

# THE MADISON

RALEIGH REVERSED

FIRE WALL REQUIREMENT NOTE: THESE PLANS WILL NOT BE BUILT INSIDE THE MINIMUM FIRE SEPARATION

These construction documents are in full compliance with the requirements of the North Carolina Residential Building Code - 2018 Edition including all amendments by the North Carolina Building Code Council adopted and effective as of the date of this issue of these documents

AREA SQ FT 1ST FLOOR 1205 2ND FLOOR 1600 TOTAL LIVING 2805

GARAGE >

LOT # >

GARAGE

**PORCHES** 

TOTAL

STRUCTURAL MEMBERS ONLY

3070 HAMMOND BUSINESS

PLACE, SUITE 171

RALEIGH, NC 27603

OFFICE: 919.380.9991

NC COL #: C-4361

RIGHT LEFT

ELEVATION # >

THE MADISON Raleigh, reversed 395 80

3280

01/24/19

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**PAGES** 

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Design Specifications

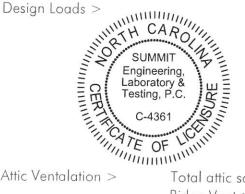
Use Group > One and two family dwelling

Construction Class > Unprotected

Height & Area Limit > 35 feet maximum 2 story height

Emergency Escape > Egress or rescue windows from sleeping rooms shall have minimum of 4.0 sq ft net clear opening. Min width 20"; min height 22"; & max sill height 44".

 $Garage > House \ ceiling > House \ assembly > 1/2" \ gypsum \ board \ wall \ \& \ 5/8" \ type \ X \ gypsum \ board \ ceiling \ where \ living \ is$ above w/ 20 minute garage/house door.



Live Load > Sleeping = 30 psfNon-sleeping = 40 psf

Dead Load > 10 psf Decks > 40 psf

Ultimate Wind Speed > 130 mph

Stair Load > 40 psf Roof Live Load > 20 psf

Assumed Lateral Soil Pressure > 30 psf

Attic Ventalation >

Total attic sq ft / 300 =sq ft free area reg'd

Ridge Vent > Linear feet of vent X 18 sq in free area / 12 = sq ft provided Soffit Vent > Linear feet of vent X 7 sq in free area / 12 = sq ft provided

Edge Shingle-Over Vent > Linear feet of vent  $\times$  9 sq in free area / 12 = sq ft provided Roof Louver Vents > 70 sq in free area X number of vents / 12 =sq ft provided

Crawl Space Ventalation > Total crawl space sq ft / 300 = sq ft free area reg'd Foundation Vents > Free space provided by vent = F

Free area reg'd / F = number of vents reg'd

# GENERAL NOTES:

SECTION THROUGH STAIR

1ST FLOOR PLAN

2ND FLOOR PLANN

- 1) DO NOT SCALE DRAWINGS; DIMENSIONS SHOWN SHALL BE USED
- IN PREFERENCE TO MEASUREMENTS BY SCALE

MONOLITHIC FOUNDATION PLAN STEMWALL FOUNDATION PLAN

- 2) CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AT SITE BEFORE PROCEEDING WITH ANY WORK
- 3) NOTES INDICATING TYPICAL CONDITIONS SHALL APPLY TO ALL LIKE AREAS UNLESS NOTED OTHERWISE

STANDARD ENGINEERING DETAILS

STANDARD ENGINEERING DETAILS

S- STRUCTRUAL PAGES BY ENGINEER

- 4) CONTRACTOR SHALL VERIFY AND COMPLY TO ALL LOCAL AND NATIONAL BUILDING CODES
- 5) ALL SUB-CONTRACTORS SHALL BE RESPONCIBLE FOR THEIR DAMAGE TO OTHER TRADES

### ELECTRICAL NOTES

- 1) ELECTRICAL INSTALLATION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE NATIONAL ELECTRIC CODE AND ALL APPLICABLE LOCAL CODES
- 2) LOCATION OF EXISTING UNDERGROUND UTILITY LINES AND STRUCTURES SHALL BE DETERMINED AND VERIFIED IN THE FIELD PRIOR TO COMMENCING WORK. ELECTRICAL CONTRACTORS SHALL BE RESPONSIBLE FOR THEIR DAMAGE TO OTHER TRADES
- 3) ELECTRICAL BOXES INSTALLED IN FLOORS, WALLS, OR CEILINGS SHALL BE MOUNTED FLUSH WITH FINISHED SURFACE AND CONDUITS AND/OR CABLES SHALL BE CONCEALED UNLESS OTHERWISE NOTED 4) EXTERIOR OUTLETS ARE WP /GFI
- 5) OUTLETS IN GARAGE OR WITHIN 6' OF A WATER SOURCE ARE GFI

G. Snow load 10 PSF or as required by Code All designs are in accordance with the 2018 North Carolina Residential Building Code, and the 2015 International Residential Code (IRC). Refer to the relevant Code for any additional information not covered in these notes or the designs.

20 PSF or as required by Code.

130 MPH, Ultimate, Exposure B

2. Engineering design is for structural information only. The Engineer of Record does not accept responsibility for dimension errors, architectural errors, detailing of waterproofing, plumbing. electrical, or mechanical information or any part of the plan not relevant to the structural information.

#### RESIDENTIAL FOUNDATIONS:

Roof live load

Wind load

- All continuous wall footings are to be installed as noted on the plans and per the standard details package. Reinforcing is to be as noted on plans and per the standrad details package.
- 2. All interior piers are 8"x16" CMU up to a maximum height of 32". All piers over 32" high must be filled solid with Type S mortar. Maximum height for 8"x16" filled pier is 5'-0". Piers larger than 8"x16" are noted on plans and must be filled with Type S mortar. For one-story structures, pier caps are to be 4" solid masonry. For two-story structures, pier caps are to be 8" of solid masonry
- 3. Footings for 8"x16" piers are as noted on plans and per standard details package. Reinforcing is to be as noted on plans and per the standard details package.
- 4. Interior thickened slab footings which occur in basements and "slab on grade" floors are 10" deep by 16" wide unless noted otherwise. Thickened footings are required under all bearing walls.
- 5. All rebar splices shall be a minimum of 2'-0" unless otherwise noted.
- Shallow foundations are designed for an assumed soil bearing capacity of 2,000 psf. The contractor is responsible for notifying the Engineer of Record if any soils are found to be unsuitable for this bearing capacity. The contractor is responsible obtaining soil testing to ensure that the bearing capacity of the soil meets or exceeds this value. All fill is to be compacted to 95% density as measured by the Standard Proctor Test (ASTM D-1557)
- 7. All soils and fill under floors within and/or under buildings shall have preconstruction soil treatment for protection against termites. Certification of Compliance shall be issued to the Building Department by a licensed pest control company.
- 8. All footing excavations shall be neat, straight, and level in the proper elevations to receive the concrete. Excessive variations in the dimensions of footings or slab will not be permitted. Reinforcing steel and mesh shall be accurately placed and supported to maintain their position during the concrete pouring. Edge forms shall be used for concrete that will be exposed.
- 9. All slab penetrations are to be the responsibility of the contractor. Penetrations interfering with reinforcing shall be approved by the Engineer of Record prior to the placement of concrete
- 10. Elevation difference between the bottom of adjacent footings shall be a maximum of one foot less than the minimum horizontal footing distance - for stepped footings. Differential heights between footings can 6 become excessive usually where a pier footing in a crawlspace or garage footing is next to a basement wall footing.

#### SPECIAL FOUNDATION CONSIDERATIONS:

- Waffle slabs are self-supporting slabs reinforced according to details and do not require firm soil for support. Soil must only be capable of supporting concrete until it hardens and develops strength.
- Caisson foundations shall be a minimum of 12" diameter drilled unreinforced concrete caissons. Caissons shall extend to a minimum depth providing 2' penetrations into good original ground. A caisson cannot be used if water rises immediately into a drilled hole. Piles will have to be used in such cases. (6'-
- 3. Treated wood piles with a minimum diameter of 8" and a minimum design load of six tons are used for all foundations with unsuitable soil deeper than 13' or with water in drilled caisson holes. Drive per North Carolina or South Carolina Code.
- 4. Sizes and reinforcing for footing caps over caissons or piles shall be as shown on plans.
- Chimney footings are to be 12" larger than the chimney footprint by 12" thick.
- 6. Foundation walls backfilled with dirt which support structural framing shall be constructed as follows: A. For earth fill up to a maximum height of 41: Use 8" Solid Grouted CMU or 8" brick with Bituthene
- membrane waterproofing on exterior. Footings are to be 8"x16" or 8"x24" as noted on the plan. B. For earth fill 4' to a maximum height of 9': Use 8"x24" footing with #4 @ 16" dowels hooked in
- footing and projecting 18" above footings. Use 12" CMU walls with #4 @ 16" vertical bars located 4" from non-dirt fill face, lap all splices 12" and use Dur-o-wall horizontal reinforcing every 8" in CMU joints. Install 1-#3 L-bar with 24" legs in every other joint horizontally at all corners; i.e., #3 corner bars @ 16" o/c vertically. Fill all open cells of CMU with either type S or M mortar or fill with 2,500 psi concrete. Install waterproof Bituthene membrane or equal.
- C. In lieu of the preceding design, basement walls may be constructed in accordance with R404.1 of the Code. However, 24"x24", #3 corner bars shall be installed at 16" o/c vertically regardless of the wall height. ERECT ALL FRAMING BEFORE BACKFILLING
- When floor joists are parallel to basement walls, block between joists @ 24" o/c for three joist spacings. For retaining walls without framing see special designs on drawings.

FRAMING CONSTRUCTION - OTHER THAN ROOF

- See Table R602.3(1) of the Code for a fastener schedule for structural members.
- Wood beams shall be supported by metal hangers of adequate capacity where framing into beams or ledgers. The following hanger schedule may be used unless noted otherwise on the plan: Simpson® Hangers

(2) 2x8 · HUS 28-2 (2) 2x10 HU 210-2 (Max) (2) 2×12 ... HU 212-2 (Max) (2) 1 3/4" x 9 1/4" LVL ... HU 410 (Max) (2) 1 3/4" x 11 7/8" IVI - HU 412 (Max) (2) 1 3/4" x 14" LVL..... HHUS 410 All Triple LVL Members ...... . HHUS 5 50/10

Note: This table shows Simpson® face hangers. Other hangers may be used so long as they are equal to or stonger than those listed.

Note: #1 or better shall be used for Southen Pine headers and girders. Other tabulated values assume #2 grade lumber per 2018 NC Residential Code Table 602.7(1) and (2) footnote B

