12 8-2-12	49-7 8-4	- <u>4</u> 8	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	*	SPACING- Plate Grip DOI Lumber DOL Rep Stress Ind Code IRC201	2-0-0 L 1.15 1.15 cr YES 5/TPI2014	CSI. TC BC WB Matrix	0.73 0.78 0.65 -S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.25 17 -0.51 17 0.16	(loc) l. 7-18 > 7-18 > 12	/defl •999 •999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 331 It	GRIP 244/190 FT = 20%	
LUMBER- TOP CHORD 2 6 BOT CHORD 2 WEBS 2 REACTIONS. (2x4 SP No 5-8: 2x6 S 2x6 SP No 2x4 SP No (Ib/size) Max Horz Max Uplif	lo.2 *Except* SP No.2 lo.2 lo.3 2=2034/0-3-8, 12 z 2=155(LC 16) ft 2=-193(LC 12), 1	2=1972/Mechanical 2=-167(LC 13)			BRACING TOP CHOP BOT CHOP WEBS	RD S 2 RD R 1	Structural 2-0-0 oc p Rigid ceili ⊢Row at	l wood sl purlins (4 ing direc midpt	neathing diri -1-11 max.) tly applied o 7·	ectly applied or 2-2-(: 6-8. r 10-0-0 oc bracing. -18, 7-15	oc purlins, excep	ot
Max Uplift 2=-193(LC 12), 12=-167(LC 13) FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD $2\cdot3=-3930/579, 3\cdot5=-3679/569, 5\cdot6=-3043/554, 6\cdot7=-2673/537, 7\cdot8=-2644/529, 8-9=-3011/545, 9\cdot11=-3549/549, 11-12=-3760/565 3OT CHORD 2\cdot20=-468/3443, 18\cdot20=-382/3032, 17\cdot18=-299/3089, 15\cdot17=-299/3089, 13\cdot15=-359/2960, 12\cdot13=-431/3246 WEBS 3\cdot20=-313/194, 5\cdot20=-39/480, 5\cdot18=-560/219, 6\cdot18=-91/1023, 7\cdot18=-719/174, 7\cdot17=0/422, 7\cdot15=-755/174, 8\cdot15=-87/1009, 9\cdot15=-501/215, 9\cdot13=-33/401 NOTES- 1) Unbalanced roof live loads have been considered for this design $													
 Wind: ASCE 7 Wind: ASCE 7 MWFRS (enver Interior(1) 24-C reactions show Provide adequiding This truss has * This truss has will fit between All bearings ar Refer to girder Provide mechanistication All 2=167. One H2.5A Sir connection is for the standard purposed 	7-10; Vult elope) ga 0-6 to 33- wn; Lumb Jate drair 5 been de as been de as been de as been de as been de n the bott re assum r(s) for tru anical co mpson Si for uplift or rulin repre	L=130mph (3-secon able end zone and (-0-0, Exterior(2) 33 ber DOL=1.60 plate nage to prevent wa signed for a 10.0 p designed for a live I tom chord and any ned to be User Defii uss to truss connect strong-Tie connecto only and does not essentation does not	d gust) Vasd=103m C-C Exterior(2) -0-1 -0-0 to 40-0-6, Inter grip DOL=1.60 ter ponding. sf bottom chord live oad of 20.0psf on ti other members, with ned crushing capacitions. s) of truss to bearing rs recommended to consider lateral force depict the size or t	spin: TCDL= $($ 0-8 to 4-1-3, ior(1) 40-0-6 e load nonco ne bottom ch h BCDL = 10 ity of 425 ps g plate capal connect tru es. ne orientatio	5.0psf; BCDL= Interior(1) 4- 5 to 49-5-8 zon ord in all area 0.0psf. i. ole of withstar ss to bearing n of the purlin	=6.0psf; h=25ft; 1-3 to 17-0-0, E> ne;C-C for memil any other live loo is where a rectain inding 100 lb uplif walls due to UPI along the top an	Cat. II; Exi terior(2) 1 bers and f ads. ngle 3-6-0 t at joint(s .IFT at jt(s nd/or botto	p B; Enc 17-0-0 to forces & 0 tall by 2 s) except s) 2. This om chore	losed; 24-0-6, MWFRS 2-0-0 wid : (jt=lb) 3	for e	A CONTRACT OF A	SEAL 044925	Net Willing

SEAL 044925 MGINEER M. SEVILITI

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	10-1-4	19-8-0	25-0-0	30-4-0	39-10-12	49-7-4						
	10-1-4	9-6-12	5-4-0	5-4-0	9-6-12	9-8-8						
Plate Olisets (X, Y)	[13:0-0-15,0-2-0], [13:Edge,	0-3-7]										
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0*8CDL	SPACING- 2 Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 CSI 1.15 TC 1.15 BC YES WB	0.87 0.72 0.71	DEFL. in Vert(LL) -0.20 Vert(CT) -0.37 Horz(CT) 0.08	i (loc) l/defl L/d 17-19 >999 240 17-19 >999 180 14 n/a n/a	PLATES MT20	GRIP 244/190					
BODL 10.0			1X-5			Weight. 529 lb	F1 = 2078					
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF SLIDER Right 2	P No.2 P No.2 P No.3 2x4 SP No.3 -H 3-10-11			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing dir 2-0-0 oc purlins (4-2-5 max.): Rigid ceiling directly applied o 1 Row at midpt 5	rectly applied or 2-5-1 o : 6-8. or 6-0-0 oc bracing. 5-19, 7-19, 7-17, 9-15	c purlins, except					
REACTIONS. All be (lb) - Max H Max U Max G	earings 7-8-12 except (jt=leng lorz 2=178(LC 16) plift All uplift 100 lb or less irav All reactions 250 lb or l	gth) 2=0-3-8, 13=0-3-1 at joint(s) except 2=-20 less at joint(s) 13 exce	0 (input: 0-3-10). 6(LC 12), 13=-2 ot 2=1679(LC 1).	224(LC 25), 14=-113(L , 14=2380(LC 2), 14=:	C 13) 2355(LC 1)							
FORCES. (lb) - Max. TOP CHORD 2-3=: 8-9=: 8-9=: BOT CHORD 2-21: 13-14 3-21: WEBS 3-21: 7-17: 11-14	*ORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. ICOP CHORD 2-3=-3098/466, 3-5=-2808/455, 5-6=-2047/435, 6-7=-1762/436, 7-8=-1344/373, 8-9=-1574/366, 9-11=-418/227, 11-13=-60/737 3OT CHORD 2-21=-411/2692, 19-21=-259/2212, 17-19=-113/1642, 15-17=-77/873, 14-15=-619/87, 13-14=-619/87 NEBS 3-21=-373/224, 5-21=-49/567, 5-19=-708/263, 6-19=-55/620, 7-19=-76/316, 7-17=-726/168, 8-17=-32/408, 9-17=-32/759, 9-15=-1481/232, 11-15=-133/1613, 11-4=-2164/310											
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) Interior(1) 26-8-6 to reactions shown; Lu 3) Provide adequate di 4) This truss has been b) * This truss has been b) * This truss has been ill fit between the b 6) All bearings are ass 7) One H2.5A Simpsor This connection is fo 8) Graphical purlin rep	a loads have been considerer /ult=130mph (3-second gust) gable end zone and C-C Exi 30-4-0, Exterior(2) 30-4-0 to mber DOL=1.60 plate grip D rainage to prevent water pon designed for a 10.0 psf botto n designed for a live load of 3 pottom chord and any other n umed to be User Defined cru h Strong-Tie connectors reco pruplift only and does not co resentation does not depict th	d for this design. Vasd=103mph; TCDL terior(2) -0-10-8 to 4-1: 37-4-6, Interior(1) 37-4 OL=1.60 ding. om chord live load none 20.0psf on the bottom nembers, with BCDL = Ishing capacity of 425 mmended to connect the nsider lateral forces. he size or the orientation	=6.0psf; BCDL= 3, Interior(1) 4-1 -6 to 49-5-7 zon oncurrent with a shord in all areas 10.0psf. usi. uss to bearing v n of the purlin a	6.0psf; h=25ft; Cat. II; -3 to 19-8-0, Exterior(le;C-C for members and any other live loads. s where a rectangle 3- walls due to UPLIFT a long the top and/or bo	Exp B; Enclosed; 2) 19-8-0 to 26-8-6, id forces & MWFRS for 6-0 tall by 2-0-0 wide jt(s) 2, 13, and 14. ttom chord.	SCOLUMNIUM SCOL	SEAL GINEER HUM					

- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 13, and 14. This connection is for uplift only and does not consider lateral forces.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

humm February 11,2020

818 Soundside Road Edenton, NC 27932

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	10-2-4	19-8-0	25-0-0	29-6-0	30-4-0		39-9-12		50-0-0				
Plate Offsets (X,Y)	[9:0-8-6.Edge]. [17:0-3-0.0-4-0	0]. [20:0-1-12.0-2-0]	5-4-0	4-0-0	0-10-0		9-0-12		10-2-4				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2- Plate Grip DOL 1. Lumber DOL 1. Rep Stress Incr Y Code IRC2015/TPI201	0-0 CSI. .15 TC (.15 BC (ES WB (14 Matrix-3	0.86 0.99 0.87 S	DEFL. Vert(LL) Vert(CT) Horz(CT) Attic	in -0.63 -1.14 0.13 -0.59	(loc) 20-22 20-22 13 18-20	l/defl >557 >309 n/a 400	L/d 240 180 n/a 360	PLATES MT20 MT18HS Weight: 356 lb	GRIP 244/190 244/190 FT = 20%			
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP 16-19, WEBS 2x4 SP 6-20,8-	P No.2 P No.2 *Except* 19-21: 2x6 SP DSS No.3 *Except* 18,9-17: 2x4 SP No.2			BRACING- TOP CHOP BOT CHOP WEBS JOINTS	RD RD	Structu 2-0-0 o Rigid cu 2-2-0 o 1 Row 1 Brace	ral wood c purling eiling dire c bracing at midpt e at Jt(s):	sheathing d (2-11-12 ma ctly applied : 20-22. 24, 25	lirectly applied, except ax.): 6-9. I or 10-0-0 oc bracing, I 5-20, 18-24, 10-17	Except:			
REACTIONS. (Ib/size Max H Max U Max G	EACTIONS. (lb/size) 2=1871/0-3-8, 18=556/0-3-8, 13=1771/0-3-8 Max Horz 2=-170(LC 17) Max Uplift 2=-255(LC 12), 18=-224(LC 13), 13=-102(LC 12) Max Grav 2=2046(LC 26), 18=848(LC 25), 13=1896(LC 2)												
FORCES. (lb) Hax. TOP CHORD 2-3=- 8-9=- BOT CHORD 2-22= 13-1 WEBS 3-22= 8-25= 10-15 10-15	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-4021/485, 3-5=-3779/474, 5-6=-2892/439, 6-7=-2947/555, 7-8=-2945/553, 8-9=-2458/431, 9-10=-2777/450, 10-12=-3375/446, 12-13=-3625/466 BOT CHORD 2-22=-502/3526, 20-22=-317/3023, 18-20=-131/2497, 17-18=-128/2466, 15-17=-178/2765, 13-15=-297/3177 WEBS 3-22=-367/225, 5-22=-62/681, 5-20=-798/270, 20-23=-65/879, 6-23=-54/899, 8-25=-163/685, 18-24=-445/141, 8-24=-421/152, 9-17=-129/1032, 10-17=-664/277, 10-15=-67/528, 12-15=-388/228, 7-25=-365/149, 6-25=-148/476												
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; V MWFRS (envelope) Interior(1) 26-8-14 to MWFRS for reaction 3) Provide adequate dr 4) All plates are MT20 5) This truss has been will fit between the b 7) Ceiling dead load (5 8) Bottom chord live load 9) All bearings are assis 10) One H2.5A Simpson This connection is 11) Graphical purlin rep	e loads have been considered /ult=130mph (3-second gust) \ gable end zone and C-C Exte o 30-4-0, Exterior(2) 30-4-0 to is shown; Lumber DOL=1.60 p rainage to prevent water pondi plates unless otherwise indica designed for a 10.0 psf botton n designed for a live load of 20 ottom chord and any other me .0 psf) on member(s). 23-25, 2 ad (40.0 psf) and additional bo umed to be User Defined crus on Strong-Tie connectors reco for uplift only and does not cop presentation does not depict to	for this design. /asd=103mph; TCDL=6. rior(2) -0-10-8 to 4-1-8, I 37-4-14, Interior(1) 37-4 Jate grip DOL=1.60 ing. ted. n chord live load noncon 0.0psf on the bottom cho mmbers, with BCDL = 10. 24-25 thom chord dead load (0 hing capacity of 425 psi. mmended to connect tru nsider lateral forces. ne size or the orientation	0psf; BCDL=6.0p Interior(1) 4-1-8 to -14 to 50-10-8 zo current with any of rd in all areas wh .0psf. .0 psf) applied or ss to bearing wal of the purlin alor	osf; h=25ft; (o 19-8-0, Ex one;C-C for r other live loa here a rectar hly to room. Ils due to UF ng the top ar	Cat. II; E tterior(2) member ads. ngle 3-6 18-20 PLIFT at nd/or bo	Exp B; E) 19-8-0 's and fo -0 tall by t jt(s) 2, ttom cho	nclosed; to 26-8-1 prces & y 2-0-0 wi 18, and 1 prd.	4, de 3.	CORDER OF SOL	SEAL GINEER HELIN			

- will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Ceiling dead load (5.0 psf) on member(s). 23-25, 24-25
- 8) Bottom chord live load (40.0 psf) and additional bottom chord dead load (0.0 psf) applied only to room. 18-20
- 9) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 18, and 13. This connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. WARNING - Verify design parameters and KEAD NOTES ON THIS AND INCLUDED MITER KETERENCE PAGE MIT-14's rev. 10/04/2013 BETORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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	8-8-4		17-0-0	23-1-4	29-4-4 33		41-3-12	50-0-0)
		8-8-4	8-3-12	6-1-4	6-3-0	3-7-12	8-3-12	8-8-4	I
Plate Offsets (2	X,Y)	[6:0-4-0,0-4-8], [7:0-4-	6,Edge]						
LOADING (ps TCLL 20. TCDL 10. BCLL 0. BCDL 10.	f) 0 .0 .0 * .0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inci Code IRC2015	2-0-0 . 1.15 1.15 r YES //TPI2014	CSI. TC 0.70 BC 0.55 WB 0.87 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.19 16-18 -0.33 16-18 0.02 16	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 330 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	No.2 *Except* : 2x6 SP No.2 No.2 No.3		· /	BRACING- TOP CHOR BOT CHOR WEBS	D Structu except 2-0-0 d D Rigid d 1 Row	ural wood sheathing coc purlins (6-0-0 ma ceiling directly appli at midpt	g directly applied or 3-11-5 ax.): 5-7. ed or 6-0-0 oc bracing. 5-18, 6-16, 7-16	oc purlins,	
REACTIONS.	(Ib/size Max H Max U Max G	e) 1=881/0-3-8, 16= orz 1=-155(LC 17) plift 1=-130(LC 12), 16 rav 1=937(LC 23), 16	2664/0-3-8(req. 0 6=-98(LC 12), 11= =2664(LC 1), 11={	-4-3), 11=492/0-3-8 -143(LC 13) 576(LC 24)			·		
FORCES. (Ib) - Max. 1-2=-	Comp./Max. Ten All 1661/261, 2-4=-1406/2	forces 250 (lb) or 242, 4-5=-714/206	less except when shown. 5, 5-6=-580/226, 6-7=-19/10	004,				

 7-8=-15/591, 8-10=-478/200, 10-11=-733/229

 BOT CHORD
 1-20=-298/1422, 18-20=-136/995, 15-16=-476/233, 11-13=-120/591

 WEBS
 2-20=-331/199, 4-20=-53/463, 4-18=-615/231, 6-18=-82/903, 6-16=-1580/347,

7-16=-1270/274, 7-15=-114/493, 8-15=-641/217, 8-13=-32/541, 10-13=-332/192

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 5-1-12, Interior(1) 5-1-12 to 17-0-0, Exterior(2) 17-0-0 to 24-0-14, Interior(1) 24-0-14 to 33-0-0, Exterior(2) 33-0-0 to 40-0-14, Interior(1) 40-0-14 to 50-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) WARNING: Required bearing size at joint(s) 16 greater than input bearing size.
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 16, and 11. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

