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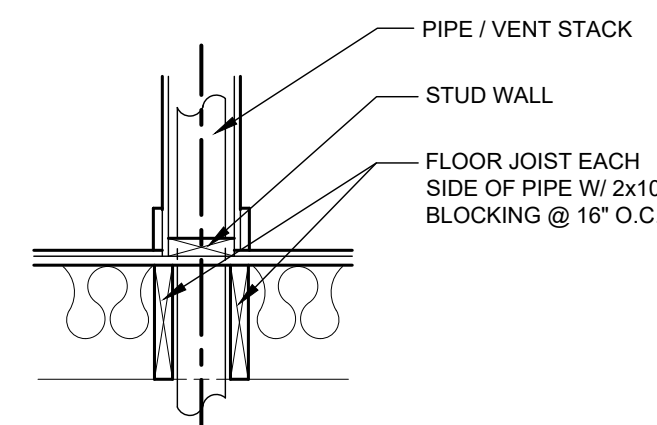
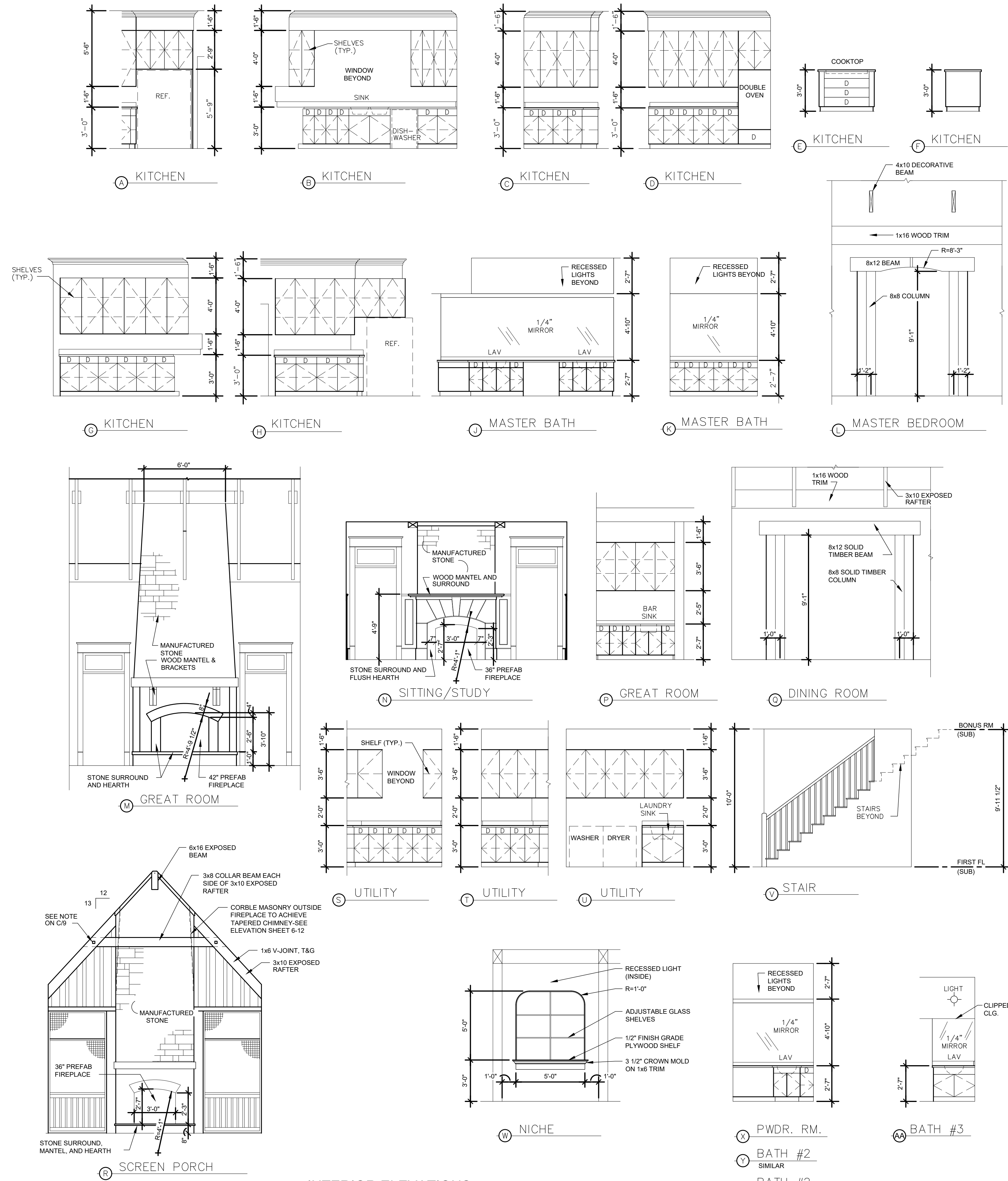
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CEDAR CREEK
DESIGN NO. M19105-959-MR
SHEET OF 1 9 ENO
COVER SHEET

MODIFICATIONS BY
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TYPICAL PIPE THRU FLOOR
3/4" = 1'-0"

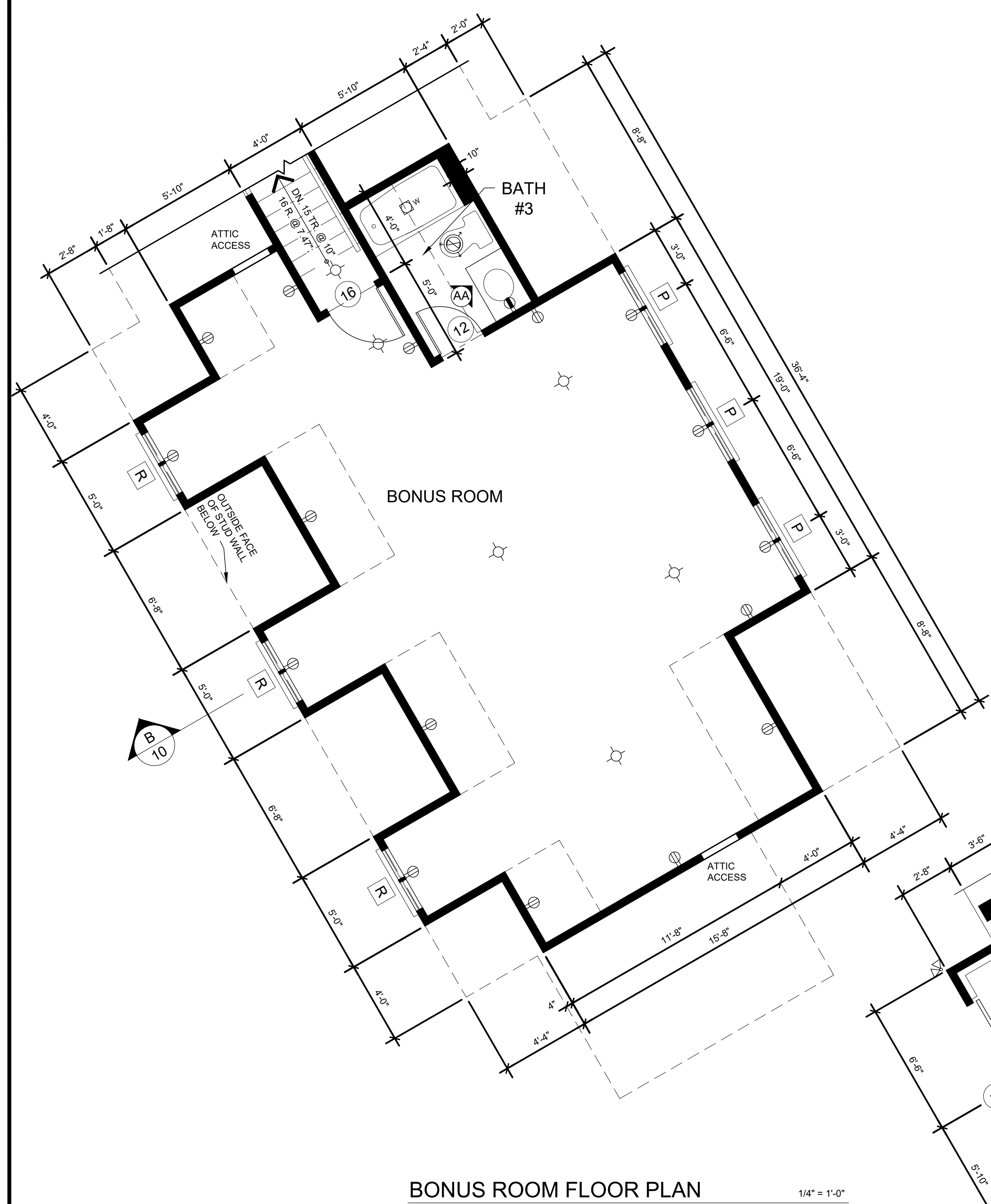
INTERIOR ELEVATIONS

1/4" = 1'-0"

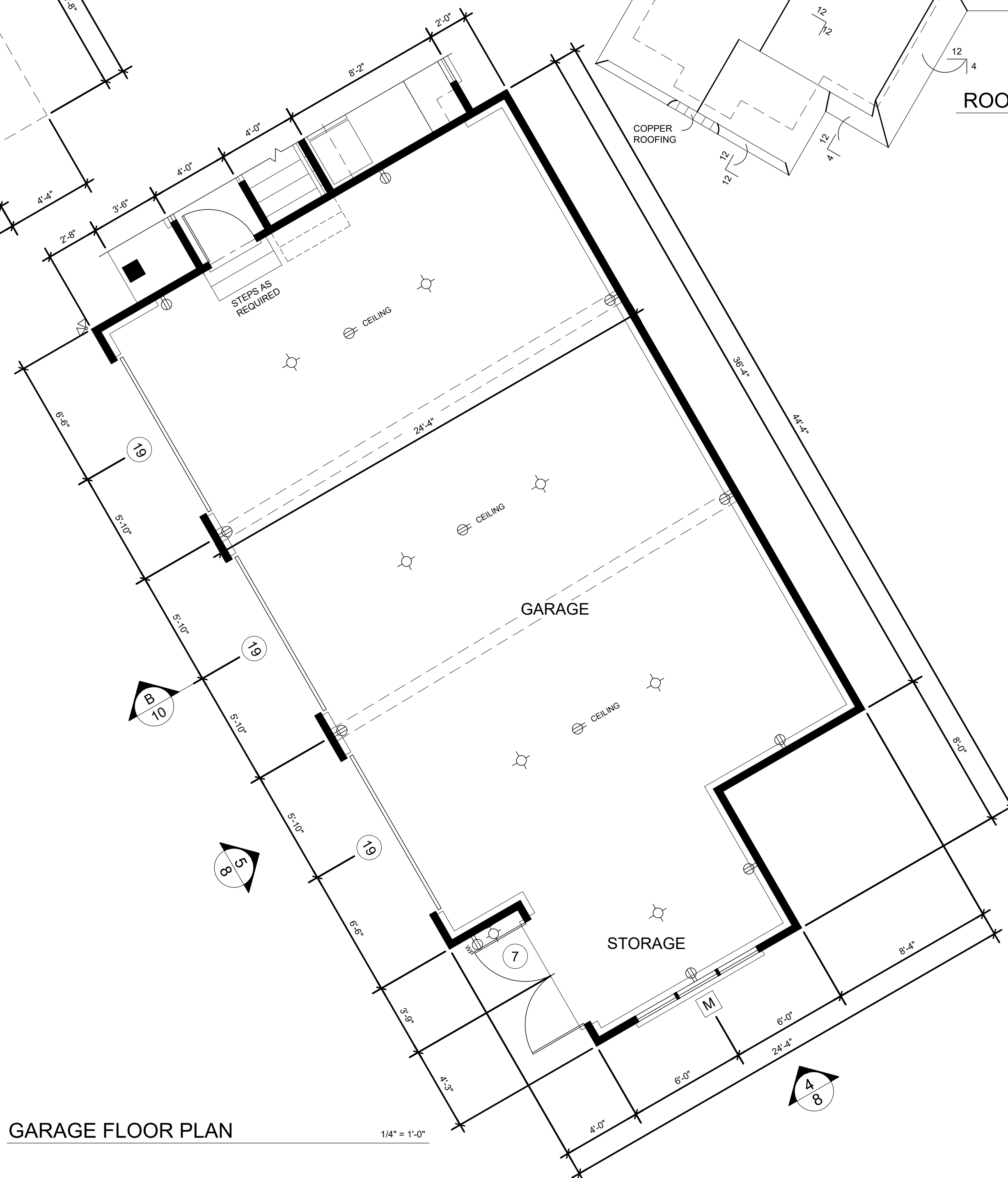
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DESIGN NO. **MT19105-959-MR** SHEET OF **2** OF **9** DR **ENO**
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 INTERIOR ELEVATIONS

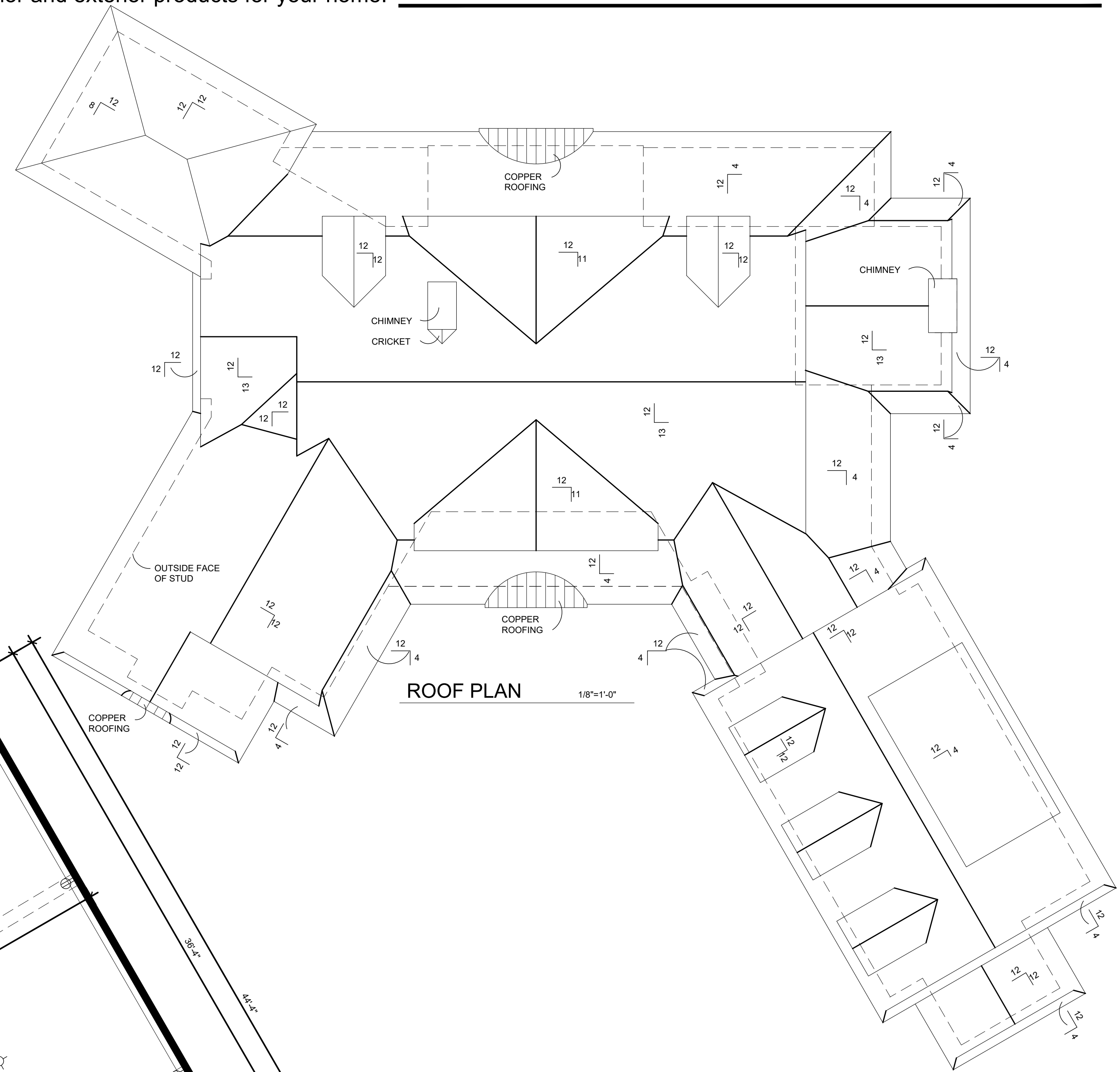
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BONUS ROOM FLOOR PLAN 1/4" = 1'-0"



GARAGE FLOOR PLAN 1/4" = 1'-0"



ROOF PLAN 1/8" = 1'-0"

PLAN NOTES:

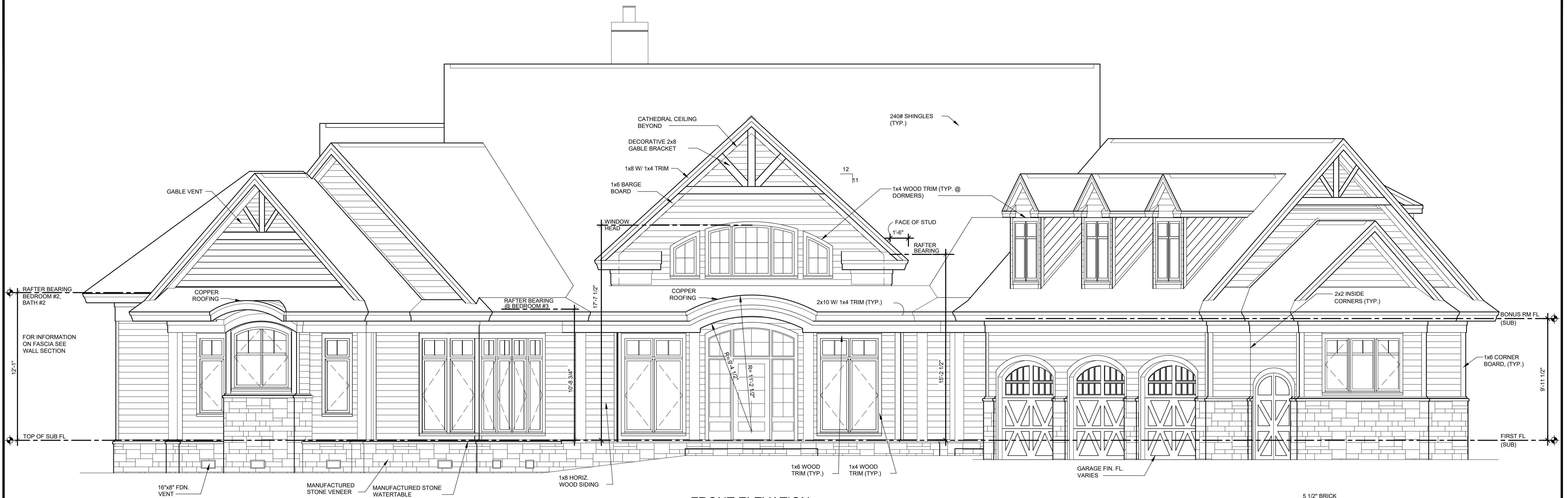
1. VERIFY ALL LOCAL CODES, ENERGY TYPES, AND SITE CONDITIONS PRIOR TO CONSTRUCTION.
2. REVIEW SELECTED MECHANICAL SYSTEMS WITH OWNER PRIOR TO CONSTRUCTION.
REVIEW SUB-CONTRACTORS LOCATIONS OF THE WATER HEATER AND HVAC UNIT(S) WITH THE OWNER PRIOR TO CONSTRUCTION. VERIFY LOCAL BUILDING CODE REQUIREMENTS AND MANUFACTURER REQUIREMENTS FOR ATTIC, CRAWL SPACE, OR GARAGE LOCATIONS.
H.V.A.C. EQUIPMENT IN THE ATTIC SPACE SHALL BE ACCESSIBLE BY AN OPENING LARGER THAN THE LARGEST PIECE OF EQUIPMENT (TO ALLOW REMOVAL OF THE EQUIPMENT) AND IN NO CASE LESS THAN 22"x36".
3. SEE STRUCTURAL DRAWINGS FOR ADDITIONAL INFORMATION.
4. INSULATE AROUND ALL BATHS AND UTILITY ROOM.
5. TYPICAL WALL, 2x4'S @ 16" O.C., UNLESS OTHERWISE DIMENSIONED.
6. PROVIDE SMOKE DETECTORS AS REQUIRED BY CODE.
7. PROVIDE DOORBELLS, TRANSFORMER, AND CHIME.