- Crawl girders and bands with 4" curtain wall and pier construction shall be 2-2x10 Southern Yellow Pine #1 unless noted otherwise. Maximum clear spans are to be 4'-8" (6'-0" o/c spacing of piers). To avoid objectionable cracking in finished hardwood floors over any girders, use the following procedure:
  - A Nailina
- i.) All floor joists must be toenailed to their support girders with a minimum of 3-8d nails at each end. Larger nails will split and render the toenail ineffective. No end nailing through the girder or band is permitted.
- ii.) If dropped girders are used, end lap all joists and side nail each with a minimum of 3-16d nails at each end of each joist. If flush girders are used, support joists at girder with hangers or ledger
- iii.) Nail multiple member built-up girders with two rows of 16d nails staggered at  $32^{\circ}$  o/c,  $2^{\circ}$ down from the top and 2" up from the bottom with 3-16d nails at each end of each piece in the joist through the members making up the multiple girder.
- iv.) This nailing pattern will ensure a tight floor from the outside of the house to the outside so that when the framing shrinks during the first heating season, the shrinkage will be uniformly distributed over the entire floor. If the girder-nailing pattern is omitted, then the shrinkage will accumulate over the girders and an objectionable crack will develop in the finished hardwood floor over the girder line.
- B. At all girders where the joists change direction, install bridging at 6' o/c for a minimum of six joist spacings beyond any joist direction change. This will insure shrinkage distribution over the floor and not let it accumulate at the girder.
- C. There must be wood blocking thru bolted to the steel beam with joists toenailed or attached to the beam with metal hangers under any hardwood floors that pass over a steel beam supporting floor joists. This condition often exists over basement areas.
- All other lumber may be Spruce #2 unless noted otherwise.
- Steel beams must have (5)-2x4 or (4)-2x6 studs under each end U.N.O. The top flange shall be covered with wood blocking fastened with 2 rows 1/2"dia. lag-screws 12" o/c staggered, or powderactuated fasteners, Simpson TB Screws, or an approved method.
- "Lam" beams must have (3)-2x4 or (2)-2x6 studs under each end U.N.O.
- Masonry lintels:
  - A. For openings up to 6': Use 3 1/2" x 3 1/2" x 1/4" steel angles.
  - B. For openings from 6' to 10': Use 5" x 3 1/2" x 5/16" steel angles.
- C. For openings from 10' to 18': Fasten 6" x 4" x 5/16" steel angle to wood header with (2) 1/2"dia. lag screws staggered @ 16" O.C. Extend angle 6" past opening to bear on masonry veneer at ends
- D. When structural steel beams with bottom plates are used to support masonry, the bottom plate must extend the full length of the steel beam. Provide a minimum 4" bearing at the end of all steel headers. This supports the ends of the plate by bearing on the adjacent masonry jambs. The beam should be temporarily shored prior to laying the masonry. The shoring may be removed five days after laying the masonry
- All brick veneer over lower roofs (brick climbs) must have a structural angle lag screwed to an adjacent stud wall in accordance with detail, with steel brick stops to prevent sliding of brick.
- All rafter braces must have two studs from plate through all floors to the foundation or supporting beam below. No braces shall be attached to top wall plate without studs directly under them.
- Where non-load bearing partitions fall between floor joists or trusses, 2x4 ladders @ 16" o/c must be placed perpendicular to the trusses to support the plywood decking. The ladders shall be supported with a Simpson "Z" clip or similar device. A double joist can also be used and is allowed to be separated 4" Max. to allow for plumbing and wiring.
- 11. All wood I-joists and open joists must be braced in accordance with the manufacturer's directions plus details shown on plans. Load-bearing partitions, jacks, beams and column supports must be solid blocked through floor. Trusses and plywood cannot carry concentrated point loads. I-joist material should not be used as blocking under concentrated point loads. All point loads must be carried to foundations with adequate blocking and/or beams.
- 12. All steel columns shall bear on concrete, masonry, or steel only. Beams that bear on top of steel columns shall be welded to the column. Where steel columns bear on concrete or masonry, unless otherwise noted, a 5/8" x 6 1/2" x 6 1/2" or 5/8" x 3 1/2" x 10" base plate shall be used to spread the column load across the bearing surface. Base plates shall be bolted with at least two 1/2"dia. anchor bolts or expansion bolts to concrete or masonry.
- 13. Unless noted otherwise on the plans, all exterior facing stud walls taller than 10' shall be constructed

A. Walls 10' to 11' high: Balloon frame 2x4 SPF#2 studs at 12" o/c with 1/2" OSB sheathing and 3 king studs on each side of each opening nailed securely to the header.

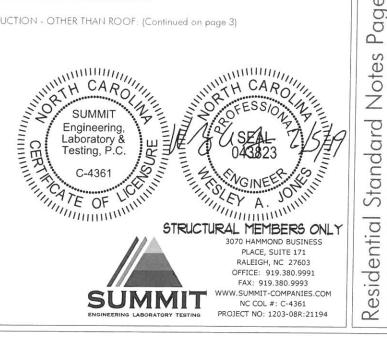
FRAMING CONSTRUCTION - OTHER THAN ROOF: (Continued)

- 14. Continuous 2x6 bridging shall be nailed to diagonal or vertical web members of all open-web floors trusses over 10' long. They shall be installed near mid-span as a load distribution member. If the 2x6 bridging is not continuous, lap ends of bridging one truss space.
- 15. Lower stud walls for buildings over two stories, but not more than three stories:
  - A. Interior walls
    - i.) Load bearing 2x4 @ 12" o/c for up to 10-0", or 2x6 @ 16" o/c if taller than 10'-0"
    - ii.) Non load bearing 2x4 @ 16" o/c under 12'-6"

Use 2x6 @ 16" o/c with 1/2" exterior grade plywood sheathing solid on walls.

- 16. Headers shall be as follows unless noted differently on plans:
  - A. Interior and exterior
  - i) Spans up to 2'-6" 2-2x6's 1 Jack Stud Each Side
  - ii) Spans 2'-6" to 3'-6" 2-2x8's 1 Jack Stud Each Side
  - iii) Spans 3'-6" to 6'-6" 2-2x10's 2 Jack Stud Each Side
  - iv) Spans 6'-6" or more See Plan See Plan
  - B. Number Of King Studs Required At Each End of wall openings shall be per footnote d of Table R602.3(5)
- 17. When ceiling joists are parallel to an exterior wall, tie the rafters near the top plate to ceiling joists with a 2x6 strongback a minimum of 6' long at 4' o/c across the top of the ceiling joists. 2x4 rafter ties shall be fastened to the side of the rafter and the strongback.
- 18. At all exterior diagonal wall panels (i.e. bay windows), each panel shall be nailed to each adjacent panel with 5-16d nails or tied together with metal strapping nailed at four locations between floors with a minimum of 2-16d nails into each panel at each strap. This will avoid vertical cracking in panel joints due to horizontal oscillating panels.
- 19. At all stairs, every stud at each stringer must be nailed to each stringer with a minimum of 2-16d nails. This will avoid cracking between wallboard and top of base molding due to vertical oscillation of stair stringers.
- 20. Roof trusses that have non-bearing partitions passing under them should be nailed to the partition plates to avoid ceiling-wall cracking.
- Roof trusses close to side walls framing and used as dead wood for sheetrock boards should be nailed to the wall framing to prevent ceiling-wall cracking.
- 22. All structural framing lumber exposed directly to the weather or bearing directly on exterior masonry piers or concrete shall be treated. All wood in contact with the ground is to be ground-contact approved. All wood exposed directly to the weather shall be protected to prevent the occurrence of
- 23. Unless otherwise detailed, all stick-built chimneys shall be constructed with 2x4 studs at 12" o/c, balloon-framed from attic ceiling or floor. Fasten 15/32" CDX plywood on all sides of the chimney along the full length of the studs. Fasten each stud to the supporting beam or ceiling joist with a 1 1/ 2"x24", 18-gauge metal strap, or a similar connector. Fasten beam down to support studs with (2)
- 24. All point loads from roof braces, jack studs, beam supports -whether wood or steel-cannot bear on sheathing alone. Blockingequal to or better than the point load supports above must be carried through all construction to the foundation.
- 25. Note to apply for all hard coat stucco exterior finishes:
  - A. Joints are necessary at the following locations:
  - Horizontally at each floor line.
  - No areas larger than 144 S.F. surface exposed.
    - No dimension longer than 18'.
  - iv) No dimension longer than 21/2 times the shortest dimension
  - B. Drip screed required at the bottom of all walls 2" above paved areas and 4" above grade.
  - C. See ASTM 926 and 1063 for further information.

FRAMING CONSTRUCTION - OTHER THAN ROOF: (Continued on page 3)





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#### FRAMING CONSTRUCTION - OTHER THAN ROOF: (Continued)

- 26. All "Self Supporting Stairs" must be connected to adequate framing to support the load of the stair case. It is the stair manufacturer's responsibility to provide the E.O.R. with all point loads prior to construction.
- 27. All studs, which support the bearing ends of steel or LVL beams, greater than four plies (i.e. 7-2x4 or 5-2xó studs etc.) are to be fastened with adhesive or equivalent nailed per R602.3(1). This adhesive is to be applied to the wide face of each stud.
- 28. Openings in garage wall shall comply with <u>Section R302.5.</u> This provision does not apply to garage walls that are perpendicular to the adjacent dwelling unit wall.

#### TABLE R302.6 DWELLING/GARAGE SERARATION

SEPARATION	MATERIAL
From the residence and attic	Not less than 1/2-inch gypsum board or equivalent applied to the garage side
From all habitable rooms above the garage	Not less than 5/8-inch Type X gypsum board or equivalent
Structure (s) supporting floor/ceiling assemblies used for separation required by this section	Not less than 1/2-inch gypsum board or equivalent
Garages located less than 3 feet from a dwelling unit on the same lot	Not less than 1/2-inch gypsum board or equivalent applied to the interior side of the exterior walls that are within this area

#### ROOF CONSTRUCTION:

- All roof trusses must be built in accordance with truss manufacturers' requirements. Tie-down connections to resist uplift shall be installed where required. When roof truss manufacturers do not provide the required connectors, it is the responsibility of the contractor to notify the roof truss engineer or the Engineer of Record to provide an adequate connector.
- Roof trussses shall be tied down to walls per truss manufacturer's specifications. Stick framed rafters shall be tied down to the top plates of walls, headers and beams per table R802.11 for 130 mph Ultimate wind speed; 33' mean roof height; exposure B. If site conditions are found to be outside the scope of this table, contact Summit Engineering, Labratory & Testing, PC, for further consultation.
- 3. Rafters shall be 2x6 SYP#2 @ 16" o/c for shingles with 7/16" OSB sheathing with one layer of 15# felt unless noted otherwise. They are to be cut into hips, ridges, etc., unless noted otherwise. Tile, slate and other heavy roof coverings shall use 2x8 SYP#2 @ 16" o/c with 5/8" minimum OSB sheathing with (2) layers of 15# felt, unless noted otherwise.
- 4. Collar ties shall be 2x6 @ 48" o/c at all ridges unless noted otherwise and located a nominal 3' below the ridge. Vaulted ceilings require special collar tie or ridge beam details. See the end of Table R802.5.1 1. Masonry walls are to be of the sizes and in the locations shown on the plans and shall be constructed in in the Code unless otherwise detailed on the plan.
- 5. A minimum of three collar ties shall be used at all ridges even if two ties must be put on one set of
- 6. All hips and ridges are 2x10 SYP#2 unless noted otherwise.
- 7. All "HOGS" shall be composed of two 2x6's or a 2x6 nailed to a 2x8, as indicated on the plan. The boards shall be fastened together at their ends with 16d nails at 4" on center to form an "L" shape (See detail at lower right this page). All hogs on ceiling joists or rafters are 12' long 2x6's unless noted otherwise. Rafters may be spliced over hogs. Splice rafter hogs only at a roof brace.
- 8. Gable end framing must be braced parallel to ridges with a minimum of 2x6 diagonal braces @ 6' o/c along the gable wall to interior ceiling joists. Braces to bear on 2x6 hogs and to the gable wall at approximately mid-height of gable walls. Braces shall be at an angle of approximately 45°. Other bracing may be used with the design engineer's approval.
- 9. Ceiling joists when erected parallel to rafters must be sistered to rafters and nailed with 3-16d nails at each rafter. If a kneewall is used and ceiling joists do not intersect with rafters, then the rafters must be tied 6. Mortar protrusion shall be less than 1/2". A protrusion of 1/2" or greater must be removed before to the ceiling joists using 2x4 kikers or rafter ties spaced no more than 48" o/c or every third rafter.
- 10. Roof Plan Legend:

#### Indicates location of roof brace point at rafter level. Arrow away from the brace point indicates direction of roof brace to partition, beam, or other brace point below. C. Arrow into brace point indicates a vertical or almost vertical roof brace to partition, beam, or other brace point below. Roof braces under 7'-0" are 2-2x4 nailed with 16 penny nails @ 9" o/c vertically from top to bottom. Braces longer than 7'-0" are (2)-2x6 T-braces. Braces longer than 10' must be braced horizontally in two directions at mid-height. Maximum spacing of roof braces are to be as follows (unless noted otherwise on plans):

#### MATERIALS SPECIFICATIONS:

#### Concrete General Notes:

- 1. Except where otherwise noted, for all concrete, the proportions of cement, aggregate, and water to attain Lumber General Notes: required plasticity and compressive strength shall be in accordance with ACI 318 Code. Concrete shall be 1. All common framing lumber is to meet the following minimum specifications at 19% moisture content. 2,500 PSI in 28 days for footings and 3,000 PSI for walls, beams and columns, unless noted otherwise
- 2. Before placing concrete, all debris, water and other deleterious material shall be removed from the places to be occupied by the concrete. The placing of all concrete shall be in accordance with ACI 318 and ASTM C94 requirements. Pumping of concrete will be permitted only with the Engineer of Record's approval of proposed concrete mix and method of pumping. Concrete shall be rapidly handled from the mixer to forms 2. All Structural Composite Lumber (LVL, LSL, PSL) is to meet the following minimum specifications: and deposited as nearly as possible to its final position to avoid segregation due to rehandling. Concrete to be spaded and worked by hand and vibrated to assure close contact with all surfaces of forms and reinforcing steel and leveled off at proper grade to receive finish. All concrete shall be placed upon clean, damp surfaces. Vibration shall be applied directly to the concrete and shall be sufficient to cause flow of settlement but not long enough to cause segregation of the mix.
- 3. Construction joints shall be located in accordance with ACI 301. All reinforcing steel shall be continuous across joints. In slabs on grade, saw contraction joints shall not be over 20 feet center to center each way. Joints shall be sawn a depth of one-third of the slab thickness. Sawing of the joints shall commence as soon as the concrete has hardened sufficiently to permit sawing without excessive raveling. Fill the saw cuts with approved joint filler after the concrete has cured.
- 4. Concrete, when deposited, shall have a temperature not below 50° F and not above 90° F. The methods and recommended practices as described in ACI 306 shall be followed for cold weather concreting and ACI
- 5. Freshly placed concrete shall be protected from premature drying by one of the following methods:
  - Ponding or continuous sprinkling.
  - Absorptive mat or fabric kept continuously wet.
  - Waterproof paper conforming to ASTM C171.
  - Application of an approved chemical curing compound.