ORT Community Street MULLIUM III SEAL 044925 S M. //////// February 11,2020

TENGINEERING BY A MiTek Affiliate 818 Soundside Road

Edenton, NC 27932

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	7-5-4	14-4-0	21-9-4	29-4-4	35-8-0	42-6-12	2 50-0	0-0
	7-5-4	6-10-12	7-5-4	7-7-0	6-3-12	6-10-12	2 7-5	5-4
Plate Offsets (X,Y)	[2:0-3-0,0-3-4], [8:0-3	3-0,0-3-4], [11:0-4-0	,0-4-8], [15:0-4-4,0-4-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress In Code IRC201	2-0-0 IL 1.15 1.15 cr YES 5/TPI2014	CSI. TC 0.77 BC 0.89 WB 0.90 Matrix-S	DEFL. i Vert(LL) -0.3 Vert(CT) -0.6 Horz(CT) 0.0	n (loc) l/defl 3 1-15 >999 9 1-15 >508 2 9 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 303 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF	 No.2 No.2 No.3 			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood 2-0-0 oc purlins Rigid ceiling dire 1 Row at midpt	sheathing direo (4-9-15 max.): ctly applied or 6-1	otty applied or 2-4-4 o 3-7. 6-0-0 oc bracing. 2, 7-12	oc purlins, except
REACTIONS. (Ib/siz Max H Max U Max C	e) 1=961/0-3-8, 12 lorz 1=-134(LC 13) Jplift 1=-118(LC 12), Grav 1=995(LC 23), 1	=2490/0-3-8(req. 0 12=-206(LC 9), 9=-1 2=2490(LC 1), 9=64	-3-14), 9=587/0-3-8 34(LC 13) 40(LC 24)					
FORCES. (lb) - Max. TOP CHORD 1-2= 8-9= 8-9= BOT CHORD 1-15: WEBS 2-15: 7-12: 7-12:	Comp./Max. Ten A -1628/329, 2-3=-1147 -716/244 =-262/1400, 14-15=-6 =-527/317, 4-15=-94/ =-1182/169, 7-11=0/6	ll forces 250 (lb) or 7/199, 3-4=-937/231 58/461, 12-14=-814/ 709, 4-14=-971/236 532, 8-11=-541/306	less except when shown. , 4-6=-450/160, 6-7=-22/8 289, 9-11=-116/567 , 6-14=-237/1576, 6-12=-1	16, 1496/341,				
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-10; \ MWFRS (envelope) Interior(1) 21-4-14 to for reactions shown 3) Provide adequate d 4) This truss has been 5) * This truss has been will fit between the b 6) WARNING: Require 7) All bearings are ass	e loads have been co /ult=130mph (3-secon gable end zone and o 35-8-0, Exterior(2) 3 ; Lumber DOL=1.60 p rainage to prevent wa designed for a 10.0 p en designed for a live bottom chord and any de bearing size at join umed to be Liser Def	nsidered for this des nd gust) Vasd=103n C-C Exterior(2) 0-1- 35-8-0 to 42-7-8, Inte late grip DOL=1.60 ther ponding. Sis foottom chord live load of 20.0psf on the other members, wit t(s) 12 greater than ined grusbing capaci	sign. hph; TCDL=6.0psf; BCDL: 12 to 5-1-12, Interior(1) 5 erior(1) 42-7-8 to 50-10-8 e load nonconcurrent with he bottom chord in all area h BCDL = 10.0psf. input bearing size. ity of 425 psi	=6.0psf; h=25ft; Cat. II -1-12 to 14-4-0, Exterio zone;C-C for members any other live loads. as where a rectangle 3	; Exp B; Enclosed; or(2) 14-4-0 to 21-4 s and forces & MWI -6-0 tall by 2-0-0 wi	-14, FRS de	Strutter H	CAROLINA ESSION NA

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 12, and 9. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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L	6-1-4	11-8-0	2	0-5-4		29-4-4			38-4-0		43-10-12	<u> </u>	50-0-0
1	6-1-4	5-6-12	1 8	3-9-4	1	8-11-0	1		8-11-12	1	5-6-12	1	6-1-4
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SP Pla Lui Re Co	ACING- ate Grip DOL mber DOL p Stress Incr de IRC2015/TPI	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matrix	0.86 0.63 0.75 <-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.14 -0.31 0.03	(loc) 1-18 1-18 14	l/defl >999 >999 n/a	L/d 240 180 n/a	P M	LATES 1T20 Veight: 295 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x 6-1 BOT CHORD 2x WEBS 2x REACTIONS (III M M	BCDL 10.0 Code IRC2015/TPI2014 Matrix-S LUMBER- TOP CHORD 2x4 SP No.2 *Except* 6-8: 2x4 SP No.1 6-8: 2x4 SP No.1 BOT CHORD 2x6 SP No.2 2x4 SP No.3 REACTIONS. (lb/size) 1=966/0-3-8, 14=2477/0-3-8 (req. 0-3-14), 10=595/0-3-8 Max Horz Max Uplift 1=-106(LC 12), 14=-286(LC 9), 10=-134(LC 13)						D	Structu except 2-0-0 c Rigid c 1 Row	iral wood oc purlins eiling dire at midpt	sheathing of (4-3-8 max ectly applied	directly ap .): 3-8. d or 6-0-0 5-14	olied or 3-11-() oc purlins,

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1719/357, 2-3=-1326/235, 3-4=-1106/251, 4-5=-591/142, 5-7=-53/950,

7-8=-305/153, 8-9=-406/129, 9-10=-799/242

BOT CHORD 1-18=-251/1492, 16-18=-152/1034, 12-14=-950/279, 10-12=-130/653

WEBS 2-18=-431/259, 3-18=-20/366, 4-16=-747/193, 5-16=-67/943, 5-14=-1429/240,

7-14=-1178/273, 7-12=-217/1382, 8-12=-309/145, 9-12=-405/244

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 5-1-12, Interior(1) 5-1-12 to 11-8-0, Exterior(2) 11-8-0 to 18-8-14, Interior(1) 18-8-14 to 38-4-0, Exterior(2) 38-4-0 to 45-4-14, Interior(1) 45-4-14 to 50-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) WARNING: Required bearing size at joint(s) 14 greater than input bearing size.

7) All bearings are assumed to be User Defined crushing capacity of 425 psi.

 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 14, and 10. This connection is for uplift only and does not consider lateral forces.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Scale = 1:87.5



	L	9-0-0	15-9-7	19-2-2	22-6-13	29-4-4		35-2-2		41-0-0	50-0-0	
	1	9-0-0	6-9-7	3-4-11	3-4-11	6-9-7	1	5-9-14	1	5-9-14	9-0-0	Ι
Plate Offset	s (X,Y)	[4:0-2-4,0-5-0], [7:0-4-6,E0	dge]									
	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 2	20.0	Plate Grip DOL Lumber DOL	1.15 1.15	BC	0.90	Vert(LL) Vert(CT)	-0.09	15-17 15-17	>999 >999	240 180	M120	244/190
BCLL	10.0	Code IRC2015/TP	12014	WB Matrix	0.95 (-S	Horz(CT)	0.06	9	n/a	n/a	Weight: 310 lb	FT = 20%
LUMBER-						BRACING-						
TOP CHOR	D 2x6 SP	No.2 *Except*				TOP CHOR	D	Structura	al wood sl	heathing d	lirectly applied or 3-10-4	oc purlins,
BOT CHOR WEBS	1-3,7-1 D 2x6 SP 2x4 SP 4-17,4-	0: 2x4 SP No.2 No.2 No.3 *Except* 13: 2x4 SP No.2		BOT CHOR WEBS	D	except 2-0-0 oc Rigid cei 1 Row a 2 Rows	purlins (4 iling direc t midpt at 1/3 pts	1-6-1 max. tly applied): 3-7. l or 10-0-0 oc bracing. 4-17 4-13, 7-13			
REACTION	S. (Ib/size Max He Max U Max G	e) 1=1037/0-3-8, 13=232 orz 1=-89(LC 13) plift 1=-86(LC 12), 13=-34 rav 1=1042(LC 23), 13=2	23/0-3-8 (req. 0 4(LC 9), 9=-116 323(LC 1), 9=6)-3-10), 9=6 6(LC 13) 93(LC 24)	379/0-3-8			2110000				
FORCES. TOP CHOR	(lb) - Max. D 1-2=- 7-8=- ⁻	- Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. 1-2=-1951/369, 2-3=-1689/278, 3-4=-1473/284, 4-5=-69/701, 5-7=-69/704, 7-8=-758/141 8-9=-1030/229										
BOT CHOR WEBS	D 1-17= 3-17= 8-11=	267/1692, 15-17=-206/14 -0/419, 4-15=0/389, 4-13= 259/190	409, 13-15=-20 -2254/359, 5-1;	6/1409, 11- 3=-730/321	13=0/629, 9-11=- , 7-13=-1389/218	144/868 , 7-11=0/481,						
NOTES-												

