

BUILDER CONTACT INFO	ELECTRICAL NOTES	PLUMBING NOTE	Ρ
GENERAL NOTES	FLOORING NOTES	TRIM NOTES	Ľ

















3,12	
	LEFT ELEVATION $1/4" = 1'-0"$
8'-0" 2ND FLR WIN. HEIGHT 9'-1 1/2" 2ND FLR PLATE HEIGHT	
8'-0" HOUSE WINDOW HEIGHT 9'-1 1/2" HOUSE PLATE HEIGHT	
	RIGHT ELEVATION



A5



	Boise Cascade		Double 1-	3/4" x	11-7/8" \	/ERSA-L	AM®	2.0 31	00 SP	I		P	ASSED		
			1st Floor	r\Dropp	ed Beams	\GDH(i181) (Droj	pped Be	am)						
BC (Buile	CALC® Member d 7493	Report		I	Dry 2 span	s No cant.					Octobe	er 6, 2020	0 08:23:52		
Job	name:					File name	e: 2	2000813A	.mmdl						
Add	ress:					Descriptio	on: 1	st Floor\[Dropped	Beams\0	GDH(i18	1)			
City	, State, Zip:					Specifier:									
Cus	tomer:					Designer:									
Cod	e reports:	ESR-1040				Company	':								
								/2 + + + +	↓ 2 ↓ ↓ ↓			16 15 + +			
]								
<u>}</u>			13-10-08						11	-10-08					
B1				Total Ha	rizontal Bra	B2 duct Longth	- 25 00	00		11-10-08					
Ros	action Summ	arv (Dowr	a / Liplift) (lb			uuci Lengin	= 25-09-	-00							
Bear	ring	Live		ə) ad	Sr	างพ		Wind		Roo	f Live				
B1,	46-1/2"		17	17/0				825 / 115	55	202	5/304				
B2,	B2, 24" 3			70/0				1563 / 20)57	383	6 / 364				
ВЗ,	22-1/2"		16	59 / 0				712 / 860)	1892	2 / 275				
Loa	ad Summarv							Live	Dead	Snow	Wind	Roof	Tributary		
neT	Description			Rof	Start	End		100%	90%	115%	160%	Live			
0	Self-Weight		Unf. Lin. (lb/ft)	L	00-00-00	25-09-00	Top	100 /0	12	11070	100 /0	12370	00-00-00		
1	Smoothed Load		Unf. Lin. (lb/ft)	L	02-09-04	14-09-04	Тор		251			296	n\a		
2	Smoothed Load		Unf. Lin. (lb/ft)	L	14-09-04	22-09-04	Тор		258			304	n\a		
3	DE(c1)		Conc. Pt. (lbs)	L	01-09-04	01-09-04	Тор		433			540	n\a		
4	DE(c1)		Conc. Pt. (lbs)	L	01-09-04	01-09-04	Тор					-52	n∖a		
5	D(c1)		Conc. Pt. (lbs)	L	03-09-04	03-09-04	Тор					-55	n∖a		
6	D(c2)		Conc. Pt. (lbs)	L	05-09-04	05-09-04	Тор					-55	n∖a		
7	D(c3)		Conc. Pt. (lbs)	L	07-09-04	07-09-04	Тор					-55	n∖a		
8	D(c4)		Conc. Pt. (lbs)	L	09-09-04	09-09-04	Тор					-55	n∖a		
9	D(c5)		Conc. Pt. (lbs)	L	11-09-04	11-09-04	Тор					-55	n∖a		
10	D(c6)		Conc. Pt. (lbs)	L	13-09-04	13-09-04	Тор					-55	n∖a		
11	D1(c1)		Conc. Pt. (lbs)	L	15-09-04	15-09-04	Тор					-57	n∖a		
12	D1(c2)		Conc. Pt. (lbs)	L	17-09-04	17-09-04	Тор					-57	n\a		
13	D1(c3)		Conc. Pt. (lbs)	L	19-09-04	19-09-04	Тор					-57	n\a		
14	D1(c4)		Conc. Pt. (lbs)	L	21-09-04	21-09-04	Тор					-57	n\a		
15	D1(c5)		Conc. Pt. (lbs)	L	23-09-04	23-09-04	Тор		502			576	n∖a		
16	D1(c5)		Conc. Pt. (lbs)	L	23-09-04	23-09-04	Тор					-53	n\a		
17	D1(c6)		Conc. Pt. (lbs)	L	25-06-12	25-06-12	Тор		425			424	n\a		
18	D1(c6)		Conc. Pt. (lbs)	L	25-06-12	25-06-12	Тор					-57	n∖a		
Co	ntrols Summa	ary Value)	% Allowa	able D	oration	Case	e Loca	tion						
Moc	. woment	30/5	3 ft-Ibe	14.ŏ% 18.∩%	1	2070 25%	ן כ	19-0 10-4	ษ-04 0_09						
End	Shoar	-403		10.070	1	2070	3	04.1	0-00						
	silieai t Shear	1042	lbe	10.0% 30.5%	1	2070 25%	I A	04-1 15 1	0-00						
Toto	L Oneal I I and Deflection	1 /000	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	00.070 n\a	1 n	2070 \a	-+ -1	10_0	9-00 9-01						
	I and Deflection	1 /000	9 (0.0 1 7) 9 (0.036")	n\a	n	\a	י 200	19-0 10_0	7-00						
Tota	I Neg. Defl	/900	9 (-0.001")	n\a	n	\a	200	, 13-0 12-1	0-08						
Max	Defl.	0.047	7"	n\a	n	\a	- - 1	19-0	9-04						
Spa	n / Depth	10.2	-					.00							
Con	c. Load (B1)	1095	lbs	11.9%	1	00%									
Con	c. Load (B2)	1095	lbs	11.9%	1	00%									
Con	c. Load (B3)	849	bs	9.2%	1	00%									



Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\Dropped Beams\GDH(i181) (Dropped Beam)

Dry | 2 spans | No cant.



October 6, 2020 08:23:52

BC CALC® Member	Report
Build 7493	
Job name:	
Address:	
City, State, Zip:	
Customer:	
Code reports:	ESR-1040

File name: 2000813A.mmdl Description: 1st Floor\Dropped Beams\GDH(i181) Specifier: Designer: Company:

Bearing	Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	46-1/2" x 3-1/2"	3668 lbs	3.1%	3.0%	Unspecified
B2	Wall/Plate	24" x 3-1/2"	7164 lbs	11.8%	11.4%	Unspecified
B3	Wall/Plate	22-1/2" x 3-1/2"	3444 lbs	6.0%	5.8%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 01-10-08, Bottom: 01-10-08.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Connection Diagram: Full Length of Member



Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.



TRANSACTION # 2000811 STATUS QUOTE QUOTE DATE

BUILDER MODEL SHIP TO

SOLD TO 2307- 84 Fayetteville 620 Belt Drive Fayetteville North Carolina 28301

Fayetteville NC

SALES REP J.P. Lilliston (910) 339-6330

CONTACT

84 Lumber Company • 200 Emmett Rd • Dunn • NC • 28334 • Phone: (910) 892-8400 • Fax: (910) 892-8343

QTY				(Shipping)	Base Span		OVERH	IANG	CANTIL	EVER	ST	UB
DIAGRAM	PLY	РІТСН	LABEL	HEIGHT	SPAN	LUMBER	LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT
	1 2-ply	8 /12	AH	10-05-03	37-06-00	2 x 4 2 x 6	10-08		-	-	-	
	1	8 /12	AHA	10-05-03	37-06-00	2 x 4	10-08		-	-	-	
	1 2-ply	8 /12	AHA1	10-01-11	37-06-00	2 x 4 2 x 6	10-08	10-08	-	-	-	
	1	8 /12	AHA2	10-01-11	37-06-00	2 x 4	10-08	10-08	-	-	-	
	1	8 /12	AHA3	10-01-11	37-06-00	2 x 4	10-08	10-08	-	-	-	
	1	8 /12	AHA4	11-05-06	37-06-00	2 x 4	10-08	10-08	-	-	-	
	1	8 /12	AHA5	11-05-06	37-06-00	2 x 4	10-08	10-08	-	-	-	
	1	8 /12	AHB	10-05-03	37-06-00	2 x 4	10-08		-	-	-	
	1	8 /12	AHC	11-05-06	37-06-00	2 x 4	10-08		-	-	-	
	1	8 /12	AHD	11-05-06	37-06-00	2 x 4	10-08	ī	-	-	-	
	1	8 /12	В	10-05-03	27-10-00	2 x 4	10-08	10-08	-	-	-	
	2	8 /12	B1	10-05-03	27-10-00	2 x 4	10-08		-	-	-	
	1 2-ply	8 /12	BH	5-05-06	27-10-00	2 x 4 2 x 6	10-08	10-08	-	-	-	
	1	8 /12	BH1	7-05-06	27-10-00	2 x 4	10-08	10-08	-	-	-	
	1	8 /12	BH2	9-05-06	27-10-00	2 x 4	10-08	10-08	-	-	-	
	3	8 /12	С	10-01-11	26-11-08	2 x 4	10-08	10-08	-	-	-	
	2	8 /12	C1	10-01-11	26-11-08	2 x 4	10-08		-	-	-	
	1 2-ply	8 /12	СН	5-05-06	26-11-08	2 x 4 2 x 6	10-08		-	-	-	
	1	8 /12	CH1	7-05-06	26-11-08	2 x 4	10-08	10-08	-	-	-	
	1	8 /12	CH2	9-05-06	26-11-08	2 x 4	10-08	10-08	-	-	-	
	3	2.12 /12	CJ1	1-08-05	5-06-06	2 x 4	1-02-14		11-04	-	-	
Δ / Δ	6	12 /12	D	10-07-00	24-00-00	2 x 4	1-02-08	1-02-08	-	-	-	



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<u>Componen</u>	t Iten	<u>n - Roc</u>	of Trusse	es								
	QTY			(Shipping)	<u>Base Span</u>		OVERH	IANG	CANTIL	EVER	ST	UB
DIAGRAM	PLY	PITCH	LABEL	HEIGHT	SPAN	LUMBER	LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT
	6	12 /12	D1	10-07-00	24-08-00	2 x 4	3-08	1-02-08	-	-	-	-
	1	12 /12	DE	10-07-00	24-00-00	2 x 4	1-02-08	1-02-08	-	-	-	-
	1	12 /12	EE	9-06-15	15-09-00	2 x 4	1-02-08	1-02-08	-	-	-	-
	1	12 /12	GE	6-11-06	11-00-00	2 x 4	10-08	10-08	-	-	-	-
\bigtriangleup	1 3-ply	12 /12	GR1	6-11-06	11-00-00	2 x 4 2 x 6			-	-	-	-
	2	8 /12	Н	5-00-15	12-00-00	2 x 4		10-08	-	-	-	-
	1	8 /12	HE	5-00-15	12-00-00	2 x 4		10-08	-	-	-	-
	נ 7	3 /12	J1	1-08-09	4-00-00	2 x 4	10-08		8-08	-	-	-
	1	8 /12	J2	2-04-15	2-00-00	2 x 4	10-08		-	-	-	-
	6	3 /12	J3	1-02-05	1-10-15	2 x 4	10-08		8-08	-	-	-
	17	12 /12	J5	5-05-06	4-00-00	2 x 4	10-08		-	-	-	-
	6	12 /12	J5A	5-01-14	4-00-00	2 x 4	10-08		-	-	-	-
	6	12 /12	J5B	3-09-14	4-00-00	2 x 4	10-08		-	-	-	-
	6	12 /12	J5C	2-05-14	4-00-00	2 x 4	10-08		-	-	-	-
	1 2-ply	12 /12	J5GR	5-05-06	4-00-00	2 x 4 2 x 6			-	-	-	-
<u> </u>	1 2-ply	12 /12	J5GR1	5-05-06	4-00-00	2 x 4 2 x 6			-	-	-	-
	1	8 /12	J6	2-05-14	2-00-00	2 x 4			-	-	-	-
	4	8 /12	J7	2-05-14	2-00-00	2 x 4	10-08		-	-	-	-
	1	6.66 /12	J8	2-05-00	2-03-14	2 x 4	1-00-00		5-13	-	-	-
	4	6.66 /12	J9	2-05-00	1-08-15	2 x 4	1-07-09		0-09	-	-	-
	1	8 /12	J10	2-05-14	2-00-00	2 x 4			-	-	-	-
	1	6.66 /12	J11	2-06-00	1-10-11	2 x 4	1-07-09		0-09	0-00	-	-



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SOLD TO 2307- 84 Fayetteville 620 Belt Drive Fayetteville North Carolina 28301

Fayetteville NC

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	QTY			(Shipping)	Base Span		OVERH	IANG	CANTIL	EVER	STI	JB
DIAGRAM	PLY	РІТСН	LABEL	HEIGHT	SPAN	LUMBER	LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT
\triangle	1	8 /12	К	5-11-10	14-08-00	2 x 4	10-08		-	-	-	
	1	8 /12	KGR	5-11-10	14-08-00	2 x 4 2 x 6	10-08		-	-	-	
	1	3 /12	LH	1-08-09	14-04-00	2 x 4 2 x 6	10-08	10-08	8-08	8-08	-	
	1	3 /12	LH1	2-02-09	14-04-00	2 x 4 2 x 6	10-08	10-08	8-08	8-08	-	
	6	3 /12	M1	2-06-07	7-03-08	2 x 4	10-08		8-08	-	-	
	1	3 /12	M1A	2-02-09	7-03-08	2 x 4	10-08		8-08	-	-	
	1	3 /12	M1B	1-08-09	7-03-08	2 x 4 2 x 6	10-08		8-08	-	-	
	6	3 /12	M2	2-05-09	7-00-00	2 x 4	10-08		8-08	-	-	
	14	3 /12	М3	2-02-01	5-10-00	2 x 4	10-08		-	-	-	
	1	3 /12	M3A	8-05-09	17-10-08	2 x 4 2 x 6	10-08		-	-	-	
	1	3 /12	M3B	7-01-09	17-10-08	2 x 4 2 x 6	10-08		-	-	-	
	1	3 /12	M3C	5-09-09	17-10-08	2 x 4 2 x 6	10-08		-	-	-	
	1	3 /12	M3D	4-05-09	17-10-08	2 x 4 2 x 6	10-08		-	-	-	
	1	3 /12	M3E	3-01-09	17-10-08	2 x 4 2 x 6	10-08		-	-	-	
	11	3 /12	M4	3-08-09	12-00-00	2 x 4 2 x 6	10-08		4-08	-	-	
	1	3 /12	M4E	2-08-09	8-00-00	2 x 4	10-08		-	-	-	
	1	3 /12	M4E1	2-02-09	6-00-00	2 x 4	10-08		-	-	-	
	10	12/12	M7	5-04-07	4-00-00	2 x 4	10-08		-	-	-	
	1	12/12	M7A	5-00-15	4-00-00	2 x 4	10-08		-	-	-	
	1	12 /12	M7B	3-08-15	4-00-00	2 x 4	10-08		-	-	-	
	1	12/12	M7C	2-04-15	4-00-00	2 x 4	10-08		-	-	-	
	1	12/12	M8	11-00-07	9-04-00	2 x 4	1-02-08		-	-	-	



TRANSACTION # 2000811 STATUS QUOTE DATE

BUILDER MODEL SHIP TO

sold to 2307- 84 Fayetteville 620 Belt Drive Fayetteville North Carolina 28301

Fayetteville NC

SALES REP J.P. Lilliston (910) 339-6330

CONTACT

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Component Item - Roof Trusses

	QTY			(Shipping)	<u>Base Span</u>		OVER	HANG	CANTIL	EVER	ST	JB
DIAGRAM	PLY	PITCH	LABEL	HEIGHT	SPAN	LUMBER	LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT
	1	12/12	PB1	(3-01-07) 2-11-15	6-02-14	2 x 4			-	5-01-08	-	-
	12	12/12	PB2	(3-01-07) 2-11-15	6-02-14	2 x 4			-	5-01-08	-	-
	2	8 /12	PB3	(2-00-00) 1-10-08	6-07-08	2 x 4			-	5-01-10	-	-
	1	12/12	V1	4-07-08	9-03-00	2 x 4			-	-	-	-
	1	12 /12	V2	3-03-08	6-07-00	2 x 4			-	-	-	-
	1	12 /12	V3	1-11-08	3-11-00	2 x 4			-	-	-	-
	199				2027.49							

Ancillary Items

QTY	Label		Description	Length
3	HHUS26-2	HHUS26-2		
4	HUS26	HUS26		

Notes: **Tax Not Included-Delivery Included**

Thank you for allowing 84 to bid this job.

Total \$15,419.00

Rodney Evans

Terms: This pricing will be honored if job delivers on or before 10/20/2020. Any layout provided with quotes will be preliminary until finalization at time of order. All girder ply fastener hardware will be provided by others. Girders ply's to be assembled in field by contractor. All girder assembly information can be found on engineered drawings provided. All field framing on layout is to be done in the field. Contact 84 engineered wood center for any needed LVL/beams.

* IMPORTANT NOTE *

184 LUMBER	JOB NAME	Hicks Residence - Milltown	TRANSACTION # 2000811 STATUS Quote
	BUILDER	MODEL	QUOTE DATE
sold to 2307- 84 Fayetteville 620 Belt Drive Fayetteville North Carolina 28301	SHIP TO	Favetteville NC	SALES REP J.P. Lilliston
			(910) 339-6330
			CONTACT

84 Lumber Company • 200 Emmett Rd • Dunn • NC • 28334 • Phone: (910) 892-8400 • Fax: (910) 892-8343

THIS PROPOSAL IS BASED ON LUMBER DESIGN VALUES IN EFFECT AT THE TIME OF THE QUOTE. IN THE EVENT ANY LUMBER DESIGN VALUES CHANGE BEFORE THE COMPLETION OF THE PROJECT, 84 LUMBER COMPANY RESERVES THE RIGHT TO MODIFY THE PRICE ACCORDINGLY.













