

Trenco RE: 1238844 Master - H&H-NC/Wilmington/ 818 Soundside Rd Site Information: Edenton, NC 27932 Project Customer: H and H Project Name: 1238844 Lot/Block: Subdivision: Model: Address: State: NC City: General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions): Design Code: IRC2009/TPI2007 Design Program: MiTek 20/20 7.6 Wind Code: ASCE 7-05 Wind Speed: 100 mph Design Method: MWFRS(low-rise)/C-C hybrid Wind ASCE 7-05 Roof Load: 40.0 psf Floor Load: N/A psf Mean Roof Height (feet): 25 Exposure Category: C No. Seal# **Truss Name Date** No. Seal# Truss Name Date 131241136 35 131241170 10/3/17 J06 10/3/17 123456789111111111122222222222333333 A01 36 37 J07 J08 A02 A03 10/3/17 31 10/3/17 10/3/17 10/3/17 10/3/17 38 39 40 131241173 131241139 J09 A04 10/3/17 131241140 A04A A05 A05A I31241174 I31241174 I31241175 J10 I31241140 I31241141 I31241142 I31241143 J11 41 42 43 44 45 131241176 131241177 10/3/17 10/3/17 J12 J13 10/3/17 A06 10/3 1144 1145 A07 A08 131241178 131241179 10/3/17 J14 J15 10/3/17 10/3/17 10/3/17 131241146 A09 131241180 **PB01** 10/3/17 10/3/17 46 47 131241147 A10 131241181 **PB02** 10/3/17 10/3/17 131241148 131241182 **PB03** A11 10/3/17 131241149 B01 I31241150 I31241151 B02 B03 10/3/17 10/3/17 10/3/17 10/3/17 131241154 10/3 131241155 C03 131241156 131241157 I31241158 I31241159 I31241160 **CP02** 10/3 131241161 FG02 I31241162 I31241163 G01 17 G02 I31241164 I31241165 G03 J01 131241166 J02 J03 J04 17 1167 168 17 131241169 .105 10/3/17 The truss drawing(s) referenced above have been prepared by ...... MiTek USA, Inc. under my direct supervision based on the parameters 20 provided by Builders FirstSource-Sumter,SC. Truss Design Engineer's Name: Galinski, John My license renewal date for the state of North Carolina is December 31, 2017 THINK WARTER **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or O

TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



October 3,2017

October 3



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Job	Truss	Truss Type	Qty	Ply	H&H-NC/Wilmington/	1312/1151
1238844_MASTER	B03	COMMON	11	1	Ich Reference (antions	SI)
Builders FirstSource,	Sumter, SC 29153			7.640	s Aug 16 2017 MiTek Ind	dustries, Inc. Tue Oct 03 07:31:27 2017 Page
		-0 <u>-10-8 5-11-9 9-11-8</u>	13-11-7	JQU8X ? XJG <u>19-11-0</u> 5-11-9	<u>20-9-8</u> 0-10-8	SA9EBX8KQGQYTLY14W4SZNgX4XMK_7KyXBac
		3x6 -	=	0.110	0.00	Scale = 1:91.3
	11-2-11	$4x6    \\ A \\ C \\ C$	I 2x4	3x6 \ E N II	4x6 II $F = Great Structure Struc$	
Plate Offsets (X,Y) [	D:0-3-0,Edge], [H:Edge,0-5-8]					
LOADING (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0- Plate Grip DOL 1.19 Lumber DOL 1.19 Rep Stress Incr YES Code IRC2009/TPI200	CSI.         DEF           5         TC 0.60         Ver           5         BC 0.44         Ver           6         WB 0.60         Hor.           7         (Matrix-M)         Win	<b>EL.</b> ir ((LL) -0.14 ((TL) -0.23 z(TL) 0.0 d(LL) 0.2	n (loc) 4 I-K 3 I-K 1 H 3 K-L	l/defi L/d >999 360 >999 240 n/a n/a >999 240	PLATES         GRIP           MT20         244/190           Weight: 128 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x6 SP WEBS 2x4 SP B-L,F-H	No.2 No.2 No.3 *Except* : 2x6 SP No.2	BR/ TOF BO	ACING- P CHORD	Structur end ver Rigid ce	al wood sheathing dire ticals. siling directly applied or	octly applied or 5-0-8 oc purlins, except 10-0-0 oc bracing.
REACTIONS. (lb/size) Max Ho Max Up Max Gr	) L=845/0-5-8, H=845/0-5-8 rz L=347(LC 7) liftL=-93(LC 8), H=-93(LC 9) av L=942(LC 2), H=942(LC 2)					
FORCES.(lb) - Max. (lb)TOP CHORDB-C=-BOT CHORDL-M=-WEBSE-I=-8	Comp./Max. Ten All forces 2 954/294, E-F=-954/294, B-L= 58/583, K-M=-58/583, J-K=-5i 2/359, C-K=-82/359, C-E=-48	50 (Ib) or less except when shown. 822/362, F-H=-822/362 3/583, I-J=-58/583, I-N=-58/583, H-N=-58/5 7/402	83			

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) L, H.