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 5-1-12, Interior(1) 5-1-12 to 9-0-0, Exterior(2) 9-0-0 to 16-0-14, Interior(1) 16-0-14 to 41-0-0, Exterior(2) 41-0-0 to 48-0-14, Interior(1) 48-0-14 to 50-10-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) WARNING: Required bearing size at joint(s) 13 greater than input bearing size.
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 13, and 9. This connection is for uplift only and does not consider lateral forces.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

 \sim Communities WWWWWWW SEAL 044925 Ô MI. MI. S February 11,2020



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Job	Truss	Truss Type	Qty	Ply	140.1582.C 12x12 CVP	
						I40215310
140_1582_C	HG1	Half Hip Girder	1	1		
					Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.	330 s Jan	22 2020 MiTek Industries, Inc. Mon Feb 10 17:04:46 2020	Page 2

ID:RUSz4LGuFS2C1bODNZWBaZyX6cZ-Lbx_2e4ftcv9beeKQ5hUn2XgUmavgdG6H8XRwfzmTxl

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-14=-60, 2-15=-20

Concentrated Loads (lb)

Vert: 25=-27(F) 27=-73(F) 6=-73(F) 23=-27(F) 11=-73(F) 18=-27(F) 21=-27(F) 28=-63(F) 29=-73(F) 30=-73(F) 31=-73(F) 32=-73(F) 33=-73(F) 34=-73(F) 35=-73(F) 35=-77(F) 55=-27(F) 5

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Truss	Truss Type	Qty	Ply	140.1582.C 12x12 CVP	
					I40215311
HG15	Hip Girder	1	1		
				Job Reference (optional)	
Dunn, NC - 28334,		8.3	330 s Jan 2	22 2020 MiTek Industries, Inc. Mon Feb 10 17:04:49 2020	Page 2
	russ IG15 Dunn, NC - 28334,	Truss Truss Type IG15 Hip Girder Dunn, NC - 28334,	Truss Truss Type Qty IG15 Hip Girder 1 Dunn, NC - 28334, 8.3	Truss Truss Type Qty Ply IG15 Hip Girder 1 1 1 Dunn, NC - 28334, 8.330 s Jan 2 8.330 s Jan 2	Truss Truss Type Qty Ply 140.1582.C 12x12 CVP IG15 Hip Girder 1 1 Job Reference (optional) Dunn, NC - 28334, 8.330 s Jan 22 2020 MiTek Industries, Inc. Mon Feb 10 17:04:49 2020

ID:RUSz4LGuFS2C1bODNZWBaZyX6cZ-mAd6gg6XAXHkS6Nv5DEBPg9C8zdhtzhYz6m5XzzmTxi

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-12=-60, 12-15=-60, 1-14=-20

Concentrated Loads (lb)

Vert: 26=-27(B) 29=-27(B) 5=-73(B) 25=-27(B) 24=-27(B) 6=-73(B) 20=-27(B) 10=-73(B) 13=-68(B) 16=-73(B) 19=-27(B) 22=-27(B) 32=-73(B) 33=-73(B) 34=-73(B) 35=-73(B) 35

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	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL	20.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
CDL	10.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	-0.06	2-4	>984	180		
CLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
SCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-P						Weight: 18 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEDGE

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-10-8 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Left: 2x4 SP No.3

REACTIONS. (lb/size) 3=133/Mechanical, 2=255/0-3-8, 4=47/Mechanical Max Horz 2=144(LC 12) Max Uplift 3=-107(LC 12), 2=-11(LC 12) Max Grav 3=148(LC 19), 2=255(LC 1), 4=93(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-9-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 107 lb uplift at joint 3.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This

connection is for uplift only and does not consider lateral forces.



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	SPACING. 2-0-0	CSI	DEEL	in	(loc)	l/defl	L/d	PI ATES	GRIP
TCLL 20.0 TCDL 10.0 BCLL 0.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	TC 0.33 BC 0.24 WB 0.00	Vert(LL) Vert(CT) Horz(CT)	-0.02 -0.05 0.03	2-5 2-5 4	>999 >999 >999 n/a	240 180 n/a	MT20	244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	BRACING-					Weight: 18 lb	FT = 20%
TOP CHORD 2x4	TOP CHOR	D	Structu	ral wood	sheathing di	rectly applied or 4-10-	8 oc purlins.		

 LUMBER BRACING

 TOP CHORD 2x4 SP No.2
 TOP CHORD
 TOP CHORD
 Structural wood sheathing directly applied or 4-10-8 oc purlins, except

 BOT CHORD
 2x4 SP No.2
 Event
 2-0-0 oc purlins: 3-4.

 WEDGE
 Left: 2x4 SP No.3
 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=123/Mechanical, 2=255/0-3-8, 5=57/Mechanical Max Horz 2=128(LC 12) Max Uplift 4=-69(LC 12), 2=-21(LC 12) Max Grav 4=123(LC 1), 2=255(LC 1), 5=88(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-1-11, Exterior(2) 4-1-11 to 4-9-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
 will be truss the better the better bed and been allowed by a second better to be the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 4.

One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
connection is for uplift only and does not consider lateral forces.

10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 2-5=-20 Concentrated Loads (lb)

Vert: 3=-41(B) 7=-36(B) 8=-24(B) 9=-16(B)



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		F		3-10-4	
LOADING TCLL TCDL BCU	(psf) 20.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr. YES	CSI. TC 0.21 BC 0.16 WB 0.00	DEFL. in (loc) I/defl I Vert(LL) -0.01 2-4 >999 2 Vert(CT) -0.02 2-4 >999 1 Horz(CT) -0.00 3 n/a 1	L/d PLATES GRIP 240 MT20 244/190 180 p/a
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 14 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-10-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=101/Mechanical, 2=216/0-3-8, 4=36/Mechanical Max Horz 2=88(LC 12) Max Uplift 3=-65(LC 12), 2=-32(LC 12) Max Grav 3=101(LC 1), 2=216(LC 1), 4=73(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-9-8 zone;C-C for members and forces
- & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 65 lb uplift at joint 3.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





		1-6-0		3-10-4	
Plate Offsets (X,Y)	[3:0-3-0,0-2-0]	1-6-0		2-4-4	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.18 BC 0.13 WB 0.00 Matrix-P	DEFL. ir Vert(LL) -0.01 Vert(CT) -0.02 Horz(CT) 0.01	n (loc) l/defl L/d 2-5 >999 240 2 2-5 >999 180 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 13 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP	No.2 No.2		BRACING- TOP CHORD	Structural wood sheathing dire	ectly applied or 3-10-4 oc purlins,

except 2-0-0 oc purlin: BOT CHORD Rigid ceiling di

2-0-0 oc purlins: 3-4. Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (lb/size) 4=96/Mechanical, 2=216/0-3-8, 5=42/Mechanical Max Horz 2=44(LC 12) Max Uplift 4=-38(LC 9), 2=-36(LC 12)

Max Grav 4=96(LC 1), 2=216(LC 1), 5=67(LC 3)

NOTES-

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be User Defined crushing capacity of 425 psi.

Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 4.
- One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This
 connection is for uplift only and does not consider lateral forces.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This
 connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.



BRACING-TOP CHORD

BOT CHORD

11	IM	R	FP)_

BCDL

TOP CHORD 2x4 SP No.2

10.0

2x4 SP No.2 BOT CHORD

REACTIONS. 3=21/Mechanical, 4=13/Mechanical, 2=125/0-3-0 (lb/size) Max Horz 2=41(LC 12) Max Uplift 3=-16(LC 12), 4=-4(LC 8), 2=-30(LC 12) Max Grav 3=21(LC 1), 4=26(LC 3), 2=125(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 3 and 4 lb uplift at ioint 4.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 1-4-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 100322010 SECORE 052. Design valid for use only with MITEK @ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD

2x4 SP No.2 WEBS

REACTIONS. 4=41/0-3-8, 2=129/0-3-0 (lb/size) Max Horz 2=43(LC 12) Max Uplift 4=-15(LC 9), 2=-29(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be User Defined crushing capacity of 425 psi.
- 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4 and 2. This connection is for uplift only and does not consider lateral forces.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPRETENCE PAGE MIT-1473 TeV. 100322010 SECORE 052. Design valid for use only with MITEK @ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-98 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl
TCLL	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	-0.00	1	n/r
TCDL	10.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	0.00	1	n/r
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00		n/a
BCDL	10.0	Code IRC2015/TF	912014	Matri	x-P				

LUMBER-		BRACING-		
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing dire	ectly applied or 4-10-8 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.	
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied o	r 10-0-0 oc bracing.
OTHERS	2x4 SP No.3			

WEDGE

Left: 2x4 SP No.3

REACTIONS. (Ib/size) 5=44/4-10-8, 2=156/4-10-8, 6=231/4-10-8 Max Horz 2=142(LC 12) Max Uplift 5=-17(LC 12), 6=-101(LC 12) Max Grav 5=46(LC 19), 2=156(LC 1), 6=250(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 4-8-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) All bearings are assumed to be User Defined crushing capacity of 425 psi.