Boise Cascade Quadruple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP												
BC (CALC® Member Re	eport	JOI \F1U	Dry 2 spar	ns L cant.) (riu	Sii Dealii))		Octobe	er 6, 2020) 07:11:07
Joh	name.				Filo nome	` .	20008134	mmdl				
000					Descriptio	7. nn:	1 of Eleon/E	.mmui Juch Roy	ame\EB1	(1162)		
City	Stata Zin:				Specifier:	JII.			anis\fdi	(1103)		
City,	, State, Zip.				Specifier.							
Cus	iomer:	CD 4040			Designer							
Cod	e reports: E	SR-1040			Company	:						
								32 15 15 17 16 16 16 16 16				
<u>}</u>												
	03-07-04 B1				24-08-0)4						B2
			Total Ho	rizontal Pro	duct Length	= 28-0	3-08					
Rea	action Summar	y (Down / Uplift) (lbs)	e,			Wind		Boo	FLivo		
B1	1119 3_1/2"	3827 / 11 649	u 4 / 0				912 / 240	0	330	7 / 80		
B2.	3-1/2"	1674 / 58 269	7/0				503 / 112	24	140	3 / 223		
	d Summary						Live	Dead	Snow	Wind	Roof	Tributary
-				• · · ·				2000			Live	moutary
Tag	Description Solf Woight	Load Type	Ref.	Start	28.03.08	Loc.	100%	<u>90%</u>	115%	160%	125%	00 00 00
1	5611-Weight E54(i55)	Unit Lin. (Ib/it)	L 1	00-00-00	28-03-08	Top		52 64				00-00-00 n\a
י 2	E34(I33) Smoothed Load	Unit Lint (Ib/it)	L 1	00-03-08	20-03-00	Top	254	04				n\a
1	Smoothed Load	Unf Lin (lb/ft)	1	01-03-00	07-03-00	Top	204	52 57			59	n\a
4 5		Unf Lin (lb/ft)	L 1	01-03-00	07-03-00	Top		90 90			95 95	n\a
5	E54(155)	Unf Lin (lb/ft)	L 1	01-10-04	05-02-04	Тор		00 74			00	n\a
7	$E_{54(155)}$	Unf Lin (lb/ft)	L 1	05-10-04	03-02-04	Top		74			93	n\a
/	E34(133)	Uni. Lin. (ID/IL)	L	07 10 04	07-02-04	тор		72			89	n\a
8	E54(155)	Unt. Lin. (ID/π)	L	07-10-04	09-02-04	тор		74			92	n\a
9	E54(155)	Unt. Lin. (ID/π)	L	09-10-04	11-02-04	тор	070	75			92	n∖a
10	Smoothed Load	Unf. Lin. (lb/ft)	L	11-03-08	17-03-08	Тор	373	136				n\a
11	Smoothed Load	Unf. Lin. (ID/π)	L	11-03-08	17-03-08	Тор		56			58	n∖a
12	E54(i55)	Unf. Lin. (lb/ft)	L	11-10-04	13-02-04	Тор		75			92	n\a
13	E54(i55)	Unf. Lin. (lb/ft)	L	13-10-04	15-02-04	Тор		72			86	n\a
14	E54(i55)	Unf. Lin. (lb/ft)	L	15-06-12	16-10-12	Тор		72			86	n\a
15	E54(i55)	Unf. Lin. (lb/ft)	L	17-02-12	23-02-12	Тор		50			60	n\a
16	Smoothed Load	Unf. Lin. (lb/ft)	L	17-03-08	24-10-04	Тор		60			61	n∖a
17	Smoothed Load	Unf. Lin. (lb/ft)	L	17-03-08	24-10-04	Тор	57					n∖a
18	E54(i55)	Unf. Lin. (lb/ft)	L	23-06-12	24-10-12	Тор		74			93	n∖a
19	E54(i55)	Unf. Lin. (lb/ft)	L	25-06-12	26-10-12	Тор		78			86	n\a
21	FB8(i170)	Conc. Pt. (lbs)	L	00-07-04	00-07-04	Тор	233	1867			1355	n∖a
22	FB8(i170)	Conc. Pt. (lbs)	L	00-07-04	00-07-04	Тор					-30	n∖a
23	-	Conc. Pt. (lbs)	L	26-01-00	26-01-00	Тор	108	185			148	n∖a
24	-	Conc. Pt. (lbs)	L	26-01-00	26-01-00	Тор					-28	n∖a
25	R1031(c1)	Conc. Pt. (lbs)	L	01-10-04	01-10-04	Тор					-3	n∖a
26	R1032(c1)	Conc. Pt. (lbs)	L	03-10-04	03-10-04	Тор					-3	n∖a
27	R1033(c1)	Conc. Pt. (lbs)	L	05-10-04	05-10-04	Тор					-3	n\a
28	R1036(c1)	Conc. Pt. (lbs)	L	11-10-04	11-10-04	Top					-3	n\a
29	R1037(c1)	Conc. Pt. (lbs)	L	13-10-04	13-10-04	Тор					-3	n\a
30	-	Conc. Pt. (lbs)	L	16-01-00	16-01-00	Top					-3	n\a
31	-	Conc. Pt. (lbs)	L	18-01-02	18-01-02	Top					-3	n\a
32	-	Conc. Pt. (lbs)	L	20-01-01	20-01-01	, aoT					-3	n\a
33	-	Conc. Pt. (lbs)	L	22-01-01	22-01-01	Top					-3	n\a
34	-	Conc. Pt. (lbs)	L	24-01-01	24-01-01	Тор					-3	n\a



Quadruple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP



Tributary

1st Floor\Flush Beams\FB1(i163) (Flush Beam) Dry | 2 spans | L cant.

Live

Dead

October 6, 2020 07:11:07

Roof

Wind

Snow

Build 7493			
Job name:		File name:	2000813A.mmdl
Address:		Description:	1st Floor\Flush Beams\FB1(i163)
City, State, Zip:		Specifier:	
Customer:		Designer:	
Code reports:	ESR-1040	Company:	

Load Summary

											Live	
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
36	E54(i55)	Conc. Pt. (lbs)	L	02-06-04	02-06-04	Тор					-21	n∖a
39	-	Conc. Pt. (lbs)	L	08-02-14	08-02-14	Тор	454	270			115	n∖a
40	-	Conc. Pt. (lbs)	L	08-02-14	08-02-14	Тор	-6					n∖a
41	-	Conc. Pt. (lbs)	L	08-02-14	08-02-14	Тор					-3	n∖a
42	-	Conc. Pt. (lbs)	L	10-02-14	10-02-14	Тор	454	269			115	n∖a
43	-	Conc. Pt. (lbs)	L	10-02-14	10-02-14	Тор	-8					n∖a
44	-	Conc. Pt. (lbs)	L	10-02-14	10-02-14	Тор					-3	n∖a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	34098 ft-lbs	45.9%	100%	4	14-03-09
Neg. Moment	-11312 ft-lbs	12.2%	125%	5	03-07-04
End Shear	4181 lbs	19.6%	100%	4	26-08-00
Cont. Shear	6289 lbs	29.6%	100%	1	05-01-00
Total Load Deflection	L/362 (0.812")	66.4%	n\a	64	15-10-03
Live Load Deflection	2xL/388 (-0.223")	92.9%	n\a	456	00-00-00
Total Neg. Defl.	2xL/244 (-0.354")	98.3%	n\a	64	00-00-00
Max Defl.	0.812"	81.2%	n\a	64	15-10-03
Cant. Max Defl.	-0.354"	35.4%	n\a	64	00-00-00
Span / Depth	18.3				

Bearing	Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column	3-1/2" x 7"	12255 lbs	69.0%	66.7%	Unspecified
B2	Wall/Plate	3-1/2" x 7"	5231 lbs	50.2%	28.5%	Spruce-Pine-Fir

Cautions

Concentrated side load(s) 9 are closer than 18" from end of member.Please consult a technical representative or Professional of Record.

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets User specified (2xL/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (1") Cantilever Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume unbraced length of Top: 00-05-08, Bottom: 00-05-08.

 $\operatorname{BC}\operatorname{CALC}\nolimits {\ensuremath{\mathbb R}}$ analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.



Quadruple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB1(i163) (Flush Beam)



0-4-4 6, 2020 07:11:07

BC CALC® Member	Report	Dry 2 spans L cant.	October 6, 20
Build 7493			
Job name:		File name:	2000813A.mmdl
Address:		Description:	1st Floor\Flush Beams\FB1(i163)
City, State, Zip:		Specifier:	
Customer:		Designer:	
Code reports:	ESR-1040	Company:	

Connection Diagram: Full Length of Member



a minimum = 2" c = 12" b minimum = 2-1/2" d = 12"

Calculated Side Load = 512.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt

Disclosure

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	Boise Cascade	Dou	ble 1-3/4" st Floor\Flu	x 16" VE		M® 2.(0 3100 Beam)	SP			P	ASSED
BC C	CALC® Member Re	port	511001411	Dry 3 spa	ns R cant.) (i iusii	Deam			Octobe	er 6, 202	0 07:11:07
Build	1 7493 namo:				Eilo nome	. 20	008134	mmdl				
n doc					Descriptio	, ∠0 nn 1si	t Floor\F	lush Re:	ams\FB2	?(i169)		
Citv.	State, Zip:				Specifier:					-(1100)		
Cust	omer:				Designer							
Code	e reports: ES	SR-1040			Company	:						
							L					
			+ + +	+ + +	0 ↓ ↓	i i i	· · ·		↓ ↓	¥ ¥	÷ ÷	,
\bowtie												
<i>k</i> —												
B1		13-00-00			B2			11-04-00				01-02-00 B3
			Total H	orizontal Pro	duct Length	= 25-06-0	0					
Rea	ction Summary	y (Down / Uplift)) (lbs)	0			line of		Dee	6 1 3		
B1 3	ing 3-1/2"	1041 / 31	391 / 0	5	now	V	vina		ROO	T LIVE		
B2. 8	3"	1878 / 0	789/0									
B3, 8	3"	325 / 172	95 / 0									
,												
Loa	d Summary					_	Live	Dead	Snow	Wind	Roof Live	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	<u>90%</u>	115%	160%	125%	00 00 00
1	EC1 Floor Material	Uni. Lin. (i Linf Lin. (i	D/IL) L	00-00-00	25-00-00	тор Тор	23	6				00-00-00 n\a
2	FC1 Floor Material	Unflin (b/ft) L	05-11-12	12-08-00	Top	30	7				n\a
3	FC1 Floor Material	Unf. Lin. (I	b/ft) L	12-08-00	25-06-00	Тор	52	, 13				n\a
4	FB9(i171)	Conc. Pt.	(lbs) L	05-11-12	05-11-12	Тор	1962	609				n\a
•	4	_										
	trois Summary	Value	% Allow	vable D	Duration	Case	Loca	tion				
POS.	Moment	7380 IL-IDS -4276 ft-Ibs	19.8%	1	100%	2 1	13-0	1-12 1-00				
Fnd	Shear	1351 lbs	12.7%	1	100%	2	01-0	7-08				
Cont	. Shear	1733 lbs	16.3%	1	100%	4	11-04	1-00				
Tota	Load Deflection	L/999 (0.064")	n\a	r	n\a	2	06-00)-12				
Live	Load Deflection	L/999 (0.048")	n∖a	r	n∖a	7	06-0	1-12				
Tota	l Neg. Defl.	L/999 (-0.019")	n\a	r	n∖a	2	17-06	6-08				
Max	Defl.	0.064"	n\a	r	n∖a	2	06-00)-12				
Cant	. Max Defl.	0.005"	n\a	r	n∖a	2	25-06	6-00				
Spar	n / Depth	9.6										
Bea	ring Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Materia	al					
B1	Wall/Plate	3-1/2" x 3-1/2"	1431 lbs	27.5%	15.6%	Spruce	e-Pine-F	ir				
B2	Column	8" x 3-1/2"	2668 lbs	13.1%	12.7%	Unspe	cified					
B3	Column	8" x 3-1/2"	420 lbs	2.1%	2.0%	Unspe	cified					
B3	Uplift		77 lbs									
Cau	itions											
Uplif	t of -77 lbs found at	bearing B3.										



Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB2(i169) (Flush Beam)

Dry | 3 spans | R cant.



October 6, 2020 07:11:07

Build 7493			
Job name:		File name:	2000813A.mmdl
Address:		Description:	1st Floor\Flush Beams\FB2(i169)
City, State, Zip:		Specifier:	
Customer:		Designer:	
Code reports:	ESR-1040	Company:	

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Design meets arbitrary (1") Cantilever Maximum Total load deflection criteria. Design meets arbitrary (0.75") Maximum live load deflection criteria. Calculations assume member is fully braced. BC CALC® analysis is based on IBC 2012.

Design based on Dry Service Condition.

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at ends.

Connection Diagram: Full Length of Member



a minimum = 2" c = 6" b minimum = 3" d = 24"

Connectors are: 3-1/4 in. Pneumatic Gun Nails

Connection Diagrams: Concentrated Side Loads



Applies to load tag(s): 3



a minimum = 2" b minimum = 4" c minimum = 4" d maximum = 12" e minimum = 4" Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

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Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB3(i167) (Flush Beam)

Dry | 2 spans | No cant.



October 6, 2020 07:11:07

BC CALC® Member	Report
Build 7493	
Job name:	
Address:	
City, State, Zip:	
Customer:	
Code reports:	ESR-1040

File name: 2000813A.mmdl Description: 1st Floor\Flush Beams\FB3(i167) Specifier: Designer:

Company:

Load Summary

Loa	d Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
37	-	Conc. Pt. (lbs)	L	20-11-06	20-11-06	Тор					-6	n∖a
38	-	Conc. Pt. (lbs)	L	22-11-05	22-11-05	Тор	631	306			79	n∖a
39	-	Conc. Pt. (lbs)	L	22-11-05	22-11-05	Тор					-6	n∖a
40	-	Conc. Pt. (lbs)	L	24-11-05	24-11-05	Тор	631	302			73	n∖a
41	-	Conc. Pt. (lbs)	L	24-11-05	24-11-05	Тор					-35	n∖a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	15364 ft-lbs	27.4%	100%	3	18-11-08
Neg. Moment	-16979 ft-lbs	30.3%	100%	1	09-05-12
End Shear	3790 lbs	23.7%	100%	3	25-04-00
Cont. Shear	5862 lbs	36.7%	100%	1	10-11-08
Total Load Deflection	L/1031 (0.201")	23.3%	n∖a	71	18-10-12
Live Load Deflection	L/999 (0.114")	n∖a	n∖a	478	18-10-12
Total Neg. Defl.	L/999 (-0.029")	n∖a	n∖a	71	05-10-12
Max Defl.	0.201"	20.1%	n∖a	71	18-10-12
Span / Depth	12.9				

Bearing	Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	3-1/2" x 5-1/4"	1758 lbs	22.5%	12.8%	Spruce-Pine-Fir
B1	Uplift		416 lbs			
B2	Column	3-1/2" x 5-1/4"	11211 lbs	84.2%	81.4%	Unspecified
B3	Wall/Plate	3-1/2" x 5-1/4"	4085 lbs	52.3%	29.6%	Spruce-Pine-Fir

Cautions

Uplift of -416 lbs found at bearing B1.

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Design meets arbitrary (0.75") Maximum live load deflection criteria. Calculations assume member is fully braced. BC CALC® analysis is based on IBC 2012. Wind loads determined from building geometry were used in selected product's verification. Design based on Dry Service Condition.

Connection Diagram: Full Length of Member





Triple 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB3(i167) (Flush Beam)

Dry | 2 spans | No cant.



October 6, 2020 07:11:07

BC CALC® Member Report Build 7493 Job name: Address: City, State, Zip: Customer: Code reports: ESR-1040

File name: 2000813A.mmdl Description: 1st Floor\Flush Beams\FB3(i167) Specifier: Designer: Company:

Connection Diagram: Full Length of Member

a minimum = 2" b minimum = 3"

c = 6" d = 12" e minimum = 3"

Calculated Side Load = 438.5 lb/ft Nailing applies to both sides of the member Connectors are: 16d Box Nails

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Boise Ca	ascade 📕	Doul	ole 1-3/4	4" x 16	" VE	RSA-LA	M® 2.	0 3100) SP			P	ASSED
		1	st Floor\	Flush Be	ams	\FB4(i166) (Flush	Beam)					
BC CALC® N Build 7493	Member Re	port		Dry 1	span	n No cant.	, ,				Octobe	er 6, 202	0 07:11:07
Job name:						File name	e: 20	000813A	.mmdl				
Address:						Descriptio	on: 1s	st Floor\F	lush Be	ams∖FB4	4(i166)		
City, State, Z	Ζip:					Specifier:							
Customer:						Designer:							
Code reports	ES	R-1040				Company	:						
													2
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B1					15-0	03-08							B2
Desetion	0		Tota	I Horizonta	al Proc	duct Length	= 15-03-0	08					
Reaction &	Summary	/ (Down / Uplift) _{Live}	(IDS) Dead		Sn	iow		Vind		Roo	f Live		
B1, 3-1/2"		2276 / 0	961/0								-		
B2, 3-1/2"		2073 / 0	985 / 0				2	41/0		84 /	0		
Load Sum	mary							Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Descript	tion	Load Type	R	ef. St	art	End	Loc.	100%	90%	115%	160%	125%	
0 Self-We	eight	Unf. Lin. (I	b/ft) L	00-0	0-00	15-03-08	Тор	•	16				00-00-00
1 FC1 Flo	or Material	Unf. Lin. (I	b/ft) L	00-0	0-00	15-03-08	Тор	6	2				n∖a
2 Smooth		Unt. Lin. (I	D/π) L	15.0	3-08	14-03-08	Тор	304	00			01	n\a n\a
5 E00(100))	Conc. Pl. (ids) L	15-0	1-12	15-01-12	тор		99			04	II\a
Controls S	Summary	Value	% Al	owable	D	uration	Case	Loca	tion				
Pos. Moment	t	12233 ft-lbs	32.7	%	10	00%	1	07-0	3-08				
End Shear	.	2990 lbs	28.1	%	10	00%	1	01-0	7-08				
Total Load D	eflection	L/886 (0.201")	27.1	%	n'	\a	1	07-0	8-00				
Live Load De	eflection	L/1258 (0.141")	28.6	%	n'	\a	40	07-0	8-00				
Max Defl.		0.201"	20.1	%	n	\a	1	07-08	8-00				
Span / Depth	1	11.1											
Bearing S	upports	Dim. (LxW)	Value	% Allo Suppo	w rt	% Allow Member	Materi	al					
ыл M	all/Plate	3-1/2" X 3-1/2"	3237 lbs	62.2%	1	35.2%	Spruc	e-Pine-F	۱ ۲				
в2 W	all/Plate	3-1/2" x 3-1/2"	3058 lbs	58.7%	1	33.3%	Spruc	e-Pine-F	Ir				
Notes													
Design meets	s Code min	imum (L/240) Total I	oad deflec	tion criter	ia.								

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.



Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB4(i166) (Flush Beam)

Dry | 1 span | No cant.



October 6, 2020 07:11:07

Build 7493			
Job name:		File name:	2000813A.mmdl
Address:		Description:	1st Floor\Flush Beams\FB4(i166)
City, State, Zip:		Specifier:	
Customer:		Designer:	
Code reports:	ESR-1040	Company:	

Connection Diagram: Full Length of Member



Calculated Side Load = 416.5 lb/ft Connectors are: 16d Common Nails

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Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP



7

					1st F	loo	r\Flu	sh E	Bea	ms\FB	5(i16	52) (F	lus	h Be	eam)									
BC CALC® Mem	ber Re	port						Dry	1 s	pan No	o can	t.								00	ctobe	er 6,	2020	0 07:	11:0
Build 7493 Job name: Address: City, State, Zip: Customer: Code reports:	ES	;R-104(D							File De Spe De Co	e nan script ecifie signe mpar	ne: tion: :r: :r: :r: ny:	2 1	20008 Ist Fl	813A loor\l	.mr Flus	ndl h Be	eam	s\FB	5(i16	62)				
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	• • 	+		<u>+</u>	<u>* *</u> 			<u>+</u>	* 		<u>+</u>	—	<u>*</u>	* 	<u>▼</u>	<u>*</u>	+	+	<u>+</u>	<u>+</u>	*	<u> </u>	<u>+</u>	<u>*</u>	*
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J																									
																									\rightarrow
B1										14-08-00															B2
						То	tal Ho	rizor	ntal I	Product I	Lengt	ih = 14	4 -0 8	-00											

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live
B1, 3-1/2"	293 / 0	3273 / 0		715 / 1599	2587 / 43
B2, 3-1/2"	293 / 0	3160 / 0		968 / 1884	2578 / 4

Loa	d Summary		Live	Dead	Snow	Wind	Roof Live	Tributary				
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-08-00	Тор		16				00-00-00
1	E55(i53)	Unf. Lin. (lb/ft)	L	00-00-00	14-08-00	Тор		64				n∖a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	14-08-00	Тор	40	10				n∖a
3	E55(i53)	Unf. Lin. (lb/ft)	L	01-10-12	03-02-12	Тор		428			442	n∖a
4	E55(i53)	Unf. Lin. (lb/ft)	L	03-10-12	05-02-12	Тор		424			436	n∖a
5	E55(i53)	Unf. Lin. (lb/ft)	L	05-06-12	11-06-12	Тор		288			300	n∖a
6	E55(i53)	Unf. Lin. (lb/ft)	L	13-11-04	14-08-00	Тор		566			573	n∖a
7	E55(i53)	Conc. Pt. (lbs)	L	00-07-08	00-07-08	Тор		937			847	n∖a
8	E55(i53)	Conc. Pt. (lbs)	L	00-07-08	00-07-08	Тор					-44	n∖a
14	E55(i53)	Conc. Pt. (lbs)	L	12-08-00	12-08-00	Тор		899			928	n∖a
15	E55(i53)	Conc. Pt. (lbs)	L	12-08-00	12-08-00	Тор					-3	n∖a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	17599 ft-lbs	37.7%	125%	2	07-03-12
End Shear	5260 lbs	39.5%	125%	2	13-00-08
Total Load Deflection	L/633 (0.27")	37.9%	n∖a	2	07-03-12
Live Load Deflection	L/1341 (0.127")	26.9%	n∖a	251	07-03-12
Max Defl.	0.27"	27.0%	n∖a	2	07-03-12
Span / Depth	10.7				

Bearing	g Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Column	3-1/2" x 3-1/2"	5860 lbs	66.0%	63.8%	Unspecified
B2	Column	3-1/2" x 3-1/2"	5750 lbs	64.7%	62.6%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification. Design based on Dry Service Condition.



Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB5(i162) (Flush Beam)

Dry | 1 span | No cant.



October 6, 2020 07:11:07

Build 7493				
Job name:		File name:	2000813A.mmdl	
Address:		Description:	1st Floor\Flush Beams\FB5(i162)	
City, State, Zip:		Specifier:		
Customer:		Designer:		
Code reports:	ESR-1040	Company:		

Connection Diagram: Full Length of Member



Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.





Boise Cascade		Double 1-3/4	" x 16" VERSA	-LAM® 2.0 310	0 SP		PASSED
		1st Floor\F	lush Beams\FB6	i165) (Flush Bear	n)		
BC CALC® Member	Report		Dry 1 span No	cant.		Octobe	er 6, 2020 07:11:07
Build 7493			File	nama: 2000812	A monodi		
Address:			File	rintion: 1st Floor	A.mmu Flush Rea	ams\FB6(i165)	
City, State, Zip;			Spe	cifier:			
Customer:			Des	gner:			
Code reports:	ESR-1040		Con	ipany:			
					٨		
8	3			6	<u>/15</u> 14)	7
	$\frac{3}{4}$			$\begin{array}{c c} & & & \\ \hline \\$	$\overline{+}$	· · · · ·	
+ $+$ $+$	+ $+$ $+$ $+$	+ + + + +	. ↓ ↓ ↓ 1 ↓	+ + + + +	¥ ¥ ,	+ + + +	+ $+$ $+$ $+$
\downarrow \downarrow \downarrow \downarrow	+ + + +	+ + + + +		\downarrow \downarrow \downarrow \downarrow \downarrow	<u>+ + ,</u>	\downarrow \downarrow \downarrow \downarrow	\downarrow \downarrow \downarrow \downarrow
é de la companya de la compa							
4 B1			14-10-12				82
51		Total	Horizontal Product L	ength = 14-10-12			D2
Reaction Summ	ary (Down	/ Uplift) (lbs)					
Bearing	Live	Dead	Snow	Wind		Roof Live	
B1, 31-1/2"	333 / 0	3484 / 0		1123 / 1	1912	2925 / 12	
B2, 10-1/4"	262 / 0	2076 / 0		410 / 10)19	1492 / 32	
Load Summary				Live	Dead	Snow Wind	Roof Tributary Live

											Live	
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	14-10-12	Тор		16				00-00-00
1	E55(i53)	Unf. Lin. (lb/ft)	L	00-00-00	14-10-12	Тор		64				n\a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	14-10-12	Тор	40	10				n\a
3	E55(i53)	Unf. Lin. (lb/ft)	L	00-00-00	04-03-04	Тор		277			288	n\a
4	E55(i53)	Unf. Lin. (lb/ft)	L	04-07-04	05-11-04	Тор		420			441	n\a
5	E55(i53)	Unf. Lin. (lb/ft)	L	06-07-04	07-11-04	Тор		412			424	n\a
6	E55(i53)	Unf. Lin. (lb/ft)	L	08-07-04	09-11-04	Тор		415			430	n\a
7	E55(i53)	Unf. Lin. (lb/ft)	L	12-07-04	13-11-04	Тор		70			112	n\a
8	-	Conc. Pt. (lbs)	L	01-04-03	01-04-03	Тор		363			492	n\a
9	-	Conc. Pt. (lbs)	L	01-04-03	01-04-03	Тор					-1	n\a
14	E55(i53)	Conc. Pt. (lbs)	L	11-02-08	11-02-08	Тор		917			822	n\a
15	E55(i53)	Conc. Pt. (lbs)	L	11-02-08	11-02-08	Тор					-43	n\a

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	11464 ft-lbs	24.5%	125%	2	08-09-04
End Shear	3371 lbs	25.3%	125%	2	12-08-08
Total Load Deflection	L/999 (0.116")	n∖a	n∖a	2	08-05-04
Live Load Deflection	L/999 (0.051")	n\a	n∖a	239	08-03-04
Max Defl.	0.116"	n∖a	n∖a	2	08-05-04
Span / Depth	8.7				
Dist. Load (B1)	659.75 lb/ft	2.1%	100%		
Conc. Load (B1)	855 lbs	9.3%	100%		

Bearing	Supports	Dim (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Wall/Plate	31-1/2" x 3-1/2"	6434 lbs	13.7%	7.8%	Spruce-Pine-Fir
B2	Wall/Plate	10-1/4" x 3-1/2"	3576 lbs	23.5%	13.3%	Spruce-Pine-Fir



Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB6(i165) (Flush Beam)



October 6, 2020 07:11:07

BC CALC® Member Report	Dry 1 span No cant.	October
Build 7493		
Job name:	File name:	2000813A.mmdl
Address:	Description:	1st Floor\Flush Beams\FB6(i165)
City, State, Zip:	Specifier:	
Customer:	Designer:	
Code reports: ESR-1040	Company:	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Connection Diagram: Full Length of Member



Calculated Side Load = 427.5 lb/ft Connectors are: 16d Common Nails

Disclosure

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Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB7(i168) (Flush Beam)



October 6, 2020 07:11:07

BC CALC® Membe	r Report	Dry 2 spans N	lo cant.	October 6
Build 7493				
Job name:		Fil	le name:	2000813A.mmdl
Address:		De	escription:	1st Floor\Flush Beams\FB7(i168)
City, State, Zip:		Sp	pecifier:	
Customer:		De	esigner:	
Code reports:	ESR-1040	Co	ompany:	

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

BC CALC® analysis is based on IBC 2012.

Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Connection Diagram: Full Length of Member



Calculated Side Load = 602.0 lb/ft Connectors are: 16d Common Nails

Disclosure

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	Boise Cascade	
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Boise Cascade BC CALC® Member F Build 7493 Job name: Address: City, State, Zip: Customer: Code reports:	Report	Double 1st F	loor\Flu	sh Beams Dry 1 spai	FB8(i170) No cant. File name Descriptio Specifier: Designer: Company	(Flus) (Flus) e: 2 on: 1	.0 310(h Beam) 2000813A st Floor\F	J SP .mmdl Flush Be	ams\FB8	Octobe 3(i170)	P/	ASSED 0 07:11:07
					$\begin{array}{c c} \hline \\ 2 \\ 1 \\ 0 \\ \hline \end{array} \\ \hline \\ 0 \\ \hline \\ \end{array} \\ \hline \\ \end{array}$						+ + + + + +	
<i>к</i> В1				11	-09-00							∕ B2
Reaction Summa	iry (Dow	n / Uplift) (lb	Total Ho S)	rizontal Pro	duct Length	= 11-09-	-00					
Bearing	Live	De	ad	S	now		Wind		Roo	f Live		
B1, 2"	213/0	18	75/0				375/945) 	136	4/31		
B2, 3-1/2"	218/0	24	70/0				744 / 128	37	196	2/14		
Load Summary												
							Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description		Load Type	Ref.	Start	End	Loc.	Live 100%	Dead 90%	Snow 115%	Wind 160%	Roof Live 125%	Tributary
TagDescription0Self-Weight		Load Type Unf. Lin. (lb/ft)	Ref.	Start 00-00-00	End 11-09-00	Loc. Top	Live 100%	Dead 90% 16	Snow 115%	Wind 160%	Roof Live 125%	Tributary
TagDescription0Self-Weight1E53(i60)		Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft)	Ref. L L	Start 00-00-00 00-00-00	End 11-09-00 11-09-00	Loc. Top Top	Live 100%	Dead 90% 16 64	Snow 115%	Wind 160%	Roof Live 125%	Tributary 00-00-00 n\a
TagDescription0Self-Weight1E53(i60)2FC1 Floor Mater	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft)	Ref. L L L	Start 00-00-00 00-00-00 00-00-00	End 11-09-00 11-09-00 11-09-00	Loc. Top Top Top	Live 100% 37	Dead 90% 16 64 9	Snow 115%	Wind 160%	Roof Live 125%	Tributary 00-00-00 n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Materia 3 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft)	Ref. L L L L	Start 00-00-00 00-00-00 00-00-00 00-09-12	End 11-09-00 11-09-00 11-09-00 02-01-12	Loc . Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70	Snow 115%	Wind 160%	Roof Live 125%	Tributary 00-00-00 n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Materia 3 E53(i60) 4 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft)	Ref. L L L L L	Start 00-00-00 00-00-00 00-09-12 04-09-12	End 11-09-00 11-09-00 11-09-00 02-01-12 06-01-12	Loc. Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428	Snow 115%	Wind 160%	Roof Live 125%	Tributary 00-00-00 n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft)	Ref. L L L L L	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12	Loc. Top Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428 424	Snow 115%	Wind 160%	Roof Live 125%	Tributary 00-00-00 n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Unf. Lin. (lb/ft)	Ref. L L L L L L	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 08-09-12	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12	Loc. Top Top Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428 424 420	Snow 115%	Wind 160%	Roof Live 125%	Tributary 00-00-00 n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft)	Ref. L L L L L L L	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 08-09-12 10-09-12	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00	<u>Loc.</u> Top Top Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428 424 440 579	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Materia 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60) 9 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Onf. Lin. (lb/ft) Conc. Pt. (lbs)	Ref. L L L L L L L L	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 08-09-12 10-09-12 03-06-08	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00 03-06-08	Loc. Top Top Top Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428 424 440 579 937	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573 846	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Materia 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60) 9 E53(i60) 10 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs)	Ref. L L L L L L L L L	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 08-09-12 10-09-12 03-06-08 03-06-08	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00 03-06-08 03-06-08	Loc. Top Top Top Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428 424 440 579 937	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573 846 -44	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60) 9 E53(i60) 10 E53(i60) 14 E53(i60)	ial	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs)	Ref. L L L L L L L L L	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 08-09-12 03-06-08 03-06-08 11-05-12	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00 03-06-08 03-06-08 11-05-12	Loc. Top Top Top Top Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428 424 440 579 937	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573 846 -44 -1	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60) 9 E53(i60) 10 E53(i60) 14 E53(i60) Controls Summa	ial ry Valu	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Conc. Pt. (lbs)	Ref. L L L L L L L L L &	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 03-06-08 03-06-08 11-05-12	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00 03-06-08 03-06-08 11-05-12	Loc. Top Top Top Top Top Top Top Top	Live 100% 37	Dead 90% 16 64 9 70 428 424 440 579 937	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573 846 -44 -1	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60) 9 E53(i60) 10 E53(i60) 14 E53(i60) Pos. Moment Pos. Moment	ial <u>ry valu</u> 1147	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Conc. Pt. (lbs)	Ref. L L L L L L L L L M Allowa 24.6%	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 03-06-08 03-06-08 11-05-12	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00 03-06-08 03-06-08 11-05-12 Duration	Loc. Top Top Top Top Top Top Top Top	Live 100% 37 <u>e Loca</u> 05-0	Dead 90% 16 64 9 70 428 424 440 579 937 tion 7-12	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573 846 -44 -1	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60) 9 E53(i60) 10 E53(i60) 14 E53(i60) Pos. Moment End Shear	ial <u>ry valu</u> 1147 3950	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Conc. Pt. (lbs) Conc. Pt. (lbs)	Ref. L L L L L L L L L L 24.6% 29.7%	Start 00-00-00 00-00-00 00-09-12 04-09-12 06-09-12 08-09-12 03-06-08 03-06-08 11-05-12 able I 1	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00 03-06-08 03-06-08 11-05-12 Duration 25% 25%	Loc. Top Top Top Top Top Top Top Top Top Zase	Live <u>100%</u> 37 <u>e Loca</u> 05-0 10-0	Dead 90% 16 64 9 70 428 424 440 579 937 tion 7-12 1-08	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573 846 -44 -1	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a n\a n\a
Tag Description 0 Self-Weight 1 E53(i60) 2 FC1 Floor Mater 3 E53(i60) 4 E53(i60) 5 E53(i60) 6 E53(i60) 7 E53(i60) 9 E53(i60) 10 E53(i60) 14 E53(i60) Pos. Moment End Shear Total Load Deflection	ial ry valu 1147 3950 L/99	Load Type Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Conc. Pt. (lbs) Conc. Pt. (lbs) 2 (0.112")	Ref. L L L L L L L L L 24.6% 29.7% n\a	Start 00-00-00 00-00-00 00-09-12 04-09-12 08-09-12 03-06-08 03-06-08 11-05-12 able I 1 1	End 11-09-00 11-09-00 02-01-12 06-01-12 08-01-12 10-01-12 11-09-00 03-06-08 03-06-08 11-05-12 Duration 25% 25%	Loc. Top Top Top Top Top Top Top Top	Live <u>100%</u> 37 <u>e Loca</u> 05-0 10-0 05-0 ²	Dead 90% 16 64 9 70 428 424 440 579 937 tion 7-12 1-08 9-12	Snow 115%	Wind 160%	Roof Live 125% 112 442 436 466 573 846 -44 -1	Tributary 00-00-00 n\a n\a n\a n\a n\a n\a n\a

n∖a

05-09-12

2

Span / D	epth	8.6				
Bearin	g Supports	Dim. (LxW)	Value	% Allow Support	% Allow Member	Material
B1	Hanger	2" x 3_1/2"	32/10 lbs	n\2	61 7%	Hanger
DI	riangei	2 × 3-1/2	52-0 103	II\a	01.7 /0	rianyei

n∖a

Cautions

Max Defl.

Hanger model Hanger was not found. Hanger has not been analyzed for adequate capacity.

0.112"



Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB8(i170) (Flush Beam)

Dry | 1 span | No cant.



October 6, 2020 07:11:07

Build 7493	•		
Job name:		File name:	2000813A.mmdl
Address:		Description:	1st Floor\Flush Beams\FB8(i170)
City, State, Zip:		Specifier:	
Customer:		Designer:	
Code reports:	ESR-1040	Company:	

Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Design meets arbitrary (0.75") Maximum live load deflection criteria. Calculations assume member is fully braced. Hanger Manufacturer: Unassigned BC CALC® analysis is based on IBC 2012. Wind loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Connection Diagram: Full Length of Member



-

Connectors are: 3-1/4 in. Pneumatic Gun Nails

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.



Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Design meets arbitrary (0.75") Maximum live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

 $\operatorname{BC}\operatorname{CALC}\nolimits \ensuremath{\mathbb{R}}$ analysis is based on IBC 2012.

Design based on Dry Service Condition.



Double 1-3/4" x 16" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\FB9(i171) (Flush Beam)



October 6, 2020 07:11:07

BC CALC® Member	^r Report	Dry 1 span No cant.	October 6,
Build 7493			
Job name:		File name:	2000813A.mmdl
Address:		Description:	1st Floor\Flush Beams\FB9(i171)
City, State, Zip:		Specifier:	
Customer:		Designer:	
Code reports:	ESR-1040	Company:	

Connection Diagram: Full Length of Member



Calculated Side Load = 482.0 lb/ft Connectors are: 16d Box Nails

Connection Diagrams: Concentrated Side Loads



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.




Bearing Post Required # 11211

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000813-2000813A	F1E	Floor Supported Gable	1	1	Job Reference (optional)
84 Components, Dunn, NC 2	8334			8.400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:46:41 2020 Page 1

ID:hjOMp88mnrQZrZEntywViFyWSr_-d2uHu0HE87zEgZXIa5qFVVG28oAu0OdTuzcLHTyWBEy



3x3 ||

1.5x4 ||

3x3 ||

			2-11-0 2-11-0					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.02 WB 0.03 Matrix-R	DEFL. vert(LL) n/ Vert(CT) n/ Horz(CT) 0.0	n (loc) a - a - 0 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 17 lb	GRIP 197/144 FT = 20%F, 115
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4	SP No.2 or 2x4 SPF No.2(flat) SP No.2 or 2x4 SPF No.2(flat)		BRACING- TOP CHORD	Struct purlin	ural wood s, except	I sheathin end vertio	g directly applied o cals.	r 2-11-0 oc

WEBS 2x4 SP No.3(flat) OTHERS 2x4 SP No.3(flat) BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 6=65/2-11-0 (min. 0-1-8), 4=79/2-11-0 (min. 0-1-8), 5=150/2-11-0 (min. 0-1-8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Gable requires continuous bottom chord bearing.

2) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

3) Gable studs spaced at 1-4-0 oc.

4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3")

nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Scale = 1:32.5



	3-0	-12		17-7-0				
	3-0	-12		14-6-4				
LOADIN TCLL TCDL BCLL BCDL	VG (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0 Plate Grip DOL 1. Lumber DOL 1. Rep Stress Incr YE Code IRC2015/TPI20	CO-0 CSI. .00 TC 0.62 .00 BC 0.43 ES WB 0.55 114 Matrix-S	DEFL. in Vert(LL) -0.09 Vert(CT) -0.12 Horz(CT) 0.02	(loc) l/defl 13-14 >999 13-14 >999 12 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 95 lb	GRIP 197/144 FT = 20%F, 119
LUMBE TOP CI BOT CI WEBS	R- HORD 2x4 \$ HORD 2x4 \$ 2x4 \$	SP No.2 or 2x4 SPF No.2(fl SP No.1(flat) SP No.3(flat)	lat)	BRACING- TOP CHORD BOT CHORD	Structural woo except end ve Rigid ceiling d	od sheathin rticals. irectly appl	g directly applied o ied or 6-0-0 oc bra	or 6-0-0 oc purlins, icing.
REACT	IONS. (Ib/s Max Max	ize) 19=-461/Mechanical, Uplift19=-575(LC 4) Grav18=1705(LC 1), 12=6	18=1705/0-3-8 (min. 0-1-8) 663(LC 4)	, 12=662/0-3-8 (min.	0-1-8)			
FORCE TOP CI BOT CI	i S. (lb) - Ma HORD 2-3 9-1 HORD 18-	ux. Comp./Max. Ten All fo =0/1465, 3-4=0/1469, 4-5= 0=-1613/0 19=-727/0, 15-16=0/1601,	orces 250 (lb) or less except -1042/0, 5-6=-1042/0, 6-7=- 14-15=0/1601, 13-14=0/160	when shown. 1601/0, 7-8=-1613/0, 8 1, 12-13=0/1061	8-9=-1613/0,			

WEBS 2-19=0/941, 2-18=-1058/0, 10-12=-1194/0, 4-18=-1679/0, 10-13=0/626, 4-16=0/1155, 9-13=-252/0, 6-16=-658/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 19=575.