7) "Semi-rigid pitchbreaks including hells" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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			_		131241153
1238844_MASTER	C01	GABLE	3	1	
					Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s	Aug 16 2017 MiTek Industries, Inc. Tue Oct 03 07:31:28 2017 Page 2

ID:XOjtQcFjQu8X?XjGN5R0bmzVOFf-P7aQ4S9mxSM2ZIJcq834tmZHeUDZ6D5DA0TYgAyXBdT

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: A-B=-60, B-D=-75(F=-15), D-F=-75(F=-15), F-I=-60, T-Z=-35(F=-15), W-Z=-20, G-K=-45(F)

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					Job Reference (optional)
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		ID:X	OjtQcFjQ	J8X?XjGN	5R0bmzVOFf-tK8olo9OimUvBSuoOraJQz6SMuaJrhGMPgD5CcyXBdS

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: A-B=-60, B-D=-75(F=-15), D-AA=-75(F=-15), G-AA=-60, U-AB=-35(F=-15), X-AB=-20, E-J=-45(F)

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#### LUMBER-

TOP CHORD 2x6 SP No.2 2x6 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) A=732/0-4-9, E=732/0-4-9 Max Horz A=-283(LC 4) Max Uplift A=-50(LC 7), E=-50(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD A-B=-846/105, B-C=-617/179, C-D=-617/179, D-E=-846/104

BOT CHORD A-I=-140/534, H-I=-140/534, G-H=-10/534, F-G=-10/534, E-F=-10/534

C-H=-135/467, D-H=-261/190, B-H=-261/189 WEBS

NOTES-(10)

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, E.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.





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 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 **NoIVIT11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

A MiTek Affil 818 Soundside Road Edenton, NC 27932





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
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October 3,2017