PLATES

Weight: 26 lb

MT20

L/d

120

120 n/a GRIP

244/190

FT = 20%

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		4-0-0	
LOADING (psf) SPACING- 2-0 TCLL 20.0 Plate Grip DOL 1.' TCDL 10.0 Lumber DOL 1.' BCLL 0.0 * Rep Stress Incr YE BCDL 10.0 Code IRC2015/TPI201- 1.'	CSI. TC 0.24 BC 0.17 WB 0.00 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) 0.02 2-4 >999 240 Vert(CT) -0.03 2-4 >999 180 Horz(CT) -0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 14 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. 3=107/Mechanical, 2=220/0-3-0, 4=38/Mechanical (lb/size) Max Horz 2=91(LC 12) Max Uplift 3=-68(LC 12), 2=-32(LC 12), 4=-12(LC 8) Max Grav 3=107(LC 1), 2=220(LC 1), 4=76(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-11-4 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 3 and 12 lb uplift at ioint 4.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.



			1-7-0 1-7-0						4-0-0 2-5-0		
Plate Offsets (X,Y)	[3:0-3-0,0-2-0]		1		1					1	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	0.01	2-7	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	-0.01	2-7	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	7	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI	2014	Matrix	(-R						Weight: 15 lb	FT = 20%
LUMBER-					BRACING-						

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins,

except end verticals, and 2-0-0 oc purlins: 3-5.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (lb/size) 7=147/Mechanical, 2=212/0-3-0 Max Horz 2=46(LC 12)

Max Uplift 7=-59(LC 9), 2=-47(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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		<u> 6-0-0</u> 6-0-0		<u>12-0-0</u> 6-0-0	———————————————————————————————————————
Plate Offsets (X,Y)	[2:0-0-0,0-0-12], [2:0-0-15,0	0-4-5], [4:Edge,0-0-12], [4:0-0-15,0-	4-5]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI2	2-0-0 CSI. 1.15 TC 0.50 1.15 BC 0.38 YES WB 0.11 2014 Matrix-S	DEFL. ir Vert(LL) 0.07 Vert(CT) -0.06 Horz(CT) 0.01	n (loc) I/defl L/d 7 4-6 >999 240 6 4-6 >999 180 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 50 lb FT = 20%
LUMBER- TOP CHORD 2x4 S	SP No.2		BRACING- TOP CHORD	Structural wood sheathing d	rectly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (Ib/size) 2=530/0-3-0, 4=530/0-3-0 Max Horz 2=-113(LC 10) Max Uplift 2=-71(LC 12), 4=-71(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-562/402, 3-4=-562/402

BOT CHORD 2-6=-224/377, 4-6=-224/377

WEBS 3-6=-290/287

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 6-0-0, Exterior(2) 6-0-0 to 9-0-0, Interior(1) 9-0-0 to 12-10-8 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.



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te Offsets (X,Y)	[3:0-2-0,Edge], [5:0-2-0,Edge]	L		10-0-0						
ADING (psf)	SPACING- 2-0	0-0 CSI .		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
LL 20.0	Plate Grip DOL 1.	.15 TC	0.14	Vert(LL)	0.00	7	n/r	120	MT20	244/190
DL 10.0	Lumber DOL 1.	.15 BC	0.19	Vert(CT)	0.00	7	n/r	120		
LL 0.0 *	Rep Stress Incr Y	ES WB	0.04	Horz(CT)	0.00	6	n/a	n/a		
DL 10.0	Code IRC2015/TPI201	14 Matr	ix-S						Weight: 30 lb	FT = 20%

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, excep 2-0-0 oc purlins (6-0-0 max.): 3-5. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=222/8-9-6 (min. 0-1-8), 6=224/8-9-6 (min. 0-1-8), 8=322/8-9-6 (min. 0-1-8) Max Horz 2=-18(LC 10) Max Uplift 2=-39(LC 12), 6=-39(LC 12), 8=-29(LC 9)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-260/134, 5-6=-260/135

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2) 0-4-11 to 6-10-15, Interior(1) 6-10-15 to 8-0-0, Exterior(2) 8-0-0 to 10-3-5 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 2, 39 lb uplift at joint 6 and 29 lb uplift at joint 8.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



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			10-8-0						
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.29	Vert(LL)	-0.01	4-6	>999	240	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.19	Vert(CT)	-0.02	4-6	>999	180		
CLL 0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT)	0.00	4	n/a	n/a		
CDL 10.0	Code IRC2015/TPI2014	Matrix-P	()					Weight: 33 lb	FT = 20%

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

REACTIONS. All bearings 10-8-0.

Max Horz 1=-44(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) except 1=-210(LC 1), 5=-146(LC 1), 5=-146(LC 1), 2=-218(LC 12), 4=-174(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 2=457(LC 1), 4=406(LC 1), 6=300(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-4-11 to 3-4-11, Interior(1) 3-4-11 to 5-4-0, Exterior(2) 5-4-0 to 8-4-0, Interior(1) 8-4-0 to 10-5-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 1 and 146 lb uplift at joint 5.

6) n/a

7) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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TREERING BY A MITEK Affiliate 818 Soundside Road

Edenton, NC 27932

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F	4-1-14		7-10-2			12-0-0	
Plate Offsets (X,Y)	[2:0-0-15,0-4-5], [2:0-0-0,0-0-12], [3:0-4-	-0,0-1-9], [4:0-4-0,0-1-9],	<u>5:0-0-0,0-0-12], [5:0-0-1</u>	5,0-4-5]		4-1-14	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.24 WB 0.06 Matrix-S	DEFL. in Vert(LL) 0.04 Vert(CT) -0.04 Horz(CT) 0.01	(loc) 2-8 5-7 5	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 51 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 3 BOT CHORD 2x4 3 WEBS 2x4 3	SP No.2 SP No.2 SP No.3		BRACING- TOP CHORD BOT CHORD	Structur 2-0-0 or Rigid cr	ral wood sheath c purlins (6-0-0 r eiling directly ap	ing directly applied or 6-0- nax.): 3-4. plied or 10-0-0 oc bracing.	0 oc purlins, except

WEDGE Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (Ib/size) 2=530/0-3-0, 5=530/0-3-0 Max Horz 2=82(LC 11) Max Horit 2= 106(LC 11)

Max Uplift 2=-106(LC 9), 5=-106(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-622/499, 3-4=-442/434, 4-5=-622/499

BOT CHORD 2-8=-321/447, 7-8=-312/442, 5-7=-320/447

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-1-14, Exterior(2) 4-1-14 to 11-10-8, Interior(1) 11-10-8 to 12-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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		I	5-7-	14		6-4-2			12-0	D-0	1	
		Γ	5-7-	·14		0-8-4			5-7-	-14	1	
Plate Off	fsets (X,Y)	[2:0-0-15,0-4-5], [2:0-0-0	,0-0-12], [3:0-	4-0,0-1-9], [4	0-4-0,0-1-9], [5:Edge,0-0-12],	5:0-0-15	,0-4-5]				
LOADIN	IG (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.44	Vert(LL)	0.06	5-7	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.31	Vert(CT)	-0.07	5-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.01	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S						Weight: 55 lb	FT = 20%

BRACING-LUMBER-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2x4 SP No.2 2-0-0 oc purlins (6-0-0 max.): 3-4. WEBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing WEDGE

Left: 2x4 SP No.3 , Right: 2x4 SP No.3

REACTIONS. (lb/size) 2=530/0-3-0, 5=530/0-3-0 Max Horz 2=-106(LC 10) Max Uplift 2=-70(LC 12), 5=-70(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-572/421, 3-4=-386/389, 4-5=-572/421