The curing shall continue until the cumulative number of days when the ambient temperature above 50° F has totaled seven. During curing, the concrete shall be protected from any mechanical injury, load stresses, 4. All other shapes not listed above shall conform to ASTM A36 having a minimum yield stress of 36,000 shock, vibration, or damage to finished surfaces.

6. Reinforcing steel bars shall be deformed in accordance with ASTM A305 and or A408 and formed of ASTM A615-78 Grade 60 steel. Welded wire fabric reinforcing to be ASTM A185 steel wire. Accessories shall conform to the CRSI "Manual of Standard Practice." The following minimum concrete cover shall be provided over reinforcing bars:

Α.	Exposed to Earth
В.	Exposed to Weather
C.	Slabs not Exposed to Weather 8"
D.	Beams and columns

#### Masonry General Notes

- accordance with the provisions of ACI 530.
- 2. Hollow Load Bearing Units: ASTM C90 made with lightweight or normal weight aggregates. Grade N-I units shall be provided for exterior and foundation walls. Grade N-I or S-I units shall be provided for other load-bearing walls or partitions.
- 3. Concrete Building Brick: ASTM C55 made with lightweight or normal aggregates, Grade N-I or S-I except that brick exposed to weather shall be N-I.
- 4. Mortar: ASTM C270-95, Type S prepackaged mortar mix which shall not contain any non-cementitious fillers combined with not more than three parts sand per one part mix.
- 5. Reinforcing Steel: ASTM A615 Grade 60 steel deformed bars where indicated on the plans. Where reinforcing bars are installed in the cells of concrete masonry units, they shall be secured with wire ties at intervals not exceeding 24" o/c to maintain the bars location in the cell. The tolerance for spacing of vertical bars is  $\pm 2$  inches along the length of the wall. The tolerance for the distance between the face of the concrete masonry unit and the center of the bar shall not exceed  $\pm 1/2^{\circ}$ .
- 7. Horizontal Joint Reinforcement: ASTM A82 fabricated from cold drawn steel wire and hot dip zinc coated (ASTM A153). It shall consist of two or more parallel, longitudinal wires 0.1875" in diameter with weld-connected cross wires 0.1483" in diameter at a maximum of 16" o/c. Joint reinforcement is to be installed in every other course and in the first two courses at the bottom and top of wall openings and shall extend not less than 24" past the opening. Splices shall overlap not less than 12".
- 8. Execution: Masonry units shall be laid in a running band pattern unless noted otherwise. The walls shall be carried up level and plumb within the tolerances specified in ACI 530.1-88, Section 2.3.3.2. If nonstandard dimensions are encountered, block shall be cut with a masonry saw to fit, not by stretching or shrinking joints. Unfinished work shall be stepped back for joining with new work. Toothing will not be permitted except where specifically approved. Damaged units are to be cut out and new units set in place.
- 9. The filled cells and bond-beam blocks of reinforced masonry walls are to be filled with ASTM C476-91 Grout for Masonry with minimum compressive stress of 2,000 psi and slump range of 8" to 11". The outside face of the bottom block of each cell is to be broken out for inspection of reinforcing and clean out of mortar droppings in cell. The grout is to be placed in cells in maximum 5' lifts and immediately vibrated to minimize voids within the grout. Reconsolidate each lift by vibrating several inches into the preceding lift before plasticity is lost. Reconsolidate the top lift and fill with grout any spaces left by settlement or shrinkage.

MATERIAL	Fb (psi)	Ft (psi)	Fc (psi)(Perp.)	E (psi)
#2 Spruce Pine Fir	875	450	425	1,400,000
#2 Southern Yellow Pine	750	450	565	1,600,000

APPLICATION	Fb (psi)	Fc (psi)(Parallel)	Fc (psi)(Perp.)	E (psi)
Girders & Beams (LVL, PSL)	2,600	2,510	750	1,900,000
Columns (LSL) & Rimboards	1,700	1,400	400	1,300,000

3. All Glue Laminated Timber (Glu-lam) is to meet the following minimum specifications:

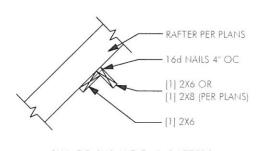
APPLICATION	Fb (psi)	Fc (psi)(Parallel)	Fc (psi)(Perp.)	E (psi)
Girders & Beams	2,400	1,700	740	1,700,000
Columns	1,600	1,550	560	1,500,000

- Three-ply side-loaded (joists frame into the side at the outside plies) or four-ply LVL beams: fasten all plies together with two rows of 1/2"dia. bolts at 12" o/c. The bolts shall be located a minimum of 2 1/2" and a maximum of 3 1/2" from the top or bottom of the beam.
- Built-up wood columns consisting of multiple studs shall have each lamination nailed with 16d nails at

#### Steel General Notes:

- All steel wide flange beams shall conform to ASTM A572 having a minimum yield stress of 50,000 psi All steel pipes shall be Schedule 40 or better with a minimum yield stress of 35,000 psi.
- All steel tubes shall conform to ASTM A500, Grade B, having a minimum yield stress of 46,000 psi.
- Unless otherwise noted, all welds shall be fillet type with a minimum 3/16" leg. Welding electrodes shall be E70xx type having a minimum yield strength of 70,000 psi. Welding work and materials shall conform to the American Welding Society Welding Code (AWS D.1).

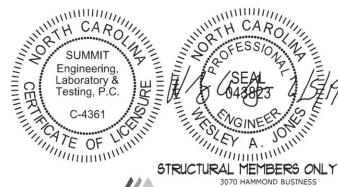
Bolted connections shall include high strength bolts conforming to ASTM A325. Foundation anchor bolts or tie rods shall conform to ASTM A36 having a minimum yield strength of 36,000 psi.



@ 9" OC T-BRACE

2X6 OR 2X8 HOG @ RAFTERS

2X4 ROOF BRACE



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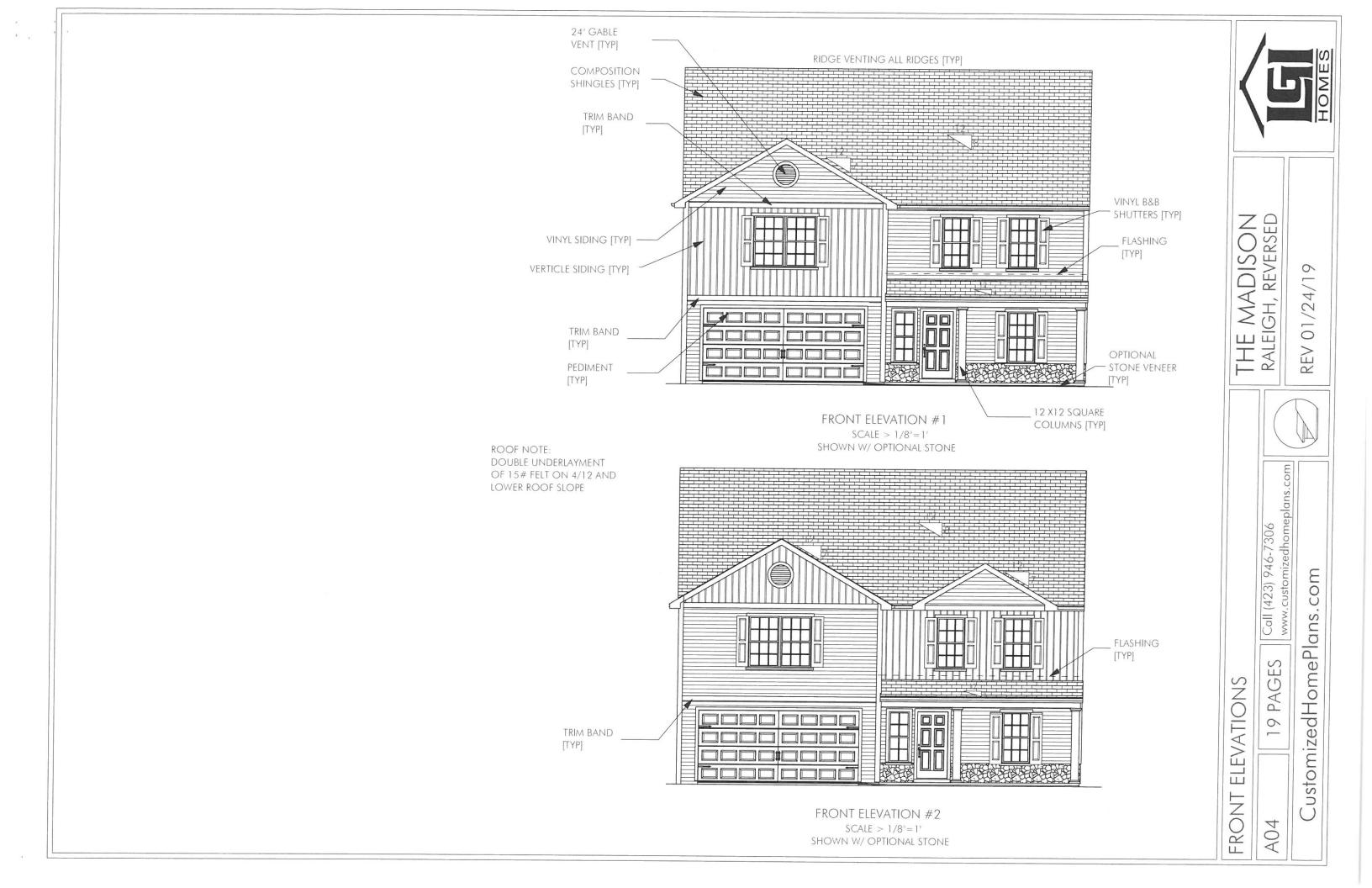
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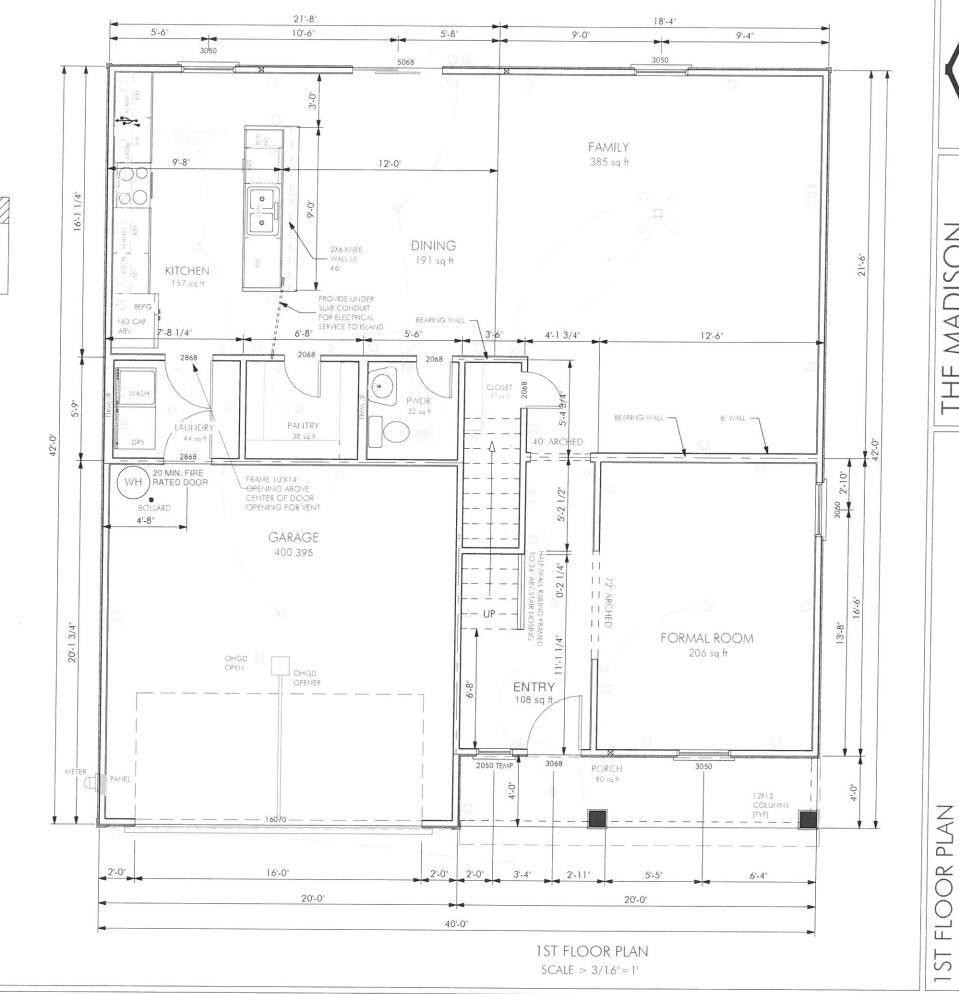
ELECTRICAL SYMBOL LEGE	ND
SWITCH	\$
3-WAY SWITCH	\$
DEWNER SWITCH	\$
WATER PROOF SWITCH	5
LTOV COUTLET	9
220V OUTLET	A
GFLOUTIET	P
WATER PROOF OUTLET	Ψ.
TELEPHONE JACK	Z
CATS HETWORK	Z,
CHRIES	6652
DOGRBEU.	(0)
THERMOSTAL	0
CARBON MONDADE DETECTOR	0
SMOKE DETECTOR	0
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ELECTRICAL SYMBOL LEGE	ND 2
STANDARD LIGHT FITTURE	-<
WALL MOUNTED LIGHT FISTURE	<
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CHANDENER	$\times$
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DECORATIVE UIGHT FIXTURE	<b>(</b>

