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**Date:** 6/6/2019

**To:** **Marty Crawford**  
KB Home  
4506 S Miami Blvd Suite 100, Durham, NC 27703  
smcrawford@kbhome.com  
910-915-0510

**Re:** **Soil Suitability for Foundation Installation**  
Location: Lot 50 Mason Pointe – 20 White Quartz Place (Fuquay-Varina, NC)  
JDS Project No.: RDU1904525  
Date of Inspection: 06/06/2019  
Foundation Type: Monolithic Slab

We are pleased to provide the evaluation of the subject and location referenced above.

**Observations:**

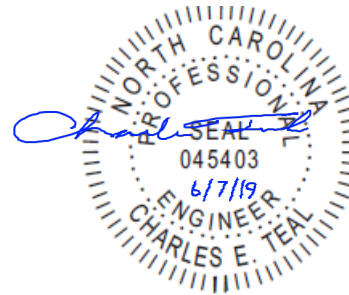
Foundation excavation observation (sub-surface testing with respect to bearing capacity).

**Recommendations:**

The exposed soils have been observed and tested for adequate bearing capacity (Probe and DCP). Based on our review and testing, the soil and conditions for the foundation are suitable for the minimum required bearing pressure of 2000 psf.

Please contact me if I can be of further assistance:

Respectfully Submitted,  
Samantha Grygoruk  
919-218-4421  
JDS Consulting & Design, PLLC



**General Notes:**

- Mechanical testing methods vary per site but always include probe rod testing across the entire excavation and augers (minimum three locations) at multiple depths with Dynamic Cone Penetrometer (DCP) testing.
- Bearing capacity test results are voided if significant precipitation or water intrusion has occurred within 48 hours of the initial testing.
- JDS is not responsible for site conditions that divert water towards the foundation or that prevent drainage away from the foundation, which can lead to soft soils and future settlement.
- This report is an assessment of vertical bearing capacity only. Unless specifically noted otherwise, neither retaining wall testing nor slope stability analysis has been evaluated. JDS shall not be held responsible for current or future retaining-wall or slope-related issues.
- It is the contractor's responsibility to ensure that all foundation areas are free of organics, loose material, standing water, and any other deleterious materials prior to placement of stone or concrete.