BOT CHORD 2-8=-240/390, 7-8=-234/386, 5-7=-239/390

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-7-14, Exterior(2) 5-7-14 to 10-7-1, Interior(1) 10-7-1 to 12-10-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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		10-1-4	19-8-0		25-0-0	30-4-0		36-8-8	42-0-4	4 49-7-4		
		10-1-4	9-6-12		5-4-0	5-4-0	I	6-4-8	5-3-12	2 7-7-0		
Plate Offse	ts (X,Y)	[6:0-3-0,0-2-7], [8:0-3-	0,0-2-7], [16:0-6-4	,0-2-4], [18:0-	-2-4,0-2-0]							
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Inci Code IRC2015	2-0-0 1.15 1.15 r YES j/TPI2014	CSI. TC BC WB Matrix-	0.93 0.83 0.90 -S	DEFL. Vert(LL) Vert(CT) Horz(CT) Attic	in (lo -0.57 18- -1.01 18- 0.10 -0.33 16-	c) I/defl 20 >885 20 >499 12 n/a 18 398	L/d 240 180 n/a 360	PLATES MT20 Weight: 352 lb	GRIP 244/190 FT = 20%	
LUMBER- TOP CHOR BOT CHOR WEBS	2x4 SP 6-8: 2x6 2x6 SP 12-15: 2 2x4 SP 6-18,8-	No.2 *Except* 5 SP No.2 DSS *Except* 2x6 SP No.2, 16-18: 2 No.3 *Except* 16,22-23: 2x4 SP No.2	x4 SP No.2			BRACING TOP CHOP BOT CHOP WEBS JOINTS	RD Str 2-0 RD Rig 1 F 1 E	uctural wood -0 oc purling id ceiling di ow at midpt race at Jt(s	d sheathing dirr s (4-0-12 max.) rectly applied c : 5 : 21, 22, 23	ectly applied, except): 6-8. or 8-5-9 oc bracing. -18		
REACTION	IS. (Ib/size Max Ho Max Uj Max G	 2=1914/0-3-8 (mi prz 2=179(LC 12) plift 2=-221(LC 12), 13 rav 2=2067(LC 2), 13 	n. 0-2-1), 13=1165 8=-343(LC 13), 12= =1572(LC 27), 12=	/0-3-8 (min. =-205(LC 12) :1157(LC 26)	0-1-14), 12=	₌1041/Mechanical						
FORCES. TOP CHOR	(lb) - Max. 2D 2-24= 5-6=-: 8-9=-: 12-29	Comp./Max. Ten All -4061/451, 3-24=-397 2965/418, 6-26=-2919 2886/425, 9-28=-2526 =-2258/456	forces 250 (lb) or 1 2/469, 3-4=-3822/4 /535, 7-26=-2920/9 /386, 10-28=-2529	ess except w 427, 4-25=-36 534, 7-27=-29 /383, 10-11=	/hen shown. 692/455, 5-2 919/534, 8-2 ⊶2633/360, 1	25=-3692/458, 17=-2918/535, 11-29=-2212/478,						
BOT CHOR	2-20= 17-18 13-14	447/3561, 20-30=-26 =-91/2557, 16-17=-91 =-363/1970, 12-13=-3	0/3082, 19-30=-26 /2557, 16-32=-151 63/1970	0/3082, 19-3 /2293, 15-32	1=-260/3082 =-151/2293,	2, 18-31=-260/308 14-15=-151/2293	2, ,					
WEBS	3-20= 7-21= 9-14=	-358/221, 5-20=-73/66 -346/162, 8-21=-177/6 -704/164, 11-14=-155	60, 5-18=-814/281, 683, 16-23=-53/740 /1006, 11-13=-133	18-22=-31/8 5, 8-23=-42/7 3/416, 6-21=	895, 6-22=-1 780, 9-16=-1 -183/509	7/915, 2/424,						
NOTES- 1) Unbalan 2) Wind: AS MWFRS Interior(1 forces & 3) Provide: 4) This trus 5) * This tru will fit be 6) Ceiling d 7) Bottom c 8) Refer to 9) Provide I 13 and 2 10) This tru referen Cohufareario	ced roof live SCE 7-10; V (envelope)) 26-8-6 to 3 MWFRS for adequate dra s has been tween the bi- lead load (5. chord live loa girder(s) for mechanical 05 lb uplift a iss is design ced standar mbagging ref	loads have been cons ult=130mph (3-second gable end zone and C 30-4-0, Exterior(2) 30- reactions shown; Lun designed for a 10.0 ps n designed for a 10.0 ps n design des net design trues to trus connect connection (by others) the sign network of the sign parameters and FEA	sidered for this des d gust) Vasd=103m -C Exterior(2) -0-1 4-0 to 37-4-6, Inter ber DOL=1.60 pla er ponding. f bottom chord live ad of 20.0psf on th ther members, wit 21-22, 21-23 ional bottom chord ions. of truss to bearing the 2015 Internati depict the size or the p NOTES ON THIS AM	ign. iph; TCDL=6 0-8 to 4-1-3, ior(1) 37-4-6 te grip DOL= load noncon le bottom cho h BCDL = 10 dead load (0 g plate capab onal Residen the orientation	.0psf; BCDL Interior(1) 4- to 49-6-4 zo -1.60 neurrent with ord in all area .0psf. 0.0 psf) appli le of withsta tial Code se	=6.0psf; h=30ft; C 1-3 to 19-8-0, Ext one; porch right ex- any other live loa as where a rectan ed only to room. 4 nding 221 lb uplift ctions R502.11.1 along the top an	eat. II; Exp E erior(2) 19- posed;C-C ds. gle 3-6-0 ta 6-18 at joint 2, 3 and R802.1 d/or bottom	3; Enclosed; 8-0 to 26-8- for member II by 2-0-0 w 43 lb uplift a 0.2 and chord.	6, s and vide at joint	Februar	CARO SSIO SEAL 44925 GINEER M. SEV y 11,2020	
Design v a truss s building	valid for use only ystem. Before u design. Bracing	y with MiTek® connectors. T use, the building designer m g indicated is to prevent buc	This design is based onl ust verify the applicabili kling of individual truss	y upon parametery ty of design para web and/or chord	ers shown, and i meters and pro d members only	is for an individual buil perly incorporate this of Additional temporary	ding componer lesign into the and permane	it, not overall nt bracing			A MiTek Affiliate	

bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI fucuality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	140.1582.C 12x12 CVP
440,4500,0	77	BIOOVEAOK BAGE			140215328
140_1582_C	17	PIGGTBACK BASE	2	1	Job Reference (optional)
		ID:RUS	Sz4LGuFS	2C1bODN	.330 s Jan 22 2020 MiTek Industries, Inc. Tue Feb 11 05:52:26 2020 Page 2 ZWBaZyX6cZ-7uasoAYShRMm3r?0rEIU5gC?oqltc61kl90kZxzmli3

