

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 21806A 240.3174.B Ext Scr Porch

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by 84 Components - #2383.

Pages or sheets covered by this seal: I38155058 thru I38155079

My license renewal date for the state of North Carolina is December 31, 2019.

North Carolina COA: C-0844



August 14,2019

Sevier, Scott

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



	1					20-8-0						
	I					20-8-0						
LOADING TCLL	(psf) 20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC	0.05	DEFL. Vert(LL)	in -0.00	(loc) 12	l/defl n/r	L/d 120	PLATES MT20	GRIP 244/190
TCDL BCLL BCDL	10.0 0.0 * 10.0	Lumber DOL Rep Stress Incr Code IRC2015/TF	1.15 YES PI2014	BC WB Matri	0.04 0.04 x-S	Vert(CT) Horz(CT)	0.00 0.00	12 12	n/r n/a	120 n/a	Weight: 91 lb	FT = 20%
I UMBER-	1			1		BRACING-					_	

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3

REACTIONS. All bearings 20-8-0.

(lb) -Max Horz 2=-62(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, 12

Max Grav All reactions 250 lb or less at joint(s) 2, 18, 19, 20, 21, 22, 17, 16, 15, 14, 12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-4-0, Exterior(2) 2-4-0 to 10-4-0, Corner(3) 10-4-0 to 13-4-0, Exterior(2) 13-4-0 to 21-6-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, and 12. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS AND INCLODED MITCR REPERVICE PAGE MIT-14/3 refer to 1000 SEC. Design valid for use only with MITER deconnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	240.3174.B Ext Scr Porch	
						138155059
21806A	AG	COMMON GIRDER	1	2		
				_	Job Reference (optional)	
84 Components (Dunn),	Dunn, NC - 28334,		8.3	10 s Jun 1	1 2019 MiTek Industries, Inc.	Wed Aug 14 08:00:42 2019 Page 2
		ID:B C	Q7f7Biu7XI	herXjarx6	dzmHHa-p0mmhsVkGqhOsDL2	ZCIdubTSU1wjB7Z5sNyC6h8yo? 3

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb)

Vert: 10=-1626(B) 12=-1626(B) 13=-1626(B) 14=-1626(B) 15=-1626(B) 16=-1626(B) 17=-1626(B) 18=-1626(B) 19=-1626(B) 20=-1632(B)

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			12-8-0 12-8-0					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (lo	c) l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.05	Vert(LL) -0.	00	8 n/r	120	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) 0.	00	8 n/r	120		
CLL 0.0 *	Rep Stress Incr YES	WB 0.04	Horz(CT) 0.	00	8 n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	· · · ·				Weight: 51 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS 2x4 SP No.3 TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 12-8-0. (lb) -

Max Horz 2=40(LC 12) Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 14, 11, 10 Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-4-0, Exterior(2) 2-4-0 to 6-4-0, Corner(3) 6-4-0 to 9-4-0, Exterior(2) 9-4-0 to 13-6-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 8, 13, 14, 11, and 10. This connection is for uplift only and does not consider lateral forces.



🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON TIPS ON TIPS REPREVED PAGE MIT-14/3 reference of the second secon fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (Ib)

Vert: 9=-1630(B) 10=-1626(B) 11=-1626(B) 12=-1626(B) 13=-1626(B) 14=-1626(B)

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818 Soundside Road

Edenton, NC 27932

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August 14,2019

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LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.25	Vert(LL) -0.01 2-4 >999 240	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.19	Vert(CT) -0.03 2-4 >999 180	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 n/a n/a	·····
BCDL	10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 16 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-4-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=149/Mechanical, 2=234/0-5-8 Max Horz 2=60(LC 8) Max Uplift 4=-33(LC 12), 2=-71(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;
- MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
- 6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



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OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL) 0.00	<u></u> 1	n/r	120	MT20	244/190
CDL 10.0	Lumber DOL 1.15	BC 0.04	Vert(CT) 0.00	1	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.06	Horz(CT) 0.00		n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P					Weight: 17 lb	FT = 20%

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD WEBS 2x4 SP No.3 TOP CHORD

Structural wood sheathing directly applied or 4-4-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=61/4-4-0, 2=133/4-4-0, 6=193/4-4-0 Max Horz 2=60(LC 8) Max Uplift 5=-15(LC 8), 2=-45(LC 8), 6=-41(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-2-4, Exterior(2) 2-2-4 to 4-2-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.