 This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.



	<u> </u>										
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.78 WB 0.58 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.27 12-13 >782 480 Vert(CT) -0.38 12-13 >546 360 Horz(CT) 0.06 11 n/a n/a	PLATES GRIP MT20 197/144 M18SHS 244/190 Weight: 91 lb FT = 20%F, 115							
LUMBER-	x4 SP No.2 or 2x4 SPF No.2(flat)		BRACING- TOP CHORD Structural wood sheathing	directly applied or 5-3-4 oc purlins.							

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2(flat

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

TOP CHORD

Structural wood sheathing directly applied or 5-3-4 oc pur except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 17=953/Mechanical, 11=953/0-3-8 (min. 0-1-8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2693/0, 3-4=-2693/0, 4-5=-3365/0, 5-6=-3365/0, 6-7=-2709/0, 7-8=-2709/0, 8-9=-2709/0

BOT CHORD 16-17=0/1618, 15-16=0/3365, 14-15=0/3365, 13-14=0/3365, 12-13=0/3294, 11-12=0/1625

WEBS 9-11=-1830/0, 2-17=-1822/0, 9-12=0/1228, 2-16=0/1218, 6-12=-662/0, 4-16=-906/0, 6-13=-221/429

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) Refer to girder(s) for truss to truss connections.

4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



F	<u> </u>											
LOADING TCLL TCDL BCLL BCDL	G (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.72 BC 0.78 WB 0.58 Matrix-S	DEFL. in (loc) l/defl L/d Vert(LL) -0.27 12-13 >782 480 Vert(CT) -0.38 12-13 >546 360 Horz(CT) 0.06 11 n/a n/a	PLATES GRIP MT20 197/144 M18SHS 244/190 Weight: 91 lb FT = 20%F, 115							
LUMBER TOP CH	8- ORD 2x4 \$	SP No 2 or 2x4 SPF No 2(flat)		BRACING- TOP CHORD Structural wood sheathir	d directly applied or 5-3-4 oc purlins							

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2(f

 BOT CHORD
 2x4 SP No.1(flat)

 WEBS
 2x4 SP No.3(flat)

TOP CHORD

Structural wood sheathing directly applied or 5-3-4 oc purl except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 17=953/Mechanical, 11=953/0-3-8 (min. 0-1-8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2693/0, 3-4=-2693/0, 4-5=-3365/0, 5-6=-3365/0, 6-7=-2709/0, 7-8=-2709/0, 8-9=-2709/0

BOT CHORD 16-17=0/1618, 15-16=0/3365, 14-15=0/3365, 13-14=0/3365, 12-13=0/3294, 11-12=0/1625

WEBS 9-11=-1830/0, 2-17=-1822/0, 9-12=0/1228, 2-16=0/1218, 6-12=-662/0, 4-16=-906/0, 6-13=-221/429

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) Refer to girder(s) for truss to truss connections.

4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Job	Truss	Truss Type		Q	ty	Ply	Hicks Residence -	Milltown		
2000813-2000813A	F5	Floor		1		1	Job Reference (optional)		
84 Components, Dunn, NC	28334	1	ID:biO	Mn88mnrOZrZ	Fntvw	8.400 ViFvWS	s Apr 7 2020 MiTe r -SBEY931 2kzk	k Industries, Inc. Tue Oct	6 09:46:47 2020 Pa	ge 1 BEs
2-2-8			0-10-0		w	vii yvvo			ra_couogoryw	DLJ
									Scale: 3/8	8"=1'
							1.5x4			
3x3	3x6 =	1.5x4	3x3 = 1.5x4	II	3x3 =	=	3x6 FP=	3x7 =	3x3	
1	2	3 T1	4 5		6		7 8	9 T2	10	
041 041	B1				æ	B2			W2	1-4-0
17		16 15	14 13				12		11	
3x6 =		3x8 M18SHS FP= 4x8 =	1.5x4	=			4x8 =		3x6 =	

	17-3-8 17-3-8											
LOADING TCLL TCDL BCLL BCDL	i (psf) 40.0 10.0 0.0 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.62 BC 0.71 WB 0.57 Matrix-S	DEFL. in (loc) l/defl Vert(LL) -0.24 12-13 >854 Vert(CT) -0.34 12-13 >596 Horz(CT) 0.06 11 n/a	L/d 480 360 n/a	PLATES MT20 M18SHS Weight: 90 lb	GRIP 197/144 244/190 FT = 20%F, 115					
LUMBER TOP CHO)RD 2x4 \$	SP No.2 or 2x4 SPF No.2(flat)		BRACING- TOP CHORD Structural wo	od sheathin	g directly applied o	r 5-8-12 oc					

BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS

BOT CHORD

purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 17=937/Mechanical, 11=937/Mechanical

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2635/0, 3-4=-2635/0, 4-5=-3258/0, 5-6=-3258/0, 6-7=-2647/0, 7-8=-2647/0, 8-9=-2647/0

BOT CHORD

16-17=0/1587, 15-16=0/3258, 14-15=0/3258, 13-14=0/3258, 12-13=0/3203, 11-12=0/1593 9-11=-1794/0, 2-17=-1787/0, 9-12=0/1194, 2-16=0/1187, 6-12=-630/0, 4-16=-835/0, 6-13=-232/392 WEBS

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are MT20 plates unless otherwise indicated.

3) Refer to girder(s) for truss to truss connections.

4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.







	1	1-7-4 1-7-4						
Plate Offsets (X,Y)) [16:0-1-8,Edge]							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.43 BC 0.37 WB 0.34 Matrix-S	DEFL. in Vert(LL) -0.05 Vert(CT) -0.07 Horz(CT) 0.01	(loc) l/defl L/d 17-19 >999 480 19-20 >999 360 12 n/a n/a	PLATES GRIP MT20 197/144 Weight: 94 lb FT = 20%F, 115			
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4	SP No.2 or 2x4 SPF No.2(flat) SP No.1(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin except end verticals. Rigid ceiling directly appl	g directly applied or 6-0-0 oc purlins, ied or 6-0-0 oc bracing.			
REACTIONS. (Ib/s	size) 20=571/Mechanical, 12=182 (Uplift12=-33(LC 3)	/Mechanical, 15=112	2/0-3-8 (min. 0-1-8)					

Max Grav 20=584(LC 10), 12=263(LC 4), 15=1122(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1302/0, 3-4=-1302/0, 4-5=-1223/0, 5-6=-1223/0, 6-7=0/617, 7-8=0/617, 8-9=0/617, 9-10=-283/167

 BOT CHORD
 19-20=0/912, 18-19=0/1223, 17-18=0/1223, 16-17=0/1223, 15-16=0/646, 14-15=-167/283, 13-14=-167/283,
 12-13=-167/283

WEBS 2-20=-1027/0, 6-15=-1215/0, 2-19=0/441, 6-16=0/717, 10-12=-316/188, 9-15=-689/0, 4-19=-110/257

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12.
 This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

6) CAUTION, Do not erect truss backwards.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000813-2000813A	F7	Floor	3	1	Job Reference (optional)
84 Components, Dunn, NC 2	28334	ID:	hjOMp88mnrQZrZEnty	8.400 wViFyW\$	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:46:50 2020 Page 1 Srsmxhn5Ou1u6yFxj1cUUMMP8X_Q6KdPhoysIK5SyWBEp
2-2	2-8	0-7-8	1-5-0		0 ₁ 18

Scale = 1:21.6



⊢					11-9-0					
Plate Offs	ets (X,Y)	[13:0-1-8,0-0-12]			11-9-0					
LOADING TCLL TCDL BCLL BCDL	(psf) 40.0 10.0 0.0 5.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.00 1.00 YES Pl2014	CSI. TC 0.33 BC 0.52 WB 0.30 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.06 9-10 -0.09 11-12 0.02 8	l/defl >999 >999 n/a	L/d 480 360 n/a	PLATES MT20 Weight: 63 lb	GRIP 197/144 FT = 20%F, 115
LUMBER- TOP CHOF BOT CHOF WEBS	RD 2x4 SI RD 2x4 SI 2x4 SI	P No.2 or 2x4 SPF No P No.2 or 2x4 SPF No P No.3(flat)	0.2(flat) 0.2(flat)		BRACING TOP CHO BOT CHO	RD Struc excep RD Rigid	tural woo ot end ve ceiling di	d sheathing rticals. irectly appli	g directly applied or ed or 10-0-0 oc bra	r 6-0-0 oc purlins, acing.

REACTIONS. (lb/size) 12=632/Mechanical, 8=626/0-3-8 (min. 0-1-8)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1484/0, 3-4=-1484/0, 4-5=-1483/0, 5-6=-1483/0

 BOT CHORD
 11-12=0/1006, 10-11=0/1484, 9-10=0/1484, 8-9=0/1002

WEBS 6-8=-1124/0, 2-12=-1133/0, 6-9=0/544, 2-11=0/558

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

3) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

a) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
5) CAUTION, Do not erect truss backwards.

Job	Truss	Truss Type	Qty	Ply Hicks Residence	- Milltown	
2000813-2000813A	F8	Floor	7	1 Job Reference	(optional)	
84 Components, Dunn,	NC 28334		ID:hjOMp88mnrQZrZEnty	8.400 s Apr 7 2020 Mi /wViFyWSrGLcqP6QmJr	Tek Industries, Inc. Tue Oct DUX6PRcHc23_1m1Ce6	6 09:46:53 2020 Page 7qkEEeqX_imyWB
2-2	2-8		0-7-12			
						Scale = 1:2
3x3	3x6 =	1.5x4	3x3 = 3x3 =	1.5x4	3x6 =	3x3
1	2	3	4 5	6	7	8
						2 W1



	<u>14-10-12</u> 14-10-12											
	3 (psf) 40.0	SPACING- Plate Grip DOL	2-0-0 1.00	CSI. TC	0.40	DEFL. Vert(LL)	in -0.13	(loc) 12	l/defl >999	L/d 480	PLATES MT20	GRIP 197/144
BCLL BCDL	0.0 5.0	Rep Stress Incr Code IRC2015/T	YES PI2014	WB Matri	0.70 0.44 x-S	Horz(CT)	-0.18 0.04	9	>975 n/a	360 n/a	Weight: 79 lb	FT = 20%F, 119
LUMBER	-		a (11 - 1)			BRACING		•				

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2(flat)

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2(flat)

 WEBS
 2x4 SP No.3(flat)

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 14=806/Mechanical, 9=806/Mechanical

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD BOT CHORD

2-3=-2148/0, 3-4=-2148/0, 4-5=-2449/0, 5-6=-2148/0, 6-7=-2148/0 13-14=0/1334, 12-13=0/2449, 11-12=0/2449, 10-11=0/2449, 9-10=0/1334

7-9=-1502/0, 2-14=-1502/0, 7-10=0/922, 2-13=0/922, 6-10=-250/0, 3-13=-250/0, 5-10=-502/16, WEBS 4-13=-502/16

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

3) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000813-2000813A	F9	Floor	6	1	
					Job Reference (optional)
84 Components, Dunn, NC	28334			8.400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:46:54 2020 Page 1
		ID:h	njOMp88mnrQZrZEntyv	vViFyWS	rIYACdSRO47cOkZ0orKZIWFICG1RUZADNtUGXEDyŴBEI
				-	-

0-11-4

Scale = 1:28.0



	<u>15-2-4</u> 15-2-4										
LOADIN TCLL	G (psf) 40.0	SPACING- 2- Plate Grip DOL 1	0-0 CSI. .00 TC	0.44	DEFL. Vert(LL)	in -0.14	(loc) 12	l/defl >999	L/d 480	PLATES MT20	GRIP 197/144
TCDL BCLL	10.0	Lumber DOL 1 Rep Stress Incr Y	.00 BC ES WB	0.76 0.46	Vert(CT) Horz(CT)	-0.19 1 0.04	1-12 9	>924 n/a	360 n/a		
BCDL	5.0	Code IRC2015/TPI20	014 Matr	ix-S	()					Weight: 80 lb	FT = 20%F, 119
	R-		ilet)	i		- 0	Ctruct	ural waa	d aboathin	a directly opplied of	

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2(flat) BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2(flat) 2x4 SP No.3(flat) WEBS

2-2-8

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) 14=822/0-3-8 (min. 0-1-8), 9=822/Mechanical

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD

2-3=-2209/0, 3-4=-2209/0, 4-5=-2540/0, 5-6=-2209/0, 6-7=-2209/0 13-14=0/1365, 12-13=0/2540, 11-12=0/2540, 10-11=0/2540, 9-10=0/1365 BOT CHORD

7-9=-1537/0, 2-14=-1537/0, 7-10=0/956, 2-13=0/956, 6-10=-257/0, 3-13=-257/0, 5-10=-552/0, WEBS 4-13=-552/0

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

3) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000813-2000813A	F9E	Floor Supported Gable	1	1	Job Reference (optional)
84 Components, Dunn, NC 2	8334			8.400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:46:56 2020 Page 1

8.400 S Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:46:56 2020 Page 1 ID:hjOMp88mnrQZrZEntywViFyWSr_-hwly28Secks6zsABylbmbgNcNrHT1ALgKoleJ5yWBEj

Scale = 1:28.0



	15-2-4 15-2-4							
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.02 WB 0.03 Matrix-R	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l/defl L/d a - n/a 999 a - n/a 999 b 14 n/a n/a	PLATES MT20 Weight: 69 lb	GRIP 197/144 FT = 20%F, 119		
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2	x4 SP No.2 or 2x4 SPF No.2(flat) x4 SP No.2 or 2x4 SPF No.2(flat) x4 SP No.3(flat)	I	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing except end verticals. Rigid ceiling directly appli	g directly applied o	r 6-0-0 oc purlins, acing.		

OTHERS 2x4 SP No.3(flat)

REACTIONS. All bearings 15-2-4.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 26, 14, 25, 24, 23, 22, 21, 20, 19, 18, 17, 16, 15

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x4 MT20 unless otherwise indicated.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Job	Truss	Trus	s Туре		Qty	Ply	Hicks Residence - Milltown	
2000813-2000813A	F10	Floor			3	1	Job Reference (optional)	
84 Components, Dunn, N	NC 28334			ID:hjOMp	88mnrQZrZEr	8.400 htywViFyW) s Apr 7 2020 MiTek Industries, Inc. Tue Oct Sr97sKFUTHN2_yb0INWS6?8twi0FS	6 09:46:57 2020 Page 1mWmpZSVBrYyWB
2-2-8				1-0-	0			
								Scale = 1:36
							1.5x4	
3x3		1.5x4	3x3 =	1.5x4	1.5x4	3x3 =	= 3x6 FP=	3x3
1	2	3	4 T1	5	6	7	8 9 ¹⁰	11
		Ê.		Ě				
-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1								
	31				В	2		
18	17	16		15	14		13	

	<u> </u>											
LOADING (p TCLL 40 TCDL 10 BCLL 0	osf) 0.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	1-4-0 1.00 1.00 YES	CSI. TC BC WB	0.41 0.77 0.47	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.25 -0.34	(loc) 15 15 12	l/defl >940 >686	L/d 480 360	PLATES MT20	GRIP 197/144
BCDL 5	5.0	Code IRC2015/T	PI2014	Matri	x-S	1012(01)	0.00	12	n/a	n/a	Weight: 101 lb	FT = 20%F, 115
LUMBER-			2(flat)				- DD	Struct	uralwoo	d choothin	a directly applied or	6 0 0 oo purlipo

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2(flat)

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2(flat)

 WEBS
 2x4 SP No.3(flat)

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 18=712/Mechanical, 12=712/0-3-8 (min. 0-1-8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2094/0, 3-4=-2094/0, 4-5=-2860/0, 5-6=-2860/0, 6-7=-2860/0, 7-8=-2094/0, 8-9=-2094/0, 9-10=-2094/0

BOT CHORD	17-18=0/1228, 16-17=0/1228, 15-16=0/2628, 14-15=0/2860, 13-14=0/2628, 12-13=0/1228
WEBS	10-12=-1382/0, 2-18=-1382/0, 10-13=0/982, 2-16=0/982, 7-13=-604/0, 4-16=-604/0, 7-14=-48/459,
	4-15=-48/459

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x6 MT20 unless otherwise indicated.

3) Refer to girder(s) for truss to truss connections.

4) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000813-2000813A	F11	Floor	5	1	
					Job Reference (optional)
84 Components, Dunn, NC 2	28334			8.400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:46:59 2020 Page 1
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0-11-0



Scale = 1:23.7



I			12-10-0		
Plate Offsets (X,Y)	[11:0-1-8,Edge], [13:0-1-8,0-0-1	2]	12-10-0		
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.48 BC 0.76 WB 0.34 Matrix-S	DEFL. in Vert(LL) -0.10 Vert(CT) -0.14 Horz(CT) 0.03	(loc) l/defl L/d 9-10 >999 480 9-10 >999 360 8 n/a n/a	PLATES GRIP MT20 197/144 Weight: 67 lb FT = 20%F, 115
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4	SP No.2 or 2x4 SPF No.2(flat) SP No.2 or 2x4 SPF No.2(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin except end verticals. Rigid ceiling directly appl	g directly applied or 6-0-0 oc purlins, ied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 12=692/Mechanical, 8=686/0-3-8 (min. 0-1-8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1744/0, 3-4=-1744/0, 4-5=-1729/0, 5-6=-1729/0 BOT CHORD 11-12=0/1122, 10-11=0/1744, 9-10=0/1744, 8-9=0/1114

6-8=-1249/0, 2-12=-1263/0, 6-9=0/697, 2-11=0/721, 5-9=-270/0, 4-9=-271/182 WEBS

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

2-2-8

-

3) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

a) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
5) CAUTION, Do not erect truss backwards.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000813-2000813A	F11E	Floor Supported Gable	1	1	Job Reference (optional)
84 Components, Dunn, NC 2	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:47:01 2020 Page 1				

8.400 S Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:47:01 2020 Page 1 ID:hjOMp88mnrQZrZEntywViFyWSr_-1u5r5rWnRHVO4e38IIBxlj5T6s_hiRaPU4TP_JyWBEe

0₁-8

Scale = 1:23.6



L			12-10-0		
			12-10-0		
Plate Offsets (X,Y) [23:0-1-8,0-0-12]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.02 WB 0.03 Matrix-R	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	l (loc) l/defl L/d - n/a 999 - n/a 999 12 n/a n/a	PLATES GRIP MT20 197/144 Weight: 59 lb FT = 20%F, 119
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4 OTHERS 2x4	SP No.2 or 2x4 SPF No.2(flat) SP No.2 or 2x4 SPF No.2(flat) SP No.3(flat) SP No.3(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing except end verticals. Rigid ceiling directly appli	directly applied or 6-0-0 oc purlins, ed or 10-0-0 oc bracing.

REACTIONS. All bearings 12-10-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) All plates are 1.5x4 MT20 unless otherwise indicated.

2) Gable requires continuous bottom chord bearing.

3) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

4) Gable studs spaced at 1-4-0 oc.

5) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
7) CAUTION, Do not erect truss backwards.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000813-2000813A	F12	Floor	3	1	Job Reference (optional)
84 Components, Dunn, NC 2	8334			8,400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 09:47:02 2020 Page 1

0-11-0

2-2-8

-

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Scale = 1:23.6



L			12-10-0		
			12-10-0		
Plate Offsets (X,Y) [11:0-1-8,Edge]				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.48 BC 0.76 WB 0.34 Matrix-S	DEFL. in Vert(LL) -0.10 Vert(CT) -0.14 Horz(CT) 0.03	(loc) l/defl L/d 9-10 >999 480 9-10 >999 360 8 n/a n/a	PLATES GRIP MT20 197/144 Weight: 68 lb FT = 20%F, 115
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4	SP No.2 or 2x4 SPF No.2(flat) SP No.2 or 2x4 SPF No.2(flat) SP No.3(flat)		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing except end verticals. Rigid ceiling directly appli	j directly applied or 6-0-0 oc purlins, ed or 10-0-0 oc bracing.

REACTIONS. (lb/size) 12=692/Mechanical, 8=692/0-3-8 (min. 0-1-8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1745/0, 3-4=-1745/0, 4-5=-1729/0, 5-6=-1729/0 BOT CHORD 11-12=0/1122, 10-11=0/1745, 9-10=0/1745, 8-9=0/1116

6-8=-1257/0, 2-12=-1263/0, 6-9=0/694, 2-11=0/721, 5-9=-269/0, 4-9=-271/181 WEBS

NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

3) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000811-2000811A	AH	Roof Special Girder	1	2	Job Reference (optional)
84 Components, Dunn, NC 2	8334	ID:hjOMp88	mnrQZrZ	8.400 EntywVil	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:55:25 2020 Page 2 FyWSrrKi4Ez0rvkM5wYsMdXIP90aoe6xGiGIwI0u3o7yW5rG

NOTES-

- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 120 lb down and 162 lb up at 29-5-4 on top chord, and 675 lb down and 225 lb up at 27-11-8, 45 lb down and 40 lb up at 29-5-4, 169 lb down and 143 lb up at 31-5-4, and 120 lb down and 102 lb up at 33-5-4, and 120 lb down and 89 lb up at 35-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-60, 2-5=-60, 5-7=-60, 7-9=-60, 9-10=-60, 11-19=-20 Concentrated Loads (lb) Vert: 12=-120(F) 20=-40(F) 22=-675(F) 23=-23(F) 24=-120(F) 25=-120(F)



	9-4-2	18-4-12		28-0-12		37-6-0		
	9-4-2	9-0-10		9-8-0	I	9-5-4	1	
Plate Offsets (X,	Y) [2:0-3-0,Edge], [4:0-4-0,Edge], [6:0-3-0,Edge], [14:0-3	3-0,0-1-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014	CSI. TC 0.81 BC 1.00 WB 0.83 Matrix-MS	DEFL. Vert(LL) -0 Vert(CT) -0 Horz(CT) 0	in (loc) l/c .32 12-14 >9 .57 12-14 >7 .10 11	defl L/d 999 240 787 180 n/a n/a	PLATES MT20 Weight: 249 lb	GRIP 197/144 DFT = 20%	
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* T2,T3: 2x6 SP No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 *Except* B2: 2x4 SP No.1 WEBS 2x4 SP No.3 BRACING- TOP CHORD Structural wood sheathing directly applied or 3-7-4 oc pur except end verticals, and 2-0-0 oc purlins (3-9-5 max.): 6- BOT CHORD B2: 2x4 SP No.1 WEBS 2x4 SP No.3 BOT CHORD WEBS BOT CHORD WEBS BOT CHORD WEBS BOT CHORD WEBS I Row at midpt 6-14, 7-12, 3-17, 9-11 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation quide.							r 3-7-4 oc purlins, -9-5 max.): 6-8. acing. 9-11 quired cross n accordance	
REACTIONS. (Ib Ma Ma Ma	REACTIONS. (lb/size) 17=1550/0-3-8 (min. 0-2-8), 11=1487/Mechanical Max Horz 17=272(LC 11) Max Uplift17=-149(LC 12), 11=-208(LC 13) Max Grav 17=1606(LC 19), 11=1487(LC 1)							
FORCES. (Ib) - M TOP CHORD 2	Max. Comp./Max. Ten All forces 25 -3=-590/255, 3-4=-1971/444, 4-5=-11 7-8=-1515/369, 8-9=-1891/399, 9-10=	0 (lb) or less except v 865/473, 5-6=-2534/6 282/92, 2-17=-543/2	vhen shown. 13, 6-7=-2102/456 242	б,				
BOT CHORD 1 1	7-18=-231/1758, 18-19=-231/1758, 1 5-20=-94/1427, 15-21=-94/1427, 14- 3-22=-260/1914, 13-23=-260/1914, 1	6-19=-231/1758, 16- 21=-94/1427, 14-22= 2-23=-260/1914, 11-	20=-94/1427, -260/1914, 12=-262/1505					
WEBS 3-16=-259/273, 5-16=-149/581, 5-14=-372/1816, 6-14=-1584/437, 7-14=-36/393, 7-12=-707/172, 8-12=-104/793, 3-17=-1618/154, 9-11=-1782/337								
NOTES- 1) Unbalanced ro 2) Wind: ASCE 7- Enclosed; MW vertical left anc grip DOL=1.60 3) Provide adequ: 4) This truss has	of live loads have been considered for 10; Vult=130mph (3-second gust) Va FRS (envelope) gable end zone and I right exposed;C-C for members and ate drainage to prevent water pondin been designed for a 10.0 psf bottom	or this design. asd=103mph; TCDL= C-C Exterior(2) zone I forces & MWFRS fo g. chord live load nonco	6.0psf; BCDL=6.0 ; cantilever left and r reactions shown; oncurrent with any	psf; h=30ft; Ca J right exposed Lumber DOL other live load	at. II; Exp B; d ; end =1.60 plate ds.			
5) * This trues have	s been designed for a live load of 20	Onst on the bottom ch	ord in all areas w	horo a roctano	10 3-6-0 tall			

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tal by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=208.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000811-2000811A	AHA1	Roof Special Girder	1	2	Job Reference (optional)
84 Components, Dunn, NC 2	8334	ID:hjOMp88mr	nrQZrZEr	8.400 htywViFy	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:55:31 2020 Page 2 WSrgU4LV15cVa7EeTJV_ospPHppgXxf6x6pgyLN0nyW5rA

NOTES-

 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 120 lb down and 162 lb up at 27-11-4, and 120 lb down and 162 lb up at 29-5-4 on top chord, and 1022 lb down and 265 lb up at 27-1-0, 45 lb down and 40 lb up at 27-11-4, 45 lb down and 40 lb up at 29-5-4, 169 lb down and 143 lb up at 31-5-4, and 120 lb down and 102 lb up at 33-5-4, and 120 lb down and 89 lb up at 35-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 12=-60, 2-5=-60, 5-7=-60, 7-9=-60, 9-10=-60, 10-11=-60, 12-20=-20 Concentrated Loads (lb) Vert: 13=-120(B) 21=-40(B) 22=-40(B) 25=-1022(B) 26=-23(B) 27=-23(B) 28=-120(B) 29=-120(B)



	8-10-14	17-6-4	22-9-8	28-0-12	37-6-0		
Plate Offsets (X	(,Y) [2:0-3-0,Edge], [4:0-4-0,Edge], [10:0-2-0,0-1-12]	5-5-4	5-5-4	9-0-4		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 * Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.76 BC 0.92 WB 0.79 Matrix-MS	DEFL. in Vert(LL) -0.25 Vert(CT) -0.42 Horz(CT) 0.10	(loc) l/defl L/d 16-18 >999 240 16-18 >999 180 12 n/a n/a	PLATES GRIP MT20 197/144 Weight: 258 lb FT = 20%		
LUMBER- TOP CHORD 2) T2 BOT CHORD 2) WEBS 2)	44 SP No.2 or 2x4 SPF No.2 *Except* 2,T3: 2x6 SP No.2 44 SP No.2 or 2x4 SPF No.2 44 SP No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing purlins, except end vertic max.): 6-8. Rigid ceiling directly appli 2-2-0 oc bracing: 18-19. 1 Row at midpt	g directly applied or 3-8-12 oc cals, and 2-0-0 oc purlins (3-10-5 ied or 10-0-0 oc bracing, Except: 6-16, 6-14, 3-19, 9-12		
REACTIONS. (Ib/size) 19=1550/0-3-8 (min. 0-2-8), 12=1550/0-3-8 (min. 0-2-7) Max Horz 19=270(LC 11) Max Uplift19=-146(LC 12), 12=-234(LC 13) Max Grav 19=1598(LC 19), 12=1550(LC 1)							
FORCES. (Ib) - TOP CHORD	Max. Comp./Max. Ten All forces 25 2-3=-550/248, 3-4=-1977/445, 4-5=-18 7-8=-1945/467, 8-9=-1870/405, 9-10=	0 (lb) or less except w 875/474, 5-6=-2510/6 =-349/120, 2-19=-517,	/hen shown. 14, 6-7=-1944/465, /236,				
BOT CHORD	10-12=-355/144 3OT CHORD 19-20=-195/1758, 20-21=-195/1758, 18-21=-195/1758, 18-22=-67/1444, 17-22=-67/1444, 17-23=-67/1444, 16-23=-67/1444, 16-24=-219/2098, 15-24=-219/2098, 14-15=-219/2098, 14-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-131/1510, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13-25=-130/1500, 13						
WEBS	3-18=-241/265, 5-18=-146/559, 5-16= 6-14=-285/59, 7-14=-383/168, 8-14=- 9-12=-1694/311	-385/1827, 6-16=-145 150/736, 8-13=0/280,	57/444, 3-19=-1658/159,				
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 							

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 19 and 12. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 21 and 13. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



¹⁰⁾ Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 9. This connection is for uplift only and does not consider lateral forces.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



	2-0-12 4-0-12	6-0-12 6 ₁ 5 ₁ 4	<u>11-4-8</u> 4-11-4	<u>16-5-8</u> 5-1-0	21	-4-12	21-9-423-9-	$\frac{-4}{0}$ + $\frac{25 \cdot 9 \cdot 4}{2 \cdot 0 \cdot 0}$ + $\frac{27 \cdot 1}{2 \cdot 0}$	10-0 -12
Plate Offsets (2	(,Y) [2:0-1-0,0-1-1	2], [3:0-4-4,0-2-4],	[6:0-4-4,0-2-4], [7:0-	-1-0,0-1-12]			010200		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	* Rep Stress Code IRC2	2-0-0 DOL 1.15 IL 1.15 Incr NO 015/TPI2014	CSI. TC 0.43 BC 0.25 WB 0.26 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.08 11-13 -0.08 11-13 0.02 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 382 I	GRIP 197/144 b FT = 20%
BRACING- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purling except end verticals, and 2-0-0 oc purling (6-0-0 max.): 3-6. BOT CHORD							r 6-0-0 oc purlins, -0-0 max.): 3-6. acing.		
REACTIONS. (lb/size) 15=1745/0-3-8 (min. 0-1-8), 9=1744/0-3-8 (min. 0-1-8) Max Horz 15=-152(LC 10) Max Uplift15=-922(LC 12), 9=-921(LC 13)									
F ORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2114/1331, 3-16=-2192/1577, 16-17=-2192/1577, 4-17=-2192/1577, 4-18=-2192/1577, 18-19=-2192/1577, 19-20=-2192/1577, 5-20=-2192/1577, 5-21=-2195/1579, 21-22=-2195/1579,									

6-22=-2195/1579, 6-7=-2112/1329, 2-15=-1555/962, 7-9=-1554/960 BOT CHORD 15-23=-313/535, 23-24=-313/535, 14-24=-313/535, 14-25=-1038/1704, 25-26=-1038/1704, 13-26=-1038/1704, 13-27=-1407/2241, 12-27=-1407/2241, 12-28=-1407/2241, 11-28=-1407/2241, 11-29=-975/1650, 29-30=-975/1650, 10-31=-261/445, 31-32=-261/445, 9-32=-261/445 WEBS 3-14=-102/280, 3-13=-663/852, 4-13=-434/442, 5-11=-456/463, 6-11=-668/858, 6-10=-98/277,

2-14=-924/1404, 7-10=-927/1402

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows:
 - Top chords connected as follows: 2x4 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
- Webs connected as follows: 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 15, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 15 and 9. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000811-2000811A	вн	Hip Girder	1	2	Job Reference (optional)
84 Components, Dunn, NC 2	28334			8.400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:55:55 2020 Page 2

8.400's Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:55:55 2020 Page 2 ID:hjOMp88mnrQZrZEntywViFyWSr_-z6WPXEOgry8qW997OqHRNKpxpCFOkuRLBh9erOyW5qo

NOTES-

12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 120 lb down and 162 lb up at 8-0-12, 120 lb down and 162 lb up at 10-0-12, 120 lb down and 162 lb up at 12-0-12, 120 lb down and 162 lb up at 13-9-4, 120 lb down and 162 lb up at 15-9-4, and 120 lb down and 162 lb up at 17-9-4, and 120 lb down and 162 lb up at 19-9-4 on top chord, and 120 lb down and 89 lb up at 2-0-12, 120 lb down and 102 lb up at 4-0-12, 169 lb down and 143 lb up at 6-0-12, 45 lb down and 40 lb up at 8-0-12, 45 lb down and 40 lb up at 12-0-12, 45 lb down and 40 lb up at 13-9-4, 45 lb down and 40 lb up at 15-9-4, 45 lb down and 40 lb up at 17-9-4, 45 lb down and 40 lb up at 12-9-4, and 120 lb down and 102 lb up at 12-9-4, and 120 lb down and 89 lb up at 25-9-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-6=-60, 6-7=-60, 7-8=-60, 9-15=-20

Concentrated Loads (lb)

Vert: 12=-23(F) 14=-120(F) 10=-120(F) 16=-40(F) 17=-40(F) 18=-40(F) 19=-40(F) 20=-40(F) 21=-40(F) 22=-40(F) 23=-120(F) 24=-120(F) 25=-23(F) 26=-23(F) 27=-23(F) 28=-23(F) 29=-23(F) 30=-23(F) 31=-120(F) 32=-120(F)



	9-5-4		18-4-12		27-10-0		
Diata Offacta ()	9-5-4		8-11-8		9-5-4		
Plate Olisets ((, f) [2.0-1-0,0-1-6], [4.0-4-0,0-1-9],	[6.0-4-0,0-1-9], [6.0-1-0	,0-1-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 * Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.48 BC 0.84 WB 0.84 Matrix-MS	DEFL. in Vert(LL) -0.30 Vert(CT) -0.40 Horz(CT) 0.05	(loc) l/defl L/d 11-13 >999 240 11-13 >830 180 10 n/a n/a	PLATES GRI MT20 197 Weight: 173 lb F	i P 7/144 T = 20%	
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2	x4 SP No.2 or 2x4 SPF No.2 x4 SP No.2 or 2x4 SPF No.2 x4 SP No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathin purlins, except end vertio max.): 4-6. Rigid ceiling directly appl 1 Row at midpt MiTek recommends that bracing be installed dur	g directly applied or 4-10 cals, and 2-0-0 oc purlins ied or 10-0-0 oc bracing. 5-13, 5-11 t Stabilizers and required ing truss erection in acc	-15 oc ; (5-8-9 d cross	
REACTIONS. (REACTIONS. (lb/size) 14=1163/0-3-8 (min. 0-1-13), 10=1163/0-3-8 (min. 0-1-13) Max Horz 14=-202(LC 10) Max Uplift14=-108(LC 12), 10=-108(LC 13)						
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-315/119, 3-4=-1264/298, 4-5=-986/286, 5-6=-986/286, 6-7=-1264/298, 7-8=-315/119, 2-14=-322/142, 8-10=-322/142 BOT CHORD 13-14=-155/1050, 12-13=-92/1089, 12-15=-92/1089, 11-15=-92/1089, 10-11=-151/1050 WEBS 4-13=-50/422, 5-13=-269/161, 5-11=-269/160, 6-11=-50/422, 3-14=-1154/211, 7-10=-1154/211							
NOTES-	and live loads have been considered	for this design					

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17 and 11. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=110.

7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13. This connection is for uplift only and does not consider lateral forces.

 This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



H	2-0-12 4-0-12 6-0-12 6-5-4 2-0-12 2-0-0 2-0-0 0-4-8	<u>11-1-0</u> 4-7-12	<u>15-10-8</u> 4-9-8	20-6-4 4-7-12	20-10-12 0-4-8 2-0-0	24-10-12 26-	11-8 D-12
Plate Offsets (X,Y)	[2:0-1-0,0-1-12], [3:0-4-4,0-2-4]	[6:0-4-4,0-2-4]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.43 BC 0.24 WB 0.27 Matrix-MS	DEFL. Vert(LL) 0. Vert(CT) -0. Horz(CT) 0.	in (loc) l/defl 07 10-12 >999 08 10-12 >999 02 8 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 370 I	GRIP 197/144 lb FT = 20%
LUMBER- BRACING- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD 2x6 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2 TOP CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 BOT CHORD							or 6-0-0 oc purlins, 5-0-0 max.): 3-6. racing.
REACTIONS. (Ib/size) 14=1712/0-3-8 (min. 0-1-8), 8=1646/0-3-8 (min. 0-1-8) Max Horz 14=146(LC 11) Max Uplift14=-925(LC 12), 8=-897(LC 13)							
CORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.							