NOTES-

12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

LOAD CASE(S) Standard

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<u> </u>	10-1-4	19-8	-0 24-5-4	29-6-0 30-4	0 39-	10-12	50-0-0	
	10-1-4	<u>9-6-1 9-6-1 </u>	2 4-9-4	5-0-12 0-10	0 9-	6-12 '	10-1-4	· · · · · · · · · · · · · · · · · · ·
Plate Offsets (X,Y)	[6:0-3-0,0-2-0], [9:0-9-8,0	0-2-4], [16:0-3-0	,0-3-12], [19:0-3-0,0-2-0]					
	0.0.0.0.0							
LOADING (pst)	SPACING-	2-0-0		DEFL.	IN (IOC) I	/defi L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	IC 0.94	Vert(LL) -0.	60 19-21 >	>589 240	MI20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.97	Vert(CT) -1	09 19-21 >	>321 180	MI18HS	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.87	Horz(CT) 0	14 13	n/a n/a		
BCDL 10.0	Code IRC2015/T	PI2014	Matrix-S	Attic -0.	57 17-19	414 360	Weight: 348 lb	FT = 20%
BCDL 10.0 LUMBER- TOP CHORD 2x4 S 11-13 BOT CHORD 2x6 S 17-19 WEBS 2x4 S 6-19,2 REACTIONS. (lb/siz Max Max Max	P No.2 *Except* : 2x4 SP No.1 P No.2 *Except* : 2x4 SP No.2, 15-18: 2x6 P No.3 *Except* 23-24,8-17: 2x4 SP No.2 ze) 2=1904/0-3-8 (min. Horz 2=180(LC 12) Uplift 2=-258(LC 12), 17=- Grav 2=2069(LC 26), 17=	SP DSS 0-2-7), 17=477/ 213(LC 13), 13= 782(LC 25), 13=	Matrix-S 0-3-8 (min. 0-1-8), 13=17 =-103(LC 12) =1888(LC 2)	Attic -0. BRACING- TOP CHORD BOT CHORD WEBS JOINTS 755/0-3-8 (min. 0-2-4)	Structural 2-0-0 oc p Rigid ceili 2-2-0 oc h 1 Row at 1 Brace a	414 360 I wood sheathing di burlins (3-0-3 max.) ing directly applied oracing: 19-21. midpt 5 tt Jt(s): 22, 23, 24	veight: 348 ib rectly applied, except : 6-9. or 10-0-0 oc bracing, 5-19, 10-16	Except:
FORCES. (lb) - Max TOP CHORD 2-22 5-6- 8-9- 12-3 BOT CHORD 2-2 BOT CHORD 2-2 18- WEBS 3-2 7-22 12-	 Comp./Max. Ten All fo 5=-4066/477, 3-25=-3977/- 2974/454, 6-27=-2868/5 2-2527/446, 9-10=-2828/44 30=-3598/505, 13-30=-371 1=-517/3566, 21-31=-337/- 19=-148/2556, 17-18=-148 34=-226/2859, 14-34=-226 1=-362/222, 5-21=-54/648, 2=-302/142, 8-22=-135/522 14=-392/235, 6-22=-133/3 	rces 250 (lb) or 494, 3-4=-3826/ 39, 7-27=-2868/ 61, 10-29=-3353 8/486 3089, 20-31=-33 5/2556, 16-17=-1 6/2859, 13-14=- 5-19=-799/272 9, 9-16=-122/10 73, 17-24=-399/	less except when shown. 452, 4-26=-3697/480, 5-2 539, 7-28=-2868/539, 8-2 8/479, 11-29=-3395/475, 1 87/3089, 20-32=-337/3088 45/2526, 16-33=-226/285 368/3276 , 19-23=-69/922, 6-23=-54 29, 10-16=-694/275, 10-1 136, 8-24=-376/147	6=-3696/484, 8=-2868/539, 11-12=-3484/447, 9, 19-32=-337/3089, 59, 15-33=-226/2859, 8/943, 4=-65/540,				
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10; MWFRS (envelope Interior(1) 26-8-14 for reactions shown 3) Provide adequate (4) All plates are MT2(5) This truss has been will fit between the 7) Ceiling dead load (8) Bottom chord live I 9) Provide mechanica 17 and 103 lb uplif 10) This truss is design referenced stand Contraction pagelize I Mathematical Standard	ve loads have been consid Vult=130mph (3-second g e) gable end zone and C-C to 30-4-0, Exterior(2) 30-4 h; Lumber DOL=1.60 plate drainage to prevent water j p plates unless otherwise i in designed for a 10.0 psf b en designed for a live load bottom chord and any oth 5.0 psf) on member(s). 22 oad (40.0 psf) and additior al connection (by others) of a t joint 13. gned in accordance with th ard ANSI/TPI 1. epresentation does not de	lered for this des ust) Vasd=103n Exterior(2) -0-1 -0 to 37-4-14, In grip DOL=1.60 ponding. ndicated. bottom chord live of 20.0psf on the er members, with -23, 22-24 al bottom chord f truss to bearing truss to bearing e 2015 Internation pict the size or the works on this and	sign. nph; TCDL=6.0psf; BCDL: 0-8 to 4-1-8, Interior(1) 4- terior(1) 37-4-14 to 49-10 e load nonconcurrent with ne bottom chord in all area h BCDL = 10.0psf. I dead load (0.0 psf) appli g plate capable of withstar onal Residential Code se the orientation of the purifir	=6.0psf; h=30ft; Cat. I 1-8 to 19-8-0, Exterio I-4 zone;C-C for mem any other live loads. as where a rectangle ed only to room. 17-1 nding 258 lb uplift at ju ctions R502.11.1 and a along the top and/or 25 PAGE MI/273 rev. 10/	; Exp B; Encl (2) 19-8-0 to bers and force 3-6-0 tall by 2- 9 int 2, 213 lb to R802.10.2 ar bottom chord	osed; 26-8-14, ss & MWFRS -0-0 wide uplift at joint id	Februar	CAROL SEAL 44925 GINEER, HERLIN M. SEVIERIUM y 11,2020
WARNING - Verify Design valid for use of a truss system. Befor building design. Brack is always required for	r design parameters and READ M inly with MiTek® connectors. This e use, the building designer must sing indicated is to prevent bucklin stability and to prevent colleges	s design is based only verify the applicabiliting of individual truss	y upon parameters shown, and i ty of design parameters and pro web and/or chord members only a linuw, and property damage.	CE PAGE MII-7473 rev. 10/0 is for an individual building of perly incorporate this design Additional temporary and For general guidance record	omponent, not into the overall permanent bracir	ng		

binding design. Biading indicates to be prevent buckning of individual duss web and/or ford internoes only. Additional emporary and permanent blading is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	140.1582.C 12x12 CVP
					140215329
140_1582_C	Т8	PIGGYBACK BASE	1	1	
					Job Reference (optional)
					200 - Jan 00 0000 Mitali la duatria da a Tua Eab 44 05:50:44 0000 Dava 0

ID:RUSz4LGuFS2C1bODNZWBaZyX6cZ-IZosEZ5h4G1Nzmxg_Bc1Q9ZhOz3GfUZM6fQceqzmlhM

NOTES-12) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





<u> </u>	10-1-4	<u>19-8-0</u> <u>24-5-4</u> 9-6-12 <u>4-9-4</u>	29-6-0 30-4-0	39-10-12		50-0-0	
Plate Offsets (X,Y)	[6:0-3-0,0-2-0], [9:0-9-8,0-2-4], [1	7:0-3-0,0-3-12], [20:0-2-12,0-2-0)]	3-0-12		10-1-4	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.94 BC 0.97 WB 0.88 Matrix-S	DEFL. ii Vert(LL) -0.6(Vert(CT) -1.1* Horz(CT) 0.14 Attic -0.58	n (loc) l/defl 0 20-22 >580 1 20-22 >316 4 13 n/a 3 18-20 406	L/d 240 180 n/a 360	PLATES MT20 MT18HS Weight: 349 lb	GRIP 244/190 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF 18-20: WEBS 2x4 SF 6-20,8 REACTIONS. (Ib/siz Max H Max U Max C	P No.2 P No.2 *Except* 2x4 SP No.2, 16-19: 2x6 SP DSS P No.3 *Except* -18,24-25: 2x4 SP No.2 e) 2=1900/0-3-8 (min. 0-2-7), 1 forz 2=172(LC 12) Jplift 2=-258(LC 12), 18=-211(LC Grav 2=2065(LC 26), 18=787(LC 2	8=485/0-3-8 (min. 0-1-8), 13=1; 13), 13=-107(LC 12) 5), 13=1934(LC 2)	BRACING- TOP CHORD BOT CHORD WEBS JOINTS 813/0-3-8 (min. 0-2-5)	Structural wood 2-0-0 oc purlins Rigid ceiling dire 2-2-0 oc bracing 1 Row at midpt 1 Brace at Jt(s):	sheathing dire (3-0-4 max.): 6 ectly applied or : 20-22. 5-2 23, 24, 25	ctly applied, except 3-9. 10-0-0 oc bracing, 20, 10-17	Except:
FORCES. (lb) - Max. TOP CHORD 2-26 5-6= 8-9= 12-3 BOT CHORD 2-22 19-2 16-3 WEBS 3-22 6-23 12-1	Comp./Max. Ten All forces 250 =-4057/479, 3-26=-3968/496, 3-4 -2963/456, 6-28=-2858/541, 7-28 -2516/448, 9-10=-2816/464, 10-30 1=-3621/484, 13-31=-3711/466 =-509/3558, 22-32=-329/3080, 21 0=-140/2545, 18-19=-140/2545, 1 5=-197/2846, 15-35=-197/2846, 1 =-362/222, 5-22=-55/649, 5-20=-8 =-134/371, 8-23=-135/531, 9-17=- 5=-380/225, 18-25=-402/136, 8-25	(lb) or less except when shown -3817/454, 4-27=-3688/482, 5-2 -2858/541, 7-29=-2858/541, 8-2)=-3334/466, 11-30=-3389/462, -32=-329/3080, 21-33=-329/308 3-15=-314/3253 01/272, 20-24=-69/919, 6-24=-5 124/1025, 10-17=-693/277, 10- i=-379/147, 7-23=-302/142	27=-3687/485, 29=-2858/541, 11-12=-3464/434, 0, 20-33=-329/3080, 46, 16-34=-197/2846, 18/940, 15=-62/536,				
NOTES- 1) Unbalanced roof liv. 2) Wind: ASCE 7-10; V MWFRS (envelope) Interior(1) 26-8-14 t for reactions shown 3) Provide adequate d 4) All plates are MT20 5) This truss has been 6) * This truss has been will fit between the t 7) Ceiling dead load (5 8) Bottom chord live lo 9) Provide mechanical 18 and 107 lb uplift 10) This truss is desig referenced standa 11) Graphical purlin re	e loads have been considered for Vult=130mph (3-second gust) Vasi) gable end zone and C-C Exterior o 30-4-0, Exterior(2) 30-4-0 to 37- ; Lumber DOL=1.60 plate grip DO Irainage to prevent water ponding. plates unless otherwise indicated d designed for a 10.0 psf bottom ch en designed for a 10.0 psf bottom ch en designed for a live load of 20.0 p bottom chord and any other memb 5.0 psf) on member(s). 23-24, 23-2 ad (40.0 psf) and additional bottom chord connection (by others) of truss to at joint 13. ned in accordance with the 2015 I rd ANSI/TPI 1. spresentation does not depict the s GWN IS DESIGNED AS UNINHA	this design. 1=103mph; TCDL=6.0psf; BCDI (2) -0-10-8 to 4-1-8, Interior(1) 4 4-14, Interior(1) 37-4-14 to 50-10 L=1.60 ord live load nonconcurrent with sf on the bottom chord in all are ers, with BCDL = 10.0psf. 5 n chord dead load (0.0 psf) appl bearing plate capable of withstant hternational Residential Code set ize or the orientation of the purli BITABLE.	.=6.0psf; h=30ft; Cat. II; -1-8 to 19-8-0, Exterior(2)-8 zone;C-C for member n any other live loads. as where a rectangle 3-1 ied only to room. 18-20 inding 258 lb uplift at joir actions R502.11.1 and R n along the top and/or b	Exp B; Enclosed; 2) 19-8-0 to 26-8-1 rs and forces & M 6-0 tall by 2-0-0 wi nt 2, 211 lb uplift at 802.10.2 and ottom chord.	4, WFRS de joint	Februar	CAROLINE SSI VILLE SEAL 44925 GINEEPHERINE M. SEVILLE VILL2020
WarNING - Verify Design valid for use or a truss system. Before building design. Braci is always required for i fabrication, storage, de Safety Information a	design parameters and READ NOTES ON hy with MITek® connectors. This design is I use, the building designer must verify the a in indicated is to prevent buckling of indivic stability and to prevent collapse with possib elivery, erection and bracing of trusses and available from Truss Plate Institute, 218 N. L	THIS AND INCLUDED MITEK REFEREN ased only upon parameters shown, and piplicability of design parameters and pro ual truss web and/or chord members on e personal injury and property damage. Truss systems, see ANSITP1 ee Street, Suite 312, Alexandria, VA 223	CE PAGE MII-7473 rev. 10/03/ is for an individual building cor operly incorporate this design ir y. Additional temporary and pe For general guidance regardin Quality Criteria, DSB-89 and I 14.	2015 BEFORE USE. mponent, not to the overall armanent bracing g the BCSI Building Compo	nent	818 Soundside I Edenton, NC 27	RING BY ENCO A MITEK Atfillate Road 932