4) Gable studs spaced at 2-0-0 oc.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 6. This connection is for uplift only and does not consider lateral forces.

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BOT CHORD

8

3x4 =

Rigid ceiling directly applied or 5-0-5 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

 $4x4 \equiv$

REACTIONS. (Ib/size) 2=890/0-3-8, 6=890/0-3-8 Max Horz 2=63(LC 12) Max Uplift 2=-348(LC 8), 6=-348(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1925/1266, 3-4=-1690/1233, 4-5=-1690/1233, 5-6=-1925/1266

BOT CHORD 2-9=-1148/1770, 8-9=-758/1203, 6-8=-1155/1770

WEBS 4-8=-459/537, 5-8=-337/180, 4-9=-459/537, 3-9=-337/180

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 10-6-0, Exterior(2) 10-6-0 to 13-6-0, Interior(1) 13-6-0 to 21-10-8 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

9

6x6 =

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.

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4x4 =

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				21-0-0						
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2 Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPI20	2-0-0 1.15 1.15 YES 014	CSI. TC 0.23 BC 0.15 WB 0.05 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.01 0.02 0.00	(loc) 11 11 10	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 90 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S OTHERS 2x4 S	5P No.2 5P No.2 5P No.3			BRACING- TOP CHOR BOT CHOR	D S D I	Structur Rigid ce	al wood seiling dire	sheathing dire	ectly applied or 6-0-0 r 10-0-0 oc bracing.) oc purlins.
REACTIONS. All (lb) - Max Max Max	bearings 20-11-12. Horz 2=63(LC 16) Uplift All uplift 100 lb or less a Grav All reactions 250 lb or l 24)	at joint(s) 2, 16, 1 less at joint(s) 2, 1	7, 18, 14, 13, 12, 10 5, 16, 17, 14, 13, 1	0 0 except 18=361(L	.C 23), 1	12=361	(LC			
FORCES. (lb) - Max WEBS 3-1	<. Comp./Max. Ten All forces 8=-257/164, 9-12=-257/164	s 250 (lb) or less e	except when shown							
NOTES- 1) Unbalanced roof li 2) Wind: ASCE 7-10; MWFRS (envelope Exterior(2) 13-6-0 Lumber DOL=1.60	ve loads have been considered Vult=130mph (3-second gust) a) gable end zone and C-C Con to 21-10-8 zone; porch left and plate grip DOL=1.60	d for this design.) Vasd=103mph; T rner(3) -0-10-8 to d right exposed;C-	CDL=6.0psf; BCDI 2-1-8, Exterior(2) 2 C for members and	L=6.0psf; h=30ft; C -1-8 to 10-6-0, Co d forces & MWFRS	at. II; Ex ner(3) 1 for read	xp B; Er 10-6-0 to ctions sl	nclosed; o 13-6-0, hown;			

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) All plates are 2x4 MT20 unless otherwise indicated.5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 16, 17, 18, 14, 13, 12, and 10. This connection is for uplift only and does not consider lateral forces.

9) Non Standard bearing condition. Review required.