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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Wilmington/
1238844_MASTER	FG01	GABLE	4	3	131241160
Builders FirstSource S	Sumter SC 29153			7 640 4	Job Reference (optional)
		1	D:XOjtQcF	jQu8X?Xj0	GN5R0bmzVOFf-IvpxwpCH?hsU2vcN3_802ck_x5ZA2zZp5eRlpxyXBdF
<ul> <li>13) This truss has been de 1-0-0, 18-11-0 to 19-11</li> <li>14) "Semi-rigid pitchbreaks</li> <li>15) Graphical purlin representation to the semi- tion of the semi-trust of the semi-tr</li></ul>	signed for a total drag load I-0 for 996.0 plf. s including heels" Member e sentation does not depict the ss is designed as an individu	of 100 plf. Lumber DOL=(1.33) Plate grip DOL nd fixity model was used in the analysis and d size or the orientation of the purlin along the t al building component. The suitability and us	=(1.33) C esign of th op and/or	Connect tr his truss. bottom c	uss to resist drag loads along bottom chord from 0-0-0 to hord.
building designer per A	NSI TPI 1 as referenced by	the building code.		omponen	for any particular ballaring to the responsionly of the
LOAD CASE(S) Standard 1) Dead + Roof Live (balar Uniform Loads (plf) Vert: A-F=-88/F	I Except: nced): Lumber Increase=1.1! =-14), F-G=-74, G-L=-88(F=	5, Plate Increase=1.15 	-)		
2) Dead + 0.75 Roof Live ( Uniform Loads (plf) Vert: A-F=-78(F	balanced) + 0.75 Uninhab. /	Attic Storage + 0.75 Attic Floor: Lumber Increa	se=1.15, M-AF=-1	Plate Incr 04(F=-70	ease=1.15 ). G-U=-27(F). F-R=-66(F)
3) Dead + Uninhabitable A Uniform Loads (plf) Vert: A-F=-48(F	ttic Without Storage: Lumbe	r Increase=1.25, Plate Increase=1.25 14), M-S=-84(F=-30), G-U=-24(F), F-R=-63(F	-)	0.(	
4) Dead + 0.6 MWFRS Win Uniform Loads (plf) Vert: A-F=25(F=	nd (Pos. Internal) Left: Lumb =-14), F-G=39, G-L=25(F=-1	er Increase=1.60, Plate Increase=1.60 4), M-S=-42(F=-30), G-U=8(F), F-R=-31(F)			
Horz: A-S=21, L 5) Dead + 0.6 MWFRS Win Uniform Loads (plf)	M=27 nd (Pos. Internal) Right: Lum	ber Increase=1.60, Plate Increase=1.60			
Vert: A-F=25(F= Horz: A-S=-27,	14), F-G=39, G-L=25(F=-1 L-M=-21	4), M-S=-42(F=-30), G-U=8(F), F-R=-31(F)			
6) Dead + 0.6 MWFRS Win Uniform Loads (plf) Vert: A-F=25(F=	nd (Pos. Internal) 1st Paralle 14) F-G=39 G-I =25(F=-1	<ul> <li>I: Lumber Increase=1.60, Plate Increase=1.60</li> <li>4) M-S=-42(F=-30) G-U=8(F) F-R=-31(F)</li> </ul>			
Horz: A-S=-26, 7) Dead + 0.6 MWFRS Win	L-M=26 nd (Pos. Internal) 2nd Paralle	el: Lumber Increase=1.60, Plate Increase=1.6	0		
Uniform Loads (plf) Vert: A-F=10(F=	=-14), F-G=24, G-L=10(F=-1	4), M-S=-42(F=-30), G-U=3(F), F-R=-36(F)			
Horz: A-S=-26, 8) Dead + 0.6 MWFRS Win Uniform Loads (plf)	L-M=26 nd (Pos. Internal) 3rd Paralle	el: Lumber Increase=1.60, Plate Increase=1.60	)		
Vert: A-F=3(F=- Horz: A-S=-26,	14), F-G=17, G-L=3(F=-14), L-M=26	M-S=-42(F=-30), G-U=1(F), F-R=-38(F)			
9) Dead + 0.6 MWFRS Win Uniform Loads (plf)	nd (Pos. Internal) 4th Paralle	el: Lumber Increase=1.60, Plate Increase=1.60	)		
Vert: A-F=-3(F= Horz: A-S=-26,	-14), F-G=11, G-L=-3(F=-14 L-M=26	), M-S=-42(F=-30), G-U=1(F), F-R=-38(F)			
Uniform Loads (plf)	Attic Storage: Lumber Increa	ase=1.25, Plate Increase=1.25	M-4E=-6	4(F=-30)	G-11=-19(F) F-R=-58(F)
11) Dead + 0.75 Roof Live Uniform Loads (plf)	(balanced) + 0.75 Uninhab.	Attic Storage + Drag LC#1 Left: Lumber Incre	ase=1.33	, Plate Ind	crease=1.33
Vert: A-F=-78( F-R=-66(F)	F=-14), F-G=-64, G-L=-78(F	=-14), S-AE=-64(F=-30), AE-AF=-94(F=-30),	M-AF=-64	(F=-30), (	G-U=-27(F),
12) Dead + 0.75 Roof Live	(balanced) + 0.75 Uninhab.	Attic Storage + Drag LC#1 Right: Lumber Inc	rease=1.3	3, Plate I	ncrease=1.33
Vert: A-F=-78( F-R=-66(F)	F=-14), F-G=-64, G-L=-78(F	=-14), S-AE=-64(F=-30), AE-AF=-94(F=-30),	M-AF=-64	(F=-30), (	G-U=-27(F),
Drag: A-L=-75 13) Dead + 0.6 MWFRS W Uniform Loads (plf)	, S-AD=747, M-AG=747 /ind (Pos. Internal) Left + Dra	ag LC#1 Left: Lumber Increase=1.33, Plate In	crease=1.	.33	
Vert: A-F=25(F Horz: A-S=21,	F=-14), F-G=39, G-L=25(F=- L-M=27	14), M-S=-42(F=-30), G-U=8(F), F-R=-31(F)			
14) Dead + 0.6 MWFRS W Uniform Loads (plf)	/ind (Pos. Internal) Left + Dra	ag LC#1 Right: Lumber Increase=1.33, Plate I	ncrease=	1.33	
Vert: A-F=25(F Horz: A-S=21,	<sup>-</sup> =-14), F-G=39, G-L=25(F=- L-M=27	14), M-S=-42(F=-30), G-U=8(F), F-R=-31(F)			
Drag: A-L=-10 15) Dead + 0.6 MWFRS W	0, S-AD=996, M-AG=996 /ind (Pos. Internal) Right + D	)rag LC#1 Left: Lumber Increase=1.33, Plate I	ncrease=	1.33	
Vert: A-F=25(F Horz: A-S=-27	<sup>=</sup> =-14), F-G=39, G-L=25(F=- , L-M=-21	14), M-S=-42(F=-30), G-U=8(F), F-R=-31(F)			
Drag: A-L=100	), S-AD=-996, M-AG=-996				

16) Dead + 0.6 MWFRS Wind (Pos. Internal) Right + Drag LC#1 Right: Lumber Increase=1.33, Plate Increase=1.33