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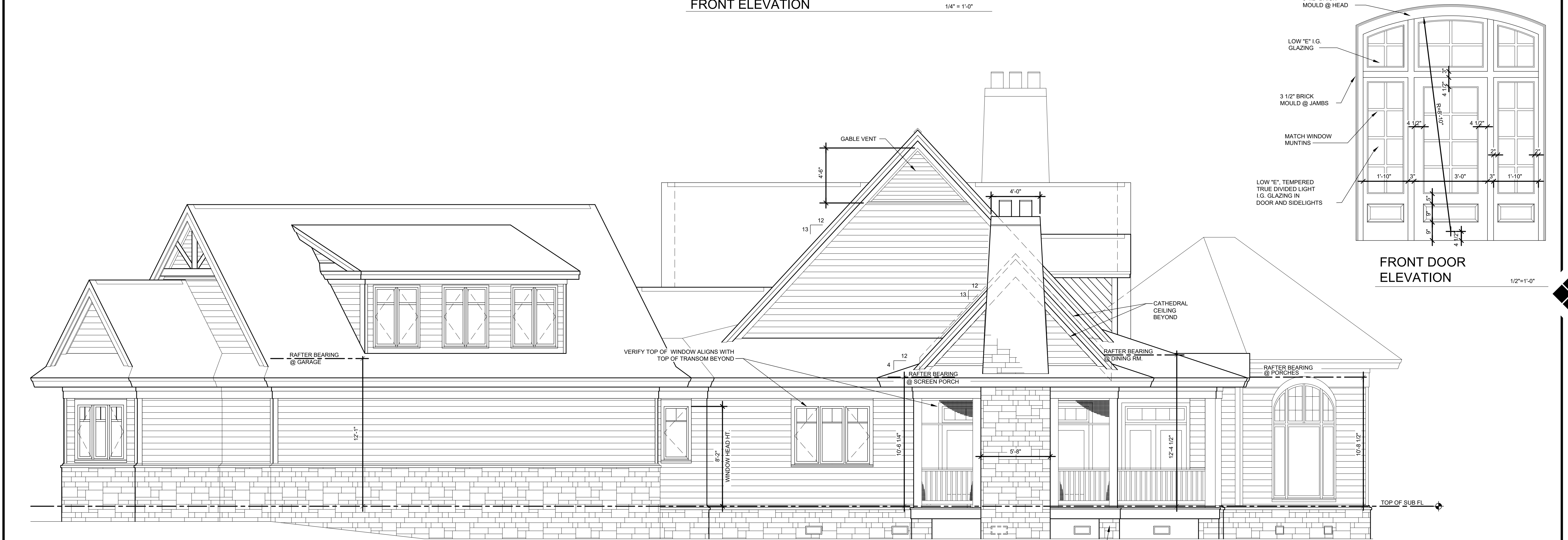
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DESIGN NO. **MT19105-959-MR**
 SHEET OF **4** OF **9** DR **ENO**
GARAGE AND BONUS ROOM FLOOR PLANS AND ROOF PLAN
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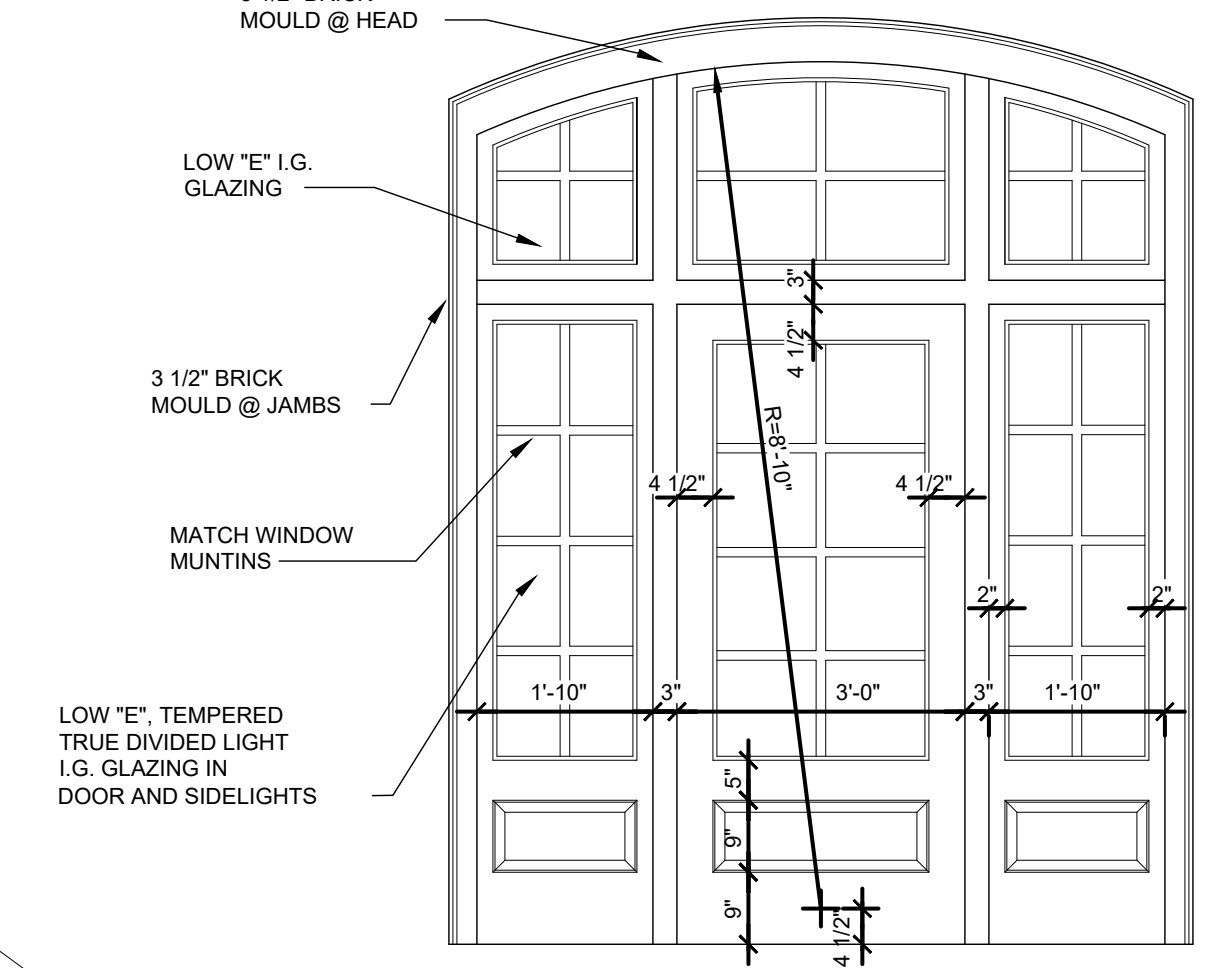
FRONT ELEVATION

1/4" = 1'-0"



RIGHT SIDE ELEVATION

1/4" = 1'-0"

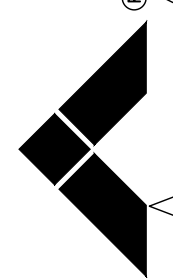


FRONT DOOR ELEVATION

1/2" = 1'-0"

I:\Modifications\19105-959-MR\19105-959-MR.dwg 06/21/19 09:01

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DESIGN NO. **M19105-959-MR**

 SHEET OF **5** OF **9**

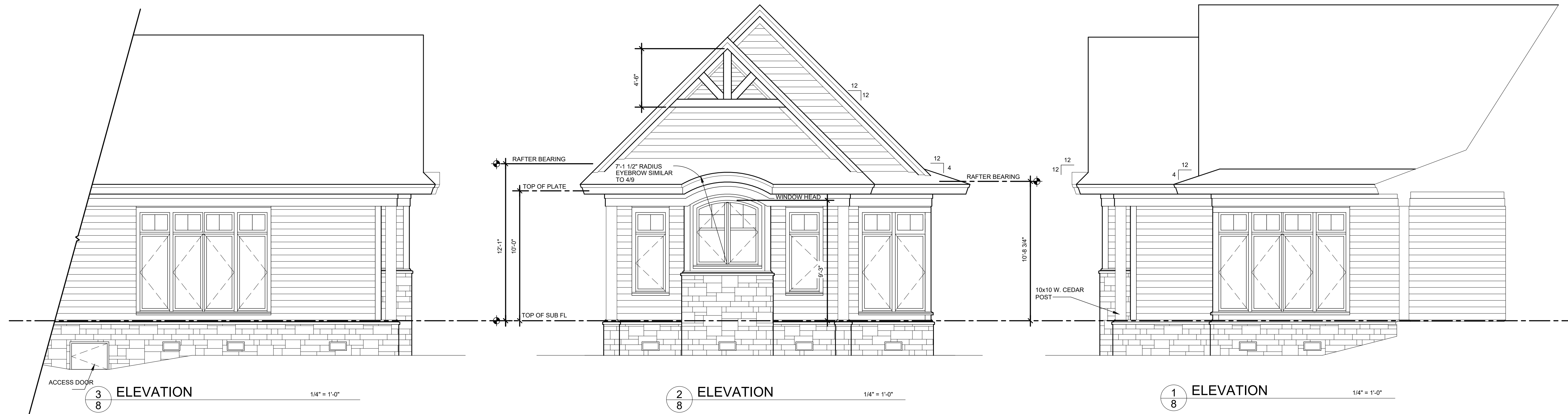
 DR **ENO**

 ELEVATIONS

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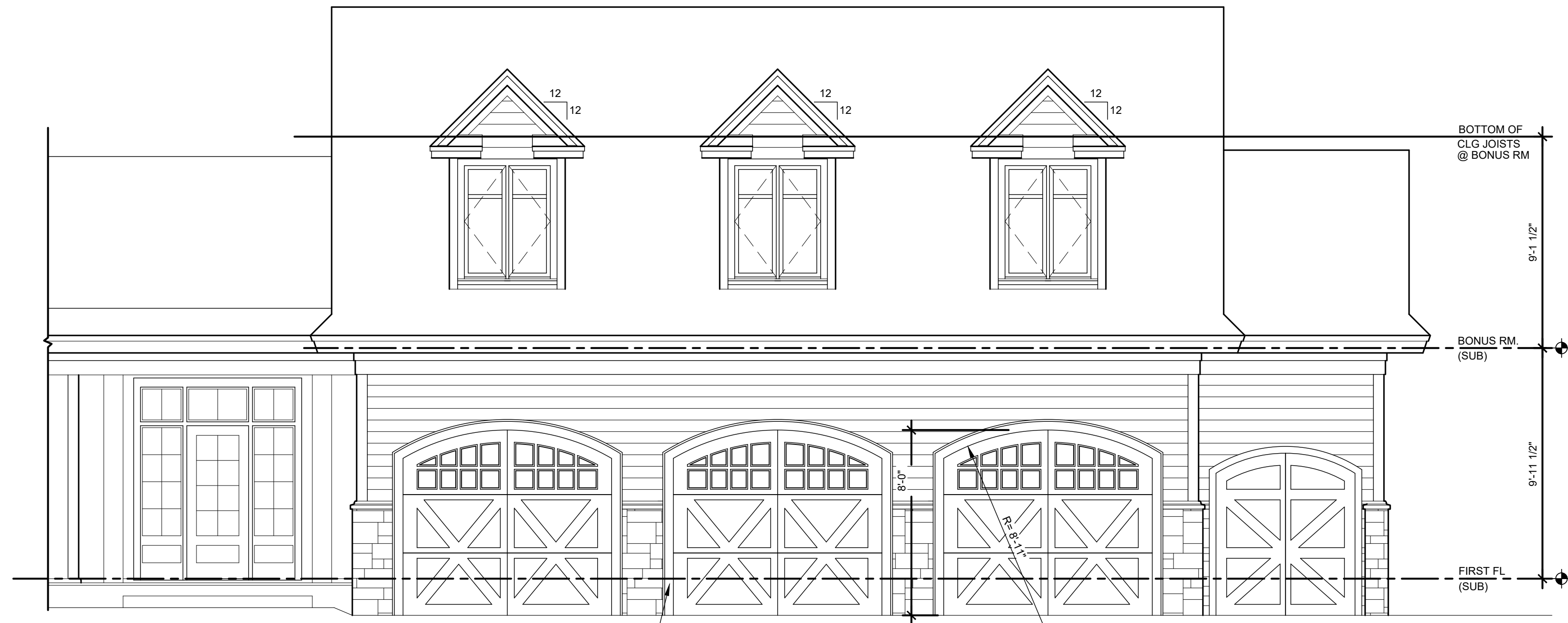
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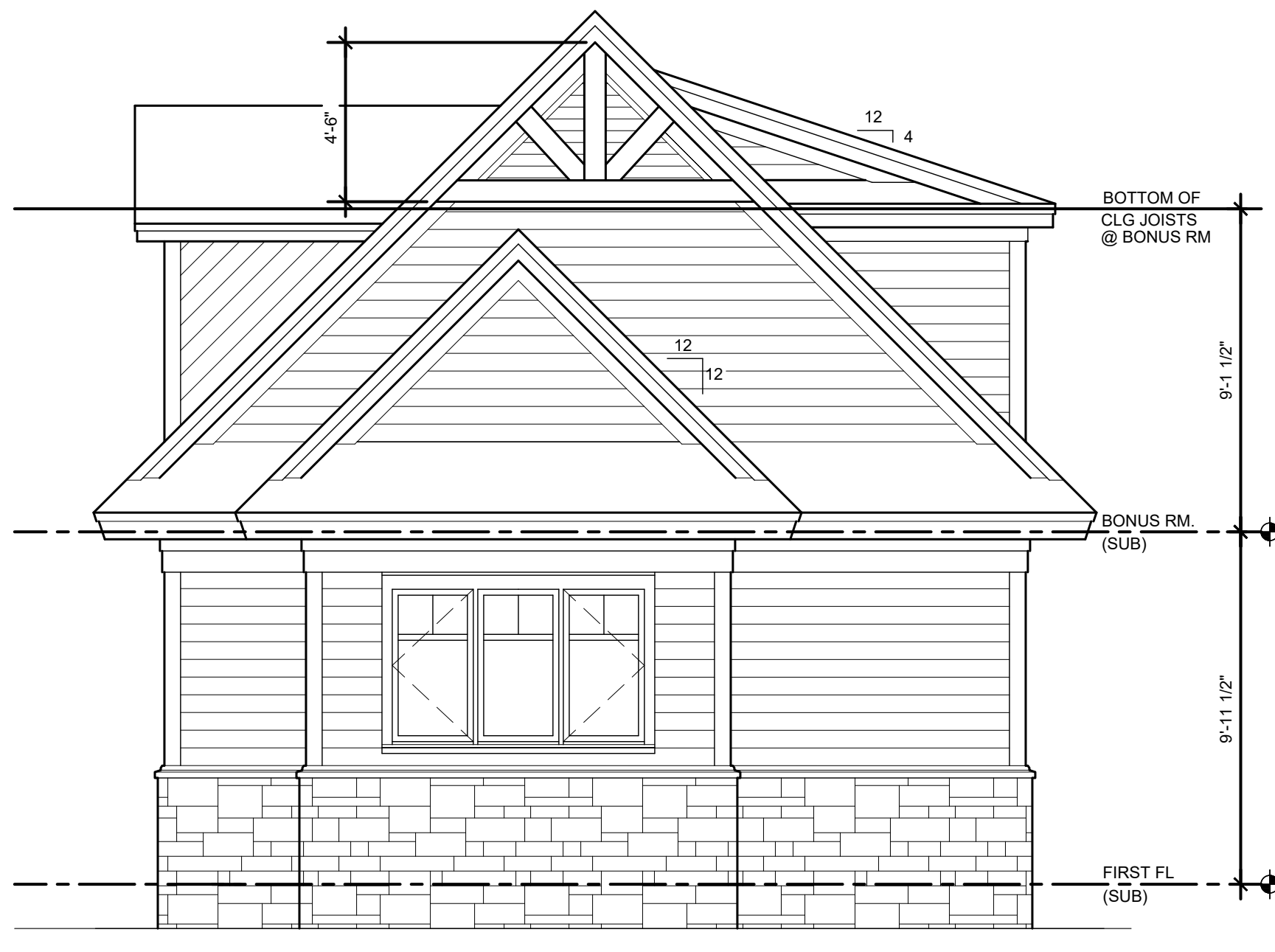
3
8 ELEVATION 1/4" = 1'-0"

2
8 ELEVATION 1/4" = 1'-0"

1
8 ELEVATION 1/4" = 1'-0"



5
8 GARAGE FRONT ELEVATION 1/4" = 1'-0"



4
8 GARAGE SIDE ELEVATION 1/4" = 1'-0"