Job	Truss	Truss Type	Qty	Ply	140.1582.C 12x12 CVP
			_		140215330
140_1582_C	19	PIGGYBACK BASE	2	1	Job Reference (optional)
				1	

8.330 s Jan 22 2020 MTEk Industries, Inc. Tue Feb 11 05:55:43 2020 Page 2 ID:RUSz4LGuFS2C1bODNZWBaZyX6cZ-so_K8Xw1HzApC7Ztu6KgaDsg2FxkGE_k3JEfz1zmlf_

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI Quality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) 0-3-14 to 3-3-14, Interior(1) 3-3-14 to 5-5-0, Exterior(2) 5-5-0 to 8-5-0,

Interior(1) 8-5-0 to 10-6-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing

LUMBER-

TOP CHORD 2x4 SP No.3 2x4 SP No.3 BOT CHORD

2x4 SP No.3 OTHERS

REACTIONS. 1=154/8-1-0, 3=154/8-1-0, 4=267/8-1-0 (lb/size) Max Horz 1=59(LC 9) Max Uplift 1=-34(LC 12), 3=-42(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-0-14, Exterior(2) 4-0-14 to 7-0-14, Interior(1) 7-0-14 to 7-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Plate Offsets (X,Y)	0-0 <u>-6</u> 0-0-6 [2:0-3-0,Edge]		5-1-12 5-1-6	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.13 BC 0.37 WB 0.00 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) n/a - n/a 999 Vert(CT) n/a - n/a 999 Horz(CT) 0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 15 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (lb/size) 1=167/5-1-0, 3=167/5-1-0 Max Horz 1=34(LC 9) Max Uplift 1=-18(LC 12), 3=-18(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



Structural wood sheathing directly applied or 5-1-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



⁴⁾ This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.



Plate Offsets (X,Y)	[3:0-3-0,Edge]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 PCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.25 BC 0.18 WB 0.05 Matrix S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l/defl L/d n - n/a 999 n - n/a 999 5 n/a n/a	PLATES GRIP MT20 244/190	
LUMBER-		Matrix 0	BRACING-	Structural wood shoothing di		
BOT CHORD 2x4 SP WEBS 2x4 SP	No.3 2 No.3		BOT CHORD	except end verticals. Rigid ceiling directly applied	or 10-0-0 oc bracing.	

WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3

(lb/size) 1=83/7-1-14, 5=137/7-1-14, 6=302/7-1-14 REACTIONS. Max Horz 1=134(LC 12) Max Uplift 5=-25(LC 12), 6=-110(LC 12) Max Grav 1=89(LC 21), 5=137(LC 1), 6=320(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 6-1-14, Exterior(2) 6-1-14 to 7-0-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REPERENCE PAGE MIL-14's rev. Invozens Derrore USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Plate Off	sets (X,Y)	[3:0-3-0,Edge]		
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.22	Vert(LL) n/a - n/a 999 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.18	Vert(CT) n/a - n/a 999
BCLL	0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT) 0.00 5 n/a n/a
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Weight: 22 lb FT = 20%
LUMBER	}-			BRACING-

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.3	TOP CHORD	Structural wood sheathing directly applied or 5-7-14 oc purlins,
BOT CHORD	2x4 SP No.3		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=-5/5-7-14, 5=138/5-7-14, 6=269/5-7-14 Max Horz 1=95(LC 12) Max Uplift 1=-36(LC 10), 5=-26(LC 12), 6=-91(LC 12)

Max Grav 1=58(LC 12), 5=138(LC 1), 6=284(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 4-7-14, Exterior(2) 4-7-14 to 5-6-2 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.18 BC 0.19 WB 0.00 Matrix-R	DEFL. ii Vert(LL) n/: Vert(CT) n/: Horz(CT) 0.00	n (loc) l/de a - n/ a - n/) 4 n/	fl L/d a 999 a 999 a n/a	PLATES MT20 Weight: 14 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 4-1-14 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.					

REACTIONS. (lb/size) 1=141/4-1-8, 4=141/4-1-8 Max Horz 1=56(LC 12)

Max Uplift 1=-12(LC 12), 4=-25(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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Plate Offsets (X,Y)	[2:0-3-0,Edge]						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.20 BC 0.14 WB 0.06	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	i (loc) l/defl i - n/a i - n/a 4 n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S				Weight: 33 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP	BRACING- TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.					
WEBS 2x4 SP	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.					

REACTIONS. (lb/size) 6=147/8-4-7, 4=133/8-4-7, 5=342/8-4-7 Max Horz 6=-112(LC 13) Max Uplift 6=-18(LC 12), 5=-104(LC 13) Max Grav 6=147(LC 1), 4=133(LC 1), 5=361(LC 20)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 WEBS
 3-5=-265/153

NOTES-

OTHERS

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 5-4-0, Interior(1) 5-4-0 to 7-11-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

2x4 SP No.3

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be User Defined crushing capacity of 425 psi.



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¹⁾ Unbalanced roof live loads have been considered for this design.



Plate Offsets (X Y)-- [4:0-3-0 Edge]

LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	9	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	912014	Matrix	-S						Weight: 75 lb	FT = 20%
LUMBER	-					BRACING-						
TOP CHORD 2x4 SP No.2				TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,								
BOT CHORD 2x4 SP No.2							except	end vert	icals.		•	

WEBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 13-11-14.

2x4 SP No.3

Max Horz 16=-131(LC 13) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10 Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

OTHERS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 5-5-0, Exterior(2) 5-5-0 to 8-5-0,
- Interior(1) 8-5-0 to 13-6-2 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) All plates are 2x4 MT20 unless otherwise indicated.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) All bearings are assumed to be User Defined crushing capacity of 425 psi.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