Continued on page 3

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Job	Truss	Truss Type	Qty	Ply	H&H-NC/Wilmington/
1238844_MASTER	FG01	GABLE	4	์ ว	I31241160
				J	Job Reference (optional)
Builders FirstSource, S	Sumter, SC 29153			7.640 s	Aug 16 2017 MiTek Industries, Inc. Tue Oct 03 07:31:32 2017 Page 3
		IC	:XOjtQcF	jQu8X?XjQ	GN5R0bmzVOFf-lvpxwpCH?hsU2vcN3_802ck_x5ZA2zZp5eRlpxyXBdP
LOAD CASE(S) Standard	Except:				

Uniform Loads (plf)
Vert: A-F=25(F=-14), F-G=39, G-L=25(F=-14), M-S=-42(F=-30), G-U=8(F), F-R=-31(F)
Horz: A-S=-27, L-M=-21
Drag: A-L=-100, S-AD=996, M-AG=996
17) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel + Drag LC#1 Left: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert A-E=25(E=-14) E-G=39 G-I=25(E=-14) M-S=-42(E=-30) G-I=8(E) E-R=-31(E)
Hore: A-S=-26 1-M=26
Drac AJ = 100 S.AD=.006 M.AC=.006
Diag. ALT 100, CAD-500, MAC-500 18) Dead + 0.6 MMCEDS (Vind (Doc. Internal) tet Parallel + Drag I C#1 Picht: Lumber Increase=1.33. Plate Increase=1.33.
Iniferminade (Af)
Velt. A-F-20(F14), F-G-39, G-L-23(F14), N-S42(F30), G-U-6(F), F-K31(F)
Drag: A-L=-100, S-AD=996, M-AG=996
19) Dead + 0.6 MWFRS wind (Pos. Internal) 2nd Parallel + Drag LC#1 Lett: Lumber increase=1.33, Plate increase=1.33
Uniform Loads (pit)
Vert: A-F=10(F=-14), F-G=24, G-L=10(F=-14), M-S=-42(F=-30), G-U=3(F), F-R=-36(F)
Horz: A-S=-26, L-M=26
Drag: A-L=100, S-AD=-996, M-AG=-996
20) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel + Drag LC#1 Right: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: A-F=10(F=-14), F-G=24, G-L=10(F=-14), M-S=-42(F=-30), G-U=3(F), F-R=-36(F)
Horz: A-S=-26, L-M=26
Drag: A-L=-100, S-AD=996, M-AG=996
21) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel + Drag LC#1 Left: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: A-F=3(F=-14), F-G=17, G-L=3(F=-14), M-S=-42(F=-30), G-U=1(F), F-R=-38(F)
Horz: A-S=-26, L-M=26
Drag: A-L=100, S-AD=-996, M-AG=-996
22) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel + Drag LC#1 Right: Lumber Increase=1.33, Plate Increase=1.33
Uniform Loads (plf)
Vert: A-F=3(F=-14), F-G=17, G-L=3(F=-14), M-S=-42(F=-30), G-U=1(F), F-R=-38(F)
Horz: A-S=-26, L-M=26
Drag: A-L=-100, S-AD=996, M-AG=996
23) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel + Drag LC#1 Left: Lumber Increase=1.33. Plate Increase=1.33
Uniform Loads (plf)
Vert A-F=-3(F=-14) F-G=11 G-I=-3(F=-14) M-S=-42(F=-30) G-II=1(F) F-R=-38(F)
Horr: A-S=-26 1
Drag: A-I =100 S-AD=-996 M-AG=-996
24) Dead + 0.6 MWERS Wind (Pos, Internal) 4th Parallel + Drag I C#1 Right: Lumber Increase=1.33, Plate Increase=1.33
Vert A-F=-3(F=-14) F-G=11 G-I=-3(F=-14) M-S=-42(F=-30) G-I=1(F) F-R=-38(F)
$1012$ , $A_{0}=20$ , $E_{0}=206$ M.AC=206
25) Dead Drag L (#1 off) Lumbar Jacoson 13. Dista Jacoson 13.
Liniteral code (all)
Onition Loads (pin)
$v_{01}$ , $v_{1} = v_{01}$ , $v_{1} = v_{1}$ , $v_{2} = v_{01}$ , $w_{1} = v_{01}$ , $w_{1} = v_{01}$ , $v_{1} = v_{1}$ , $v_{1} = v_{01}$ , $v_{1} = v_{1}$ , $v_{1} = v_{01}$ , $v_{1} = v_{1}$ , $v_{2} = v_{1}$ , $v_{2} = v_{2}$ , $v_{2}$
Diag. ALT IVV, CHUMAR JARGE-390 28) Daad Iraq Lift Diath Lumbar Jacapaga 1.3. Diata Jacapaga 1.3.
Velt. A-F=-40(F=-14), F-G=-34, G-L=-48(F=-14), M-S=-04(F=-3U), G-U=-19(F), F-K=-08(F)
Drag: A-L=-100, S-AD=996, M-AG=996