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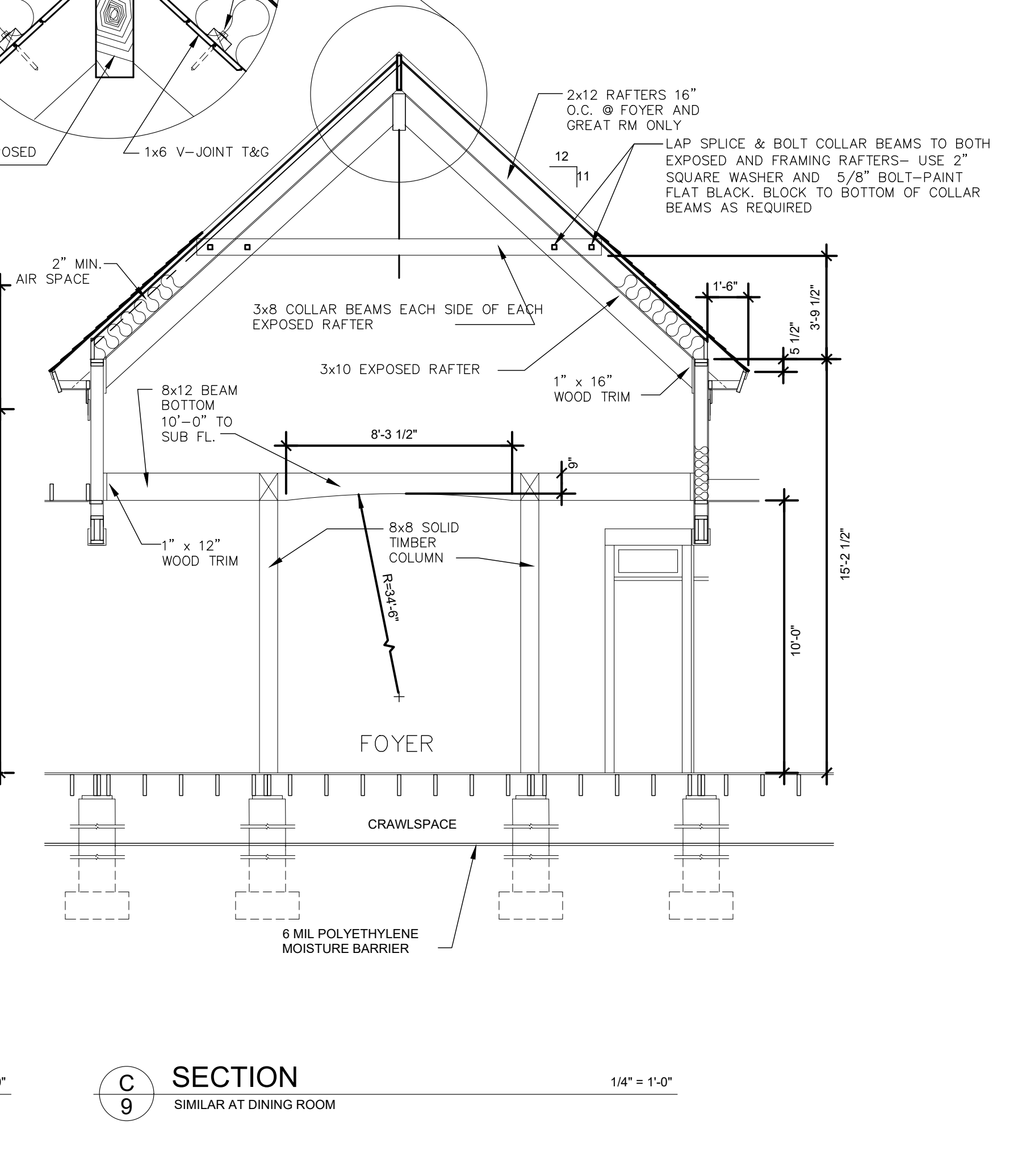
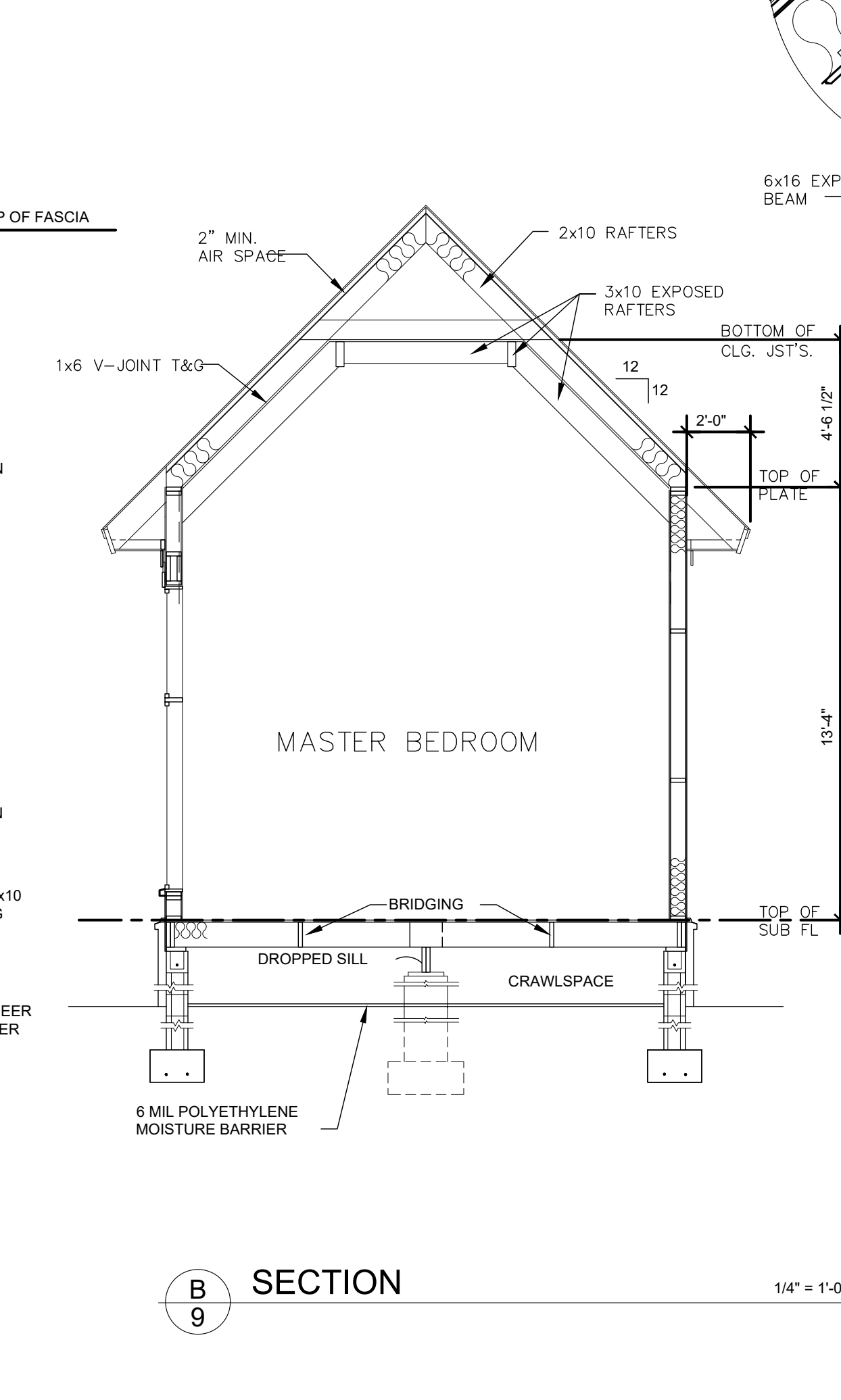
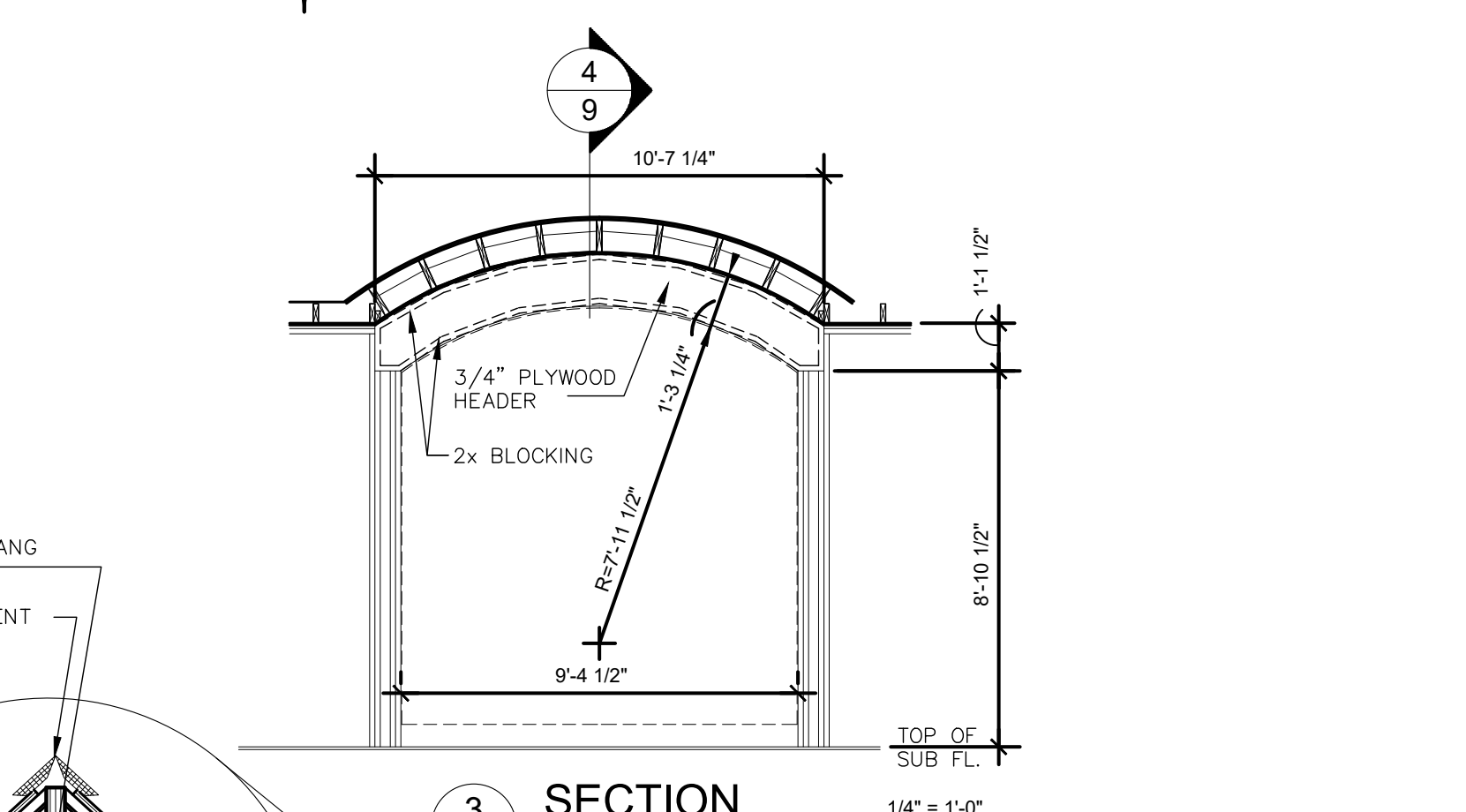
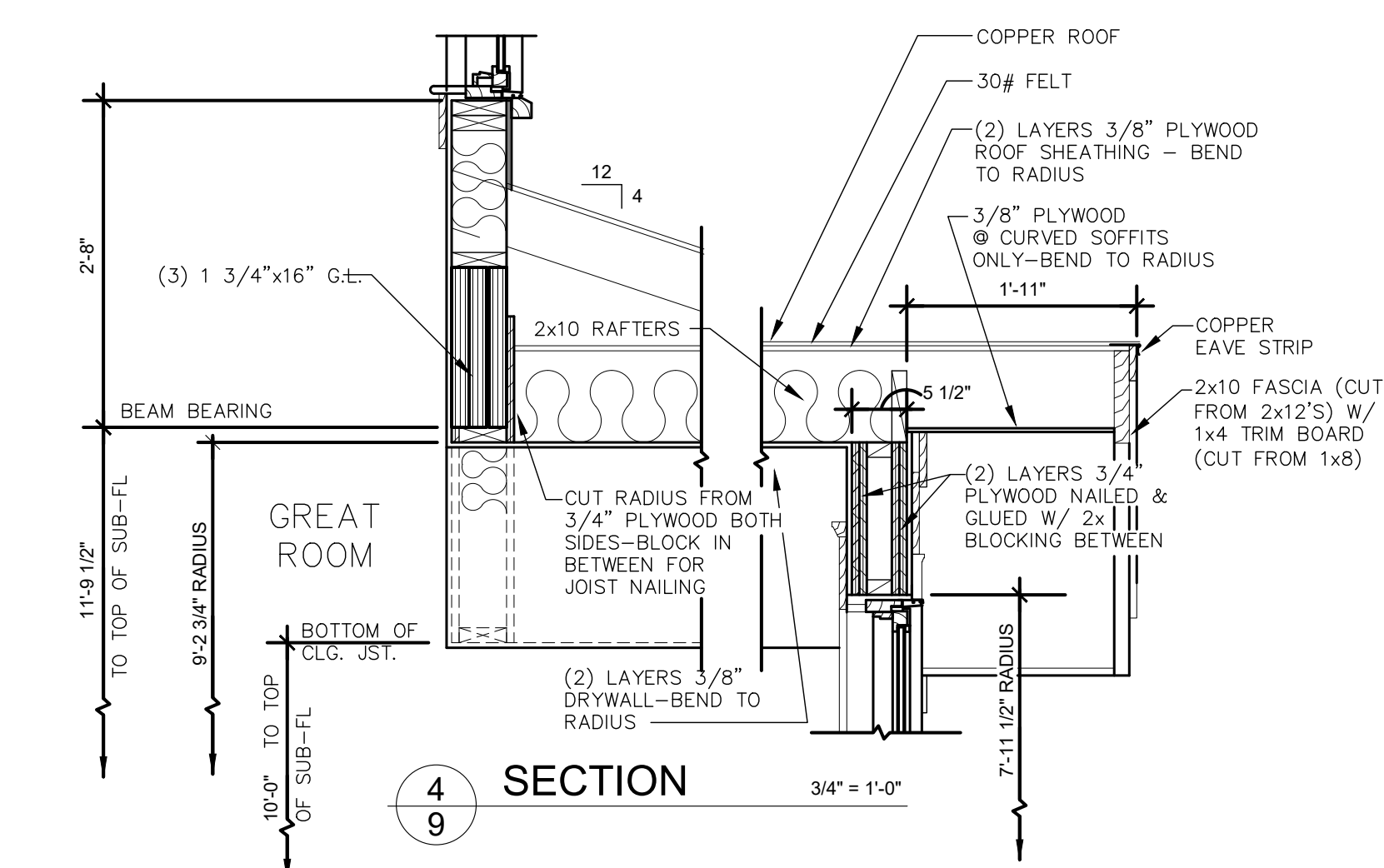
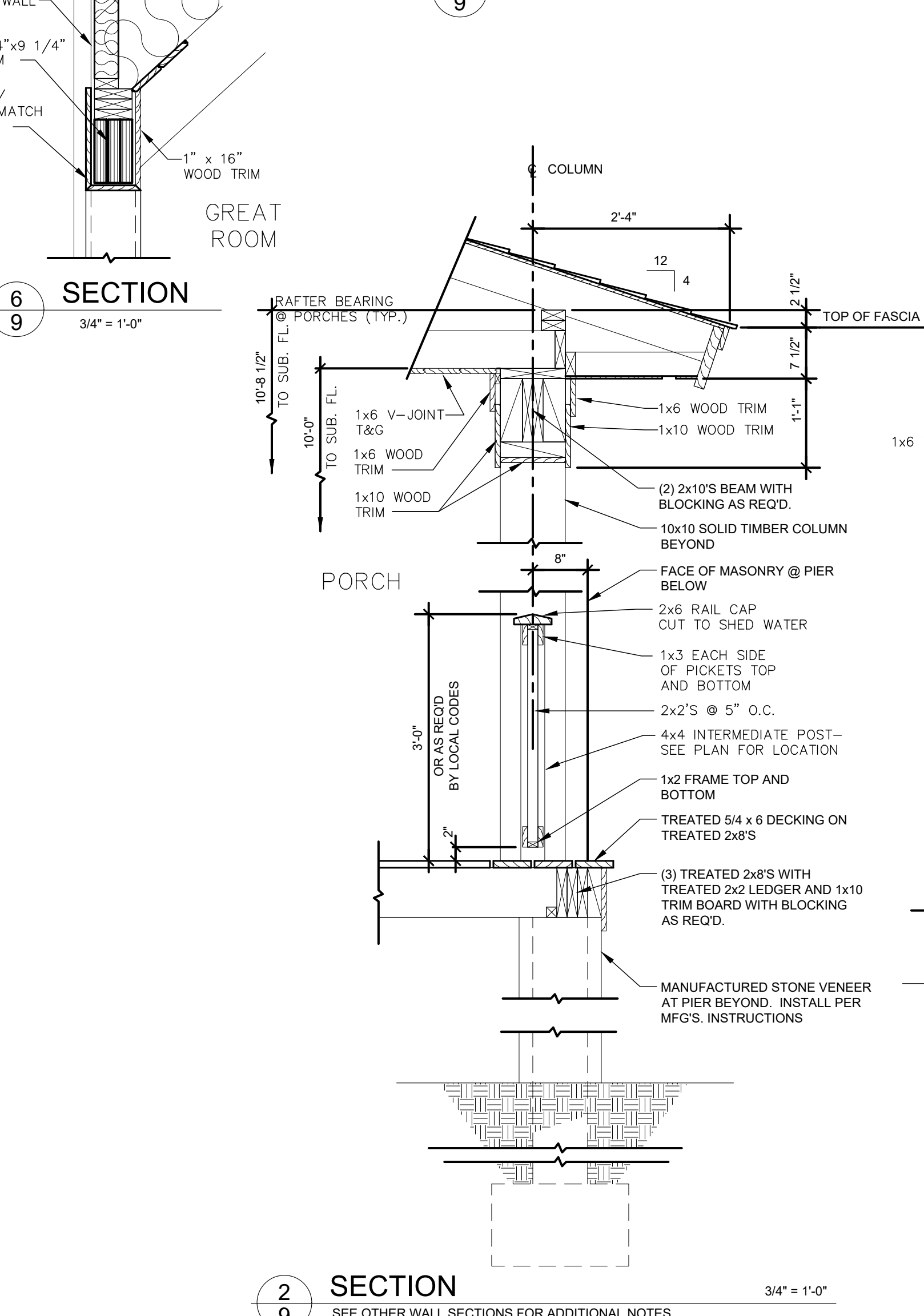
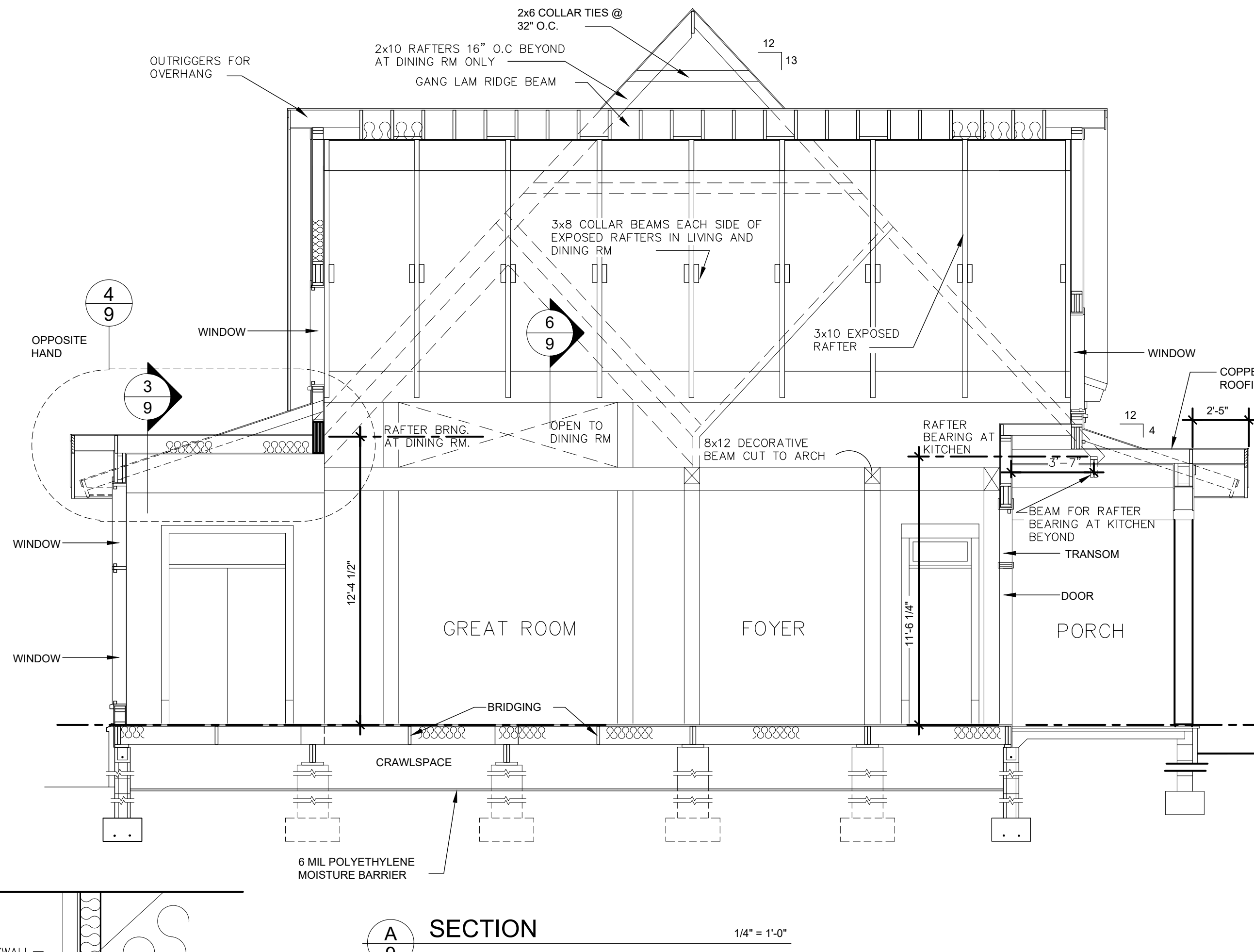
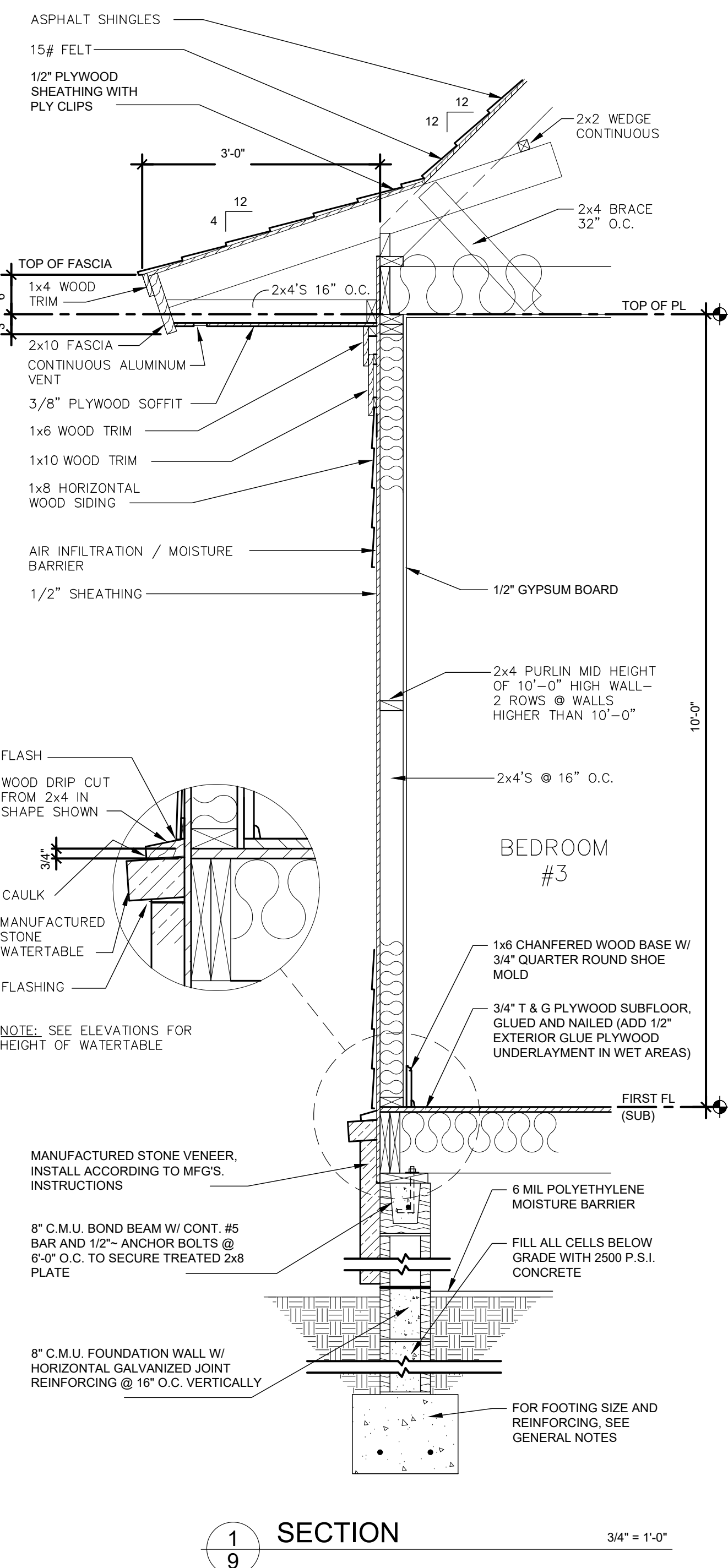
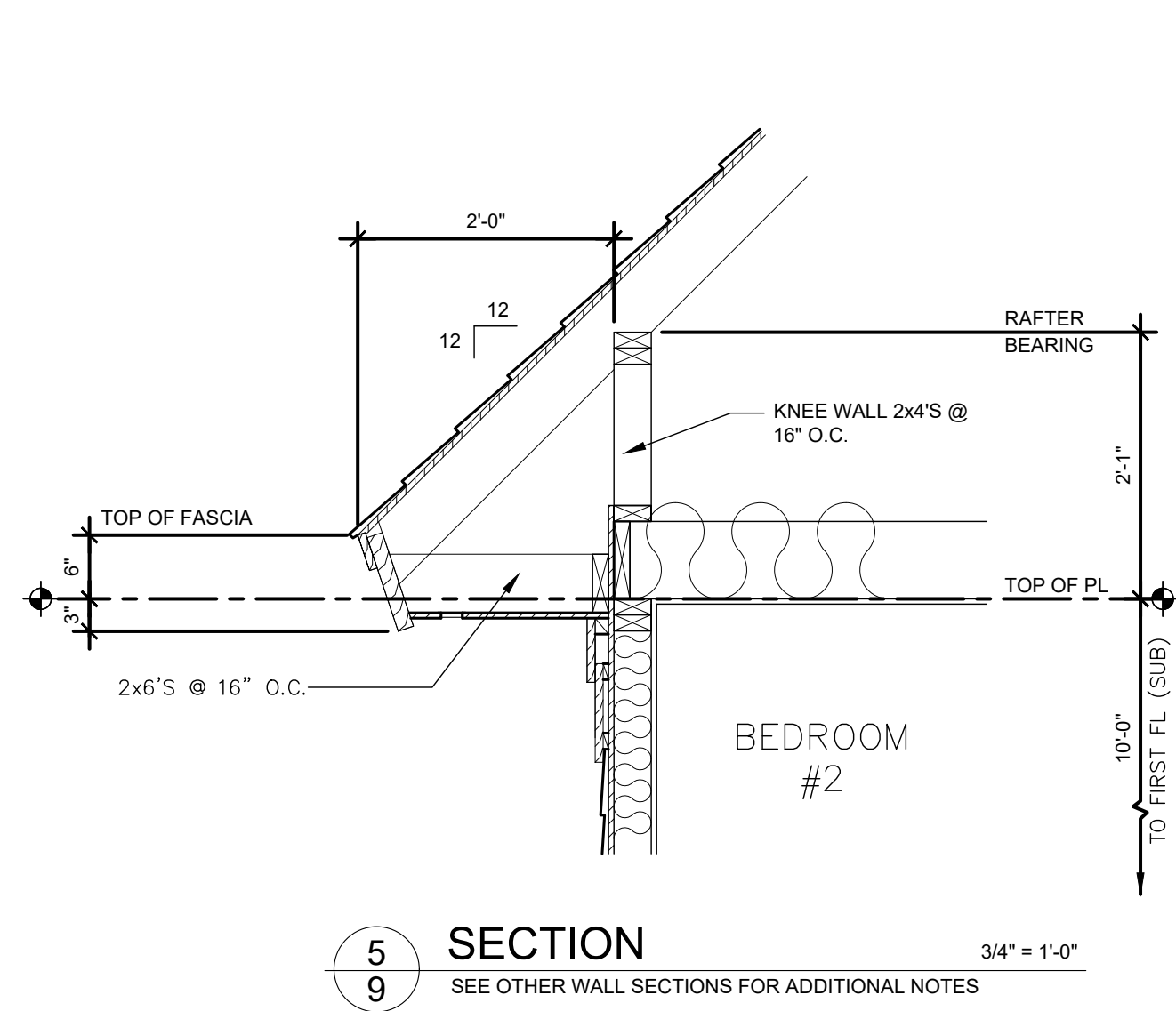