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0-4-8	7-5-12		13-3-4			20-0-0	
Plate Offsets (X,Y)	[5:0-3-6,Edge]		5-9-8			0-0-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.47 BC 0.44 WB 0.28 Matrix-S	DEFL.inVert(LL)0.16Vert(CT)-0.16Horz(CT)0.01	(loc) // 1-9 > 1-9 > 5	/defl L/d /547 240 /537 180 n/a n/a	PLATES MT20 Weight: 81 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.2 P No.2 P No.3		BRACING- TOP CHORD BOT CHORD	Structural Rigid ceili	wood sheathing dire	ectly applied or 6-0-0 r 6-0-0 oc bracing.	oc purlins.
REACTIONS. (Ib/siz Max H Max L Max C	e) 9=1038/0-3-8, 5=465/0-3-8, 1=135/ lorz 1=-57(LC 13) Jplift 9=-202(LC 8), 5=-112(LC 9), 1=-92 Grav 9=1038(LC 1), 5=473(LC 24), 1=18	0-3-8 (LC 8) 5(LC 23)					
FORCES. (lb) - Max. TOP CHORD 1-2= BOT CHORD 1-9= WEBS 2-9=	Comp./Max. Ten All forces 250 (lb) or -43/285, 2-3=-64/551, 3-4=-565/123, 4-5 -253/83, 5-7=-124/729 -439/216, 3-9=-826/142, 3-7=-85/589, 4-	less except when shown. =-801/178 7=-353/171					
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-10; \ MWFRS (envelope) 12-11-15, Interior(1) Lumber DOL=1.60 3) This truss has been 1 * This truss has been	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=103 gable end zone and C-C Exterior(2) 0-1) 12-11-15 to 20-10-8 zone; porch left ex plate grip DOL=1.60 designed for a 10.0 psf bottom chord liv n designed for a load of 20 Opef on 1	sign. mph; TCDL=6.0psf; BCDL= -12 to 3-1-12, Interior(1) 3-1 posed;C-C for members and e load nonconcurrent with a	6.0psf; h=30ft; Cat. II; E 1-12 to 9-11-15, Exterior d forces & MWFRS for r any other live loads.	xp B; Encl r(2) 9-11-1 reactions s	losed; 15 to shown;		

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=202.

6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 1. This connection is for uplift only and does not consider lateral forces.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

				7-3-0					
	(psf)	SPACING- 2-0-0 Plate Grip DOI 1 15	CSI.	DEFL. ir	n (loc)	l/defl	L/d	PLATES	GRIP
TCDL	10.0	Lumber DOL 1.15	BC 0.64	Vert(CT) -0.28	1-3	>297	180	WIT20	244/130
BCLL BCDL	0.0 * 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.00 Matrix-P	Horz(CT) 0.00		n/a	n/a	Weight: 25 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (Ib/size) 1=278/0-3-8, 3=278/0-3-8 Max Horz 1=85(LC 8) Max Uplift 1=-101(LC 8), 3=-128(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed;

MWFRS (envelope) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 7-1-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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			20-0-0					
Plate Offsets (X,Y)	[10:0-3-0,Edge]		20-0-0					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.79 BC 0.91 WB 0.19 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.50 12-13 -0.70 12-13 0.04 10	l/defl >328 >237 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 81 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP 10-17: OTHERS 2x4 SP	2 No.2 2 No.2 *Except* 2x4 SP No.1 2 No.3		BRACING- TOP CHOR BOT CHOR	D Structu D Rigid c	ral wood : eiling dire	sheathing dir ctly applied o	rectly applied or 2-10 or 6-1-2 oc bracing.	-4 oc purlins.
REACTIONS. All be (lb) - Max H Max U Max G	earings 7-5-8 except (jt=length) 10=0-3-8 orz 2=-53(LC 13) plift All uplift 100 lb or less at joint(s) 1 rav All reactions 250 lb or less at joint(7, 18 except 2=-152(LC 8) s) 17 except 2=566(LC 1)), 10=-195(LC 8)), 18=279(LC 1), 1	0=774(LC 1)				
FORCES. (lb) - Max. TOP CHORD 2-3=- 7-8=- 7-8=- BOT CHORD 2-18= 13-1. WEBS	Comp./Max. Ten All forces 250 (lb) or 1339/901, 3-4=-1330/930, 4-5=-1308/93 1290/913, 8-9=-1314/911, 9-10=-1366/9 824/1253, 17-18=-824/1253, 16-17=-8 4=-825/1250, 12-13=-825/1250, 10-12=- -371/450	less except when shown. 3, 5-6=-1307/956, 6-7=-1 11 25/1250, 15-16=-825/1250 825/1250	287/935, 0, 14-15=-825/125	50,				
 NOTES- Unbalanced roof live Wind: ASCE 7-10; V MWFRS (envelope) , Interior(1) 12-11-15 DOL=1.60 plate grip Truss designed for v Gable End Details at All plates are 2x4 M Gable studs spaced This truss has been This truss has been * This truss has been * This truss has been * This truss has been will fit between the b Provide mechanical 2=152. One RT7A USP con for uplift only and do 	e loads have been considered for this de (ult=130mph (3-second gust) Vasd=103r gable end zone and C-C Exterior(2) -0- 5 to 20-10-8 zone; porch left exposed;C- 0 DDL=1.60 vind loads in the plane of the truss only. s applicable, or consult qualified building T20 unless otherwise indicated. at 2-0-0 oc. designed for a 10.0 psf bottom chord liv. n designed for a live load of 20.0psf on t bottom chord and any other members. connection (by others) of truss to bearin nectors recommended to connect truss to less not consider lateral forces.	sign. nph; TCDL=6.0psf; BCDL 0-8 to 2-1-8, Interior(1) 2 C for members and forces For studs exposed to wir designer as per ANSI/TF e load nonconcurrent with he bottom chord in all are g plate capable of withsta o bearing walls due to UF	=6.0psf; h=30ft; C -1-8 to 9-11-15, E & MWFRS for re ad (normal to the f P 1. any other live loa as where a rectan nding 100 lb uplift PLIFT at jt(s) 17, 1	at. II; Exp B; E kterior(2) 9-11- actions shown; ace), see Stand ds. gle 3-6-0 tall b at joint(s) exco 8, and 10. This	inclosed; 15 to 12-1 Lumber dard Indus y 2-0-0 wi ept (jt=lb) s connecti	1-15 etry de on is	A Standard Stand Standard Standard Stand Standard Standard Stand	H CAROL EESSIG SEAL 044925 VGINEERIE