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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Wilmington/	
1238811 MASTED	EG02	CARLE	3		1312411	61
1230044_WASTER	1 602	GABLE	5	3	Job Reference (optional)	
Builders FirstSource, S	Sumter, SC 29153			7.640 s	Aug 16 2017 MiTek Industries, Inc. Tue Oct 03 07:31:33 2017 Pag	ge 2
<b>NOTES-</b> (16)		ID:X	OjtQcFjQı	J8X?XjGN	5R0bmzVOFf-m5NJ79Cvm?_Lf3BadhfFapHAoVuVnQsyKIBJLOyXE	3dC

13) This truss has been designed for a total drag load of 100 plf. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 1-0-0, 18-11-0 to 19-11-0 for 996.0 plf.

14) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

16) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard Except:

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: A-L=-74, M-S=-79(F=-45), G-U=-30(F), F-R=-69(F) 2) Dead + 0.75 Roof Live (balanced) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor: Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: A-F=-114(F=-50), F-G=-64, G-L=-114(F=-50), S-AE=-149(F=-115), AE-AF=-179(F=-115), M-AF=-149(F=-115), G-U=-27(F), F-R=-66(F) 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: A-F=-84(F=-50), F-G=-34, G-L=-84(F=-50), M-S=-99(F=-45), G-U=-24(F), F-R=-63(F) 4) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: A-F=-11(F=-50), F-G=39, G-L=-11(F=-50), M-S=-57(F=-45), G-U=8(F), F-R=-31(F) Horz: A-S=21, L-M=27 5) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: A-F=-11(F=-50), F-G=39, G-L=-11(F=-50), M-S=-57(F=-45), G-U=8(F), F-R=-31(F) Horz: A-S=-27, L-M=-21 6) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: A-F=-11(F=-50), F-G=39, G-L=-11(F=-50), M-S=-57(F=-45), G-U=8(F), F-R=-31(F) Horz: A-S=-26, L-M=26 7) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60. Plate Increase=1.60 Uniform Loads (plf) Vert: A-F=-26(F=-50), F-G=24, G-L=-26(F=-50), M-S=-57(F=-45), G-U=3(F), F-R=-36(F) Horz: A-S=-26, L-M=26 8) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: A-F=-33(F=-50), F-G=17, G-L=-33(F=-50), M-S=-57(F=-45), G-U=1(F), F-R=-38(F) Horz: A-S=-26, L-M=26 9) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: A-F=-39(F=-50), F-G=11, G-L=-39(F=-50), M-S=-57(F=-45), G-U=1(F), F-R=-38(F) Horz: A-S=-26, L-M=26 10) Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: A-L=-34, S-AE=-119(F=-85), AE-AF=-159(F=-85), M-AF=-119(F=-85), G-U=-19(F), F-R=-58(F) 11) Dead + 0.75 Roof Live (balanced) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + Drag LC#1 Left: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: A-F=-114(F=-50), F-G=-64, G-L=-114(F=-50), S-AE=-149(F=-115), AE-AF=-179(F=-115), M-AF=-149(F=-115), G-U=-27(F), F-R=-66(F) Drag: A-L=75, S-AD=-747, M-AG=-747 12) Dead + 0.75 Roof Live (balanced) + 0.75 Uninhab. Attic Storage + 0.75 Attic Floor + Drag LC#1 Right: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: A-F=-114(F=-50), F-G=-64, G-L=-114(F=-50), S-AE=-149(F=-115), AE-AF=-179(F=-115), M-AF=-149(F=-115), G-U=-27(F), F-R=-66(F) Drag: A-L=-75, S-AD=747, M-AG=747 13) Dead + 0.6 MWFRS Wind (Pos. Internal) Left + Drag LC#1 Left: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: A-F=-11(F=-50), F-G=39, G-L=-11(F=-50), M-S=-57(F=-45), G-U=8(F), F-R=-31(F) Horz: A-S=21, L-M=27 Drag: A-L=100, S-AD=-996, M-AG=-996 14) Dead + 0.6 MWFRS Wind (Pos. Internal) Left + Drag LC#1 Right: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: A-F=-11(F=-50), F-G=39, G-L=-11(F=-50), M-S=-57(F=-45), G-U=8(F), F-R=-31(F) Horz: A-S=21, L-M=27 Drag: A-L=-100, S-AD=996, M-AG=996 15) Dead + 0.6 MWFRS Wind (Pos. Internal) Right + Drag LC#1 Left: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: A-F=-11(F=-50), F-G=39, G-L=-11(F=-50), M-S=-57(F=-45), G-U=8(F), F-R=-31(F) Horz: A-S=-27, L-M=-21 Drag: A-L=100, S-AD=-996, M-AG=-996 16) Dead + 0.6 MWFRS Wind (Pos. Internal) Right + Drag LC#1 Right: Lumber Increase=1.33, Plate Increase=1.33