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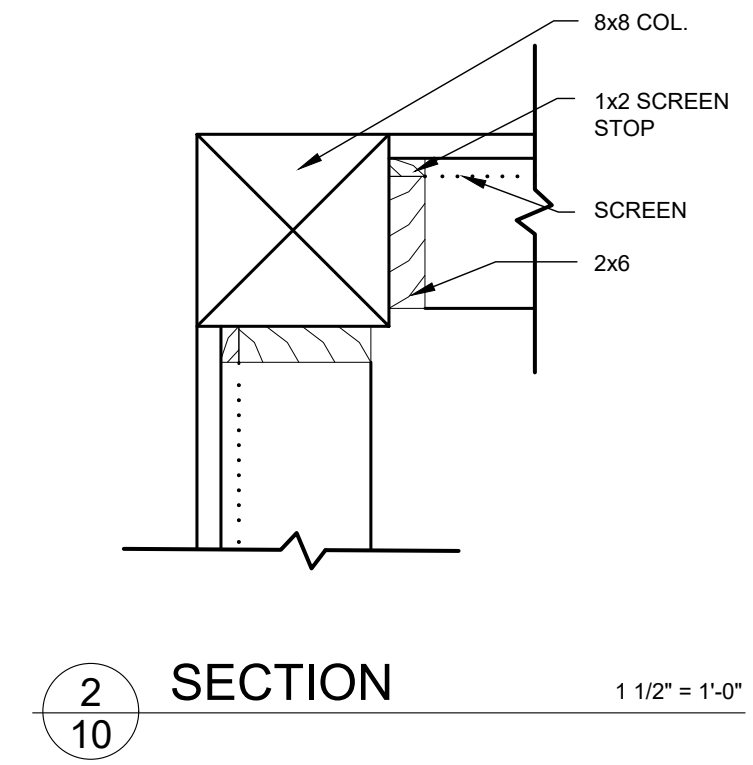
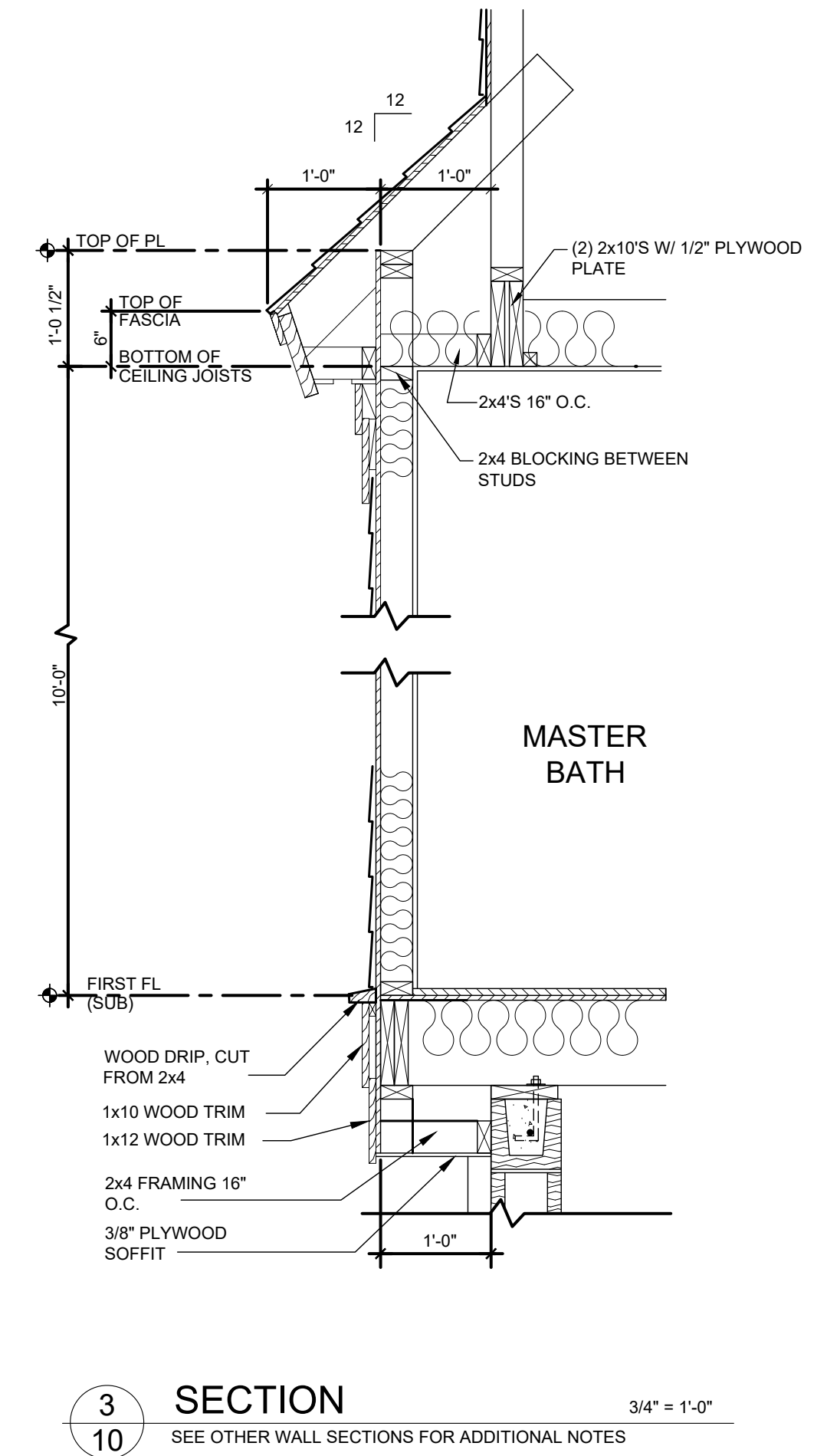
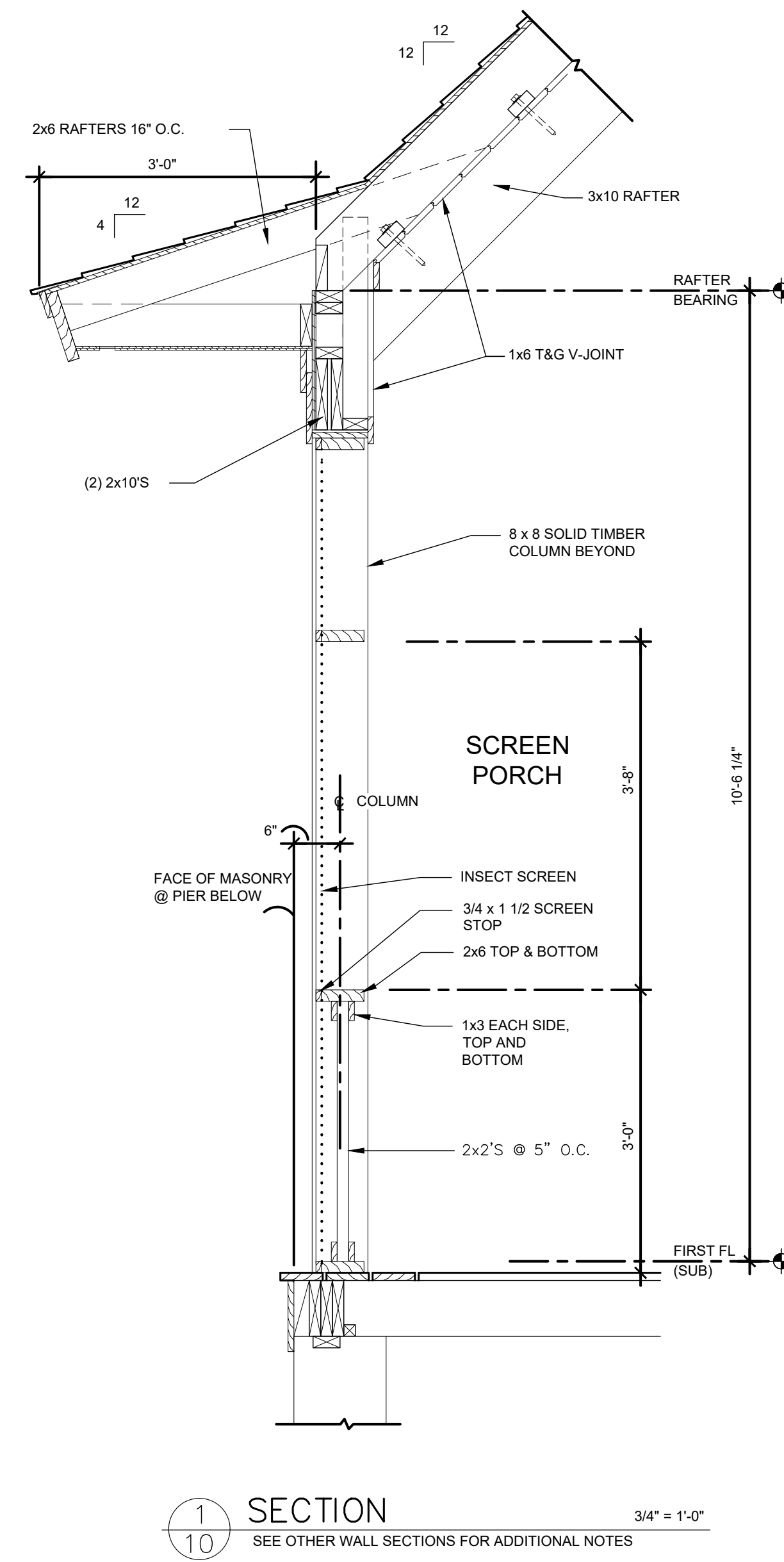
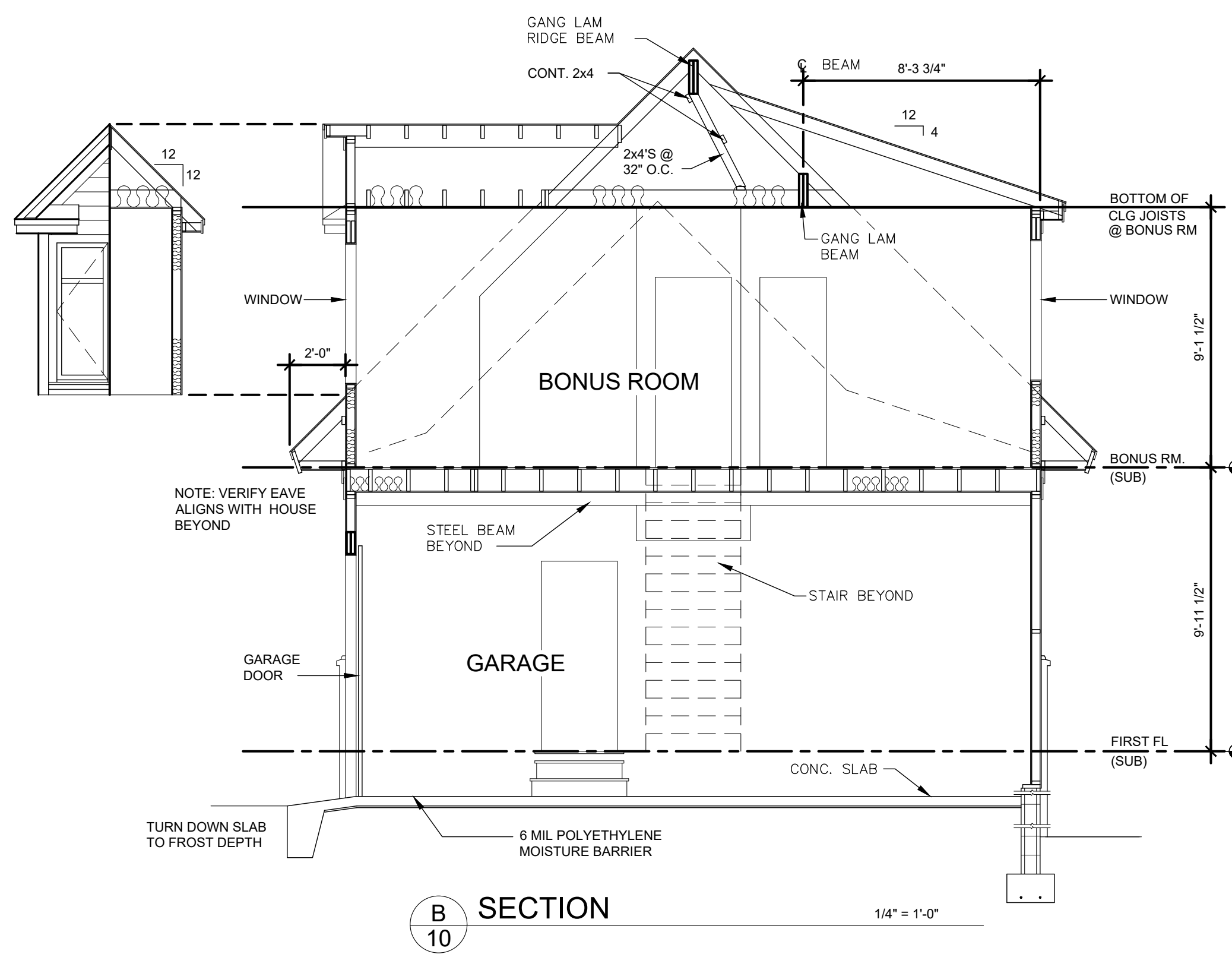
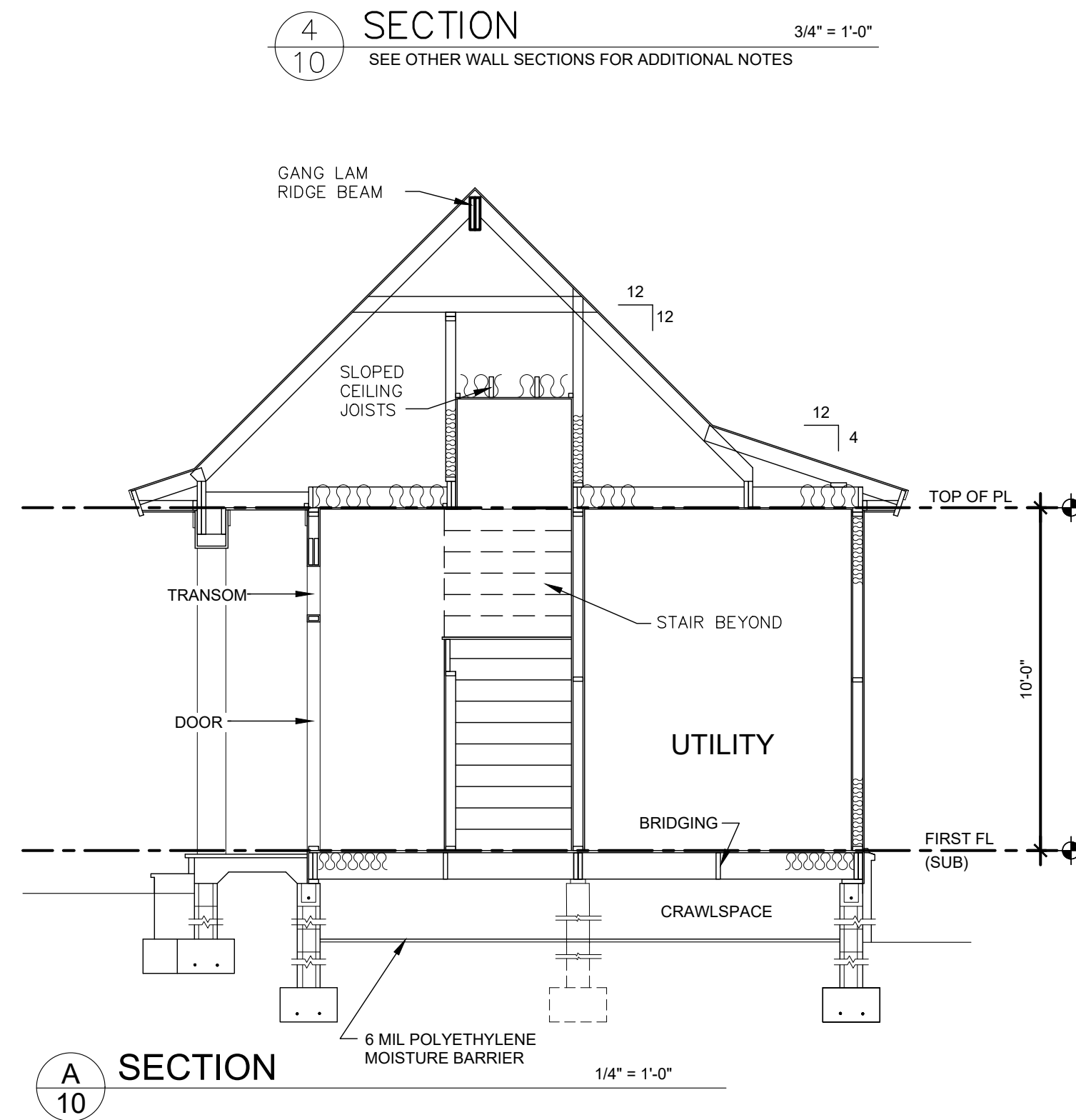
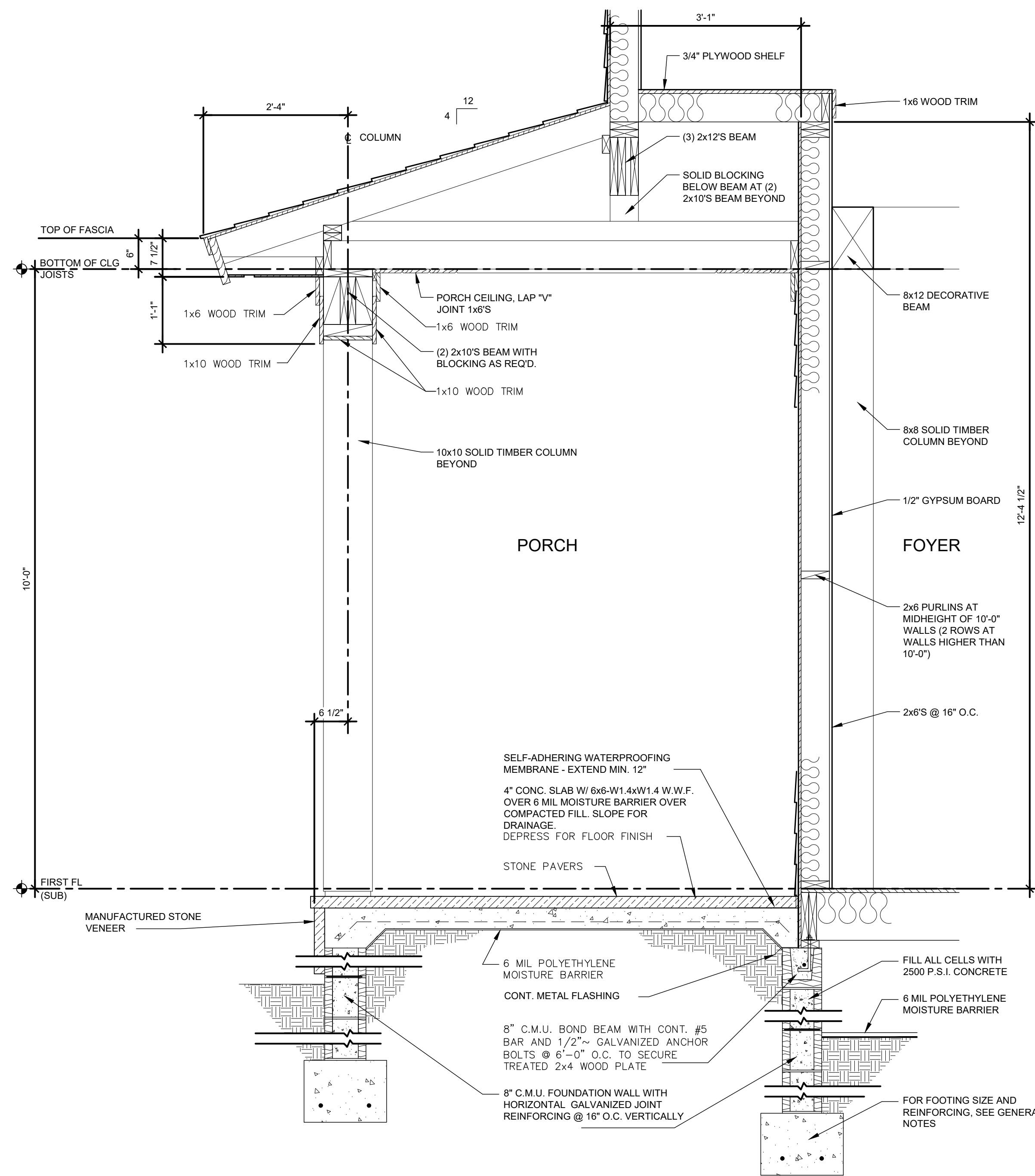
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DESIGN NO. **M19105-959-MR**
SHEET OF **8** DR **9** ENO
SECTIONS