22-10-12

- TOP CHORD 2-3=-2064/1324, 3-15=-2110/1554, 15-16=-2110/1554, 4-16=-2110/1554, 4-17=-2110/1554, 17-18=-2110/1554, 18-19=-2110/1554, 5-19=-2110/1554, 5-20=-2103/1544, 20-21=-2103/1544, 6-21=-2103/1544, 6-7=-2057/1310, 2-14=-1522/957, 7-8=-1455/906
- BOT CHORD 14-22=-327/528, 22-23=-327/528, 13-23=-327/528, 13-24=-1037/1670, 24-25=-1037/1670, 12-25=-1037/1670, 12-26=-1404/2143, 11-26=-1404/2143, 11-27=-1404/2143, 10-27=-1404/2143, 10-28=-1004/1612, 28-29=-1004/1612, 9-29=-1004/1612, 9-30=-206/348, 30-31=-206/348, 8-31=-206/348 WEBS 3-13=-101/277, 3-12=-660/818, 4-12=-429/456, 5-10=-440/466, 6-10=-658/813, 6-9=-88/271, 2-13=-915/1370, 7-9=-893/1389

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows:
- Top chords connected as follows: 2x4 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
- Webs connected as follows: 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasl=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 14, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Two H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 8. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
| Job | Truss | Truss Type | Qty | Ply | Hicks Residence - Milltown |
|---------------------------|-------|------------|-----|-------|--------------------------------------------------------------------|
| 2000811-2000811A | СН | Hip Girder | 1 | 2 | Job Reference (optional) |
| 84 Components, Dunn, NC 2 | 8334 | | | 8.400 | s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:56:13 2020 Page 2 |

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NOTES-

12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 120 lb down and 162 lb up at 8-0-12, 120 lb down and 162 lb up at 10-0-12, 120 lb down and 162 lb up at 12-0-12, 120 lb down and 162 lb up at 12-10-12, 120 lb down and 162 lb up at 14-10-12, and 120 lb down and 162 lb up at 16-10-12, and 120 lb down and 162 lb up at 18-10-12 on top chord, and 120 lb down and 89 lb up at 2-0-12, 120 lb down and 102 Ib up at 4-0-12, 169 lb down and 143 lb up at 6-0-12, 45 lb down and 40 lb up at 8-0-12, 45 lb down and 40 lb up at 10-0-12, 45 lb down and 40 lb up at 12-0-12, 45 lb down and 40 lb up at 12-10-12, 45 lb down and 40 lb up at 14-10-12, 45 lb down and 40 lb up at 16-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 lb down and 40 lb up at 18-10-12, 45 chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-6=-60, 6-7=-60, 8-14=-20

Concentrated Loads (lb)

Vert: 11=-23(B) 13=-120(B) 9=-120(B) 15=-40(B) 15=-40(B) 17=-40(B) 18=-40(B) 19=-40(B) 20=-40(B) 21=-40(B) 22=-120(B) 23=-120(B) 24=-23(B) 25=-23(B) 26=-23(B) 27=-23(B) 28=-23(B) 29=-23(B) 30=-120(B) 31=-120(B)



	<u>9-5-4</u> 9-5-4		<u>17-6-4</u> 8-1-0	2	6-11-8 9-5-4	
Plate Offsets (2	(,Y) [2:0-1-0,0-1-8], [4:0-4-0,0-1-9],	6:0-4-0,0-1-9], [8:0-1	-0,0-1-8]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 * Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.50 BC 0.76 WB 0.80 Matrix-MS	DEFL. in Vert(LL) -0.20 Vert(CT) -0.35 Horz(CT) 0.04	(loc) l/defl L/d 111-13 >999 240 13-14 >904 180 10 n/a n/a	PLATES GRI MT20 197 Weight: 169 lb F	Ρ /144 Γ = 20%
LUMBER- BRACING- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.						
REACTIONS.	lb/size) 14=1128/0-3-8 (min. 0-1-12) /lax Horz 14=202(LC 11) /lax Uplift14=-108(LC 12), 10=-108(LC	, 10=1128/0-3-8 (mi ; 13)	n. 0-1-12)	bracing be installed duri with Stabilizer Installatio	ng truss erection, in acconn guide.	ordance

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-317/115, 3-4=-1206/289, 4-5=-937/278, 5-6=-937/278, 6-7=-1206/289, 7-8=-317/115, 2-14=-322/139, 8-10=-322/140
- BOT CHORD
 13-14=-149/1009, 12-13=-76/1021, 12-15=-76/1021, 15-16=-76/1021, 11-16=-76/1021, 10-11=-144/1009

 WEBS
 4-13=-49/400, 6-11=-50/400, 3-14=-1098/206, 7-10=-1098/205

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall
- by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14 and 10. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17 and 11. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

 Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



		0-11-4	1-1-9	2-9-8	5-6-6	
		0-11-4	0-2-5	1-7-15	2-8-14	1
Plate Offsets (X,Y)-	- [8:0-3-12,0-1-8]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/T	2-0-0 1.15 1.15 NO PI2014	CSI. TC 0.37 BC 0.37 WB 0.04 Matrix-MS	DEFL. ir Vert(LL) -0.04 Vert(CT) 0.04 Horz(CT) -0.00	i (loc) I/defl L/d 7-8 >999 240 7-8 >999 180 7 n/a n/a	PLATES GRIP MT20 197/144 Weight: 21 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	SP No.2 or 2x4 SPF No SP No.2 or 2x4 SPF No SP No.3	0.2 0.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing except end verticals. Rigid ceiling directly applie	directly applied or 5-6-6 oc purlins, ed or 10-0-0 oc bracing.

REACTIONS. (lb/size) 7=79/Mechanical, 8=290/0-4-10 (min. 0-1-8) Max Horz 8=53(LC 11) Max Uplift7=-17(LC 12), 8=-128(LC 8) Max Grav 7=89(LC 43), 8=290(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 81 lb down and 94 lb up at 2-9-8, and 81 lb down and 94 lb up at 2-9-8 on top chord, and 3 lb down and 63 lb up at 2-9-8, and 3 lb down and 63 lb up at 2-9-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf)
 - Vert: 1-4=-60, 4-5=-20, 6-9=-20 Concentrated Loads (lb)
 - Vert: 10=64(F=32, B=32) 11=70(F=35, B=35)



 BOT CHORD
 13-14=-1/3//05, 14-15=-1/3//05, 12-15=-1/3//05, 11-12=-36/587, 11-16=-36/587, 10-16=-36/587, 10-17=-23/577, 17-18=-23/577, 9-18=-23/577

 WEBS
 4-12=-65/317, 5-10=-92/386, 3-13=-806/76, 6-9=-805/76

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13 and 9. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top an chord.



One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s)
 This connection is for uplift only and does not consider lateral forces.

11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Scale = 1:57.1



			<u>15-9-0</u> 15-9-0					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0 Plate Grip DOL 1. Lumber DOL 1. Rep Stress Incr YI Code IRC2015/TPI20	0-0 CSI. 15 TC 0.30 15 BC 0.17 ES WB 0.20 14 Matrix-R	DEFL. in Vert(LL) -0.01 Vert(CT) -0.01 Horz(CT) -0.00	(loc) l/defl L/d 11 n/r 120 11 n/r 90 12 n/a n/a	PLATES GRIP MT20 197/144 Weight: 124 lb FT = 20%			
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD WEBS	 Structural wood sheathing directly applied or 6-0-0 except end verticals. ORD Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 6-16 				
				MiTek recommends that bracing be installed dur with Stabilizer Installation	It Stabilizers and required cross ing truss erection, in accordance on guide.			

REACTIONS. All bearings 15-9-0.

6-16=-454/274

- Max Uplift All uplift 100 lb or less at joint(s) 18, 15, 14 except 20=-198(LC 8), 12=-184(LC 9), 17=-100(LC 12), 19=-193(LC 12), 13=-190(LC 13) Max Grav All reactions 250 lb or less at joint(s) 17, 18, 15, 14 except 20=271(LC 20), 12=260(LC 19),
 - 16=386(LC 13), 19=280(LC 10), 13=271(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 4-5=-203/282, 5-6=-283/373, 6-7=-283/373, 7-8=-203/281

WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- a) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 b) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 20, 12, 17, 18, 19, 15, 14, and 13. This connection is for uplift only and does not consider lateral forces.
- 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

⁽lb) - Max Horz 20=279(LC 11)



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.18 BC 0.10 WB 0.25 Matrix-R	DEFL. in Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) -0.00	(loc) l/defl 9 n/r 9 n/r 10 n/a	L/d 120 90 n/a	PLATES MT20 Weight: 77 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4	4 SP No.2 or 2x4 SPF No.2 4 SP No.2 or 2x4 SPF No.2 4 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wo except end v Rigid ceiling	ood sheathin erticals. directly appl	g directly applied c ied or 6-0-0 oc bra	or 6-0-0 oc purlins, cing.
OTHERS 2x4	4 SP No.3			MiTek reco bracing be with Stabili	mmends tha installed dur zer Installatio	t Stabilizers and re ing truss erection, on quide.	equired cross in accordance

REACTIONS. All bearings 11-0-0.

(lb) - Max Horz 16=-203(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 14, 12 except 16=-140(LC 8), 10=-128(LC 9), 15=-162(LC 12), 11=-160(LC 13) Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 5-13=-301/154

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16, 10, 14, 15, 12, and 11. This connection is for uplift only and does not consider lateral forces.
- 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Millto	own
2000811-2000811A	GR1	Common Girder	1	3	Ich Reference (ontio	nal)
84 Components, Dunn, I	NC 28334	1		8.400	s Apr 7 2020 MiTek Ind	lustries, Inc. Tue Oct 6 15:56:31 2020 Page 1
		0-1 <u>-12 5-6-0</u>		11-0	-0	
		0-1-12 5-4-4	'	5-6-	0	
			4x4			Scale = 1:41.7
	T		2 1			
		12 00 12				
		12.00 12				
				$\backslash /$		
	ې ب	r1		M	\backslash	
	6-11		W1			
		. //			3x6 \\	
	3x6	1			3	
	Ī					I
	မု	W3 W2		W2		မှ
			BI			-
		7	⁸ 5 ⁹	10	11 🕅	
		4x6 =	8x8 =		4 4x6 =	
		2-2-0 4-1-4	5-6-0 6-1-4 8-1	-4	10-1-4 .11-0-0	
Plate Offecte (V.V)	. [4:Edgo 0-2 0] [5:0 4 0	2-2-0 1-11-4	1-4-12 0-7-4 2-0)-0	2-0-0 0-10-12	
Plate Ulisets (X,Y)-	- [4.⊏uge,∪-∠-∪], [5:∪-4-0,	<u>U-4-12j</u>				
LOADING (psf)	SPACING- 2-0 Plate Grip DOI 1	0-0 CSI.	DEFL. ii	n (loc)	I/defI L/d	PLATES GRIP
TCDL 10.0	Lumber DOL 1	15 BC 0.43	Vert(CT) -0.0	9 5-6	>999 180	131/144
BCLL 0.0 * BCDL 10.0	Rep Stress Incr I Code IRC2015/TPI20	NO WB 0.60 Matrix-MS	Horz(CT) 0.00	0 4	n/a n/a	Weight: 230 lb FT = 20%
TOP CHORD 2x4 S	P No.2 or 2x4 SPF No.2		TOP CHORD	Struct	ural wood sheathin	g directly applied or 6-0-0 oc purlins.
BOT CHORD 2x6 S	P DSS			excep	ot end verticals.	light or 10-0-0 oc broging
VVLDO 2x4 S	0.001		DOT GHUKD	ixigid	cening unectly appl	neu or ro-o-o oc braciliy.
REACTIONS. (Ib/siz	ze) 6=4433/0-3-8 (min. (Horz 6=-174(I C 31)	0-2-5), 4=4687/0-3-8 (min. 0	-2-7)			
Max	Uplift6=-928(LC 13), 4=-63	38(LC 12)				
FORCES. (Ib) - Max	x. Comp./Max. Ten All fo	prces 250 (lb) or less except	when shown.			
TOP CHORD 1-2	=-3409/648, 2-3=-3408/64	7, 1-6=-2911/519, 3-4=-2889	0/565		405	
БОТСНОКО 6-7= 4-11	=-361/652, 7-8=-361/652, 1=-117/495	5-0=-301/052, 5-9=-11//495,	, 9-10=-117/495, 10-1	11=-117/	490,	
WEBS 2-5=	=-736/4354, 1-5=-294/176	7, 3-5=-436/1896				
NOTES-						
1) 3-ply truss to be c Top chords conne	connected together with 10	d (0.120"x3") nails as follows w at 0-9-0 oc.	s:			
Bottom chords co	nnected as follows: 2x6 - 3	3 rows staggered at 0-4-0 oc				
2) All loads are cons	as tollows: 2x4 - 1 row at (idered equally applied to a	0-9-0 oc. all plies, except if noted as fro	ont (F) or back (B) fa	ce in the	LOAD CASE(S)	
section. Ply to ply	connections have been p	rovided to distribute only load	ds noted as (F) or (B)), unless	otherwise	
3) Unbalanced roof I	live loads have been consi	dered for this design.				
4) Wind: ASCE 7-10 Enclosed: MW/EP	; Vult=130mph (3-second	gust) Vasd=103mph; TCDL=	=6.0psf; BCDL=6.0ps	f; h=30f	t; Cat. II; Exp B;	
vertical left and rig	ght exposed;C-C for memb	pers and forces & MWFRS for	or reactions shown; L	umber D	OCL=1.60 plate	
grip DOL=1.60	en designed for a 10.0 psf	bottom chord live load nonce	oncurrent with any of	her live	loads	
6) * This truss has b	een designed for a live loa	d of 20.0psf on the bottom c	hord in all areas whe	ere a rec	tangle 3-6-0 tall	
by 2-0-0 wide will 7) Two H2.5A Simps	tit between the bottom cho son Strona-Tie connectors	ord and any other members. recommended to connect tr	uss to bearing walls o	due to U	PLIFT at it(s) 6	
and 4. This conne	ection is for uplift only and	does not consider lateral for	ces.			
8) This truss is designed and referenced st	ned in accordance with th andard ANSI/TPI 1.	e 2015 International Resider	ntial Code sections R	502.11.	1 and R802.10.2	
9) Hanger(s) or othe	r connection device(s) sha	all be provided sufficient to su	upport concentrated I	oad(s) 2	343 lb down and	
770 lb up at 2-2-0), 1467 lb down and 228 lb	o up at 4-1-4, 1467 lb down a	and 228 lb up at 6-1	-4, and 1	467 lb down and	

158 lb up at 8-1-4, and 1470 lb down and 155 lb up at 10-1-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

- LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-60, 2-3=-60, 4-6=-20

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown	
2000811-2000811A	GR1	Common Girder	1	3	Job Reference (optional)	
84 Components, Dunn, NC 28334 ID:hjOMp88mnrQZrZEntywViFyWSrv1i05YqFN0hkqhv1VOxtrwI_BjU_A55eTIt						

LOAD CASE(S) Standard Concentrated Loads (Ib) Vert: 1=-48 7=-2343(B) 8=-1467(B) 9=-1467(B) 10=-1467(B) 11=-1470(B)



BOT CHORD 6-7=-135/263, 5-6=-117/260

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.
- 6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



						12-0-0						
LOADING (P TCLL 20 TCDL 10 BCLL 0 BCDL 10	sf)).0).0).0 *).0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.08 0.06 0.05 x-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 8 8 9	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 65 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORE BOT CHORE WEBS OTHERS) 2x4 S) 2x4 S 2x4 S 2x4 S 2x4 S	P No.2 or 2x4 SPF No P No.2 or 2x4 SPF No P No.3 P No.3	.2 .2			BRACING TOP CHO BOT CHO	RD RD	Structo except Rigid o MiTe braci	ural woo t end ve ceiling di k recom ng be in	d sheathin rticals. irectly appl mends tha stalled dur	g directly applied o ied or 10-0-0 oc bra t Stabilizers and re ing truss erection, i	r 6-0-0 oc purlins, acing. quired cross n accordance

with Stabilizer Installation guide.

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 15=-139(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 15, 9, 13, 14, 11, 10 Max Grav All reactions 250 lb or less at joint(s) 15, 9, 12, 13, 14, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 15, 9, 13, 14, 11, and 10. This connection is for uplift only and does not consider lateral forces.
- 11) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



	0-8-8 0-8-8		<u>4-0-0</u> 3-3-8
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.14 BC 0.08 WB 0.06 Matrix-MP	DEFL. in (loc) I/defl L/d Vert(LL) -0.00 5-6 >999 240 Vert(CT) -0.01 5-6 >999 180 Horz(CT) 0.00 4 n/a n/a Weight: 14 lb FT = 20%
LUMBER-	SP No 2 or 2x4 SPF No 2		BRACING- TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins

 IOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance

with Stabilizer Installation guide.

REACTIONS. (lb/size) 4=72/Mechanical, 5=22/Mechanical, 6=270/0-3-0 (min. 0-1-8) Max Horz 6=43(LC 8) Max Uplift4=-38(LC 12), 6=-92(LC 8) Max Grav 4=72(LC 1), 5=53(LC 3), 6=270(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6.

This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.13 0.07 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 -0.00 -0.00	(loc) 5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2015/TF	PI2014	Matr	ix-MR						Weight: 9 lb	FT = 20%

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=152/0-3-8 (min. 0-1-8), 3=41/Mechanical, 4=16/Mechanical Max Horz 5=59(LC 12) Max Uplift3=-44(LC 12), 4=-5(LC 12) Max Grav 5=152(LC 1), 3=50(LC 19), 4=34(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.11 WB 0.02 Matrix-MP	DEFL. in (loc) I/defl L Vert(LL) 0.00 7 >999 24 Vert(CT) 0.00 7 >999 18 Horz(CT) -0.00 4 n/a n	/d PLATES GRIP 40 MT20 197/144 30 //a Weight: 9 lb FT = 20%
--------------------------------------------------------------------	---------------------------------------------------------------------------------------------------------	----------------------------------------------------	----------------------------------------------------------------------------------------------------------------	--------------------------------------------------------------------------

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-10-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=-9/Mechanical, 5=-35/Mechanical, 7=245/0-3-0 (min. 0-1-8) Max Horz 7=30(LC 8) Max Uplift4=-9(LC 1), 5=-35(LC 1), 7=-109(LC 8) Max Grav 4=12(LC 8), 5=18(LC 8), 7=245(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.3 WEBS

BOT CHORD

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=221/0-3-8 (min. 0-1-8), 3=100/Mechanical, 4=43/Mechanical Max Horz 5=164(LC 12) Max Uplift3=-130(LC 12), 4=-20(LC 12) Max Grav 5=221(LC 1), 3=127(LC 19), 4=73(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=130.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



with Stabilizer Installation guide.

(lb/size) 5=218/0-3-8 (min. 0-1-8), 4=140/Mechanical Max Horz 5=202(LC 9)

Max Uplift5=-25(LC 8), 4=-123(LC 9) Max Grav 5=254(LC 20), 4=202(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

BOT CHORD 4-5=-346/316

WEBS 2-4=-259/299

NOTES-

REACTIONS.

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 4. This connection is for uplift only and does not consider lateral forces.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

Job	Truss	Truss Type	Qty	Ply		Hicks Residence - Millto	wn
2000811-2000811A	J5B	Half Hip	6		1		
84 Components, Dunn,	NC 28334			8	3.400 s	Job Reference (optio s Apr 7 2020 MiTek Indu	nal) ustries, Inc. Tue Oct 6 15:56:44 2020 Page 1
		0.40.0	ID:hjOMp88mnrQZrZI	EntywV	/iFyW	/Sr1X_xpPJ?Juu	hOXmdgwsgLMmzzlj5gYSpXk42yW5q1
		0-10-8	2-4-8		<u>4-0-0</u> 1-7-8	<u> </u>	
			4x4 = 3			4	Scale = 1:20.6
	Ī		Æ	1	T2	1.5x4	
				<u>h</u> —		MU	
		12.00 12					
		/					
	4	3x6 // T1				W2	
	6 *		w	2			
		2					
	ې	W3					
	1-2-1	1	W1				
		\bowtie	6			5	
			3x4	=		2.4	
		1.5x4				584	
			249		100		
			2-4-8		1-7-8		
Plate Offsets (X,Y)	[3:0-2-8,0-2-0]						
LOADING (psf)	SPACING- 2-0	-0 CSI.	DEFL.	in (l	loc)	l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.1	15 TC 0.22	Vert(LL) 0	.02	6-7	>999 240	MT20 197/144
BCLL 0.0 *	Rep Stress Incr YE	S WB 0.07	Horz(CT) -0	.02 0	7-8 5	>999 180 n/a n/a	
BCDL 10.0	Code IRC2015/TPI20	14 Matrix-MS					Weight: 30 lb FT = 20%
LUMBER-	1	I	BRACING-				
TOP CHORD 2x4 S	SP No.2 or 2x4 SPF No.2		TOP CHORD	St	tructu	ural wood sheathin	g directly applied or 4-0-0 oc purlins,
WFBS 2x4 S	SP No.2 or 2x4 SPF No.2 SP No.3		BOT CHORD	ex Ri	iaid a	end verticals, and	2-0-0 oc purlins: 3-4. ed or 10-0-0 oc bracing
					MiTe	k recommends that	t Stabilizers and required cross
				1	braci	ng be installed duri	ng truss erection, in accordance
				1	with \$	Stabilizer Installatio	on guide.