August 14,2019

818 Soundside Road Edenton, NC 27932

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 			12-8-0 12-8-0						
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15	CSI. TC 0.21 BC 0.15	DEFL. Vert(LL) Vert(CT)	in 0.01 0.01	(loc) 7 7	l/defl n/r n/r	L/d 120 120	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.06 Matrix-S	Horz(CT)	0.00	6	n/a	n/a	Weight: 47 lb	FT = 20%
I UMBER-			BRACING-						

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS. All bearings 12-8-0.

(lb) - Max Horz 2=-35(LC 13) Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 9, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 10=357(LC 1), 8=357(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. 3-10=-252/216, 5-8=-252/216 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 2-1-8, Exterior(2) 2-1-8 to 6-3-15, Corner(3) 6-3-15 to 9-3-15, Exterior(2) 9-3-15 to 13-6-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9, 10, and 8. This connection is for uplift only and does not consider lateral forces.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

	<u>10-4-11</u> 10-4-11		20-10-0 10-5-5		3	81-3-5 0-5-5		41-4-8 10-1-3	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TI	2-0-0 1.15 1.15 YES Pl2014	CSI. TC 0.84 BC 0.90 WB 0.52 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (-0.23 13 -0.46 13 0.12	(loc) l/de 3-15 >999 3-15 >999 10 n/	fl L/d 9 240 9 180 a n/a	PLATES MT20 Weight: 236 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S 8-10: BOT CHORD 2x6 S WEBS 2x4 S REACTIONS. (lb/si Max Max	P No.2 *Except* 2x4 SP No.1 P No.2 P No.3 ze) 2=1709/0-3-8, 10=16 Horz 2=154(LC 12) Uplift 2=-222(LC 12), 10=-	346/Mechanical 199(LC 13)		BRACING- TOP CHOR BOT CHOR WEBS	D S D R 1	Structural wo igid ceiling Row at mic	od sheathing d directly applied lpt	irectly applied or 2-2-0 c or 10-0-0 oc bracing. 5-13, 7-13	oc purlins.
FORCES. (lb) Har TOP CHORD 2-3 9-1 BOT CHORD 2-1 9-1 WEBS 3-13 7-1	 c. Comp./Max. Ten All foi =-3457/438, 3-5=-3146/374)=-3400/433 5=-475/3085, 13-15=-292/2 5=-351/226, 5-15=-24/591, 1=-22/561, 9-11=-321/227 	rces 250 (lb) or 4, 5-6=-2200/34 2560, 11-13=-19 5-13=-824/274	less except when shown 6, 6-7=-2200/350, 7-9=-3 01/2541, 10-11=-324/302 , 6-13=-101/1261, 7-13=	n. 3107/380, 21 -802/271,					
NOTES- 1) Unbalanced roof lin 2) Wind: ASCE 7-10; MWFRS (envelope	ve loads have been consid Vult=130mph (3-second g e) gable end zone and C-C	ered for this des ust) Vasd=103n Exterior(2) -0-1	sign. nph; TCDL=6.0psf; BCD 0-8 to 3-3-2, Interior(1) 3	L=6.0psf; h=30ft; C 3-3-2 to 20-10-0, E	cat. II; Exp xterior(2)	p B; Enclose 20-10-0 to	ed; 24-11-10		

MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-3-2, Interior(1) 3-3-2 to 20-10-0, Exterior(2) 20-10-0 to 24-1 , Interior(1) 24-11-10 to 41-3-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=199.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

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14

5x9 =

23

13 24

5x9 =

12

4x4 =

F	2-3-8	8-6-12	14-10-0		20-10-0	23-7-0		31-3	3-5		41-4-8	
Plate Offs	ets (X,Y)	[2:0-3-6,0-1-3], [2:0-11-	6,0-0-9], [16:0-2·	-12,0-4-0]	0-0-0	2-5-0		7-0	-5		10-1-0	
LOADING TCLL TCDL BCLL BCDL	(psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/7	2-0-0 1.15 1.15 YES IPI2014	CSI. TC BC WB Matrix	0.80 0.97 0.59 <-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.27 -0.55 0.19	(loc) 6 6 11	l/defl >999 >906 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 265 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHO BOT CHO WEBS	RD 2x4 SF 9-11: 2 RD 2x6 SF 2-18: 2 2x4 SF	⁻ No.2 *Except* 2x4 SP No.1 P DSS *Except* 2x8 SP No.2, 6-15: 2x4 S P No.3	SP No.3, 13-15: 2	2x6 SP No.2		BRACING- TOP CHOR BOT CHOR WEBS	D D	Structu Rigid ce 1 Row a	ral wood eiling dire at midpt	sheathing d ectly applied	lirectly applied or 2-2-0 o l or 2-2-0 oc bracing. 4-16, 8-14	c purlins.
REACTIO	NS. (Ib/size Max H Max U	e) 2=1709/0-3-8, 11=1 forz 2=155(LC 16) Jplift 2=-223(LC 12), 11=	1646/Mechanical =-199(LC 13)									
FORCES. TOP CHO	(lb) - Max. RD 2-3≕ 8-10	. Comp./Max. Ten All fe -4267/601, 3-4=-4049/48 =-3108/377, 10-11=-340	orces 250 (lb) or 33, 4-6=-2994/38 0/431	less except 82, 6-7=-2979	when shown. 9/477, 7-8=-2	194/353,						
BOT CHO WEBS	RD 2-17 11-12 4-17 8-14	=-654/3861, 16-17=-482 2=-321/3020 =0/590, 4-16=-1124/231 =-807/267, 8-12=-16/576	/3708, 6-16=-39 , 14-16=-28/139 5, 10-12=-319/22	5/222, 14-15 3, 7-16=-323 28	=-58/558, 12· /1276, 7-14=·	-14=-191/2540, -77/604,						

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-3-2, Interior(1) 3-3-2 to 20-10-0, Exterior(2) 20-10-0 to 24-11-10 , Interior(1) 24-11-10 to 41-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

16

15

3x4 ||

8x8

17

4x4 =

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

2x4 =

3

18

8x8 =

4x8

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=199.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