Continued on page 3



Job	Truss	Truss Type	Qty	Ply	H&H-NC/Wilmington/	
1238844_MASTER	FG02	GABLE	3	2		31241161
				J	Job Reference (optional)	
Builders FirstSource,	Sumter, SC 29153			7.640	s Aug 16 2017 MiTek Industries, Inc. Tue Oct 03 07:31:33 20	17 Page
			ID.AOJIQCF	JQuoxinjo		JLOyABut
LOAD CASE(S) Standa	ard Except:					
Uniform Loads (plf)						
Vert: A-F=-1	1(F=-50), F-G=39, G-L=-1	11(F=-50), M-S=-57(F=-45), G-U=8(F)	, F-R=-31(F)			
Horz: A-S=-	27, L-M=-21	06				
17) Dead + 0.6 MWERS	Wind (Pos Internal) 1st F	2 Parallel + Drag I C#1 Left: Lumber Inci	rease=1.33 Plate In	crease=1 ?	33	
Liniform Loads (nlf)	Wind (1 03. Internal) 13(1	araller · Drag Lo# r Left. Lamber mer				
Vert: A-F=-1	1(F=-50), F-G=39, G-L=-'	11(F=-50), M-S=-57(F=-45), G-U=8(F)	. F-R=-31(F)			
Horz: A-S=-	26, L-M=26		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			
Drag: A-L=1	00, S-AD=-996, M-AG=-9	96				
18) Dead + 0.6 MWFRS	Wind (Pos. Internal) 1st F	Parallel + Drag LC#1 Right: Lumber In	crease=1.33, Plate	Increase=1	.33	
Uniform Loads (plf)						
Vert: A-F=-1	1(F=-50), F-G=39, G-L=-1	11(F=-50), M-S=-57(F=-45), G-U=8(F)	, F-R=-31(F)			
Horz: A-S=-	26, L-M=26					
10) Dead + 0.6 MWERS	Wind (Pos Internal) 2nd	10 Parallel + Drag I C#1 Left: Lumber Inc	rosco=1.33 Plato I	ncrosco=1	33	
Uniform Loads (plf)	Wind (1 03. Internal) Zha					
Vert: A-F=-2	26(F=-50), F-G=24, G-L=-2	26(F=-50), M-S=-57(F=-45), G-U=3(F)	, F-R=-36(F)			
Horz: A-S=-	26, L-M=26		, ()			
Drag: A-L=1	00, S-AD=-996, M-AG=-9	96				
20) Dead + 0.6 MWFRS	Wind (Pos. Internal) 2nd	Parallel + Drag LC#1 Right: Lumber Ir	ncrease=1.33, Plate	Increase=	1.33	
Uniform Loads (plf)						
Vert: A-F=-2	.6(F=-50), F-G=24, G-L=-2	26(F=-50), M-S=-57(F=-45), G-U=3(F)	), F-R=-36(F)			
HUIZ. A-S= Drag: A-L=-	20, L-1VI=20 100 S-AD=006 M-AC=00	26				
21) Dead + 0.6 MWERS	Wind (Pos Internal) 3rd [	Parallel + Drag I C#1 Left: Lumber Inc	rease=1.33 Plate Ir	ncrease=1 '	33	
Uniform Loads (plf)		araller * Brag Een + Eent Eamber me				
Vert: A-F=-3	3(F=-50), F-G=17, G-L=-?	33(F=-50), M-S=-57(F=-45), G-U=1(F)	, F-R=-38(F)			
Horz: A-S=-	26, L-M=26					
Drag: A-L=1	00, S-AD=-996, M-AG=-9	96				
22) Dead + 0.6 MWFRS	Wind (Pos. Internal) 3rd F	Parallel + Drag LC#1 Right: Lumber In	crease=1.33, Plate	Increase=1	.33	
Uniform Loads (plf)		2/(- EO) M O- E7/(- 4E) O H-1/(-)				
Horz A-S=-	3(r50), r-G-17, G-L3 26 1-M=26	55(F = -50), M = 3 = -57(F = -45), G = 0 = 1(F)	, r- <del>r</del> 30(r)			
Drag: A-L=-	100. S-AD=996. M-AG=98	96				
23) Dead + 0.6 MWFRS	Wind (Pos. Internal) 4th F	Parallel + Drag LC#1 Left: Lumber Inc	rease=1.33, Plate Ir	crease=1.3	33	
Uniform Loads (plf)						
Vert: A-F=-3	9(F=-50), F-G=11, G-L=-3	39(F=-50), M-S=-57(F=-45), G-U=1(F)	, F-R=-38(F)			
Horz: A-S=-	26, L-M=26					
Drag: A-L=1	00, S-AD=-996, M-AG=-9	96 Parallal - Drag I Otti Diabte Lumbar In	areas -1.22 Dista	l	22	
Liniform Loads (nlf)	wind (POS. Internal) 4th F	araller + Drag LC#1 Right. Lumber in	crease=1.55, Flate	Increase- I	.55	
Vert: A-F=-3	9(F=-50). F-G=11. G-L=-(	39(F=-50), M-S=-57(F=-45), G-U=1(F)	. F-R=-38(F)			
Horz: A-S=-	26, L-M=26					
Drag: A-L=-	100, S-AD=996, M-AG=99	96				
25) Dead-Drag LC#1 Le	ft: Lumber Increase=1.33,	Plate Increase=1.33				
Uniform Loads (plf)						