REACTIONS. (lb/size) 5=140/Mechanical, 7=218/0-3-8 (min. 0-1-8) Max Horz 7=148(LC 9) Max Uplift5=-82(LC 9), 7=-18(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. BOT CHORD 6-7=-269/260

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 6) Refer to girder(s) for truss to truss connections.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 7. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Max Uplift5=-69(LC 9), 7=-50(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall
- by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 7. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 94 lb down and 61 lb up at 1-0-8, and 88 lb down and 68 lb up at 2-0-12 on top chord, and 15 lb down and 17 lb up at 2-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb)
 - Vert: 9=1(F)



WEBS 2x4 SP No.3 BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=1350/0-3-8 (min. 0-1-8), 3=1042/Mechanical Max Horz 4=128(LC 12) Max Uplift4=-100(LC 8), 3=-245(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 3=245.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 4. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1064 lb down and 126 lb up at 0-8-12, and 1060 lb down and 130 lb up at 2-8-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 3-4=-20

Concentrated Loads (lb) Vert: 5=-1050(F) 6=-1045(F)

Joh	Truss	Truss Type	Oty Plv	Hicks Residence - Milltown
000	11055			
2000811-2000811A	J5GR1	Jack-Open Girder	1 2	Job Reference (optional)
84 Components, Dunn, N	C 28334		8.40	0 s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:56:49 2020 Page 1
		ID:njOMp 4-	88mnrQZrZEntywViFy\ .0-0	WSrNVnqsi2X7XyA_SHVYAF6Zj2By_ajONbHc5EVkGyW5py
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		3x4	2x4	
			100	
		2-0-4	+ 4-0-0	
		204		

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.32 BC 0.61 WB 0.02 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) -0.02 3-4 >999 240 Vert(CT) -0.04 3-4 >999 180 Horz(CT) 0.00 n/a n/a Weight: 53 lb FT =	20%
LUMBER-			BRACING-	

I UMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.3 WEBS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 4=682/0-3-8 (min. 0-1-8), 3=695/Mechanical Max Horz 4=128(LC 12) Max Uplift4=-24(LC 8), 3=-205(LC 12) Max Grav 4=682(LC 1), 3=695(LC 41)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) 2-ply truss to be connected together with 10d (0.120"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=205.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1105 lb down and 134 lb up at 2-0-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 3-4=-20

Concentrated Loads (lb)

Vert: 5=-1081(B)



⁴ 1.5x4 || 3

2-0-0

	2-0-0								
LOADING (psf)	SPACING- 2-0-0 Plate Grip DOI 1.15	CSI. TC 0.12	DEFL. in (loc) I/defl L/d PLATES GRIP Vert(LL) -0.00 4 >999 240 MT20 197/144						
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.07 WB 0.00	Vert(CT) -0.00 3-4 >999 180 Horz(CT) 0.01 2 n/a n/a						
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MR	Weight: 8 lb FT = 20%	%					

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 4=74/0-3-8 (min. 0-1-8), 2=52/Mechanical, 3=21/Mechanical Max Horz 4=46(LC 9) Max Uplift2=-47(LC 12), 3=-4(LC 12) Max Grav 4=74(LC 1), 2=61(LC 19), 3=36(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 3.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



-				200	
LOADING TCLL TCDL BCLL	i (psf) 20.0 10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.14 BC 0.08 WB 0.00	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) -0.00 5 >999 240 MT20 197/144 Vert(CT) -0.00 4-5 >999 180 Horz(CT) -0.01 3 n/a n/a	
BCDL	10.0	Code IRC2015/TPI2014	Matrix-MR	Weight: 9 lb FT = 20%	

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=152/0-3-8 (min. 0-1-8), 3=41/Mechanical, 4=16/Mechanical Max Horz 5=58(LC 12) Max Uplift3=-45(LC 12), 4=-6(LC 12) Max Grav 5=152(LC 1), 3=51(LC 19), 4=34(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



W2

Β1

2-3-14

5

1.5x4 ||

W3

7

1.5x4 ||

0-5-13

1.5x4 ||

					0-5-1	3 '	1-10-1					
LOADING	i (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	ТС	0.13	Vert(LL)	0.00	6	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	0.00	6	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	-0.01	4	n/a	n/a		
BCDL	10.0	Code IRC2015/TF	912014	Matr	ix-MR						Weight: 14 lb	FT = 20%

LUMBER-

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-3-14 oc purlins, except end verticals.

C-0-2

Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=-25/Mechanical, 6=241/0-6-5 (min. 0-1-8), 4=15/Mechanical Max Horz 6=86(LC 9) Max Uplift5=-33(LC 20), 6=-43(LC 12), 4=-33(LC 9) Max Grav 5=23(LC 8), 6=241(LC 1), 4=29(LC 19)

1

1-1-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall
- by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4 and 6. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.09 WB 0.00 Matrix-MR	DEFL. in (loc) I/defl L/d Vert(LL) -0.00 5 >999 240 Vert(CT) -0.00 5 >999 180 Horz(CT) -0.01 3 n/a n/a	PLATES GRIP MT20 197/144 Weight: 10 lb FT = 20%

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-8-15 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=230/0-5-3 (min. 0-1-8), 3=1/Mechanical, 4=3/Mechanical Max Horz 5=61(LC 9) Max Uplift5=-26(LC 12), 3=-27(LC 12), 4=-10(LC 9) Max Grav 5=230(LC 1), 3=16(LC 10), 4=26(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



LOADING	(psf) 20.0	SPACING- 2-0-0 Plate Grip DOI 1 15	CSI. TC 0.12	DEFL. in (loc) I/defl L/d	PLATES GRIP MT20 197/144			
TCDL	10.0	Lumber DOL 1.15	BC 0.07	Vert(CT) -0.00 3-4 >999 180				
BCLL BCDL	0.0 * 10.0	Code IRC2015/TPI2014	WB 0.00 Matrix-MR	Horz(CT) 0.01 2 n/a n/a	Weight: 8 lb FT = 20%			

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 4=74/0-3-8 (min. 0-1-8), 2=52/Mechanical, 3=21/Mechanical Max Horz 4=46(LC 9) Max Uplift2=-47(LC 12), 3=-4(LC 12) Max Grav 4=74(LC 1), 2=61(LC 19), 3=36(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 3.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



LOADING	i (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	IC 0.23	Vert(LL) -0.00	5	>999	240	M120	197/144
TCDL	10.0	Lumber DOL 1.15	BC 0.09	Vert(CT) -0.00	5	>999	180		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix-MR					Weight: 11 lb	FT = 20%

 TOP CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 BOT CHORD
 2x4 SP No.2 or 2x4 SPF No.2

 WEBS
 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-10-11 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=230/0-5-3 (min. 0-1-8), 3=10/Mechanical, 4=5/Mechanical Max Horz 5=63(LC 9) Max Uplift5=-25(LC 12), 3=-31(LC 12), 4=-8(LC 9) Max Grav 5=230(LC 1), 3=20(LC 19), 4=29(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

6) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



			7-4-0 7-4-0					14-8 7-4	3-0 -0	I	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Ipcr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.93 0.47 0.11	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.05 -0.11 0.01	(loc) 5-6 5-6	l/defl >999 >999	L/d 240 180 p/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/T	PI2014	Matri	x-MS		0.01	0	17/4	n/a	Weight: 81	lb FT = 20%
I LIMBER.					BRACING	i-					

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.3 WEBS

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-11-15 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 7=638/0-3-8 (min. 0-1-8), 5=573/0-3-8 (min. 0-1-8) Max Horz 7=161(LC 9) Max Uplift7=-79(LC 12), 5=-58(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-620/129, 3-4=-612/124, 2-7=-575/180, 4-5=-509/135

- BOT CHORD 6-7=-260/464, 5-6=-118/257
- WEBS 3-6=0/290, 2-6=-109/268, 4-6=-58/251

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.

6) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



- BOT CHORD
- 4-9=-104/331, 4-10=-102/321, 3-12=-665/91, 5-7=-677/110 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 12, 7, 11, and 8. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

- Uniform Loads (plf)
 - Vert: 1-2=-60, 2-4=-60, 4-6=-60, 7-12=-54(F=-34)



0	8-8 4-0-0	10-4-0		1	3-7-8 14	1-4-0 ₁
0	8-8 3-3-8	6-4-0			3-3-8 0	-8-8
Plate Offsets (X,Y) [2:0-2-15,0-9-8], [2:0-0-0,0-1-4]	[3:0-3-0,0-2-12], [5:0-0-0,0-1-4], [[5:0-2-15,0-9-8]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI.DEFL.TC0.98Vert(LL)BC0.61Vert(CTWB0.05Horz(CTMatrix-MSVert(CT	in (loc) //dr) -0.05 7-8 >99) -0.12 7-8 >99 T) 0.01 5 n	efl L/d 99 240 99 180 /a n/a	PLATES MT20 Weight: 74 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x6 WEBS 2x4 WEDGE	SP No.2 or 2x4 SPF No.2 SP No.2 SP No.3	BRACIN TOP CH BOT CH	IG- IORD Structural ¹ purlins, ex 2-0-0 oc p HORD Rigid ceilir	Structural wood sheathing directly applied or 5-1-15 oc purlins, except 2-0-0 oc purlins (2-9-12 max.): 3-4. Rigid ceiling directly applied or 10-0-0 oc bracing.		
Left: 2x4 SP No.3	Right: 2x4 SP No.3	-710/0.2.0 (min 0.1.8)	MiTek red bracing b with Stab	commends that the installed dur tilizer Installation	at Stabilizers and re ring truss erection, on guide.	quired cross in accordance

REACTIONS. (lb/size) 2=719/0-3-0 (min. 0-1-8), 5=719/0-3-0 (min. 0-1-8) Max Horz 2=-21(LC 32) Max Uplift2=-184(LC 8), 5=-184(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1169/292, 3-19=-1118/309, 19-20=-1118/309, 20-21=-1118/309, 4-21=-1118/309, 4-5=-1185/298

BOT CHORD 2-8=-238/1116, 8-22=-260/1133, 22-23=-260/1133, 23-24=-260/1133, 7-24=-260/1133, 5-7=-250/1131

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 31 lb down and 67 lb up at 4-0-0, 28 lb down and 65 lb up at 6-0-12, 28 lb down and 65 lb up at 7-2-0, and 28 lb down and 65 lb up at 8-3-4, and 31 lb down and 67 lb up at 10-4-0 on top chord, and 64 lb down and 28 lb up at 4-0-0, 18 lb down at 6-0-12, 18 lb down at 7-2-0, and 18 lb down at 8-3-4, and 64 lb down and 28 lb up at 10-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf)
 - Vert: 1-3=-60, 3-4=-60, 4-6=-60, 9-14=-20

Job	Truss	Truss Type	Qty	Ply	Hicks Residence - Milltown
2000811-2000811A	LH	Hip Girder	1	1	Job Reference (optional)
84 Components, Dunn, NC 2	8334			8.400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:57:04 2020 Page 2

ID:hjOMp88mnrQZrZEntywViFyWSr_-RNBV0qDyb8r2ImwNxq0dguAau1iuP9qV3wNomuyW5pj

LOAD CASE(S) Standard Concentrated Loads (Ib) Vert: 3=-12(F) 8=-58(F) 4=-12(F) 7=-58(F) 19=-12(F) 20=-12(F) 21=-12(F) 22=-3(F) 23=-3(F) 24=-3(F)



Scale = 1:30.0



Plate Offsets (X Y))-8-8 6-0-0)-8-8 5-3-8 [2:0-0-0 0-1-8] [2:0-2-15 0-9-8]	[5:0-0-0 0-1-8] [5:0-2:	8-4-0 2-4-0 -15 0-9-81	<u>13-7-8</u> 5-3-8	14-4-0 0-8-8
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.35 BC 0.31 WB 0.04 Matrix-MS	DEFL. in Vert(LL) -0.03 Vert(CT) -0.06 Horz(CT) 0.01	(loc) l/defl L/d 7-8 >999 240 7-8 >999 180 5 n/a n/a	PLATES GRIP MT20 197/144 Weight: 70 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x6 S WEBS 2x4 S WEDGE	SP No.2 or 2x4 SPF No.2 SP No.2 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin purlins, except 2-0-0 oc purlins (6-0-0 m Rigid ceiling directly app	g directly applied or 5-10-5 oc ax.): 3-4. lied or 10-0-0 oc bracing.
Left: 2x4 SP No.3 , I REACTIONS. (Ib/si	Right: 2x4 SP No.3 ze) 2=626/0-3-0 (min. 0-1-8), 5=	=626/0-3-0 (min. 0-1-8)	MiTek recommends that bracing be installed dur with Stabilizer Installation	at Stabilizers and required cross ing truss erection, in accordance on guide.

Max Horz 2=28(LC 12) Max Uplift2=-126(LC 8), 5=-126(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-897/233, 3-4=-833/246, 4-5=-899/234 BOT CHORD 2-8=-162/835, 7-8=-173/836, 5-7=-170/837

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasts deset103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord



0-8-8 0-10-0 0-8-8 0-1-8		<u> </u>				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.61 BC 0.43 WB 0.10 Matrix-MS	DEFL. in Vert(LL) -0.07 Vert(CT) -0.15 Horz(CT) -0.00	(loc) I/defl L/d 5-6 >999 240 5-6 >501 180 5 n/a n/a	PLATES GRIP MT20 197/144 Weight: 27 lb FT = 20%	
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 6-0-0 oc except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.		

bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 5=238/0-3-8 (min. 0-1-8), 6=383/0-3-0 (min. 0-1-8) Max Horz 6=88(LC 11) Max Uplift5=-49(LC 12), 6=-111(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 3-6=-364/351

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 6. This connection is for uplift only and does not consider lateral forces.

5) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



LOADING (psf) SPACING- TCLL 20.0 Plate Grip DC	2-0-0	CSI.	DEFL		<i>a</i> >				
	L 1.15	TC 0.50	Vert(LL)	-0.07	(loc) 7-8	l/defl >999	L/d 240	PLATES MT20	GRIP 197/144
TCDL 10.0 Lumber DOL BCLL 0.0 * Rep Stress Ir BCDL 10.0 Code IBC201	1.15 cr YES 5/TPI2014	BC 0.48 WB 0.08 Matrix-MS	Vert(CT) Horz(CT)	-0.16 -0.00	7-8 6	>459 n/a	180 n/a	Weight [,] 29 lb	FT = 20%
BCLL 0.0 * Rep Stress Ir BCDL 10.0 Code IRC201	cr YES 5/TPI2014	WB 0.08 Matrix-MS	Horz(CT)	-0.00	6	n/a	n/a	Weight: 29 lb	FT = 2

BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3

BOT CHORD

except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5. Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 6=238/0-3-8 (min. 0-1-8), 8=383/0-3-0 (min. 0-1-8) Max Horz 8=77(LC 11) Max Uplift6=-43(LC 8), 8=-113(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 3-8=-273/267

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 8. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



2-0-12



1-2-12



4-0-0

Plate Offsets (X,Y)-	- [2:0-0-0,0-1-4]	4-0-0 3-3-8	4-0 ₁ 12 0-0-12	6-0-12 2-0-0	7-3-8 1-2-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014	CSI. TC 0.23 BC 0.14 WB 0.10 Matrix-MP	DEFL. in Vert(LL) -0.00 Vert(CT) -0.01 Horz(CT) 0.00	(loc) I/defl L/d 6 >999 240 6 >999 180 5 n/a n/a	PLATES GRIP MT20 197/144 Weight: 39 lb FT = 20%	
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 6-0-0 oc pr except end verticals, and 2-0-0 oc purlins: 3-4. Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance		

REACTIONS. (lb/size) 5=293/0-3-8 (min. 0-1-8), 2=418/0-3-0 (min. 0-1-8) Max Horz 2=54(LC 11) Max Uplift5=-67(LC 9), 2=-115(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-376/123

BOT CHORD 2-6=-152/340, 6-13=-155/334, 5-13=-155/334

0-10-8

WEBS 3-5=-363/151

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5 and 2. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 31 lb down and 70 lb up at 4-0-0, and 28 lb down and 67 lb up at 6-0-12 on top chord, and 64 lb down and 28 lb up at 4-0-0, and 18 lb down at 6-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-3=-60, 3-4=-60, 5-7=-20
 - Concentrated Loads (lb)
 - Vert: 6=-58(B) 3=-12(B) 12=-12(B) 13=-3(B)


Plate Offsets (X,Y)	- [5:0-3-8,Edge]		7-0-0 6-2-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.56 BC 0.38 WB 0.10 Matrix-MS	DEFL. in Vert(LL) -0.05 Vert(CT) -0.12 Horz(CT) -0.00	(loc) l/defl L/d 5-6 >999 240 5-6 >582 180 5 n/a n/a	PLATES GRIP MT20 197/144 M18SHS 197/144 Weight: 26 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin except end verticals. Rigid ceiling directly app MiTek recommends tha	g directly applied or 6-0-0 oc purlin lied or 10-0-0 oc bracing. It Stabilizers and required cross
				bracing be installed dur with Stabilizer Installation	ing truss erection, in according truss erection, in according to the second s

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REACTIONS. (lb/size) 6=372/0-3-0 (min. 0-1-8), 5=226/0-1-8 (min. 0-1-8)
Max Horz 6=85(LC 11)
Max Uplift6=-109(LC 8), 5=-46(LC 12)
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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-6=-344/333

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) All plates are MT20 plates unless otherwise indicated.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 5. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



			5-10-0		
			5-10-0		1
Plate Offsets (X,Y)	[2:0-4-3,0-0-13]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.48 BC 0.37 WB 0.00	DEFL. in Vert(LL) 0.06 Vert(CT) -0.11 Horz(CT) 0.03	(loc) l/defl L/d 5-8 >999 240 5-8 >643 180 2 n/a n/a	PLATES GRIP MT20 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-MP			Weight: 23 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S	SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 SP No.3 W4 SP No.2	· · · · · · · · · · · · · · · · · · ·	BRACING- TOP CHORD BOT CHORD	Structural wood sheathin purlins, except end verti Rigid ceiling directly appl	g directly applied or 5-10-0 oc cals. ied or 10-0-0 oc bracing.
SLIDER LETT 2	224 SP NO.3 -U 1-6-0			MiTek recommends that bracing be installed dur with Stabilizer Installation	t Stabilizers and required cross ing truss erection, in accordance on guide.

REACTIONS. (lb/size) 2=284/0-3-8 (min. 0-1-8), 5=223/0-1-8 (min. 0-1-8) Max Horz 2=73(LC 11) Max Uplift2=-73(LC 8), 5=-46(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer
- should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



		1	7-0-0		1	13-0-0		1		17-10-8	1	
			7-0-0		1	6-0-0		1		4-10-8		
Plate C	Offsets (X,Y)-	- [2:0-0-0,0-1-4], [4:0-	-4-4,0-1-12]	, [6:0-4-7,0	-1-7]							
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.71	Vert(LL)	-0.06) ģ	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.38	Vert(CT)	-0.12	7-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.90	Horz(CT)	0.02	6	n/a	n/a		
BCDL	10.0	Code IRC2015/T	PI2014	Matri	x-MS						Weight: 123	3 lb FT = 20%

LUMBER-	BRACING-	
TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 3-11-4 oc
BOT CHORD 2x6 SP No.2		purlins, except end verticals, and 2-0-0 oc purlins (6-0-0
WEBS 2x4 SP No.3		max.): 4-5.
WEDGE	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
Left: 2x4 SP No.3	WEBS	1 Row at midpt 5-6, 4-6
		MiTek recommends that Stabilizers and required cross

bracing be installed during truss erection, in accordance

with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=763/0-3-8 (min. 0-1-8), 6=708/0-1-8 (min. 0-1-8) Max Horz 2=311(LC 11) Max Uplift2=-102(LC 12), 6=-124(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

2-3=-1563/294, 3-4=-612/165 TOP CHORD

BOT CHORD 2-9=-387/1462, 8-9=-390/1457, 7-8=-390/1457, 7-13=-202/405, 6-13=-202/405

WEBS 3-7=-1192/349, 4-7=-85/560, 4-6=-633/235

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2

and 6. This connection is for uplift only and does not consider lateral forces. 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



	7-0-0	1	11-8-0	17-10-8	
	7-0-0	Ι	4-8-0	6-2-8	
Plate Offsets (X,Y)-	- [2:0-0-0,0-1-4], [4:0-4-4,0-1-12],	[6:0-4-7,0-1-7]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.66 BC 0.36 WB 0.54 Matrix-MS	DEFL. in Vert(LL) -0.05 Vert(CT) -0.10 Horz(CT) 0.02	(loc) l/defl L/d 9 >999 240 9 >999 180 6 n/a n/a	PLATES GRIP MT20 197/144 Weight: 116 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x6 S WEBS 2x4 S WEDGE Left: 2x4 SP No.3	SP No.2 or 2x4 SPF No.2 SP No.2 SP No.3		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing purlins, except end vertio max.): 4-5. Rigid ceiling directly appli 1 Row at midpt	g directly applied or 3-10-8 oc als, and 2-0-0 oc purlins (6-0-0 ed or 10-0-0 oc bracing. 4-6
				MiTek recommends that bracing be installed duri with Stabilizer Installation	t Stabilizers and required cross ng truss erection, in accordance n guide.