22

11

4x6 =

-15 -15 -15

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1		8-5-2	15-4-0	1	26-4-0	1	33-2-1	14	41-8-0	1
		8-5-2	6-10-14	1	11-0-0	1	6-10-1	14	8-5-2	1
LOADING TCLL TCDL BCLL BCDL	i (psf) 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DC Lumber DOL Rep Stress In Code IRC20 ⁻	2-0-0 DL 1.15 1.15 https://www.action.org/ 15/TPI2014	CSI. TC 0.95 BC 0.77 WB 0.47 Matrix-S	DEFL. Vert(LL) -0 Vert(CT) -0 Horz(CT) 0	in (loc) .21 14-17 .43 14-17 .10 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 268 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHC BOT CHC	- RD 2x4 SI 1-4,8- RD 2x6 SI	P DSS *Except* 11: 2x4 SP No.2 P No.2			BRACING- TOP CHORD BOT CHORD	Structur Rigid ce	ral wood s eiling direc	heathing dire	ctly applied. 10-0-0 oc bracing.	
WEBS	2x4 SI	⁵ No.3								
REACTIO	NS. (Ib/siz Max H Max U	e) 2=1716/0-3-8, 1 lorz 2=-151(LC 13) Jplift 2=-223(LC 12),	0=1716/0-3-8 10=-223(LC 13)							
FORCES. TOP CHC	(lb) - Max. RD 2-3= 9-10	Comp./Max. Ten A -3485/417, 3-5=-3309 =-3485/417	All forces 250 (lb) or 9/439, 5-6=-2715/40	less except when s 8, 6-7=-2715/408,	shown. 7-9=-3309/440,					
BOT CHC WEBS	RD 2-19 6-14 5-17	=-449/3105, 17-19=-3 =-175/941, 7-14=-67 =-671/280, 5-19=-99/	310/2670, 14-17=-11 1/280, 7-12=-100/53 /533, 3-19=-264/178	0/1931, 12-14=-19 3, 9-12=-264/178,	99/2670, 10-12=-299/3105 6-17=-175/941,					
NOTES- 1) Unbala 2) Wind: / MWFR Interior DOL=1 2) All plat	nced roof live ASCE 7-10; N S (envelope) (1) 25-0-0 to .60	e loads have been co /ult=130mph (3-seco) gable end zone and 42-6-8 zone;C-C for	onsidered for this dee nd gust) Vasd=103n C-C Exterior(2) -0-1 members and forces	sign. nph; TCDL=6.0psf 0-8 to 3-3-8, Interi s & MWFRS for rea	; BCDL=6.0psf; h=30ft; Cat. or(1) 3-3-8 to 20-10-0, Exter actions shown; Lumber DOL	II; Exp B; E rior(2) 20-10 _=1.60 plate	nclosed; I-0 to 25-0 grip	-0,		
5) All plat	53 ale 414 W	1 20 ULIESS ULIEI WISE	- muicaleu.							

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for

uplift only and does not consider lateral forces.

7) ATTIC SPACE SHOWN IS DESIGNED AS UNINHABITABLE.

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	8-5-2 8-5-2	15-4-0 6-10-14		26-4-0 11-0-0		33-2-14 6-10-14	41-4-8 8-1-10	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DO Lumber DOL Rep Stress Ind Code IRC201	2-0-0 L 1.15 1.15 Sr YES 5/TPI2014	CSI. TC 0.94 BC 0.91 WB 0.47 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.20 13-16 -0.41 13-16 0.10 10	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 265 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x6 WEBS 2x4	SP DSS *Except* : 2x4 SP No.2, 8-10: 2x4 SP No.2 SP No.3	SP No.1		BRACING- TOP CHOR BOT CHOR	D Structur D Rigid ce	al wood sheathing d iling directly applied	lirectly applied. or 10-0-0 oc bracing.	
REACTIONS. (Ib, Ma Ma	/size) 2=1709/0-3-8, 10 x Horz 2=154(LC 12) x Uplift 2=-222(LC 12), 1	0=1646/Mechanical 0=-199(LC 13)						
FORCES.(lb) -TOP CHORD29BOT CHORD2WEBS37	ax. Comp./Max. Ten A -3=-3467/416, 3-5=-3291 -10=-3408/409 -18=-452/3088, 16-18=-3 -18=-264/178, 5-18=-99/5 -13=-650/276, 7-11=-95/4	ll forces 250 (lb) or le /439, 5-6=-2696/407 13/2653, 13-16=-11: 533, 5-16=-671/280, 192	ess except when shown , 6-7=-2683/413, 7-9=- 3/1914, 11-13=-201/26 6-16=-175/941, 6-13=-	n. ·3236/433, 31, 10-11=-300/302 ·173/921,	23			
 NOTES- 1) Unbalanced roof 2) Wind: ASCE 7-1 MWFRS (envelo , Interior(1) 24-1 DOL=1.60 3) All plates are 4x. 4) This truss has be 5) * This truss has low will fit between th 6) Refer to girder(s 7) Provide mechan 10=199. 8) One RT7A USP and does not cous 9) ATTIC SPACE S 	live loads have been cor 0; Vult=130mph (3-secor pe) gable end zone and (1-10 to 41-3-12 zone;C-C 4 MT20 unless otherwise been designed for a 10.0 p been designed for a live I ne bottom chord and any) for truss to truss connec ical connection (by others connectors recommende nsider lateral forces. HOWN IS DESIGNED A	nsidered for this desi d gust) Vasd=103m C-C Exterior(2) -0-10 for members and for indicated. sf bottom chord live oad of 20.0psf on the other members, with tions. s) of truss to bearing d to connect truss to S UNINHABITABLE	gn. -8 to 3-3-2, Interior(1) rces & MWFRS for rea load nonconcurrent wi a bottom chord in all ar BCDL = 10.0psf. plate capable of withst bearing walls due to U	DL=6.0psf; h=30ft; C 3-3-2 to 20-10-0, E actions shown; Lum th any other live loa eas where a rectar tanding 100 lb upliff JPLIFT at jt(s) 2. Th	Cat. II; Exp B; Er xterior(2) 20-10 ber DOL=1.60 p ds. gle 3-6-0 tall by at joint(s) exce is connection is	nclosed; -0 to 24-11-10 plate grip ? 2-0-0 wide pt (jt=lb) s for uplift only	A DIMORTH A DIMORTH	SEAL GINEER