Vert: A-L=-34, M-S=-79(F=-45), G-U=-19(F), F-R=-58(F)

- Drag: A-L=100, S-AD=-996, M-AG=-996 26) Dead-Drag LC#1 Right: Lumber Increase=1.33, Plate Increase=1.33
- Uniform Loads (plf) Vert: A-L=-34, M-S=-79(F=-45), G-U=-19(F), F-R=-58(F) Drag: A-L=-100, S-AD=996, M-AG=996

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 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E, B except (jt=lb) F=119.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

SEAL 28677 October 3,2017

ENGINEERING BY ERENCO AMITEK Affiliate

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818 Soundside Road Edenton, NC 27932



REACTIONS. (Ib/size) B=320/0-3-0, D=201/0-1-8 Max Horz B=65(LC 6) Max UpliftB=-165(LC 6), D=-107(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-** (9)

Left: 2x4 SP No.3

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=165, D=107.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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1 1010 011									
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP					
TCLL	20.0	Plate Grip DOL 1.15	TC 0.24	Vert(LL) -0.01 D-I >999 360 MT20 244/190					
TCDL	10.0	Lumber DOL 1.15	BC 0.25	Vert(TL) -0.04 D-I >999 240					
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.01 B n/a n/a					
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.05 D-I >999 240 Weight: 20 lb FT = 20%					

#### LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

 WEDGE
 Left: 2x4 SP No.3

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (Ib/size) B=283/0-3-0, D=158/0-1-8 Max Horz B=56(LC 6) Max UpliftB=-148(LC 6), D=-84(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-** (9)

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) B=148.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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LOADING (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.25 BC 0.18 WB 0.00 (Matrix)	<b>DEFL.</b> in (loc) I/defl L/d Vert(LL) 0.00 A n/r 120 Vert(TL) 0.01 A n/r 120 Horz(TL) 0.00 D n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 15 lb         FT = 20%
			BRACING-	

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) D=148/4-0-0, B=213/4-0-0 Max Horz B=48(LC 7) Max Uplift D=-21(LC 6), B=-64(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(10)

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, B.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



October 3,2017





LOADING	G (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.17	Vert(LL) -0.01	D-G	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.17	Vert(TL) -0.02	D-G	>999	240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.01	С	n/a	n/a		
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.01	D-G	>999	240	Weight: 32 lb	FT = 20%

#### LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEDGE Left: 2x4 SP No.3

### Left. 274 OF 110.5

REACTIONS. (lb/size) C=128/Mechanical, B=242/0-5-8, D=62/Mechanical Max Horz B=216(LC 8) Max UpliftC=-123(LC 8) Max Grav C=128(LC 1), B=242(LC 1), D=92(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES- (8)

 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This true has been designed for basic lead combinations. Which includes a second with real reliance for multiple computer time leads.

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) C=123.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum

sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied. Rigid ceiling directly applied.



## NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;

end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, F.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum

sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, B, E.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum

sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







BOT CHORD

C-D: 2x4 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.2 WEBS

REACTIONS. (lb/size) B=239/0-5-8, E=186/Mechanical Max Horz B=79(LC 6) Max Uplift B=-37(LC 6), E=-39(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-(11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, E.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

except end verticals, and 2-0-0 oc purlins: C-D.

Rigid ceiling directly applied or 10-0-0 oc bracing.

October 3,2017

818 Soundside Road Edenton, NC 27932

SEAL 28677 C



TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEDGE Left: 2x4 SP No.3

BOT CHORD

Structural wood sheathing directly applied or 1-5-4 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (Ib/size) C=28/Mechanical, B=126/0-5-8, D=13/Mechanical Max Horz B=75(LC 8) Max Uplift C=-19(LC 8), B=-38(LC 8)

Max Grav C=28(LC 1), B=126(LC 1), D=23(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-(8)

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;

end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C, B.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular

building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING-TOP CHORD

TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD Rigid ceiling directly applied.