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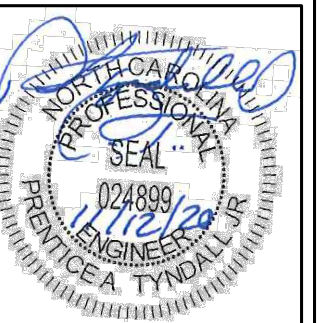
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DESIGN NO. **M19105-959-MR**
SHEET OF **9** ENO
SECTIONS

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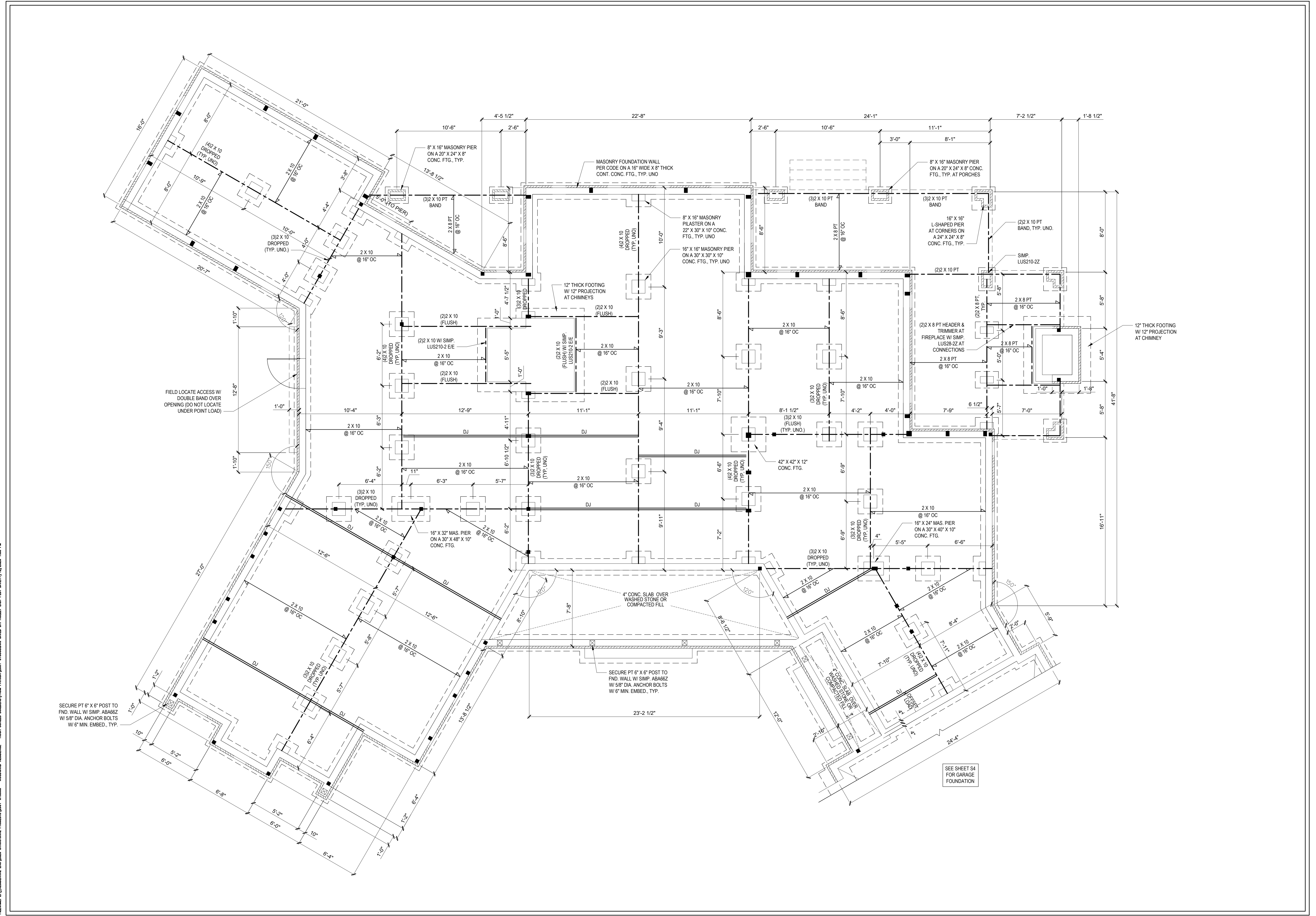
Client: **MITCH WOODWARD**
Project: **WOODWARD RESIDENCE**

**FOUNDATION PLAN
1ST FLOOR FRAMING**

Project #: 2001-010503
Date: 11/11/2020
Drawn/Design By: PSE
DWG. Checked By: PAT
Scale: SEE PLAN

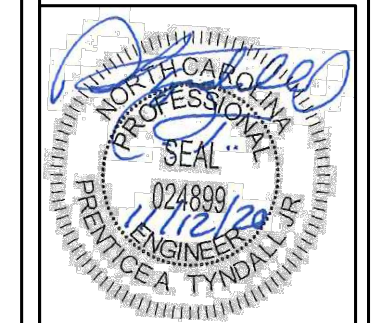
REVISIONS		
No.	Date	Remarks

Sheet Number
S1
1 of 8



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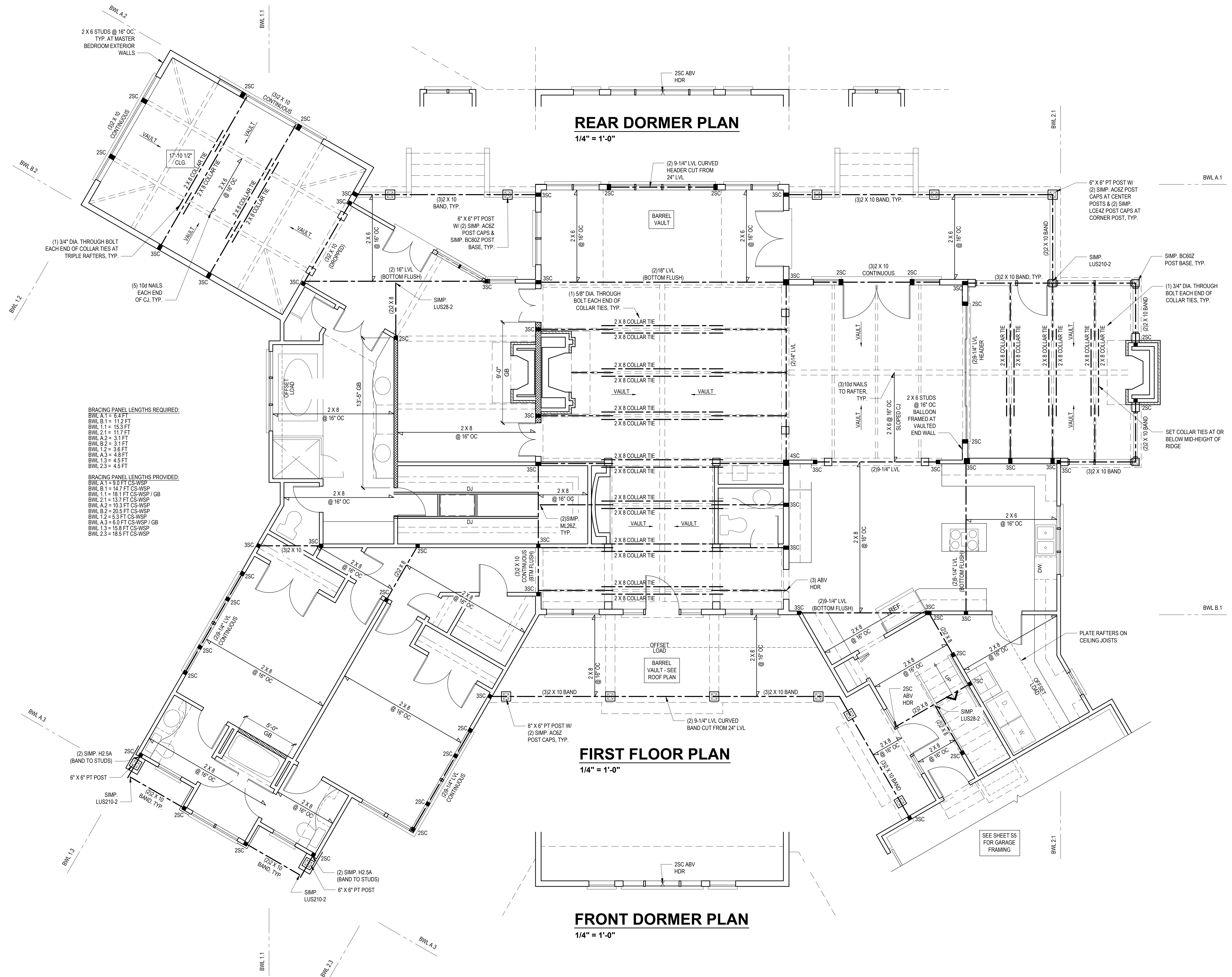
Client: MITCH WOODWARD
 Project: WOODWARD RESIDENCE

1ST FLOOR HEADER
 2ND FLOOR FRAMING

Project #: 2001-010503
 Date: 11/11/2020
 Drawn/Design By: PSE
 DWG. Checked By: PAT
 Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

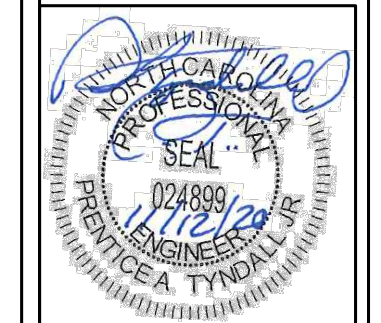
Sheet Number
S2
 2 of 8



- BRACING PANEL LENGTHS REQUIRED:**
 BWL A.1 = 6.4 FT
 BWL B.1 = 11.2 FT
 BWL 1.1 = 15.3 FT
 BWL 2.1 = 11.7 FT
 BWL A.2 = 3.1 FT
 BWL B.2 = 3.1 FT
 BWL 1.2 = 3.6 FT
 BWL A.3 = 4.8 FT
 BWL 1.3 = 4.5 FT
 BWL 2.3 = 4.5 FT
- BRACING PANEL LENGTHS PROVIDED:**
 BWL A.1 = 9.0 FT CS-WSP
 BWL B.1 = 14.7 FT CS-WSP
 BWL 1.1 = 18.1 FT CS-WSP / GB
 BWL 2.1 = 13.7 FT CS-WSP
 BWL A.2 = 10.3 FT CS-WSP
 BWL B.2 = 20.5 FT CS-WSP
 BWL 1.2 = 5.3 FT CS-WSP
 BWL A.3 = 5.0 FT CS-WSP / GB
 BWL 1.3 = 15.8 FT CS-WSP
 BWL 2.3 = 18.5 FT CS-WSP

FILENAME: Z:\RESIDENTIAL ENR\2020 STRUCTURAL PROJECTS\2001-010503 - WOODWARD RESIDENCE - HIGH SHELVE BUILDERS\FROM PATRICK\2001-010503\DWG 06: HOLEY LAST PUT DATE: 11/12/2020 1:53 PM