REACTIONS. (lb/size) 2=763/0-3-8 (min. 0-1-8), 6=708/0-1-8 (min. 0-1-8) Max Horz 2=261(LC 11) Max Uplift2=-106(LC 12), 6=-119(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1546/298, 3-4=-737/190

BOT CHORD 2-9=-408/1444, 8-9=-410/1440, 7-8=-410/1440, 7-13=-213/471, 6-13=-213/471

WEBS 3-7=-1086/313, 4-7=-81/626, 4-6=-641/209

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2

and 6. This connection is for uplift only and does not consider lateral forces. 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



F	7-0-0		10-4-0			17-10-8		4
Plate Offsets (X,Y)) [2:0-0-0,0-1-4], [4:0-4-4,0-1-12],	[5:Edge,0-1-8], [6:0-	4-7,0-1-7]			7-0-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.84 BC 0.35 WB 0.32 Matrix-MS	DEFL. Vert(LL) -C Vert(CT) -C Horz(CT) C	in).05).10).02	(loc) l/defl 9 >999 9 >999 6 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 110 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x6 WEBS 2x4 WEDGE Left: 2x4 SP No.3	SP No.2 or 2x4 SPF No.2 SP No.2 SP No.3		BRACING- TOP CHORE BOT CHORE WEBS	2 S F 7 2 I	Structural woo purlins, excep max.): 4-5. Rigid ceiling d 1 Row at midp	d sheathing t end vertio irectly appli t	g directly applied or cals, and 2-0-0 oc pu ied or 10-0-0 oc brad 4-6	3-10-0 oc ırlins (6-0-0 cing.
REACTIONS. (Ib/s Max Max	ize) 2=763/0-3-8 (min. 0-1-8), 6= ∶Horz 2=210(LC 11) ∶Uplift2=-109(LC 8), 6=-115(LC 9)	708/0-1-8 (min. 0-1-	8)		MiTek recom bracing be in with Stabilize	mends tha stalled duri er Installatic	t Stabilizers and req ing truss erection, in n guide.	uired cross accordance

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1535/305, 3-4=-927/221
- BOT CHORD 2-9=-414/1432, 8-9=-416/1430, 7-8=-416/1430, 6-7=-226/598
- WEBS 3-7=-971/280, 4-7=-83/687, 4-6=-706/199

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 6.
 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



F	7-0-0 7-0-0	9-0-0		<u> </u>	
Plate Offsets (X,Y)	[2:0-0-0,0-1-4], [4:0-2-4,0-1-12],	[7:0-4-7,0-1-7]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.67 BC 0.39 WB 0.54 Matrix-MS	DEFL. in Vert(LL) -0.05 Vert(CT) -0.12 Horz(CT) 0.02	(loc) I/defl L/d 7-8 >999 240 7-8 >999 180 7 n/a n/a	PLATES GRIP MT20 197/144 Weight: 107 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x6 S WEBS 2x4 S WEDGE	SP No.2 or 2x4 SPF No.2 SP No.2 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin purlins, except end vertio max.): 4-6. Rigid ceiling directly appl	g directly applied or 3-10-9 oc als, and 2-0-0 oc purlins (6-0-0 ied or 10-0-0 oc bracing.
REACTIONS. (Ib/si	ize) 2=763/0-3-8 (min. 0-1-8), 7=	708/0-1-8 (min. 0-1-8)		MiTek recommends tha bracing be installed dur with Stabilizer Installation	t Stabilizers and required cross ing truss erection, in accordance on guide.

Max Horz 2=160(LC 11) Max Uplift2=-126(LC 8), 7=-114(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 2-3=-1534/314, 3-4=-1153/256, 4-5=-797/207
- TOP CHORD BOT CHORD 2-10=-408/1432, 9-10=-409/1434, 8-9=-409/1434, 7-8=-214/579
- WEBS 3-8=-918/279, 4-8=-109/630, 5-8=-20/351, 5-7=-767/234

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 7.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Plate Offsets (X,Y)	7-0-0 7-0-0 [2:0-0-0,0-1-4], [4:0-6-4,0-1-12],	7-8-0 0-8-0 [7:0-2-7,0-1-7], [10:0-3-	<u>12-9-4</u> 5-1-4 -4,0-4-8]		17-10-8 5-1-4	_
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.66 BC 0.40 WB 0.49 Matrix-MS	DEFL. in Vert(LL) -0.06 Vert(CT) -0.12 Horz(CT) 0.02	(loc) l/defl L/d 11 >999 240 11-14 >999 180 7 n/a n/d	d PLATES 0 MT20 a Weight: 105 II	GRIP 197/144 p FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x6 S WEBS 2x4 S WEDGE	SP No.2 or 2x4 SPF No.2 SP No.2 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wood she purlins, except end max.): 4-6. Rigid ceiling directly	eathing directly applied o I verticals, and 2-0-0 oc p y applied or 10-0-0 oc bra	r 3-10-0 oc ourlins (5-7-12 acing.
Left: 2x4 SP No.3 REACTIONS. (lb/si	ize) 2=763/0-3-8 (min. 0-1-8), 7=	708/0-1-8 (min. 0-1-8)		MiTek recommend bracing be installe with Stabilizer Inst	ds that Stabilizers and re ed during truss erection, i tallation guide.	quired cross n accordance

Max Horz 2=110(LC 11) Max Uplift2=-149(LC 8), 7=-118(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD BOT CHORD 2-3=-1506/309, 3-4=-2111/537, 4-5=-965/241, 5-6=-965/241, 6-7=-650/172
- 2-11=-371/1404, 10-11=-310/1161, 9-10=-310/1161, 8-9=-311/1176
- WEBS 3-11=-1314/418, 4-11=-320/1186, 4-9=-39/290, 5-8=-337/159, 6-8=-237/1072

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 7.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 7. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



0-4	I-8 <u>6-0-</u> I-8 <u>5-7-</u>) 3		<u>12-0-0</u> 6-0-0	
Plate Offsets (X,Y)	- [2:0-0-0,0-1-4], [2:0-2-15,0-9-8]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.38 BC 0.22 WB 0.56 Matrix-MS	DEFL. in Vert(LL) -0.02 Vert(CT) -0.04 Horz(CT) 0.01	(loc) l/defl L/d 6 >999 240 5-6 >999 180 5 n/a n/a	PLATES GRIP MT20 197/144 Weight: 64 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x6 S WEBS 2x4 S	SP No.2 or 2x4 SPF No.2 SP No.2 SP No.3 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin except end verticals. Rigid ceiling directly appl	g directly applied or 6-0-0 oc purlins, ied or 10-0-0 oc bracing.
WEDGE Left: 2x4 SP No.3				MiTek recommends tha bracing be installed duri with Stabilizer Installation	t Stabilizers and required cross ing truss erection, in accordance on guide.

REACTIONS. (lb/size) 2=546/0-3-0 (min. 0-1-8), 5=455/0-1-8 (min. 0-1-8) Max Horz 2=134(LC 11) Max Uplift2=-118(LC 8), 5=-92(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-834/238

- BOT CHORD 2-6=-322/780, 5-6=-322/780
- WEBS 3-5=-785/284

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Diata Offacta (X X)			<u>8-0-0</u> 8-0-0		
Plate Olisets (A, f)	[[2.0-4-3,0-0-13]				1
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.19 BC 0.13 WB 0.06 Matrix-P	DEFL. in Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) -0.00	(loc) l/defl L/d 1 n/r 120 1 n/r 90 7 n/a n/a	PLATES GRIP MT20 197/144 Weight: 35 lb FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4	SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 SP No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathin except end verticals. Bigid ceiling directly appl	g directly applied or 6-0-0 oc purlins,

OTHERS 2x4 SP No.3 SLIDER Left 2x4 SP No.3 -ü 1-6-0

REACTIONS. All bearings 7-7-8.

(lb) - Max Horz 2=93(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2, 8, 9

Max Grav All reactions 250 lb or less at joint(s) 7, 2, 8 except 9=308(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7, 2, 8, and 9. This connection is for uplift only and does not consider lateral forces.

7) Non Standard bearing condition. Review required.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



			000		
	I		6-0-0		I
Plate Offsets (X, Y	() [2:0-4-3,0-0-13]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.19 BC 0.13 WB 0.06	DEFL. in Vert(LL) -0.00 Vert(CT) 0.00 Horz(CT) 0.00	(loc) l/defl L/d 1 n/r 120 1 n/r 90 6 n/a n/a	PLATES GRIP MT20 197/144
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P			Weight: 26 lb FT = 20%
LUMBER-			BRACING-		
TOP CHORD 2x4 BOT CHORD 2x4	SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2		TOP CHORD	Structural wood sheathin except end verticals.	g directly applied or 6-0-0 oc purlins,
WEBS 2x4 OTHERS 2x4	SP No.3 SP No.3		BOT CHORD	Rigid ceiling directly app	ied or 10-0-0 oc bracing.
SLIDER Left	t 2x4 SP No.3 -ü 1-6-0				

6-0-0

REACTIONS. (lb/size) 6=16/5-7-8 (min. 0-1-8), 2=191/5-7-8 (min. 0-1-8), 7=313/5-7-8 (min. 0-1-8) Max Horz 2=73(LC 9) Max Uplift6=-2(LC 9), 2=-53(LC 8), 7=-72(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6, 2, and 7. This connection is for uplift only and does not consider lateral forces.

7) Non Standard bearing condition. Review required.

8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



REACTIONS. (lb/size) 5=218/0-3-8 (min. 0-1-8), 4=140/0-1-8 (min. 0-1-8) Max Horz 5=199(LC 9) Max Uplift5=-23(LC 8), 4=-119(LC 9) Max Grav 5=251(LC 20), 4=200(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

BOT CHORD 4-5=-343/312

WEBS 2-4=-254/294

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 5 and 4. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Max Horz 5=199(LC 9) Max Uplift5=-23(LC 8), 4=-119(LC 9) Max Grav 5=251(LC 20), 4=200(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

BOT CHORD 4-5=-343/312

WEBS 2-4=-254/294

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 5 and 4. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



REACTIONS. (lb/size) 7=218/0-3-8 (min. 0-1-8), 5=140/0-1-8 (min. 0-1-8) Max Horz 7=145(LC 11) Max Uplift7=-18(LC 12), 5=-79(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. BOT CHORD $\,$ 6-7=-266/258 $\,$

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer
- should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Max Grav T = 210(LC T), S = 141(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -0-10-8 to 3-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 7 and 5. This connection is for uplift only and does not consider lateral forces.
- 9) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 88 lb down and 67 lb up at 2-0-12 on top chord, and 14 lb down and 15 lb up at 2-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb) Vert: 9=1(F)

Job	Truss	Truss Type		Qty	Ply	Hicks Residence - Milltown
2000811-2000811A	M8	Monopitch Supported Gable		1	1	
2000011200001111						Job Reference (optional)
84 Components, Dunn, NC 2	28334				8.400	s Apr 7 2020 MiTek Industries, Inc. Tue Oct 6 15:57:39 2020 Page 1
			ID:hjOMp88mnr0	ZrZEnty	wViFyWS	Srw6pkMoevMvF4_856Ug9qbG7lw7nP6xHe6tL9WMyW5pA
		<mark>1-2-8 ا</mark>	9-4-0			
		1-2-8	9-4-0			

Scale = 1:60.7

7 6 12.00 12 5 11-0-7 g 3 3x6 // P 1-8-7 13 12 11 10 9 8 3x6 || 6x6 =

					1							
LOADING TCLL TCDL BCLL	(psf) 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.22 0.08 0.19	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 -0.00	(loc) 1 1 8	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2015/TF	712014	Matri	x-S						Weight: 89 lb	FI = 20%
LUMBER-		·				BRACING	-					
TOP CHO	RD 2x4 S	SP No.2 or 2x4 SPF No SP No 2 or 2x4 SPF No	.2			TOP CHO	RD	Struct	ural woo t end ve	od sheathin rticals.	g directly applied	or 6-0-0 oc purlins,
WEBS	2x4 9	SP No.3				BOT CHO	RD	Rigid	ceiling d	irectly app	lied or 10-0-0 oc b	racing, Except:
OTTIERO	274 (WEBS		1 Row	at midp	ot 12 10.	7-8, 6-9	
								MiTe brac with	ek recom ing be ir Stabilize	nmends tha Istalled dui er Installati	at Stabilizers and r ring truss erection, on guide.	equired cross in accordance

REACTIONS. All bearings 9-4-0. (lb) - Max Horz 13=374(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 8 except 13=-228(LC 10), 9=-109(LC 12), 10=-106(LC 12), 11=-114(LC 12), 12=-527(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 8, 9, 10, 11 except 13=737(LC 12), 12=284(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-13=-781/581, 2-3=-466/415, 3-4=-400/330, 4-5=-280/232

BOT CHORD 12-13=-407/322

WEBS 2-12=-518/653

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 1.5x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall
- by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 13,
- 8, 9, 10, 11, and 12. This connection is for uplift only and does not consider lateral forces.

10) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Plate Offsets (X,Y)	[2:0-2-6,0-1-0], [6:0-2-6,0-1-0]						-	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.03 BC 0.01 WB 0.03 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loo 0.00 0.00 0.00	c) l/defl 6 n/r 6 n/r 6 n/a	L/d 120 90 n/a	PLATES MT20 Weight: 27 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S OTHERS 2x4 S	SP No.2 or 2x4 SPF No.2 SP No.2 or 2x4 SPF No.2 SP No.3 SP No.3		BRACING- TOP CHOR BOT CHOR	D Stru D Rig M br	uctural woo id ceiling di iTek recom racing be in	d sheathin rectly appl mends tha stalled dur	g directly applied o ied or 10-0-0 oc br t Stabilizers and re ing truss erection,	r 6-0-0 oc purlins. acing. quired cross in accordance
REACTIONS AII	bearings 5-1-8					motanatic	Jir guluo.	

(lb) - Max Horz 2=-72(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 6, 10, and 8. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 10, 8 Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8



REACTIONS. (lb/size) 2=146/5-1-8 (min. 0-1-8), 4=146/5-1-8 (min. 0-1-8), 6=159/5-1-8 (min. 0-1-8) Max Horz 2=-72(LC 10) Max Uplift2=-34(LC 13), 4=-40(LC 13) Max Grav 2=146(LC 1), 4=146(LC 1), 6=160(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



			6-7-8						
	1		6-7-8					1	
Plate Offsets (X,Y)-	- [3:0-2-0,Edge], [5:0-2-0,Edge]								
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.07 BC 0.07 WB 0.02	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 7 7 6	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20	GRIP 197/144
BCDL 10.0	Code IRC2015/1912014	Matrix-S						weight. Z i b	FI = 20%
LUMBER- TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 OTHERS 2x4 SP No.3				RD RD	Structural wood sheathing directly applied or 6-0-0 oc purlins except 2-0-0 oc purlins (6-0-0 max.): 3-5. Rigid ceiling directly applied or 10-0-0 oc bracing.				r 6-0-0 oc purlins, acing.
		100/5 4 40 (1010 0 4 6		40 (MiTe braci with	k recon ing be ir Stabilize	nmends than Istalled du er Installati	at Stabilizers and re ring truss erection, on guide.	equired cross in accordance

-8), 6=136/5-1-10 (min. 0-1-8), 8=196/5-1-10 (min. 0-1-8) Max Horz 2=-43(LC 10) Max Uplift2=-33(LC 12), 6=-39(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 8) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



	0-0-4 0-0-4		9-3-0		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.44 BC 0.31 WB 0.07 Matrix-S	DEFL. ir Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	n (loc) l/defl L/d a - n/a 999 a - n/a 999 0 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 38 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S OTHERS 2x4 S	P No.3 P No.3 P No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathii Rigid ceiling directly app MiTek recommends th bracing be installed du with Stabilizer Installat	ng directly applied or 6-0-0 oc purlins. Jlied or 10-0-0 oc bracing. at Stabilizers and required cross ring truss erection, in accordance ion guide.

REACTIONS. (Ib/size) 1=190/9-2-8 (min. 0-1-8), 3=190/9-2-8 (min. 0-1-8), 4=304/9-2-8 (min. 0-1-8) Max Horz 1=106(LC 9) Max Uplift1=-37(LC 13), 3=-37(LC 13), 4=-5(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 3, and 4. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



	0-Ö-4		6-6-12				1	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.28 BC 0.15 WB 0.03 Matrix-P	DEFL. Vert(LL) r Vert(CT) r Horz(CT) 0.	in (loc) n/a - n/a - 00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 26 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 \$ BOT CHORD 2x4 \$ OTHERS 2x4 \$	SP No.3 SP No.3 SP No.3 SP No.3		BRACING- TOP CHORD BOT CHORD	Struct Rigid MiTe brac with	ural wood ceiling di ek recomi ing be ins Stabilize	d sheathing rectly appli mends tha stalled duri r Installatic	g directly applied o ied or 10-0-0 oc br t Stabilizers and re ng truss erection, ng quide.	or 6-0-0 oc purlins. acing. equired cross in accordance

REACTIONS. (Ib/size) 1=140/6-6-8 (min. 0-1-8), 3=140/6-6-8 (min. 0-1-8), 4=189/6-6-8 (min. 0-1-8) Max Horz 1=73(LC 9) Max Uplift1=-35(LC 13), 3=-35(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



	0- <u>0-4</u> 0-0-4		3-11-0 3-10-12		—
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.08 BC 0.05 WB 0.01 Matrix-P	DEFL. in Vert(LL) n/a Vert(CT) n/a Horz(CT) 0.00	(loc) l/defl L/d - n/a 999 - n/a 999 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 15 lb FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S OTHERS 2x4 S	SP No.3 SP No.3 SP No.3	<u> </u>	BRACING- TOP CHORD BOT CHORD	Structural wood sheathin purlins. Rigid ceiling directly appl	g directly applied or 3-11-0 oc ied or 10-0-0 oc bracing.
				MiTek recommends tha bracing be installed dur with Stabilizer Installation	t Stabilizers and required cross ing truss erection, in accordance on guide.

REACTIONS. (Ib/size) 1=77/3-10-8 (min. 0-1-8), 3=77/3-10-8 (min. 0-1-8), 4=103/3-10-8 (min. 0-1-8) Max Horz 1=-40(LC 8) Max Uplift1=-19(LC 13), 3=-19(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

7) This truss is designed in accordance with the 2015 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