REACTIONS. (Ib/size) I=255/0-5-8, D=68/Mechanical, E=112/Mechanical Max Horz I=242(LC 8) Max Uplift D=-68(LC 8), E=-65(LC 8) Max Grav I=255(LC 1), D=68(LC 1), E=113(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

BOT CHORD H-I=-339/0

WEBS C-F=-154/262

### **NOTES-** (8)

 Wind: ASCÉ 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 This true has been depleted for basic load applications, which include approximate for multiple properties (load).

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, E.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum

sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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#### LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2 \*Except\*

 B-E: 2x4 SP No.3

### BRACING-

 TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

#### REACTIONS. (Ib/size) F=152/0-5-8, C=38/Mechanical, D=19/Mechanical Max Horz F=147(LC 8) Max Uplift C=-29(LC 7), D=-54(LC 8) Max Grav F=152(LC 1), C=38(LC 1), D=38(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES- (8)

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;

end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C, D.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular

building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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LOADING	(psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) I/c	lefi L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.14	Vert(LL) -0.00	F >9	999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.10	Vert(TL) -0.00	F >9	999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	E	n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL) 0.00	F >9	999 240	Weight: 13 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2 \*Exc

WEBS 2x4 SP No.2 \*Except\* D-E: 2x4 SP No.3

REACTIONS. (lb/size) E=50/Mechanical, F=148/0-5-8 Max Horz F=98(LC 7)

Max Uplift E=-57(LC 7), F=-44(LC 8)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E, F.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 2-0-0 oc purlins, except

end verticals, and 2-0-0 oc purlins: C-D.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Job	Truss	Truss Type	Qty	Ply	H&H-NC/Wilmington/
1238844 MASTER	J15	Half Hip	2	1	131241179
		- F			Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s	s Aug 16 2017 MiTek Industries, Inc. Tue Oct 03 07:31:40 2017 Page 1



LOADING TCLL TCDL BCLL	<b>G</b> (psf) 20.0 10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.12 BC 0.04 WB 0.00	DEFL.         in         (loc)         l/defl         L/d         PLATES         GRIP           Vert(LL)         -0.00         F         >999         360         MT20         244/190           Vert(TL)         -0.00         F         >999         240         MT20         244/190           Horz(TL)         -0.00         E         n/a         n/a         MT20         244/190	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL)         0.00         F         >999         240         Weight: 12 lb         FT = 20%	

#### LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\* C-D: 2x6 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

Plate Offsets (X,Y)-- [C:0-2-0,Edge]

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: C-D. Rigid ceiling directly applied or 6-0-0 oc bracing.

2x4 SP No.2 \*Except\* D-E: 2x4 SP No.3

REACTIONS. (lb/size) E=50/Mechanical, F=140/0-5-8 Max Horz F=59(LC 7)

Max Uplift E=-25(LC 7), F=-45(LC 8)

Max Grav E=62(LC 14), F=140(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2)

-0-10-8 to 1-10-4 zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber

DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E, F.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Scale = 1:12.3





<u>4-6-11</u> 4-6-11

Plate Of	TSETS (X, Y)	[ <u>C:0-2-8,0-0-5], [D:0-2-8,0-0</u>	1-5]								
LOADIN TCLL TCDL BCLL	I <b>G</b> (psf) 20.0 10.0 0.0 *	SPACING- 2- Plate Grip DOL 1 Lumber DOL 1 Rep Stress Incr Y	-0-0 <b>C</b> 1.15 T 1.15 B 7ES W	<b>SI.</b> C 0.04 C 0.07 B 0.00	DEFL. Vert(LL) Vert(TL) Horz(TL)	in n/a n/a 0.00	(loc) - - F	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0 Code IRC2009/TPI2007 (Matrix)								Weight: 13 lb	FT = 20%		
LUMBER- TOP CHORD 2x4 SP No.2				BRACING- TOP CHOF	RD	Structu	iral wood	sheathing	directly applied or 4-6	-11 oc purlins, except	

BOT CHORD

2-0-0 oc purlins: C-D.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES- (14)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 5) Provide adequate drainage to prevent water ponding.6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, F, B, E.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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REACTIONS. (Ib/size) A=-11/4-6-11, F=-11/4-6-11, B=175/4-6-11, E=175/4-6-11 Max Horz A=-35(LC 6) Max UpliftA=-30(LC 6), F=-25(LC 3), B=-34(LC 7), E=-13(LC 6) Max Grav A=21(LC 7), B=175(LC 1), E=175(LC 1)



- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- 12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, E, B, D.

10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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