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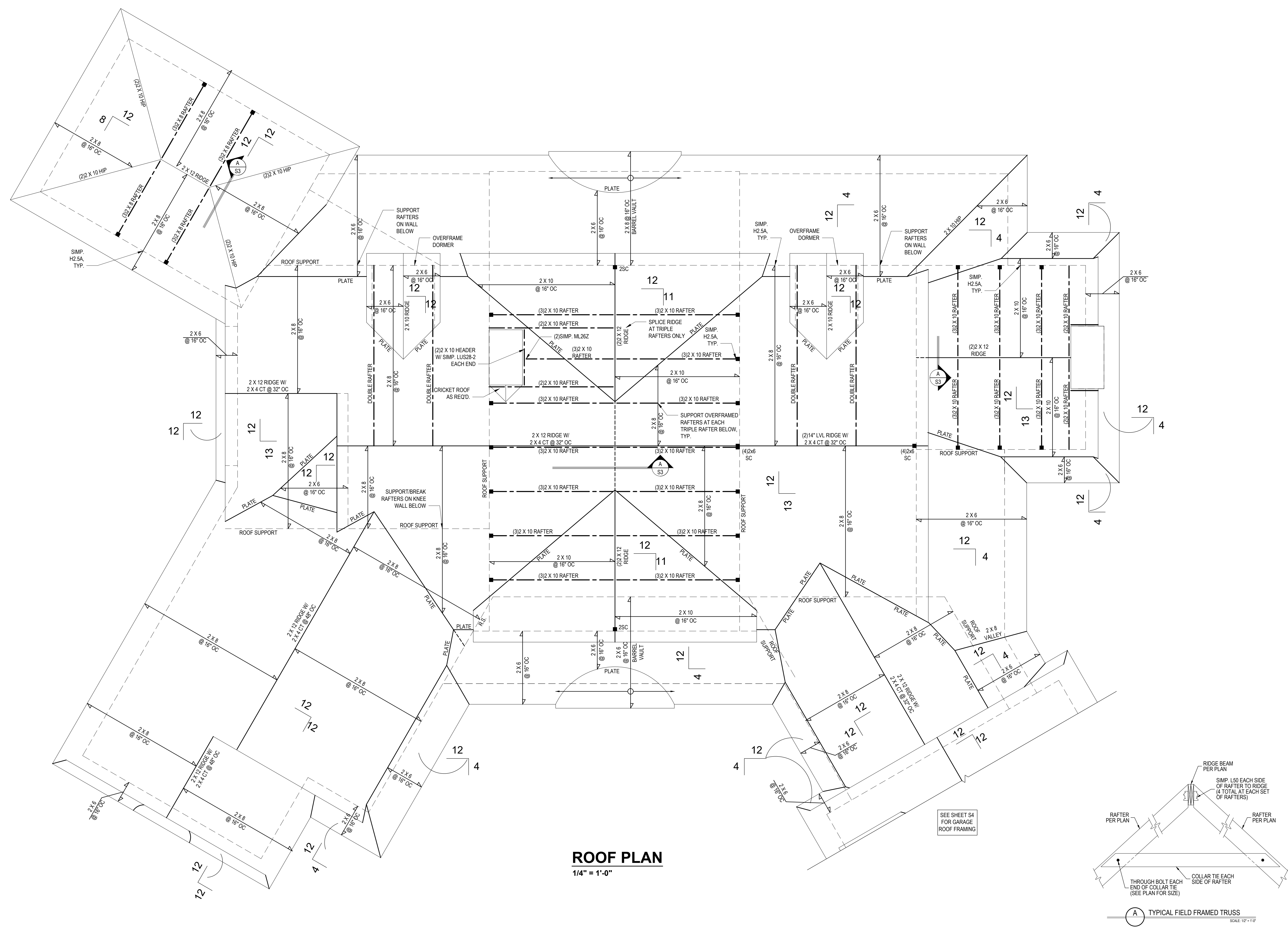
Client: **MITCH WOODWARD**
 Project: **WOODWARD RESIDENCE**

ROOF PLAN

Project #: 2001-010503
 Date: 11/11/2020
 Drawn/Design By: PSE
 DWG. Checked By: PAT
 Scale: SEE PLAN

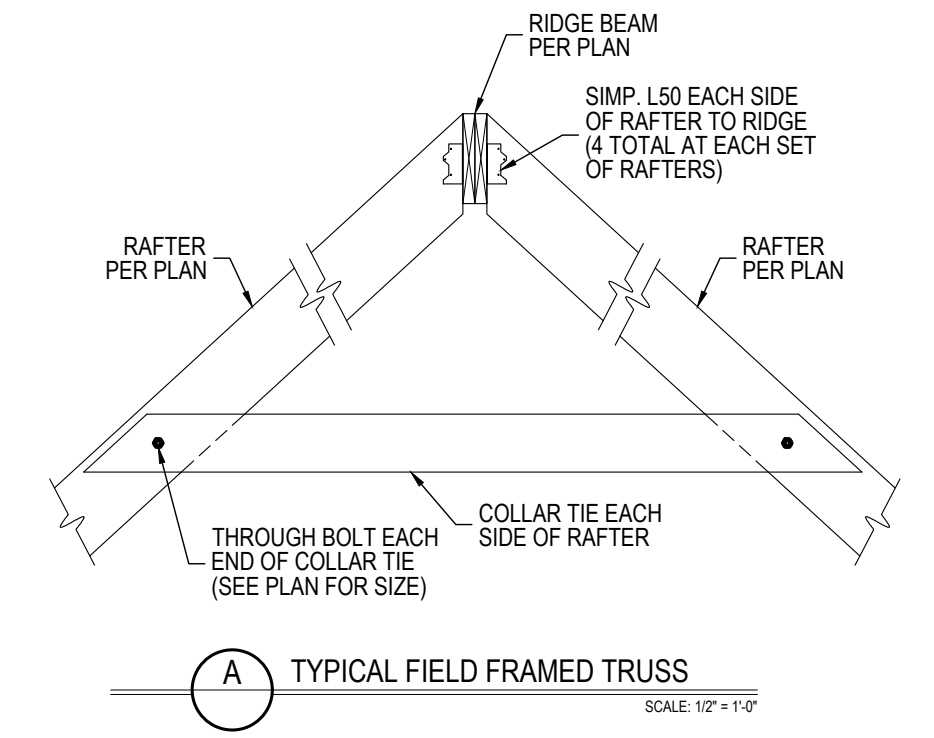
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No.	Date	Remarks

Sheet Number
S3
 3 of 8



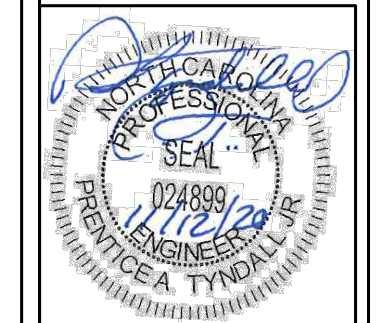
ROOF PLAN
 1/4" = 1'-0"

SEE SHEET S4
 FOR GARAGE
 ROOF FRAMING



FILENAME: Z:_RESIDENTIAL_ENR\2020_STRUCTURAL_PROJECTS\2001-010503 - WOODWARD RESIDENCE - HIGH SINGLE BUILDERS\FRAM PART\001-010503-01-ROOFPLAN.dwg
 PROJECT: 2001-010503 - WOODWARD RESIDENCE - HIGH SINGLE BUILDERS
 DRAWN: PAT
 CHECKED: PAT
 DATE: 11/11/2020 1:53 PM

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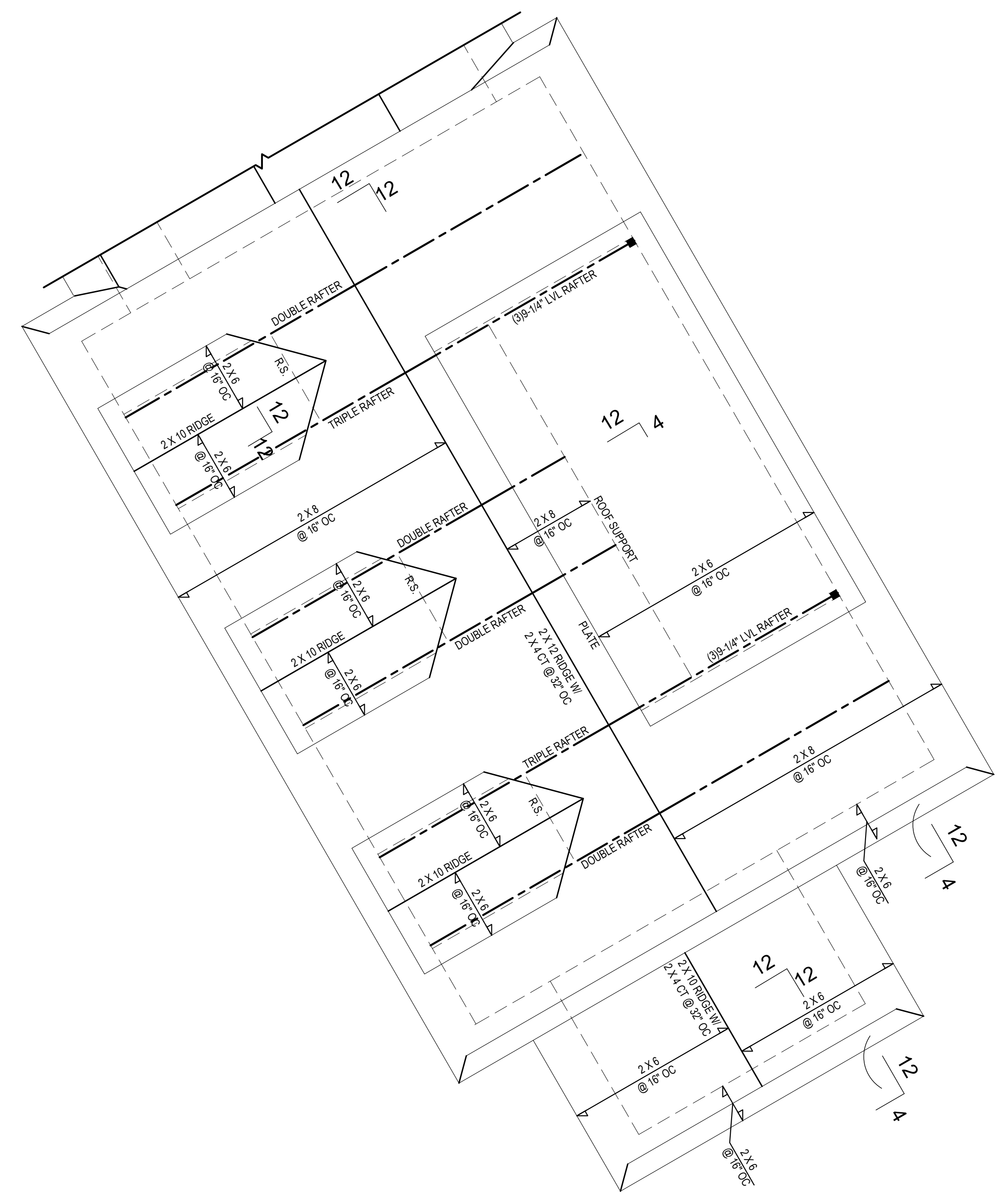
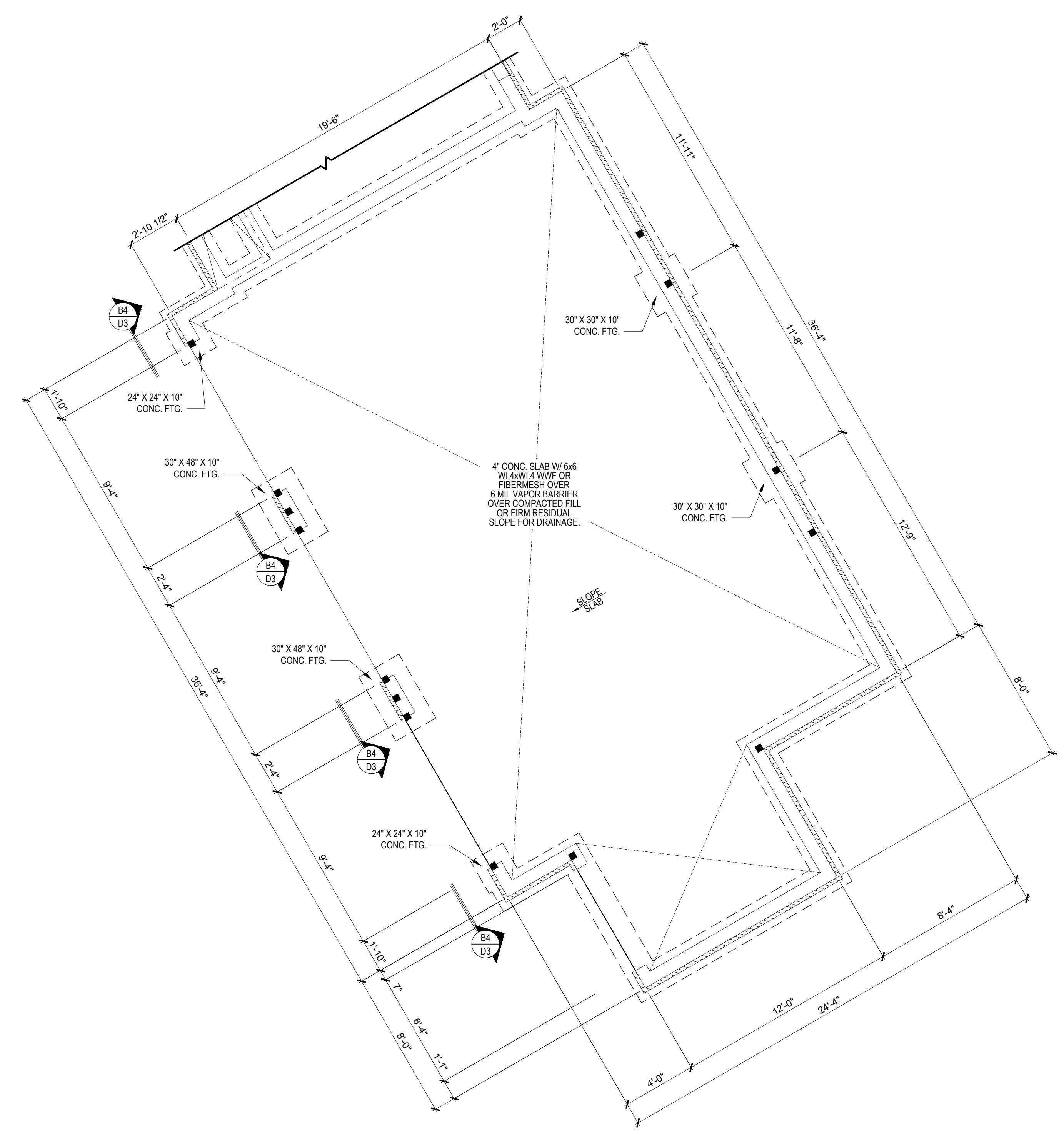
CLIENT: MITCH WOODWARD
 PROJECT: WOODWARD RESIDENCE

GARAGE ROOF PLAN & FOUNDATION PLAN

Project #: 2001-010503
 Date: 11/11/2020
 Drawn/Design By: PSE
 DWG. Checked By: PAT
 Scale: SEE PLAN

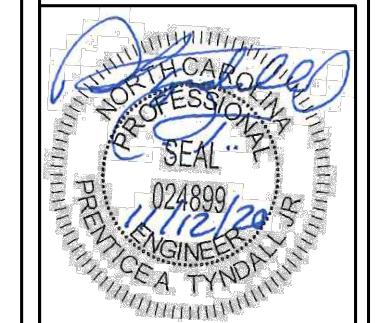
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No.	Date	Remarks

Sheet Number
S4
 4 of 8



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Raleigh, NC 27601
919.775.2500 • 919.775.4444
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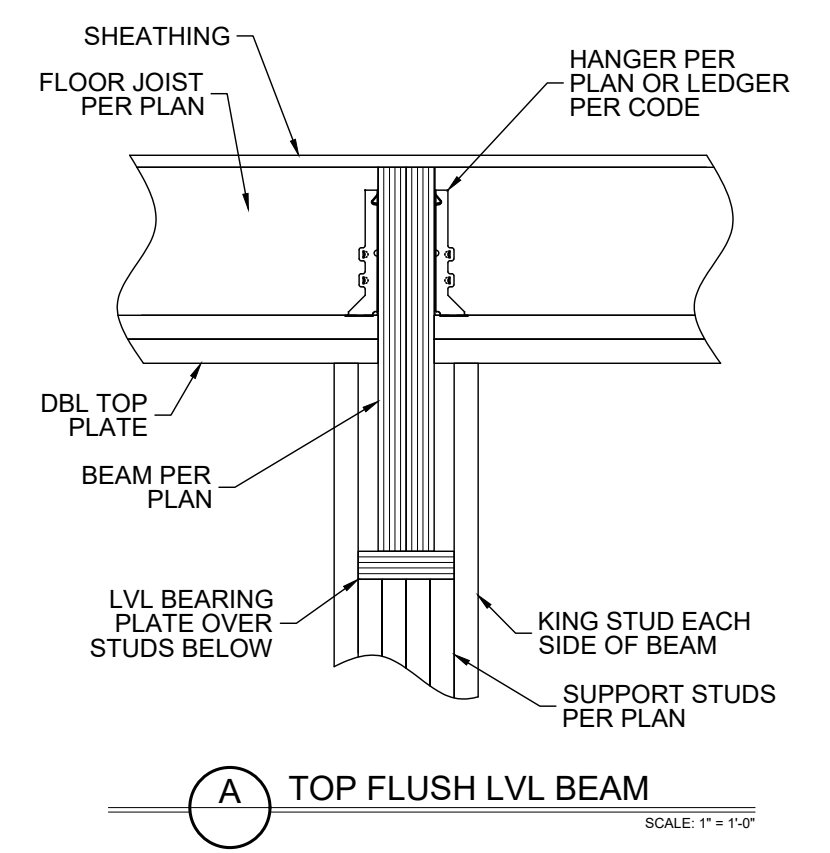
Client: MITCH WOODWARD
Project: WOODWARD RESIDENCE

**GARAGE 2ND FLR FRMG
& 2ND FLR CLG. FRMG**

Project #: 2001-010503
Date: 11/11/2020
Drawn/Design By: PSE
DWG. Checked By: PAT
Scale: SEE PLAN