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BRACING-

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 10-0-0 oc bracing

LOWDER

- TOP CHORD 2x4 SP No.2 *Except* 8-10: 2x4 SP No.1
- BOT CHORD2x6 SP No.2WEBS2x4 SP No.3
- REACTIONS. (lb/size) 2=1709/0-3-8, 10=1646/Mechanical Max Horz 2=154(LC 12) Max Uplift 2=-222(LC 12), 10=-199(LC 13)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-3482/420, 3-5=-3305/446, 5-6=-2616/396, 6-7=-2605/402, 7-9=-3250/440,
- 9-10=-3424/413
- BOT CHORD
 2-16=-458/3106, 14-16=-289/2574, 13-14=-101/1944, 11-13=-188/2556, 10-11=-305/3041

 WEBS
 3-16=-310/204, 5-16=-121/630, 5-14=-677/280, 6-14=-174/933, 6-13=-171/913, 7-13=-657/277, 7-11=-117/588, 9-11=-285/206

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-3-2, Interior(1) 3-3-2 to 20-10-0, Exterior(2) 20-10-0 to 24-11-10 , Interior(1) 24-11-10 to 41-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=199.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

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			41-4-8			
I			41-4-8			
Plate Offsets (X,Y)	[2:0-0-0,0-1-2], [2:0-1-11,0-6-0]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.16 Matrix-S	DEFL. ir Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) 0.01	n (loc) l/defl L/d) 1 n/r 120) 1 n/r 120 25 n/a n/a	PLATES GRIP MT20 244/190 Weight: 265 lb FT = 2	20%
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI WEBS 2x4 SI OTHERS 2x4 SI	P No.2 P No.2 P No.3 P No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing dir except end verticals. Rigid ceiling directly applied o 1 Row at midot 1	ectly applied or 6-0-0 oc purlins, or 10-0-0 oc bracing. 3-36	

WEDGE Left: 2x4 SP No.3

- REACTIONS. All bearings 41-4-8. (lb) - Max Horz 2=162(LC
 - Max Horz 2=162(LC 12) Max Uplift All uplift 100 lb or less at joint(s) 2, 37, 38, 39, 41, 42, 43, 44, 45,

46, 35, 34, 33, 31, 30, 29, 28, 27, 26

Max Grav All reactions 250 lb or less at joint(s) 25, 2, 36, 37, 38, 39, 41, 42, 43, 44, 45, 46, 35, 34, 33, 31, 30, 29, 28, 27, 26

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 11-12=-99/277, 12-13=-112/312, 13-14=-112/318, 14-15=-99/283

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -0-10-8 to 3-3-2, Exterior(2) 3-3-2 to 20-10-0, Corner(3) 20-10-0 to 24-10-0, Exterior(2) 24-10-0 to 41-2-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 37, 38, 39, 41, 42, 43, 44, 45, 46, 35, 34, 33, 31, 30, 29, 28, 27, and 26. This connection is for uplift only and does not consider lateral forces.