CEILING NOTE:

9'-1 1/8" CEILINGS ON
FIRST FLOOR
8' 1 1/8" CEILINGS ON
SECOND FLOOR



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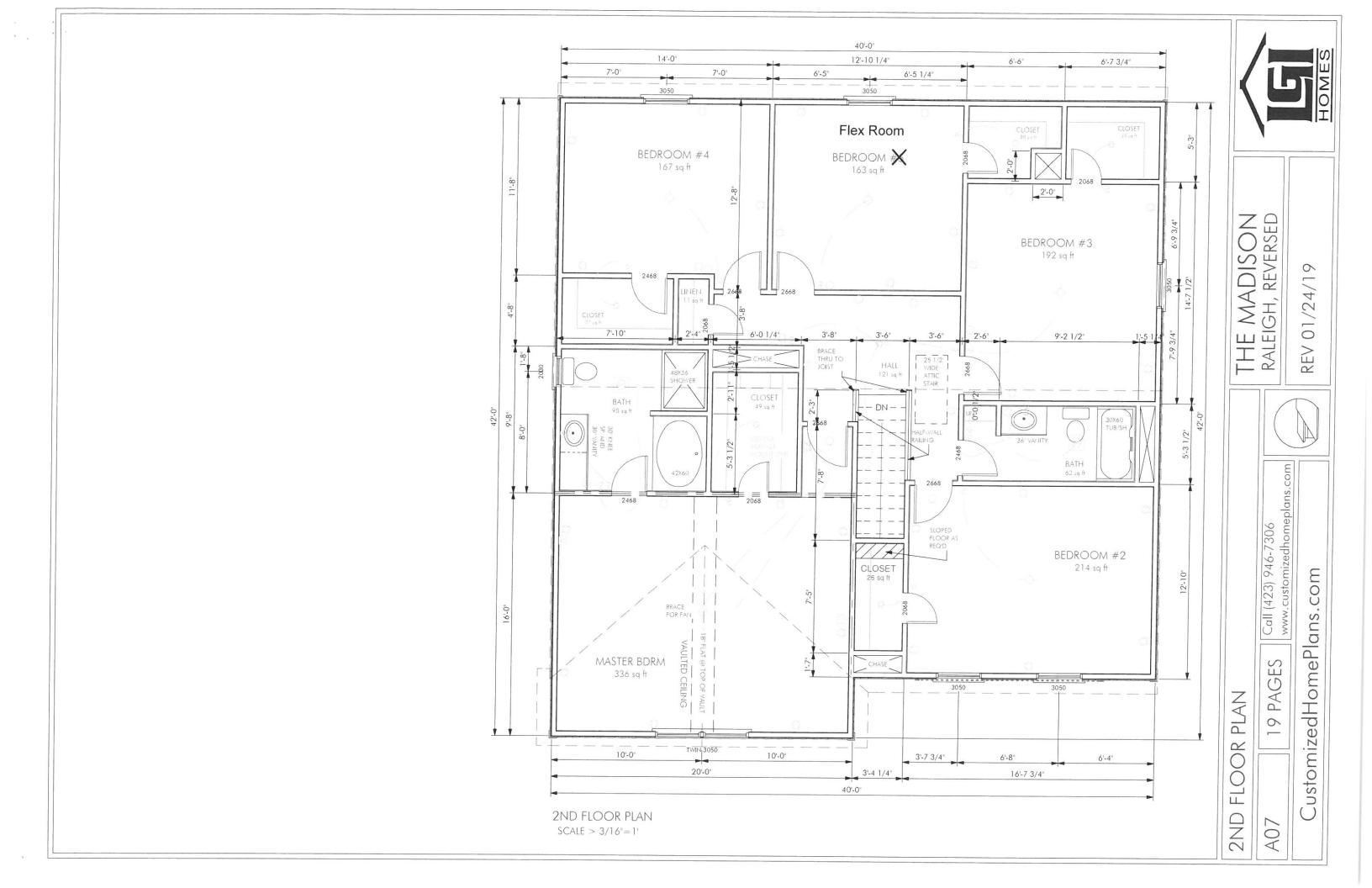
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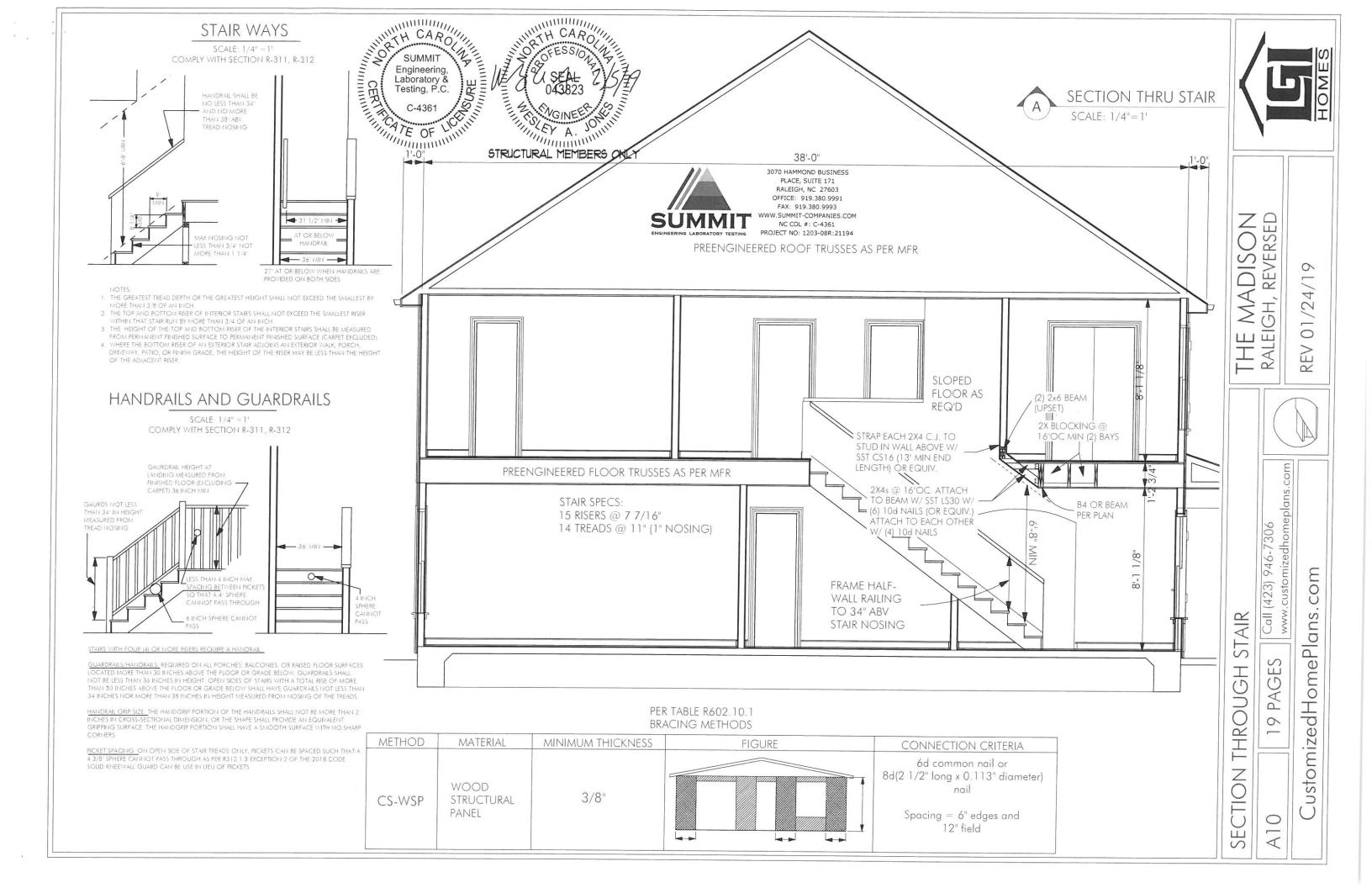
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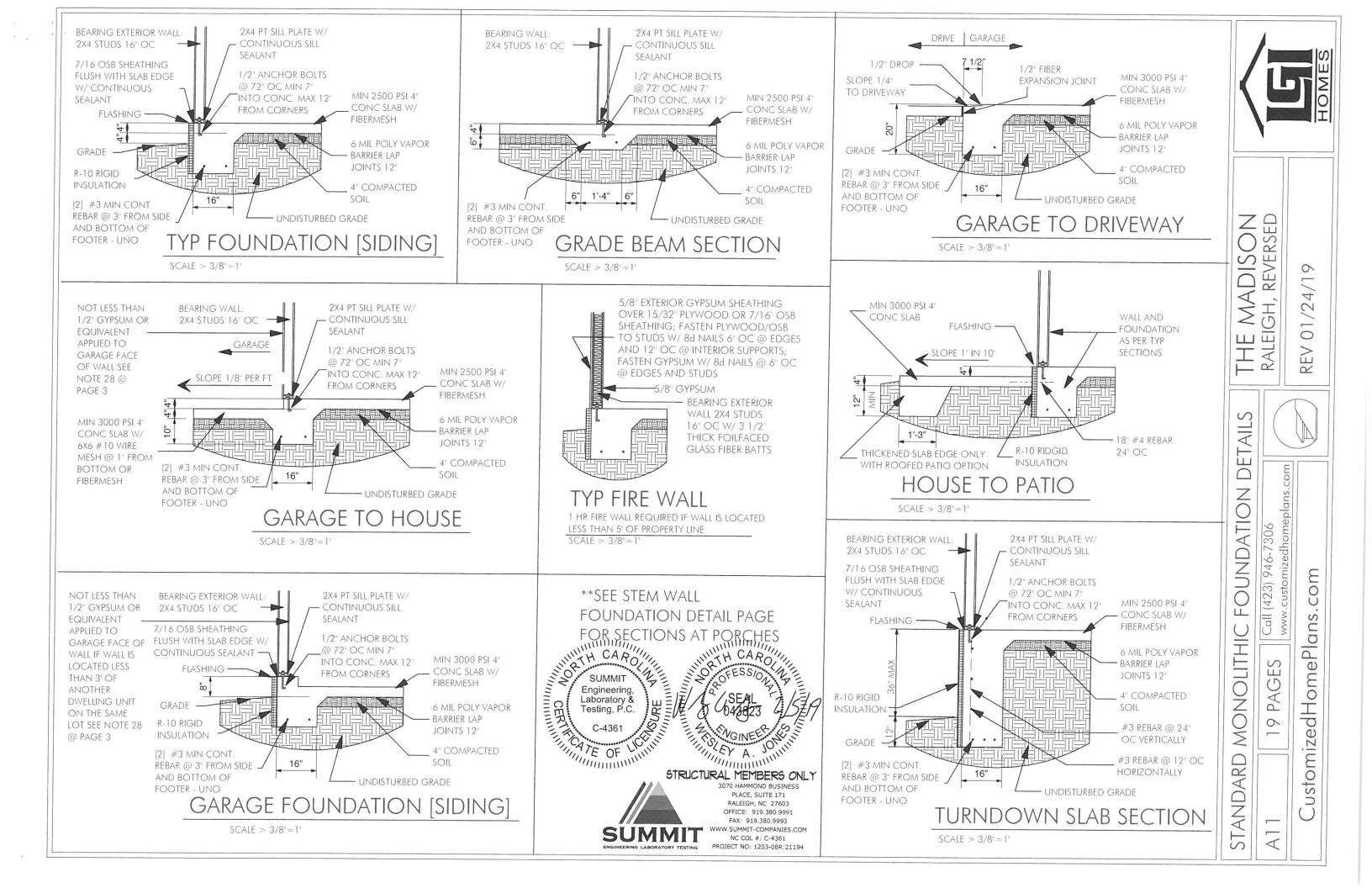
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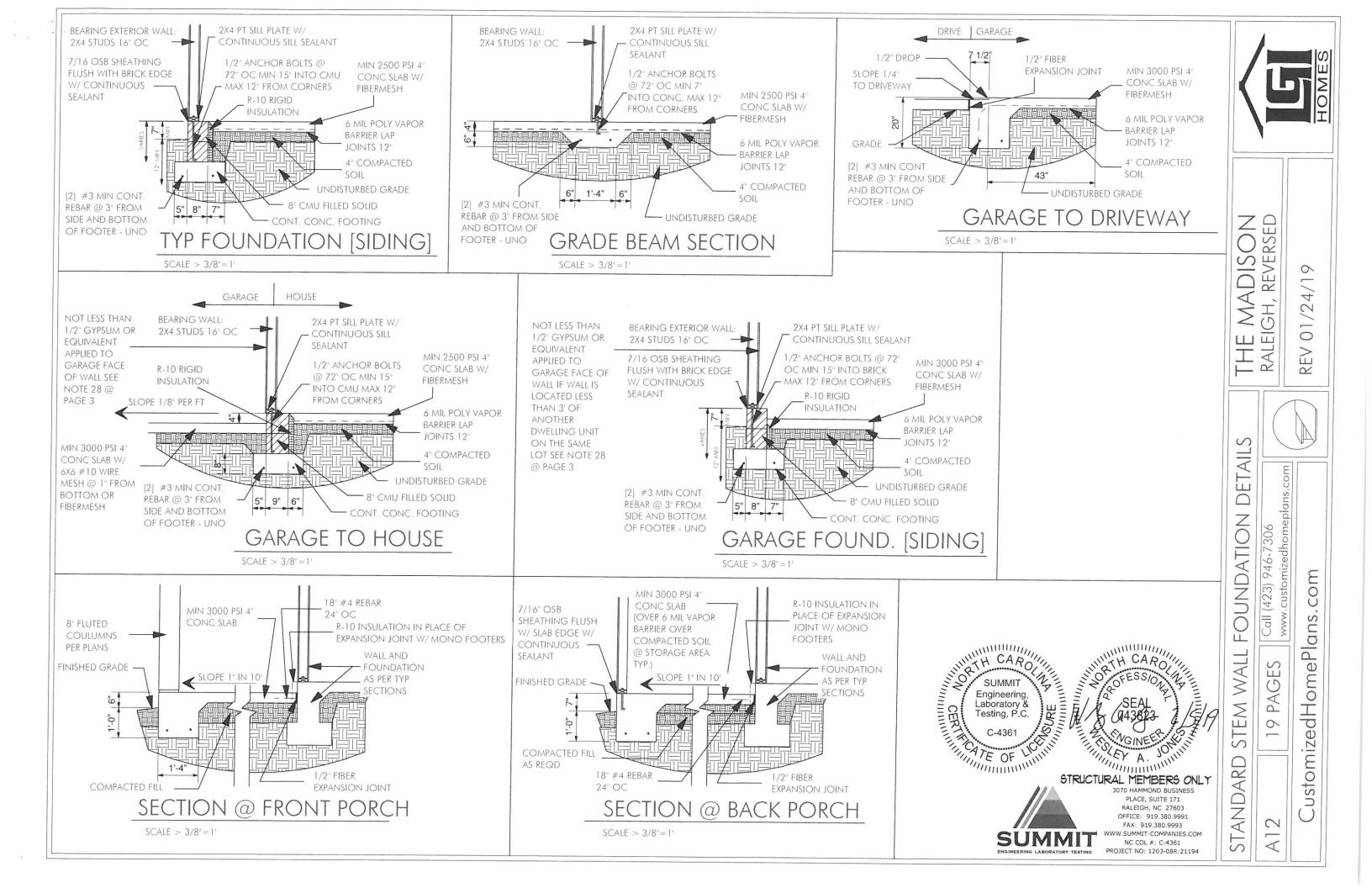
# COLUMN NOTE:

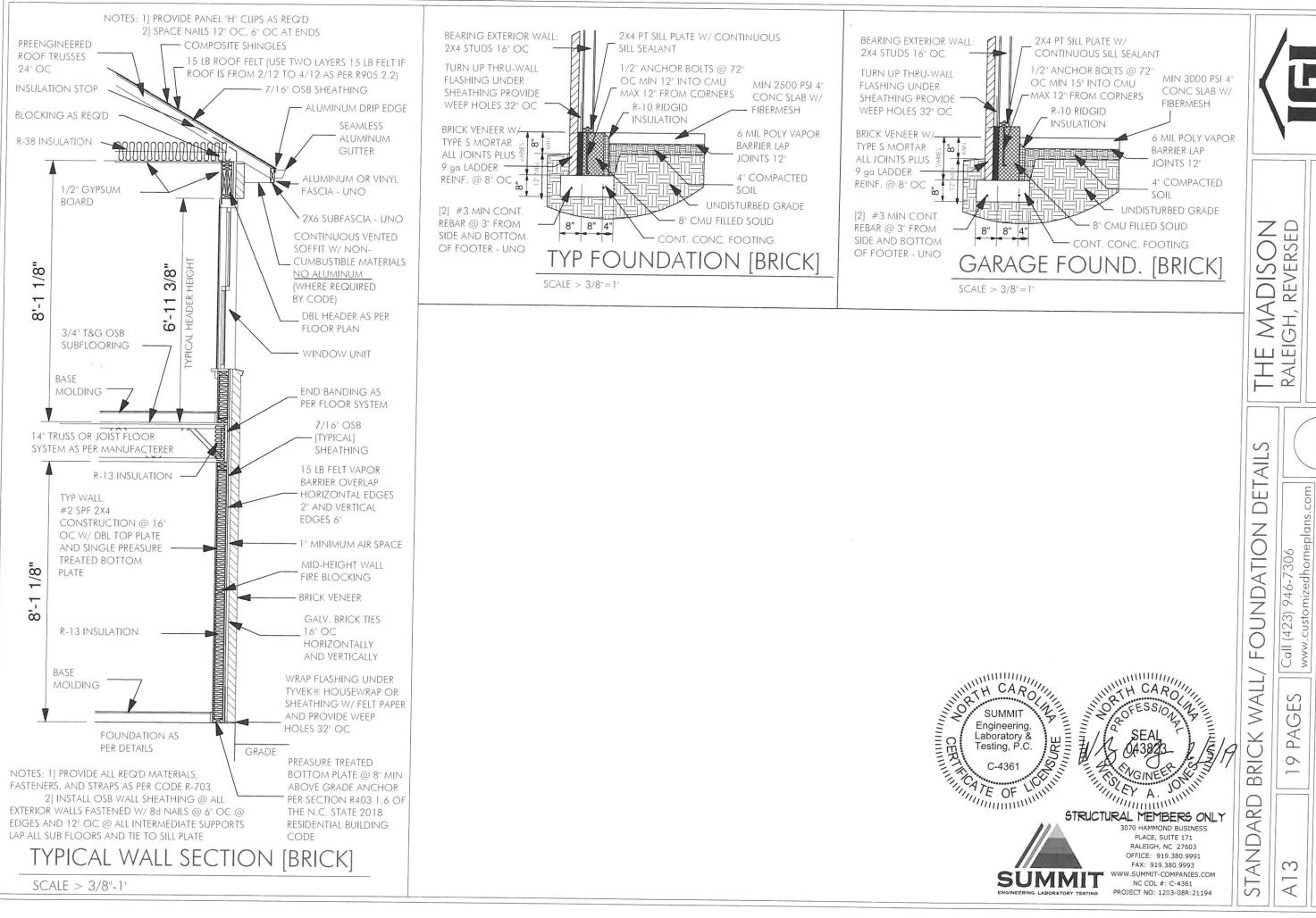
Columns to be: AFCO or column of equal bearing capacity. (6000# MIN)
Top connection: [2] #8- 1/4" x 3" stainless steel screws per side inserted into beam.
Bottom connection: [3] UBS-#18043 brackets fastened with [2] 1/4" x 1 1/4" screws into column and [2] 1/4" x 3 3/4" concrete screws through fastener into concrete.