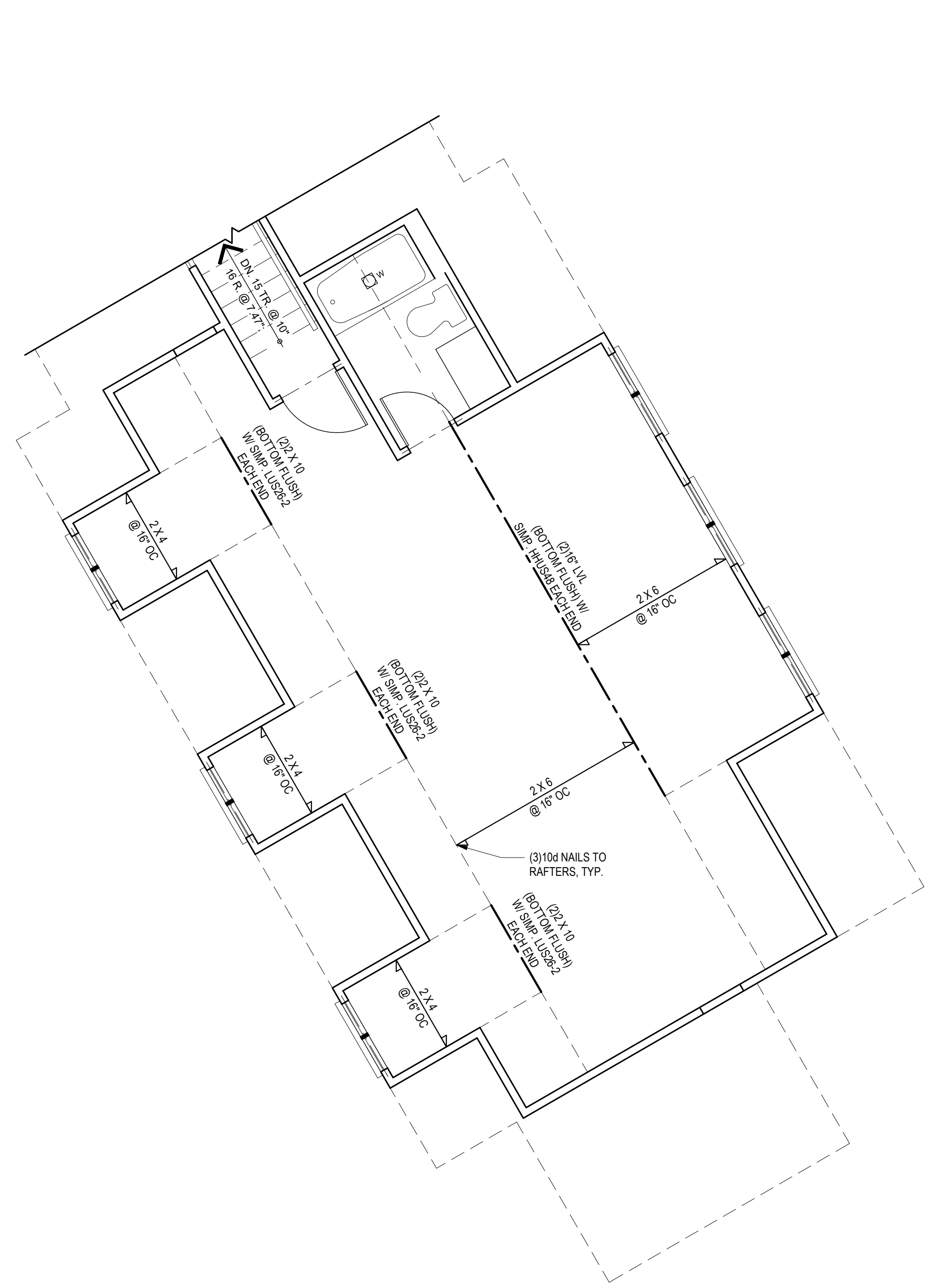
REVISIONS		
No.	Date	Remarks

Sheet Number
S5
5 of 8

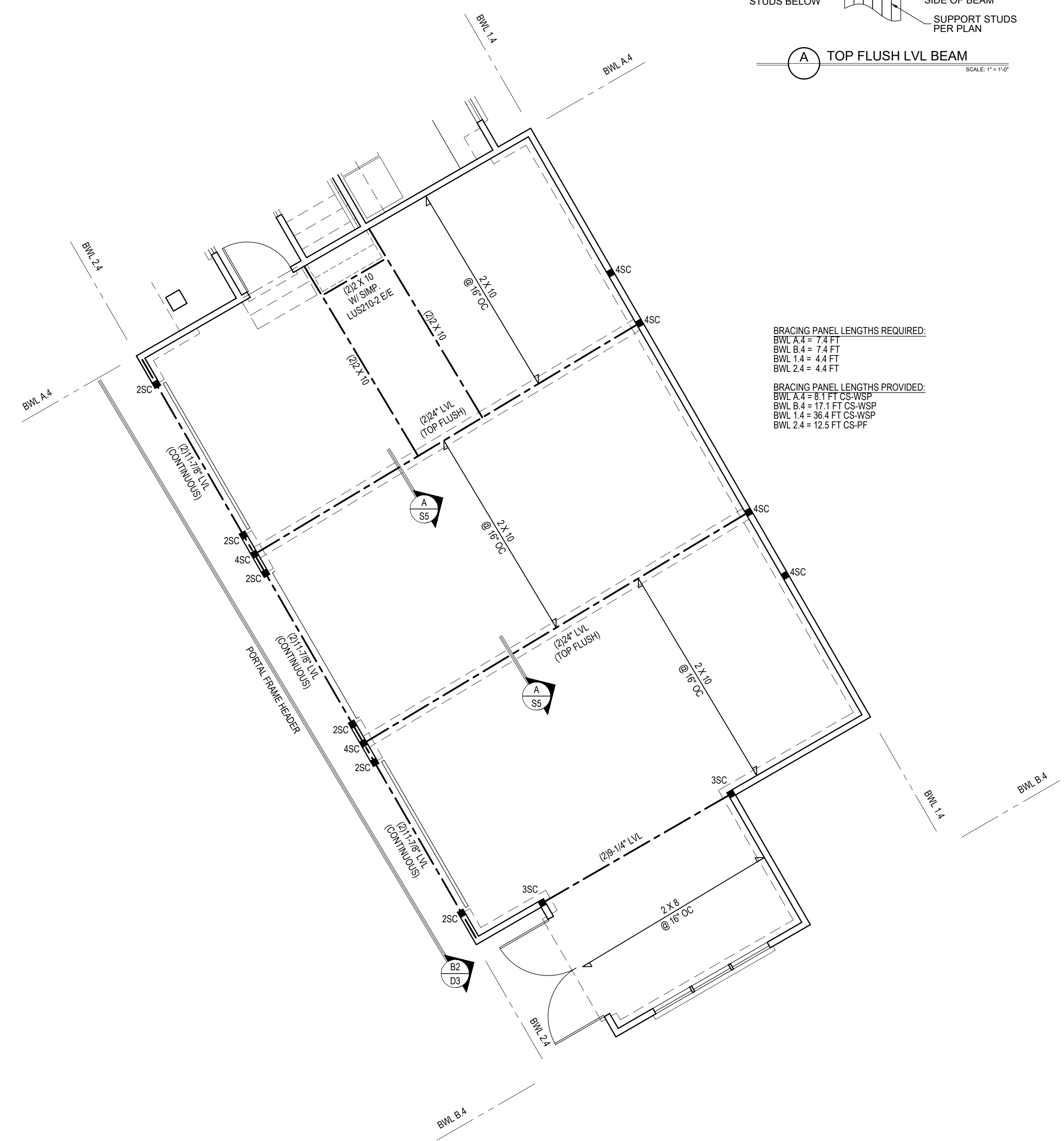


BRACING PANEL LENGTHS REQUIRED:
BWL 2.4 = 7.4 FT
BWL 2.4 = 7.4 FT
BWL 1.4 = 4.4 FT
BWL 2.4 = 4.4 FT

BRACING PANEL LENGTHS PROVIDED:
BWL 2.4 = 8.1 FT CS-WSP
BWL 2.4 = 17.1 FT CS-WSP
BWL 1.4 = 36.4 FT CS-WSP
BWL 2.4 = 12.5 FT CS-PF



SECOND FLOOR PLAN
1/4" = 1'-0"



FIRST FLOOR PLAN
1/4" = 1'-0"

FILENAME: Z:\RESIDENTIAL ENR\2020 STRUCTURAL PROJECTS\2001-010503 - WOODWARD RESIDENCE - HIGH SHELVE BUILDERS\FROM PATRICK\2001-010503-010503-11122020 153.PLM

STRUCTURAL NOTES

- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF 'NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE', IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
- DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
			LL	TL
ALL FLOORS	40	10	L/360	L/240
ATTIC (w/ walk up stairs)	30	10	L/360	L/240
ATTIC (pull down access)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	20	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	SEISMIC ZONES A, B & C			
- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE (U.N.C.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R602.3 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT, WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON D/JO) (U.N.)
ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2000 PSI, E = 1.9M PSI (U.N.O.)
ALL L.S. LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2325 PSI, E = 1.8M PSI (U.N.O.)
ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10 (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50.
ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36.
ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1 1/2" x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 12"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.
- WALL AND ROOF CLADDING VALUES:
WALL CLADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
ROOF WALLS BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:
39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1/12
36.0 LBS/SQFT FOR ROOF PITCHES 1/12 TO 2/12
18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12
*MEAN ROOF HEIGHT 3/12" OR LESS
- FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- UPLIFT LOADS GREATER THAN 50#F SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA
- PSL COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- PROVIDE A MINIMUM OF 50#F UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- MAXIMUM MASONRY PER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

DEFINITIONS FOR COMMON ABBREVIATIONS

ALT = ALTERNATE	MAX = MAXIMUM
CANT = CANTILEVER	MIN = MINIMUM
CJ = CEILING JOIST	NOM = NOMINAL
CMU = CONCRETE MASONRY UNIT	O.C. = ON CENTER
COL = COLUMN	PL = POINT LOAD
CONC = CONCRETE	PT = PRESSURE TREATED
CONT = CONTINUOUS	REIN = REINFORCED
CT = COLLAR TIE	REQD = REQUIRED
DBL = DOUBLE	RJ = ROOF JOIST
DIA = DIAMETER	RS = ROOF SUPPORT
DJ = DOUBLE JOIST	SC = STUD COLUMN
DR = DOUBLE RAFTER	SCH = SCHEDULE
EACH = EACH END	SPEC = SPECIFIED
FJ = FLOOR JOIST	THK = THICK
FND = FOUNDATION	TRD = TREATED
FTG = FOOTING	TYP = TYPICAL
GALV = GALVANIZED	UNO = UNLESS NOTED OTHERWISE
HORIZ = HORIZONTAL	W = WIDE FLANGE BEAM
HT = HEIGHT	WWF = WELDED WIRE FABRIC
MANUF = MANUFACTURER	XJ = EXTRA JOIST

1) MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

POST SIZE	MAX. POST HEIGHT**
4 x 4	8'-0"
6 x 6	20'-0"
***	OVER 20'-0"

* THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS. MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
** FROM TOP OF FOOTING TO BOTTOM OF GIRDER.
*** DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.

2) DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THESE METHODS:

A. THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION (4) ABOVE. LATERAL BRACING IS NOT REQUIRED.

B. 4 x 4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED BOLT AT EACH END OF THE BRACE.

C. FOR FREESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO (2) PERPENDICULAR DIRECTIONS FOR FREESTANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6 SHALL BE ATTACHED TO THE POSTS WITH ONE 5/8"Ø HOT DIPPED GALVANIZED BOLT AT EACH END OF EACH BRACING MEMBER.