MILLIN August 14,201 WWWWWWWW

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0-0-12						14-9-0						
0-0-12		Γ		1		14-8-4						
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	PI2014	Matri	x-S						Weight: 47 lb	FT = 20%
LUMBER-				•		BRACING-						

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.3 2x4 SP No.2 BOT CHORD 2x4 SP No.3 OTHERS

REACTIONS.

All bearings 14-7-8. (lb) - Max Horz 1=-36(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7, 6, 8

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 6=305(LC 24), 8=305(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-11-5 to 4-0-0, Interior(1) 4-0-0 to 7-4-8, Exterior(2) 7-4-8 to 10-4-8, Interior(1) 10-4-8 to 13-9-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7, 6, and 8. This connection is for uplift only and does not consider lateral forces.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

1			9-9-0					1	
			9-9-0					1	
LOADING (psf)	SPACING- 2-0-0 Plate Grip DOI 1 15	CSI. TC 0.21	DEFL. Vert(LL)	in (loc	:) l/defl	L/d 999	PLATES MT20	GRIP 244/190	
TCDL 10.0 BCLL 0.0 * BCDL 10.0	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.15 WB 0.05	Vert(CT) Horz(CT)	n/a - 0.00 :	- n/a 3 n/a	999 n/a	Walaht: 29 lb	ET - 20%	
	Code IRC2015/1PI2014	Matrix-S	PRACINC				weight: 28 lb	F1 = 20%	

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD OTHERS

2x4 SP No.3

REACTIONS. 1=135/9-9-0, 3=135/9-9-0, 4=360/9-9-0 (lb/size) Max Horz 1=-22(LC 13) Max Uplift 1=-29(LC 8), 3=-32(LC 13), 4=-30(LC 8) Max Grav 1=139(LC 23), 3=139(LC 24), 4=360(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-11-5 to 3-11-5, Interior(1) 3-11-5 to 4-10-8, Exterior(2) 4-10-8 to 7-10-8, Interior(1) 7-10-8 to 8-9-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

7) One RT7A USP connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 4. This connection is for uplift only and does not consider lateral forces.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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2x4 ⋍

2x4 🗢

Rigid ceiling directly applied or 10-0-0 oc bracing.

0-0-12 late Offsets (X,Y)	[2:0-2-0.Edge]		4-9-0 4-8-4	
DADING (pst)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL) n/a - n/a 999	MT20 244/190
CDL 10.0	Lumber DOL 1.15	BC 0.17	Vert(CT) n/a - n/a 999	
CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a	
CDL 10.0	Code IRC2015/TPI2014	Matrix-P		Weight: 11 lb FT = 20%

BOT CHORD

TOP CHORD2x4 SP No.3BOT CHORD2x4 SP No.3

REACTIONS. (lb/size) 1=115/4-7-8, 3=115/4-7-8 Max Horz 1=8(LC 12) Max Uplift 1=-16(LC 8), 3=-16(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

0-0-12	3	-4-8 -3-12			3-4-8		
Plate Offsets (X,Y)	[2:0-2-0,Edge]						
LOADING (psf)	SPACING- 2-0-	csi.	DEFL. in	(loc) l/d	lefl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.1	5 TC 0.10	Vert(LL) n/a	I	n/a 999	MT20	244/190
TCDL 10.0	Lumber DOL 1.1	5 BC 0.31	Vert(CT) n/a	- 1	n/a 999		
BCLL 0.0 *	Rep Stress Incr YES	S WB 0.00	Horz(CT) 0.00	3 1	n/a n/a		
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P				Weight: 18 lb	FT = 20%
LUMBER-			BRACING-			1	
TOP CHORD 2x4 S	SP No.2		TOP CHORD	Structural v	wood sheathing dire	ectly applied or 6-0-0	oc purlins.

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 1=195/6-7-8, 3=195/6-7-8 Max Horz 1=-14(LC 13) Max Uplift 1=-27(LC 8), 3=-27(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED INTER REPERENCE PAGE MIL-14's rev. Invozen's Derrore USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