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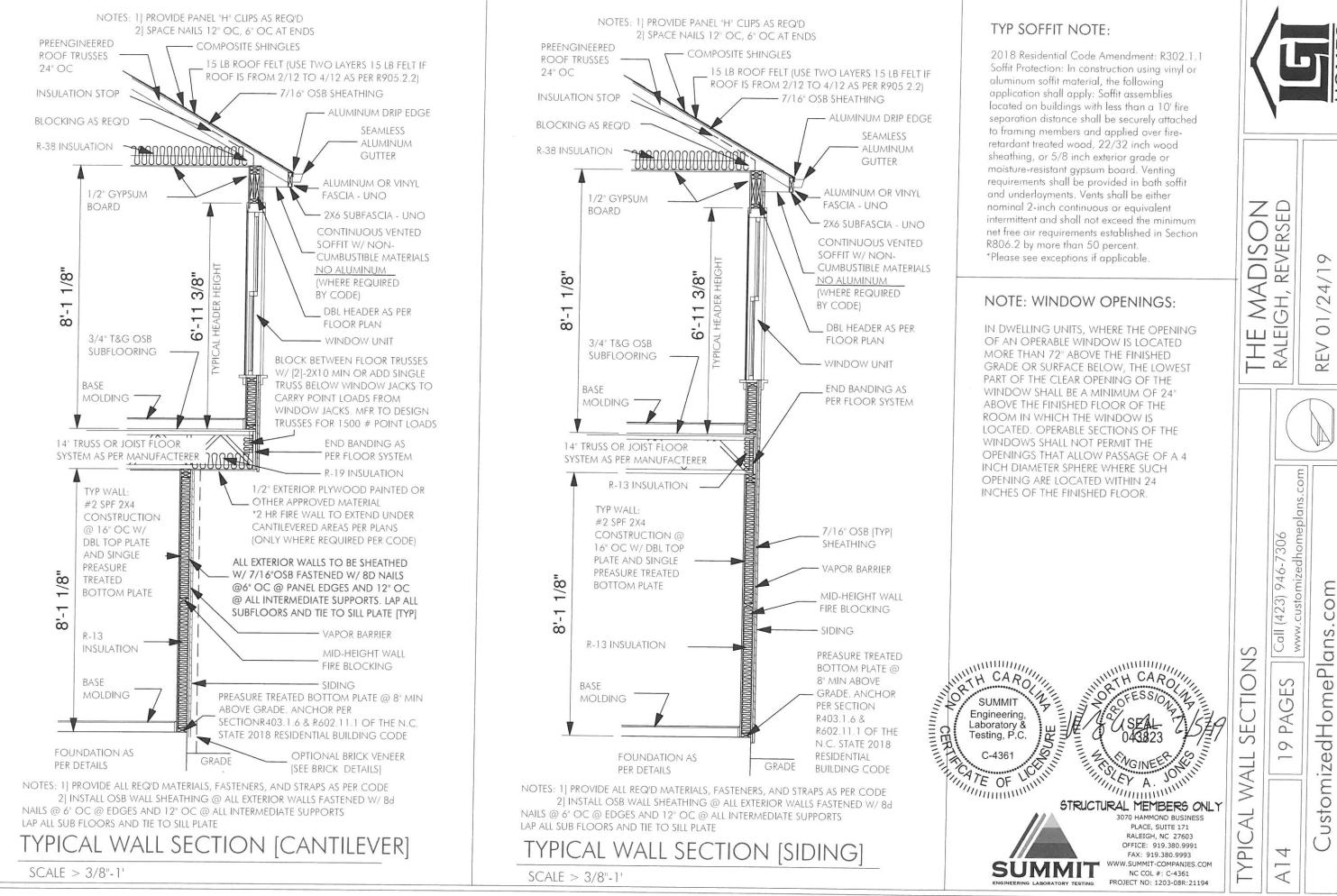
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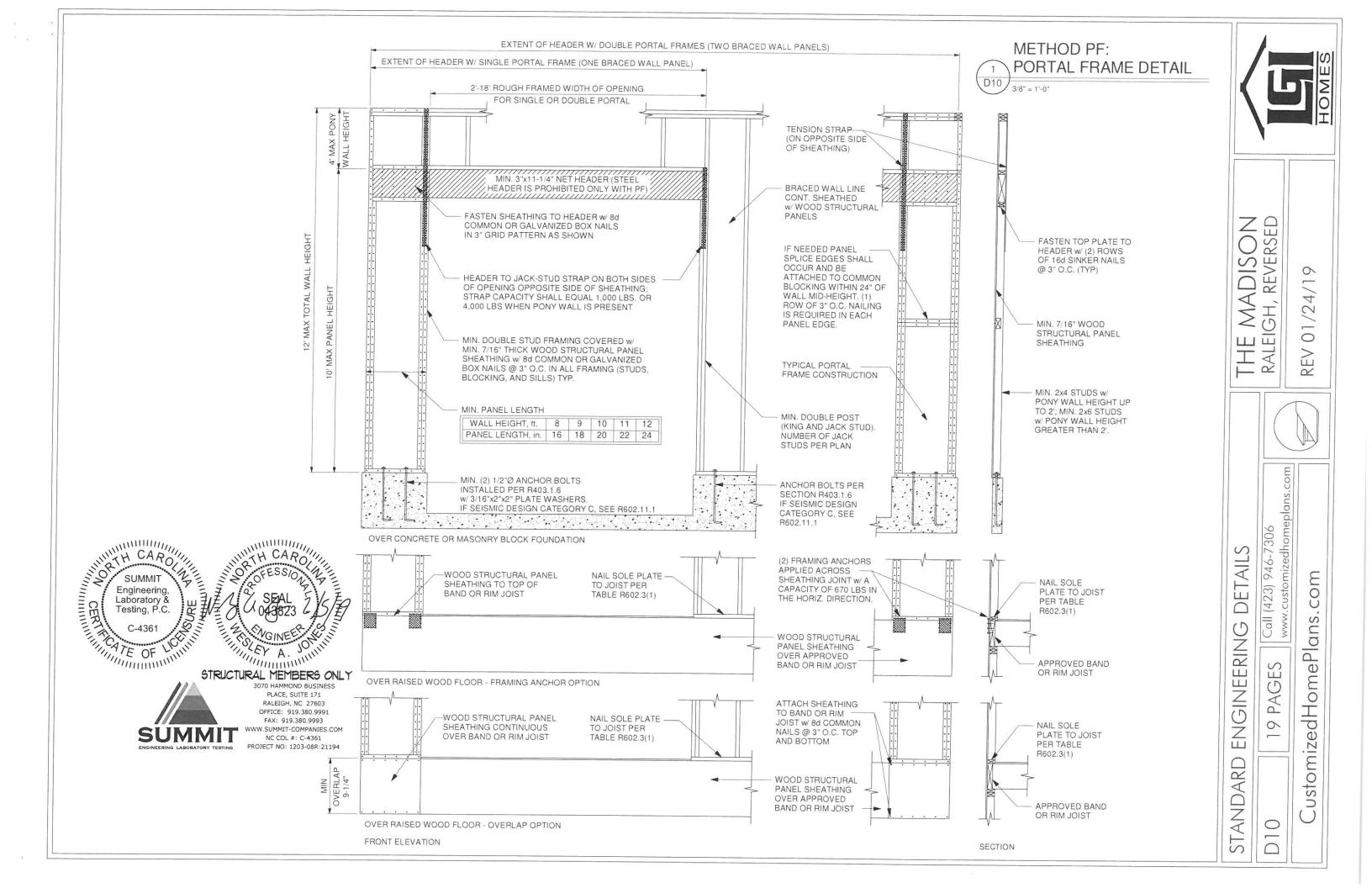
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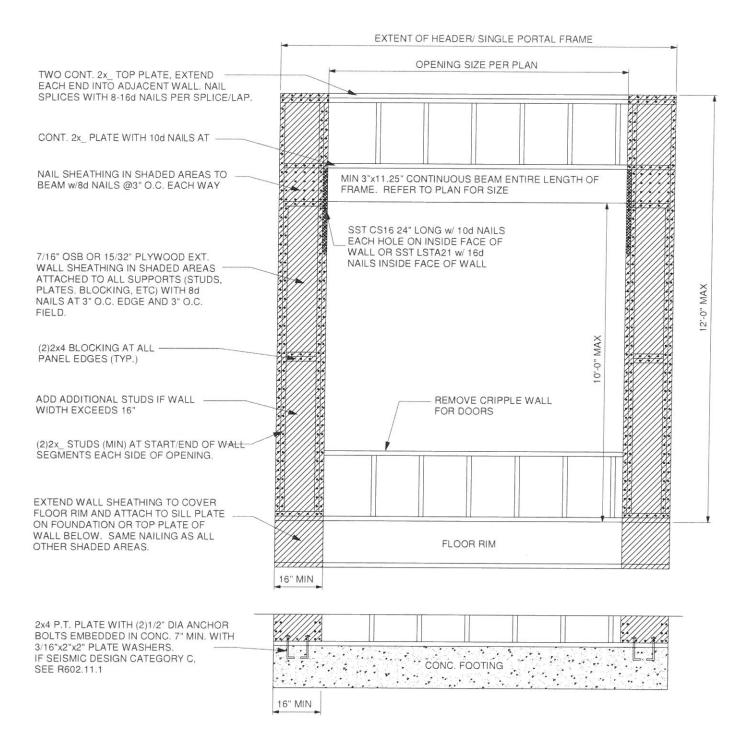
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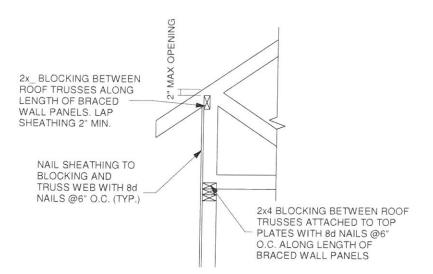




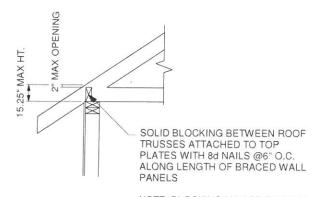




METHOD PF: PORTAL FRAME DETAIL D12 OPENINGS UNDER 8'-0"



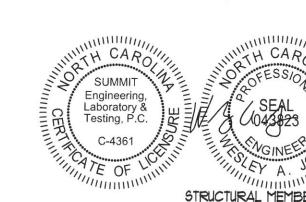
HEEL HEIGHT GREATER THAN 15.25"



NOTE: BLOCKING MAY BE OMITTED IF HEEL HEIGHT IS LESS THAN 9.25"

HEEL HEIGHT LESS THAN 15.25"







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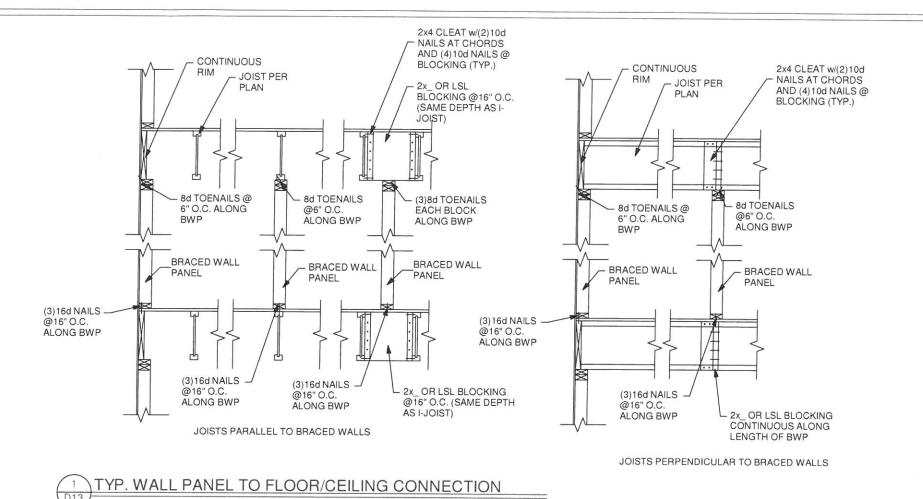
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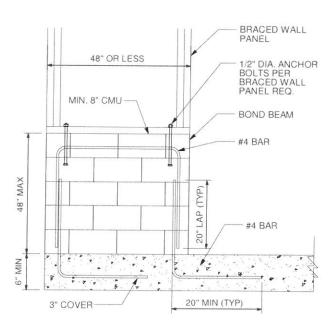
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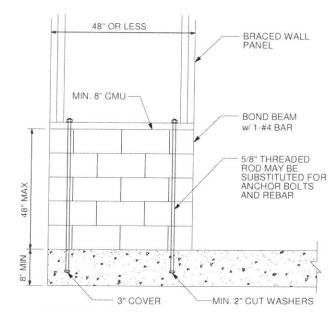
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48" OR LESS **BRACED WALL** PANEL 1/2" DIA. ANCHOR BOLTS PER BRACED WALL PANEL REQ. MIN. 8" CMU BOND BEAM w/ 1-#4 BAR #4 BAR MIN FIELD BEND 6" EXTENSION INTO BOND BEAM 20" MIN (TYP) 3" COVER

SHORT STEM WALL REINFORCEMENT



TALL STEM WALL REINFORCEMENT



RODS MAY BE INSTALLED USING AN ADHESIVE ANCHORING SYSTEM WITH A MINIMUM TENSILE CAPACITY OF 3,750 LBS AND INSTALLED IN ACCORDANCE WITH MANUFACTURER'S

OPTIONAL STEM WALL REINFORCEMENT

NOTE: GROUT BOND BEAMS AND ALL CELLS WHICH CONTAIN REBAR, THREADED RODS AND ANCHOR BOLTS.

2018 NCRC FIGURE R602.10.4.3 - MASONRY STEM WALLS SUPPORTING BRACED WALL PANELS

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8.2,-19.0	19.2,-20.0	19.9,-20.7	20.4,-21.3
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DESIGNER: My Custom Home Design 227 East 6th Avenue Tallahassee FL

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory & Testing, P.C. before construction begins.

## PLAN ABBREVIATIONS:

AB	ANCHOR BOLT	PT	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	SJ	SINGLE JOIST
DJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
DSP	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
EE	EACH END	SYP	SOUTHERN YELLOW PINE
EW	EACH WAY	TJ	TRIPLE JOIST
NTS	NOT TO SCALE	TSP	TRIPLE STUD POCKET
OC	ON CENTER	TYP	TYPICAL
PSF	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
P51	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by LGI HOMES. Subsequent plan revisions based on roof truss and floor joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify SUMMIT immediately.

1	2/5/19	21194	Updated plan per 2018 NCRC & moved engineering to SUMMIT template
		_	

s shall extend below the frost line for ucture is to be constructed. However. 3 shall be a minimum of 12" below grade. ider the direction or recommendation l engineer.

compacted to a minimum of 95%

shall be lined temporarily with a 6 mil placement of concrete does not occur

ed against any subgrade containing

3 bricated and erected in accordance of Steel Construction "Code of zel Buildings and Bridges" and the on "Load Resistance Factor Design"

5. Concrete slabs-on-grade shall be constructed in accordance with ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction".

6. The concrete slab-on-grade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported conditions not in accordance with the above assumptions.

Control or saw cut joints shall be spaced in interior slabs-on-grade at a maximum of 15'-0" O.C. and in exterior slabs-on-grade at a maximum of 10'-0" unless otherwise noted.

8. Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished

Reinforcing steel may not extend through a control joint. Reinforcing steel may extend through a saw cut joint.

10. All welded wire fabric (W.W.F.) for concrete slabs-on-grade shall be placed at mid-depth of slab. The W.W.F. shall be securely supported during the concrete pour.

Where reinforcing dowels are required, they shall be equivalent in size and spacing to the vertical reinforcement. The dowel shall extend 48 bar diameters vertically and 20 bar diameters into the footing.

10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted

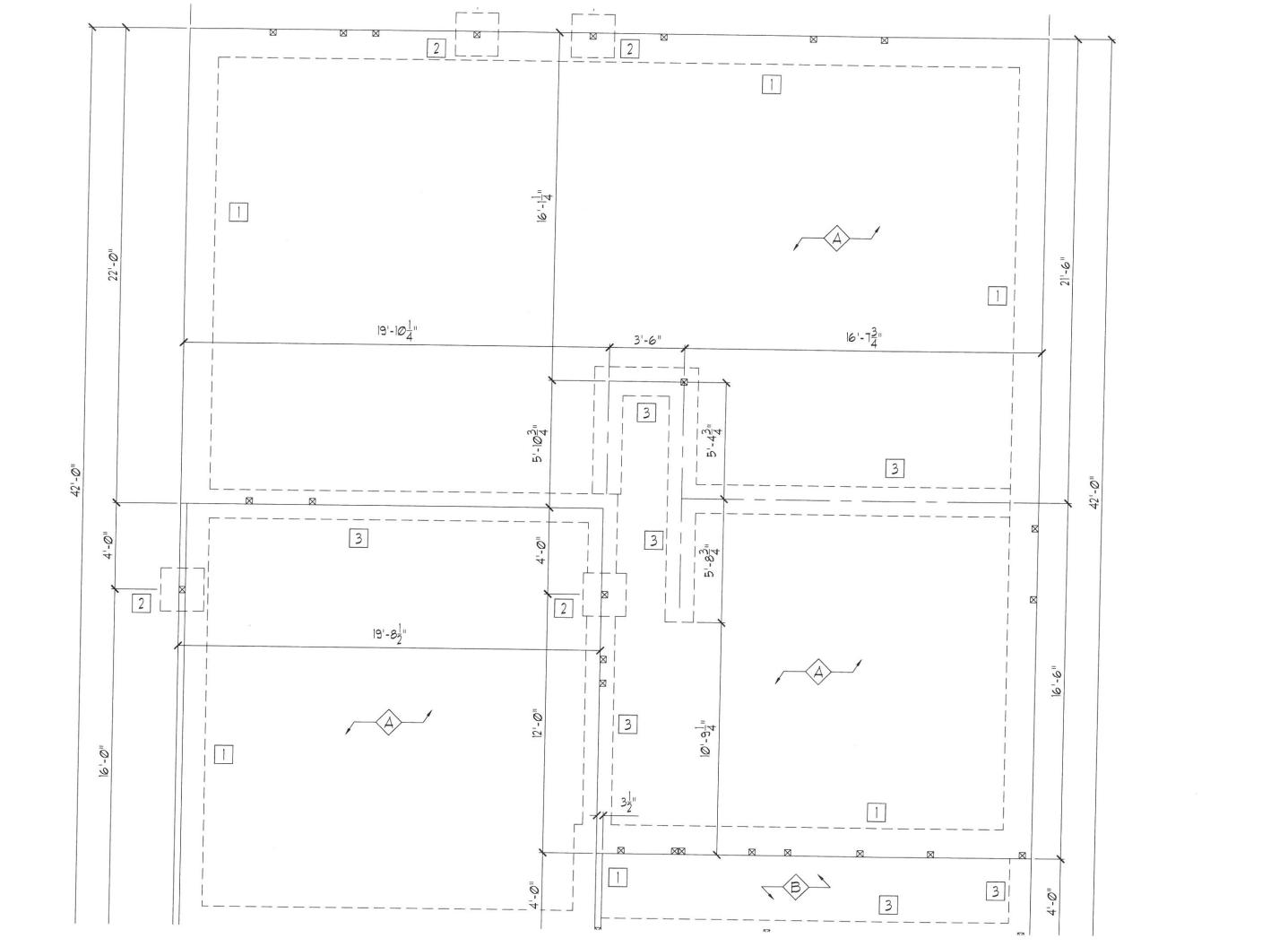
### WOOD FRAMING:

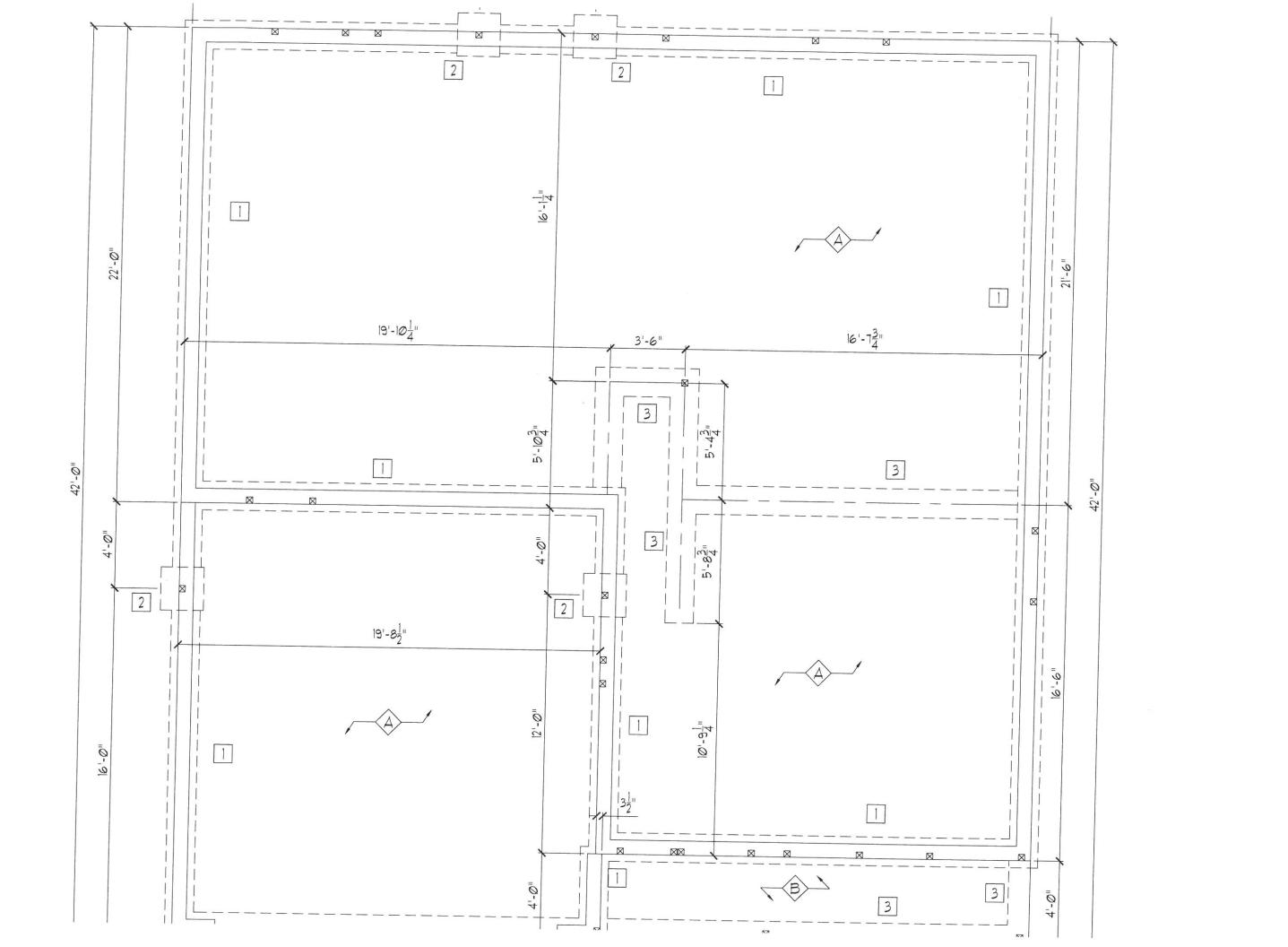
- Solid sawn wood framing members shall conform to the specifications listed in the latest edition of the "National Design Specification for Wood Construction" (NDS). Unless otherwise noted, all wood framing members are designed to be Southern-Yellow-Pine (SYP) #2.
- 2. LVL or PSL engineered wood shall have the following minimum design values:
  - 2.1. E = 1,900,000 psi

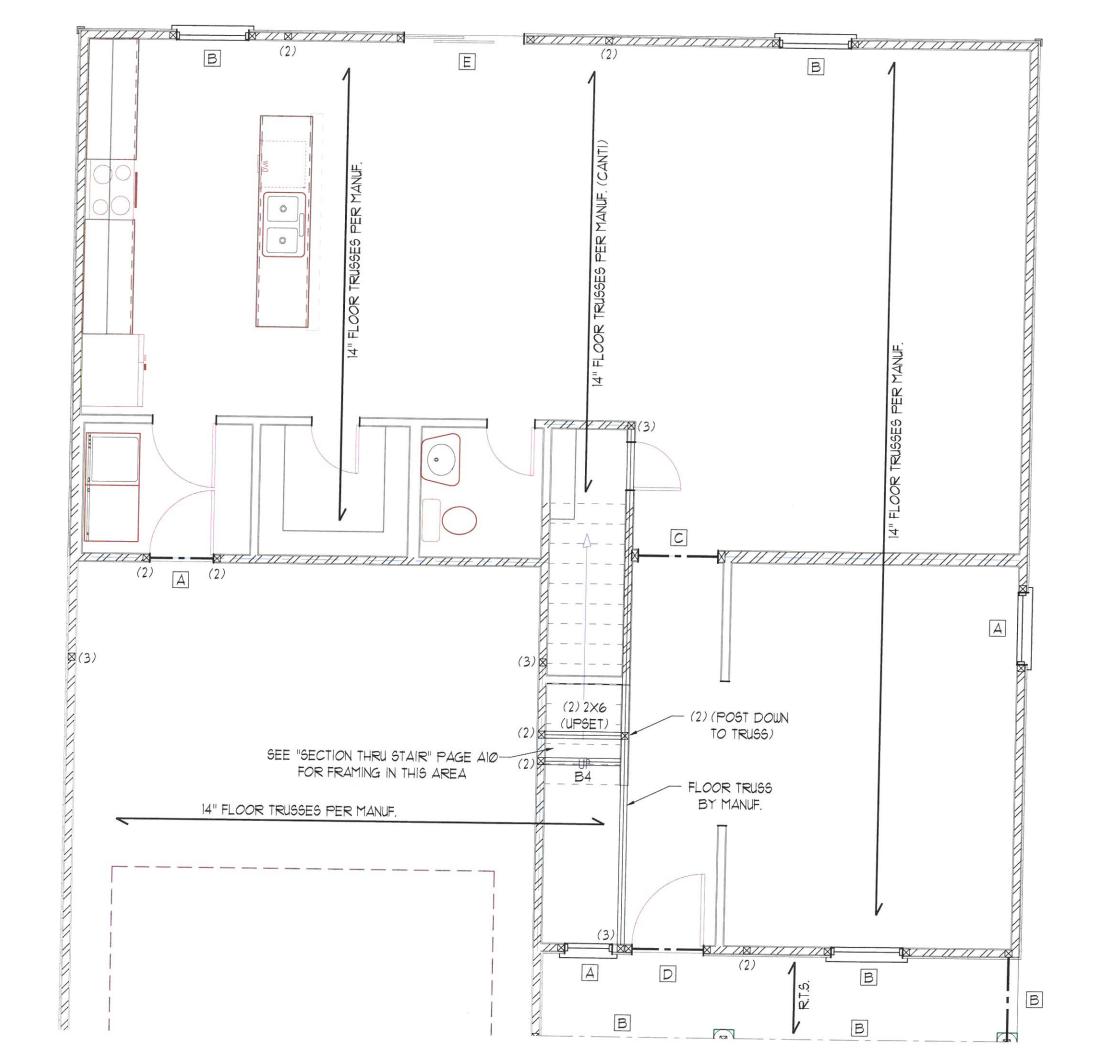
  - 24 Fc = 700 nsi
  - 2.2. Fb = 2600 bsi 2.3. Fy = 285 bsi

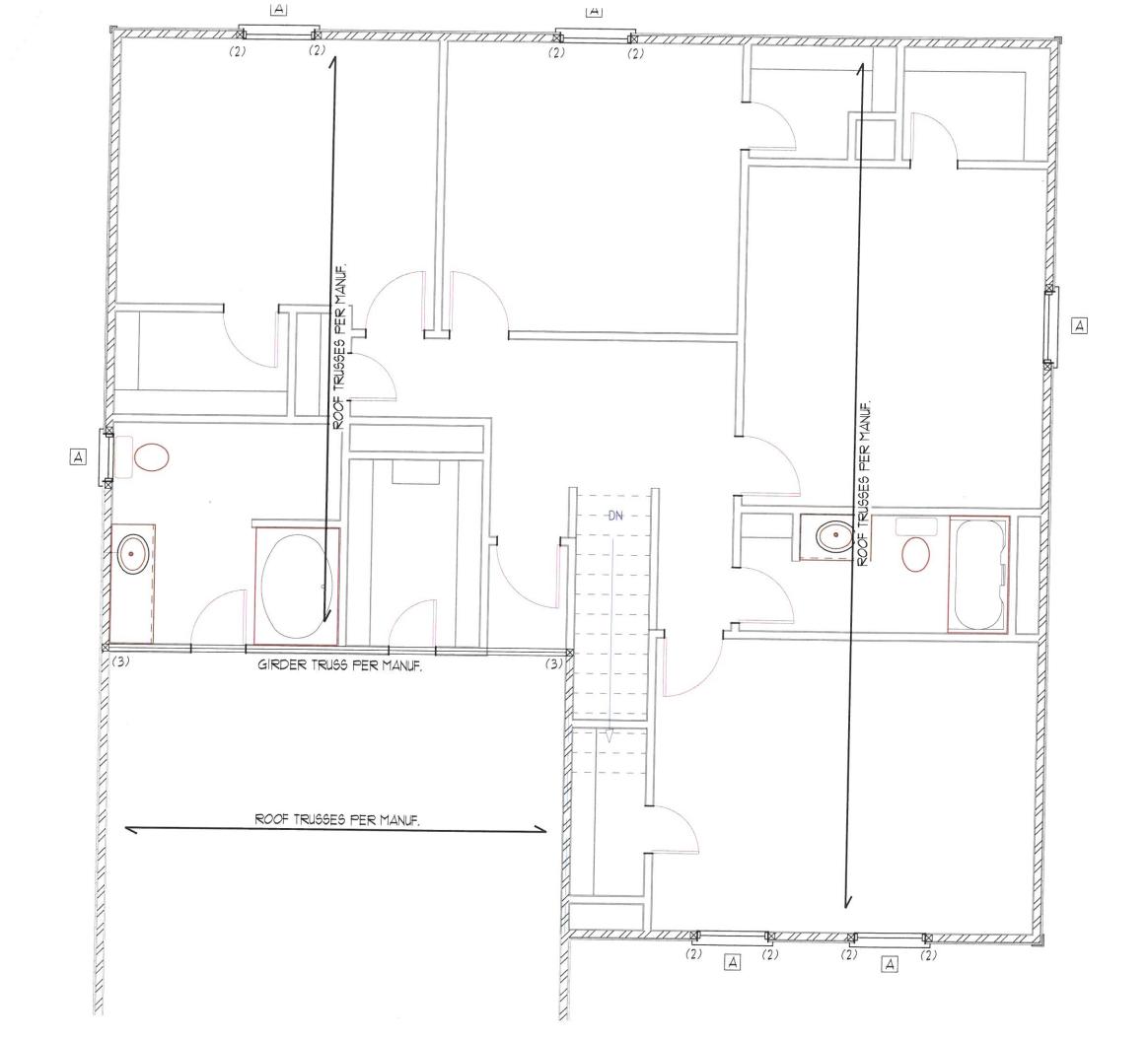
## WOOD TRUSSES:

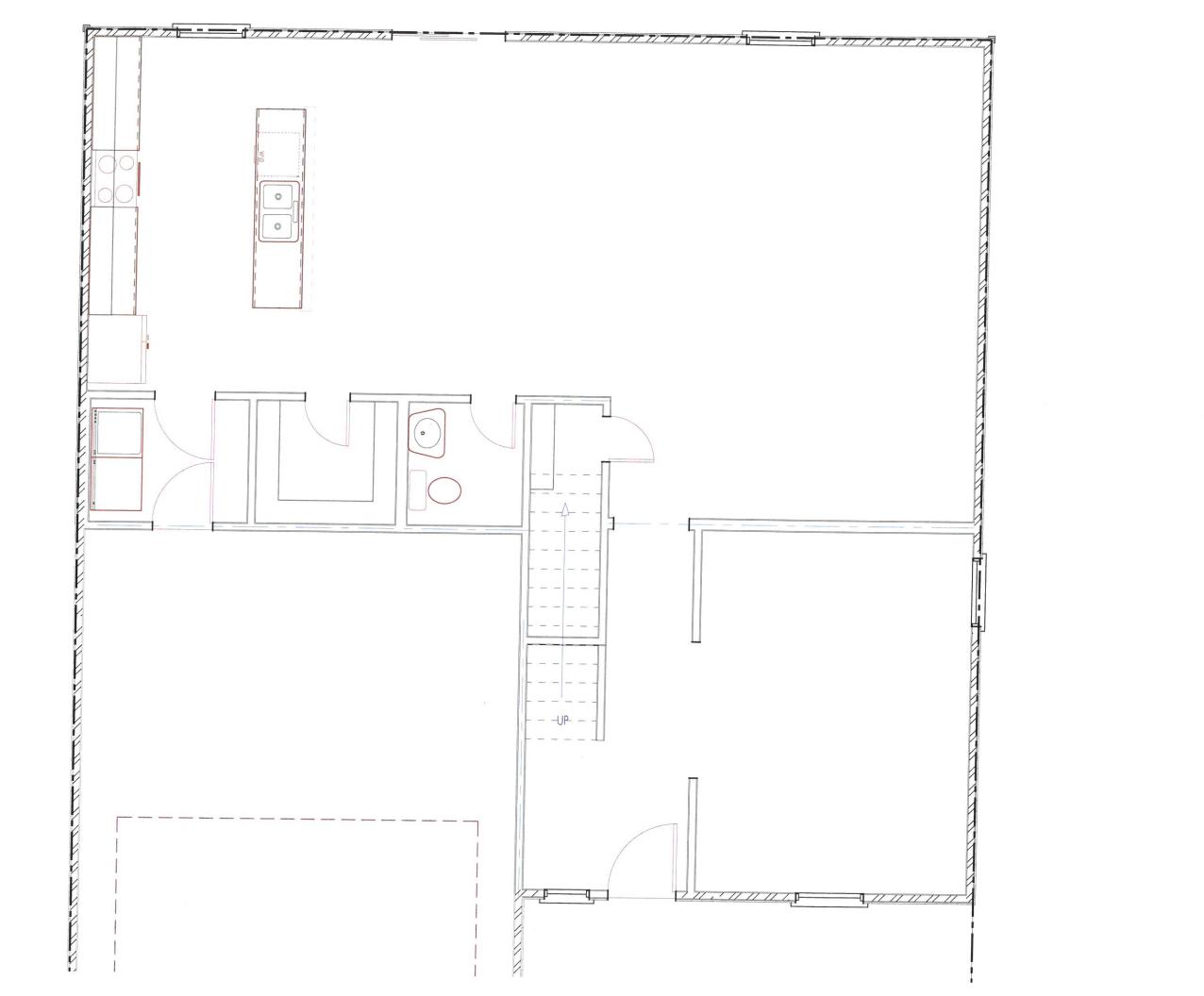
- The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overall compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses.
- The wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 7-10), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to the trusses.
- The trivage ahall he decided fabricated and erected in



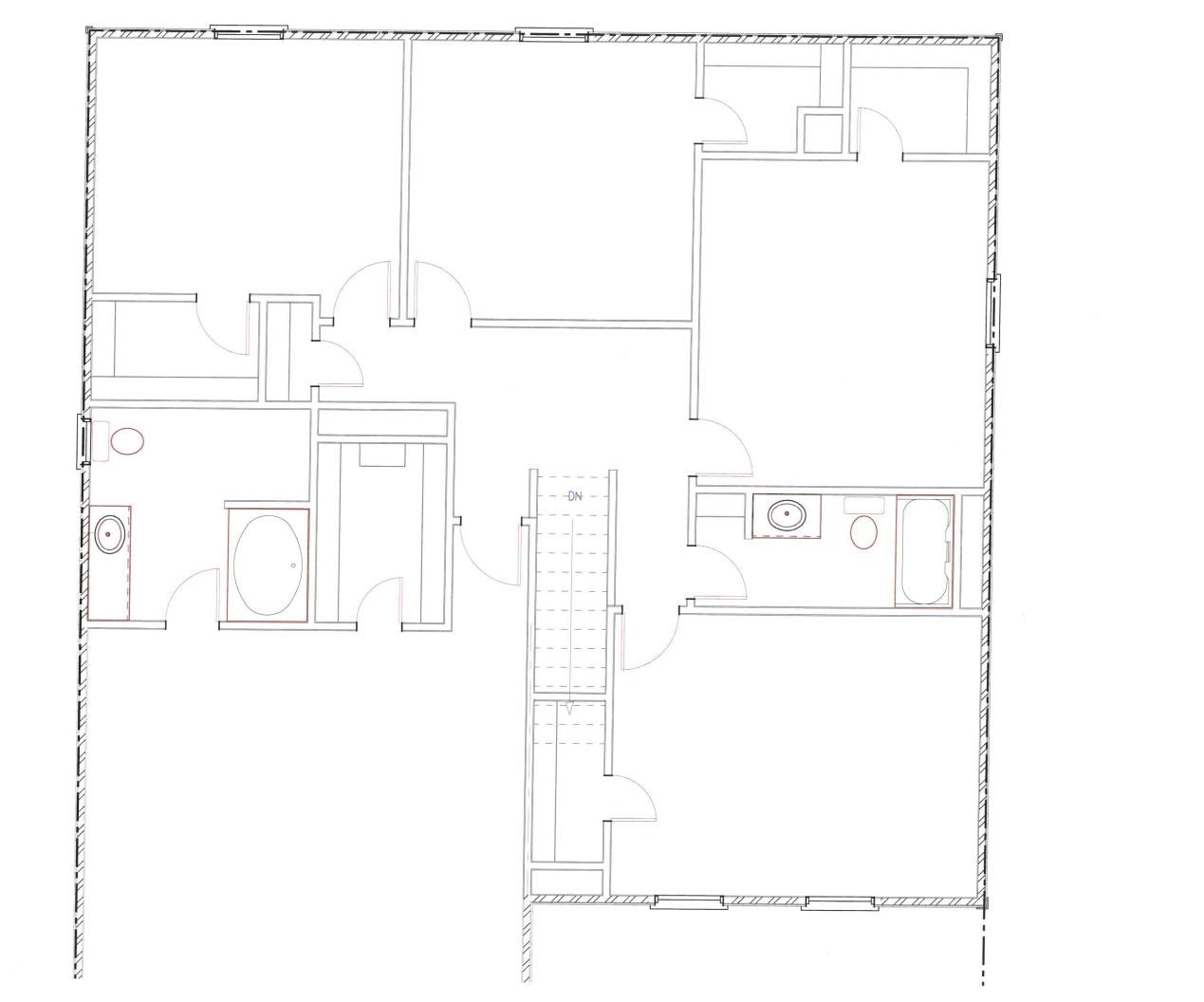








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