E. FOR EMBEDMENT OF PILLS IN COASTAL REGIONS, SEE CHAPTER 46.

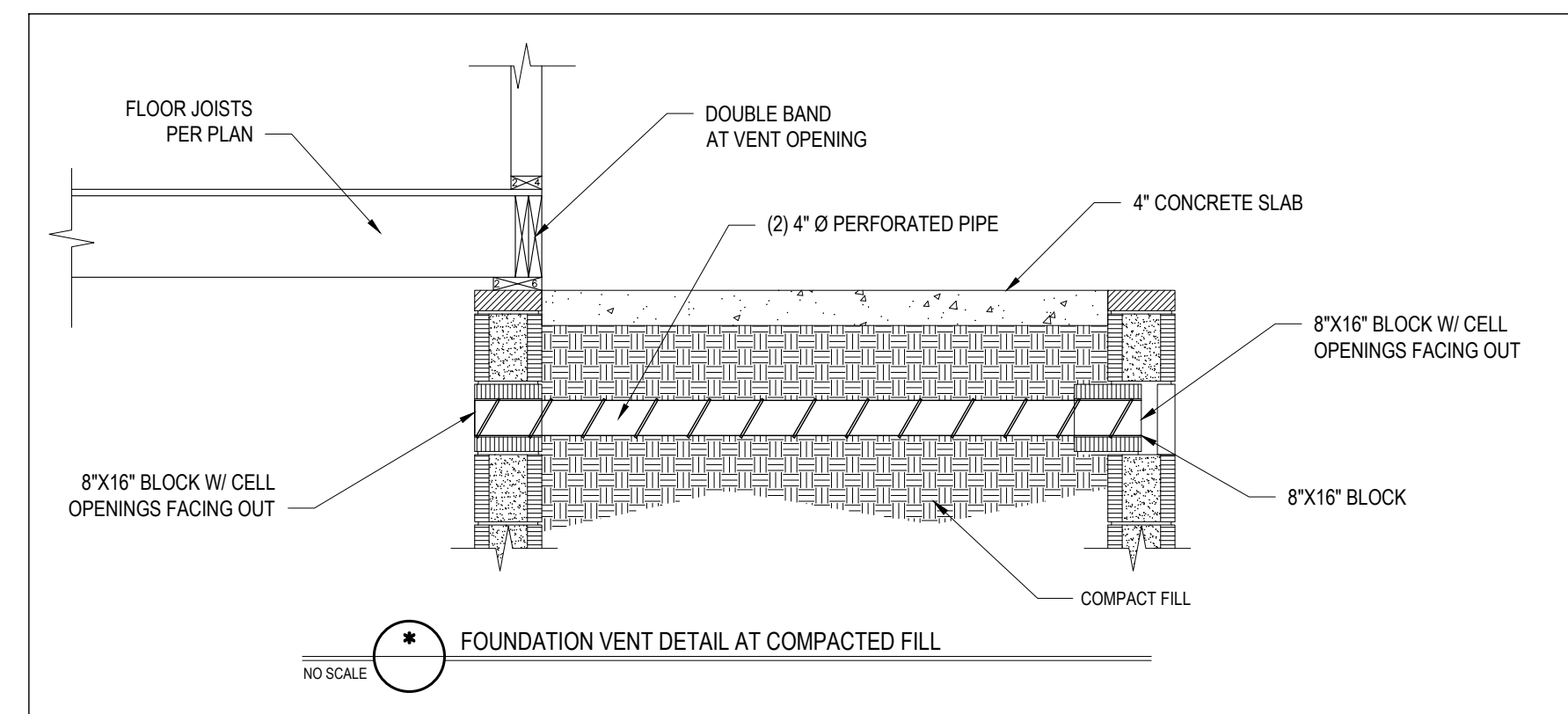
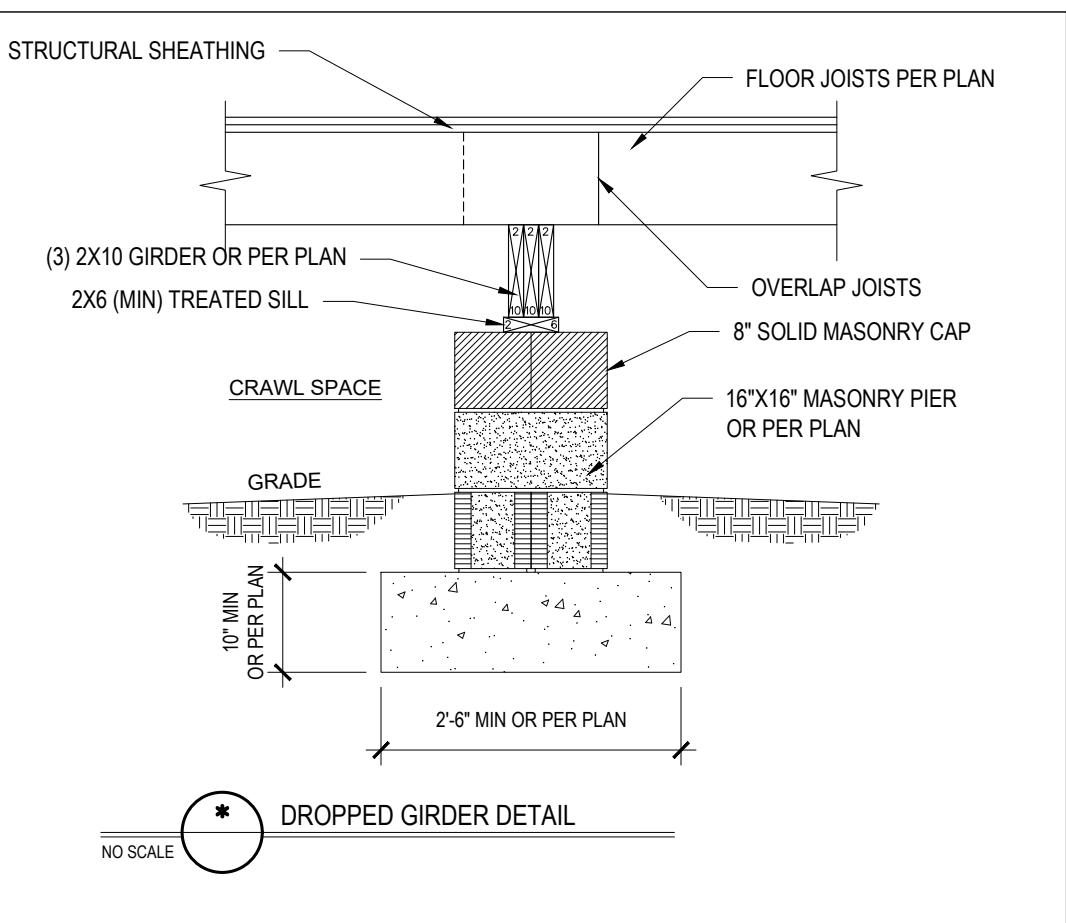
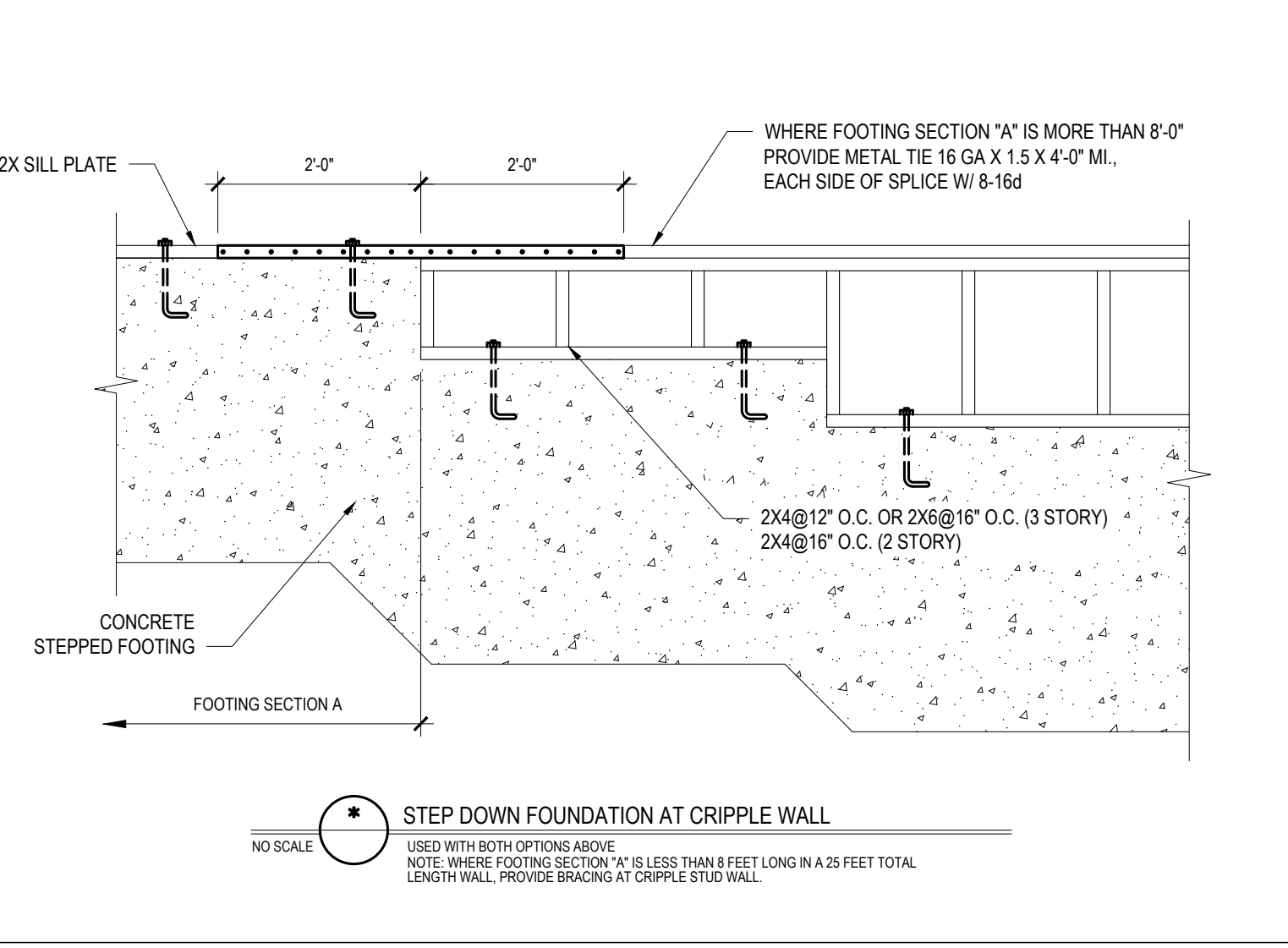
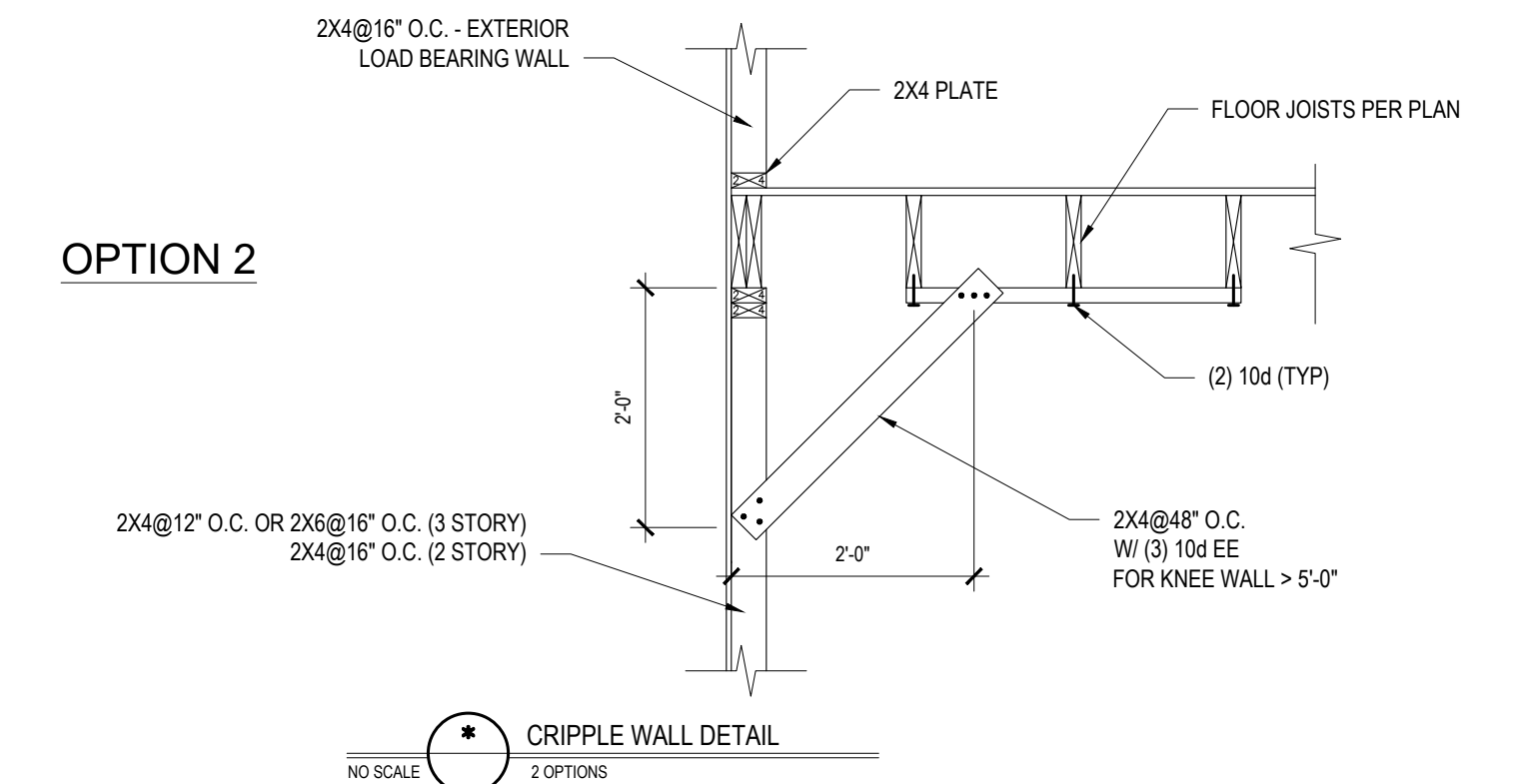
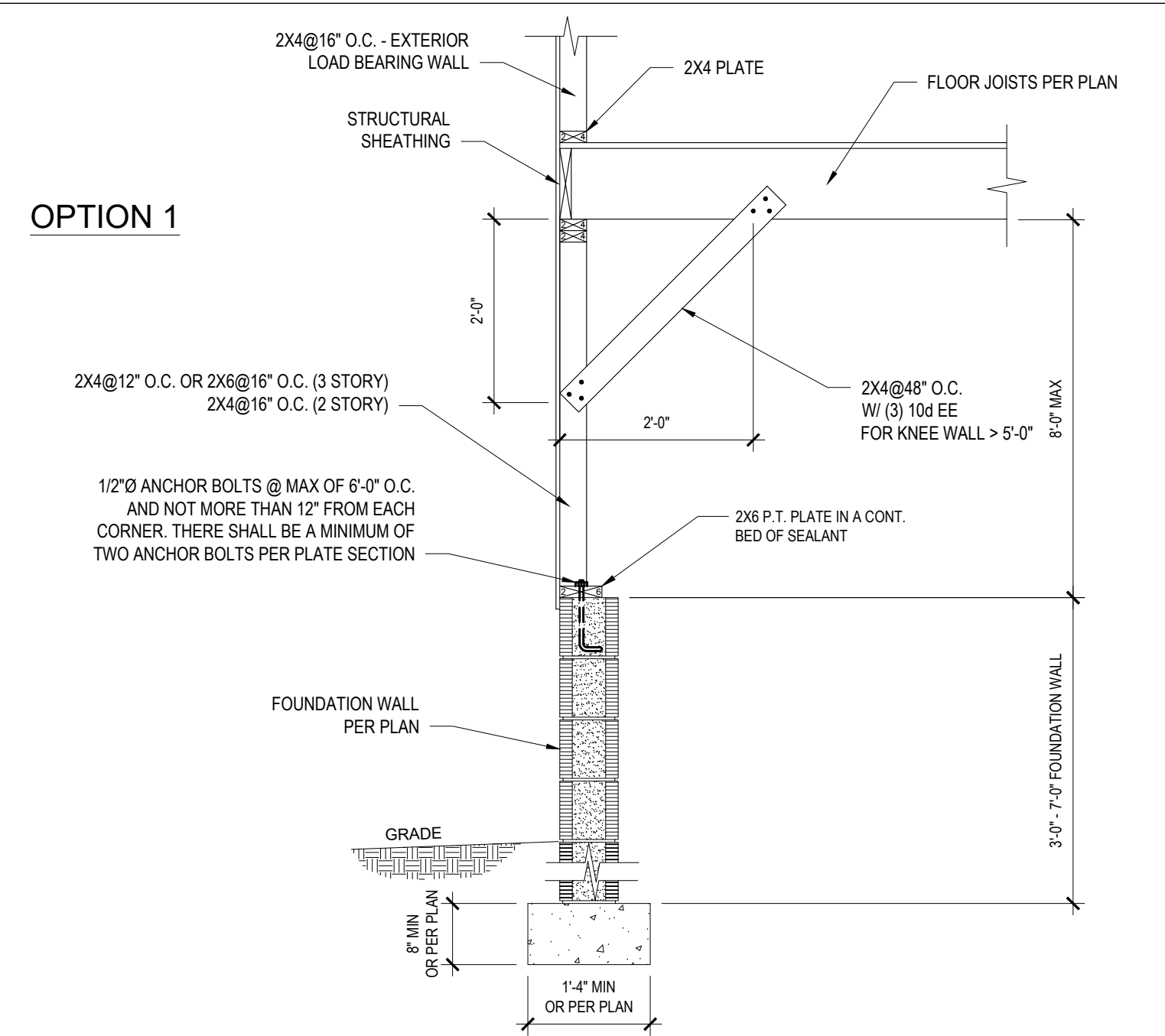
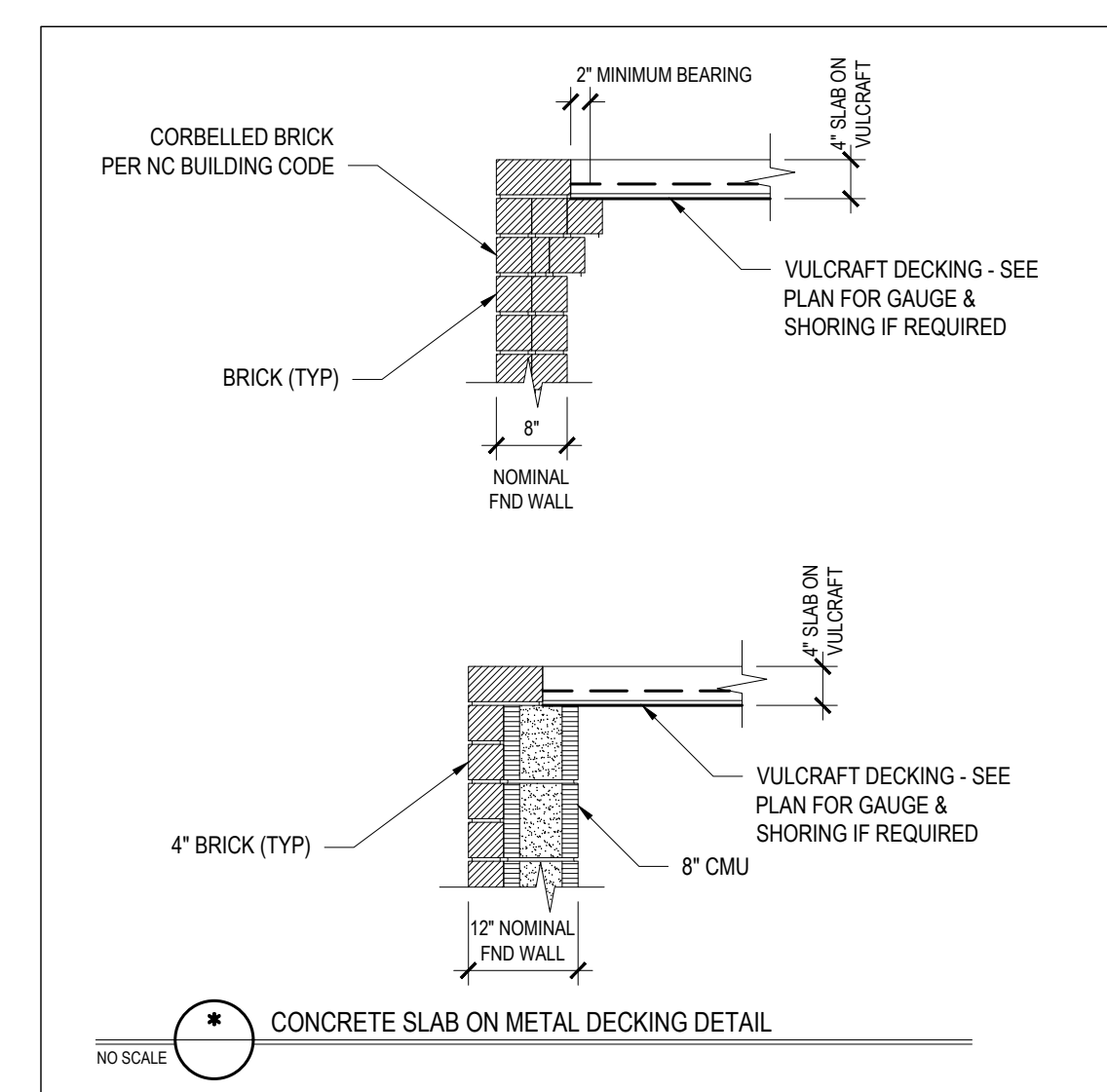
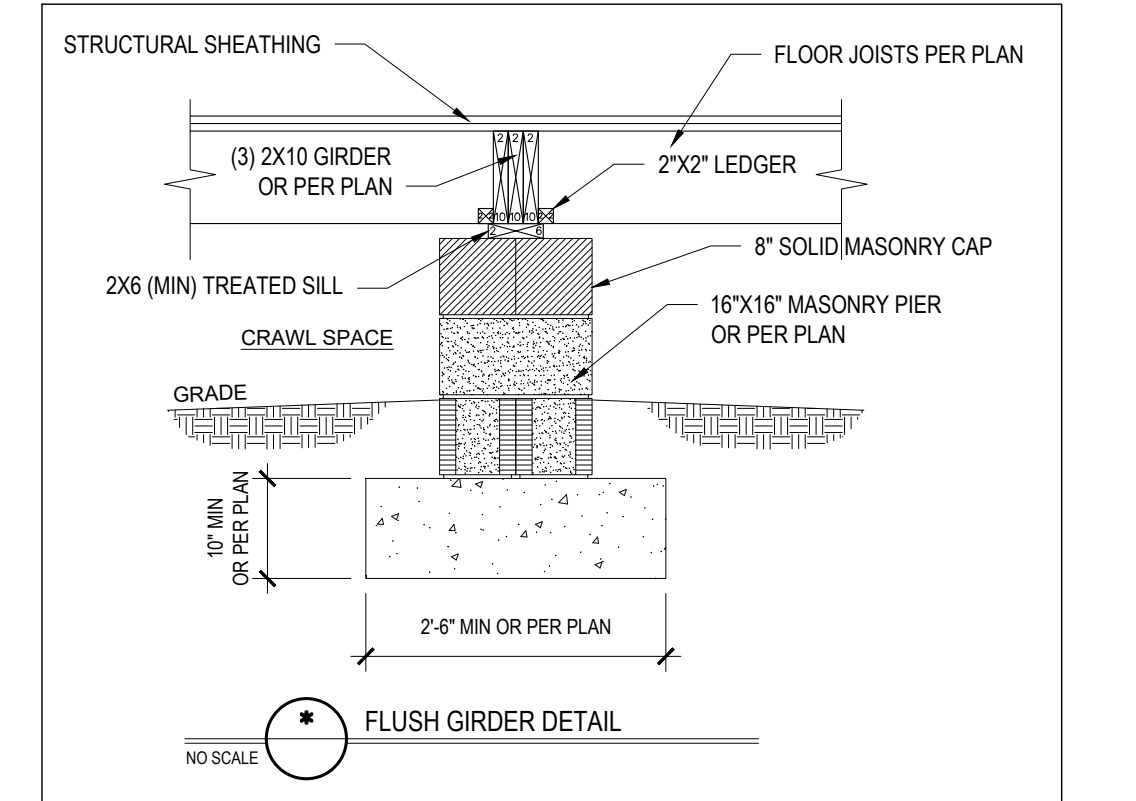
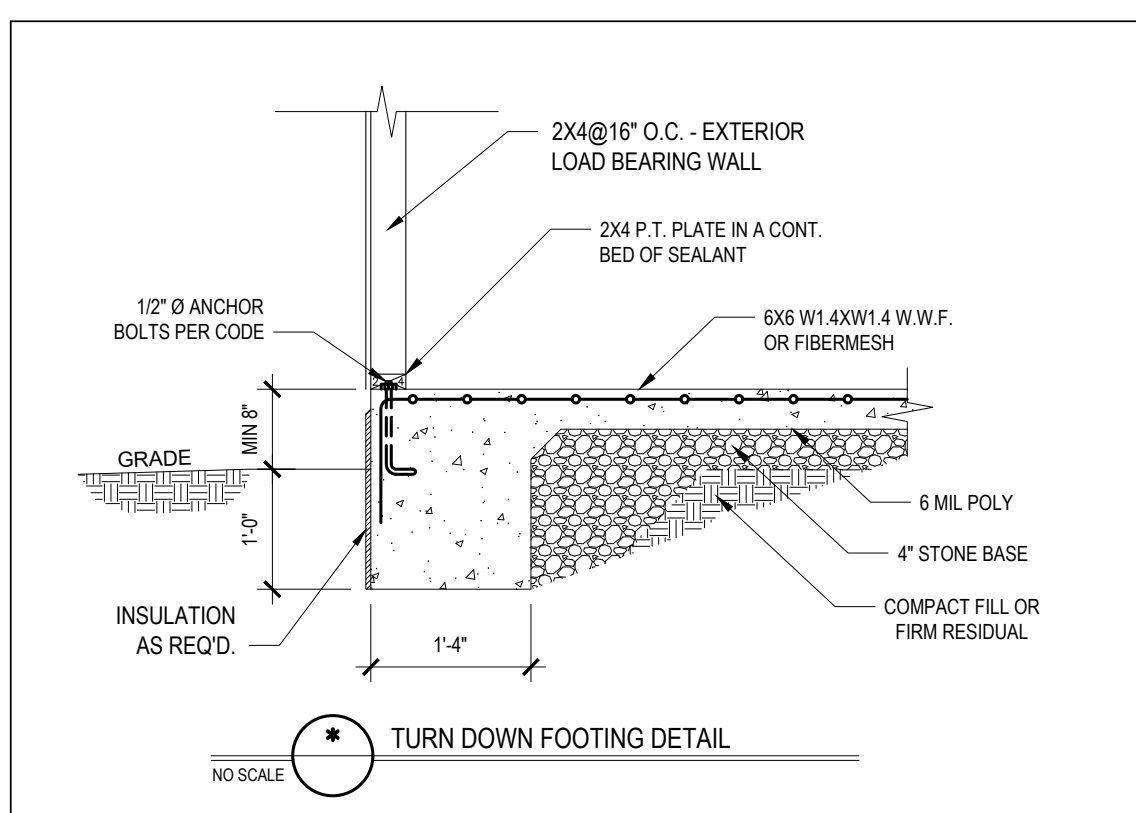
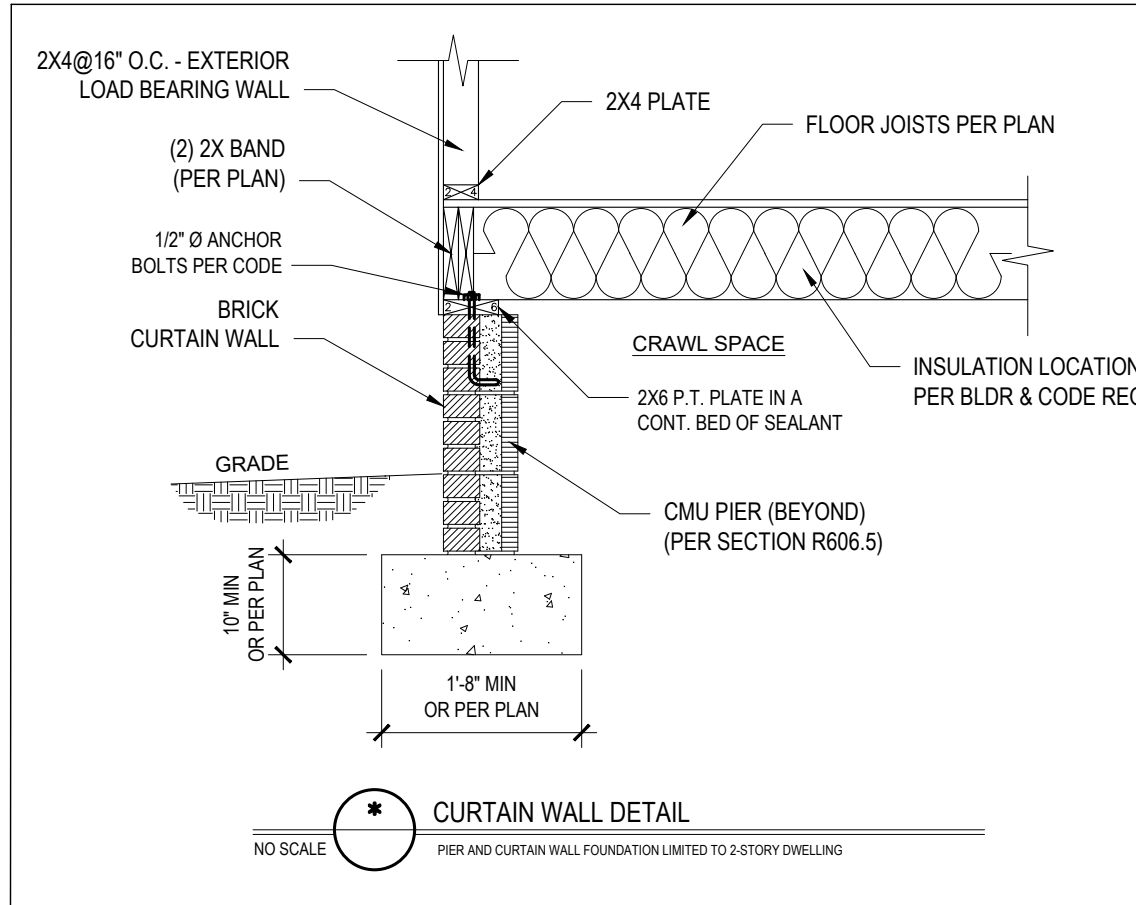
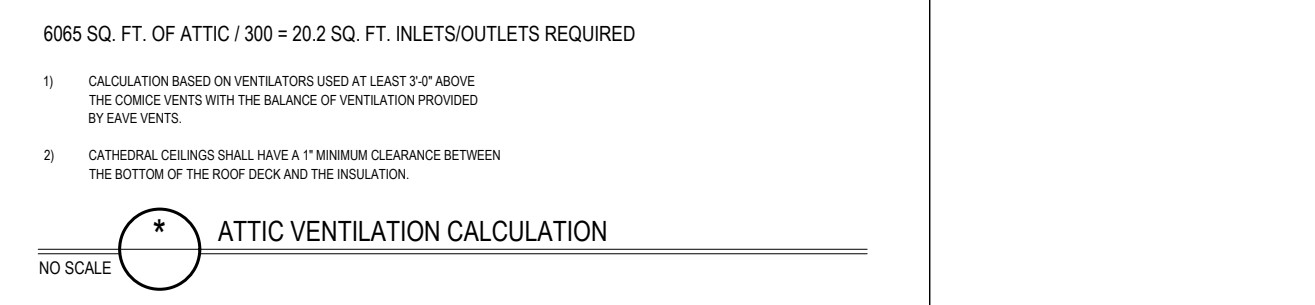
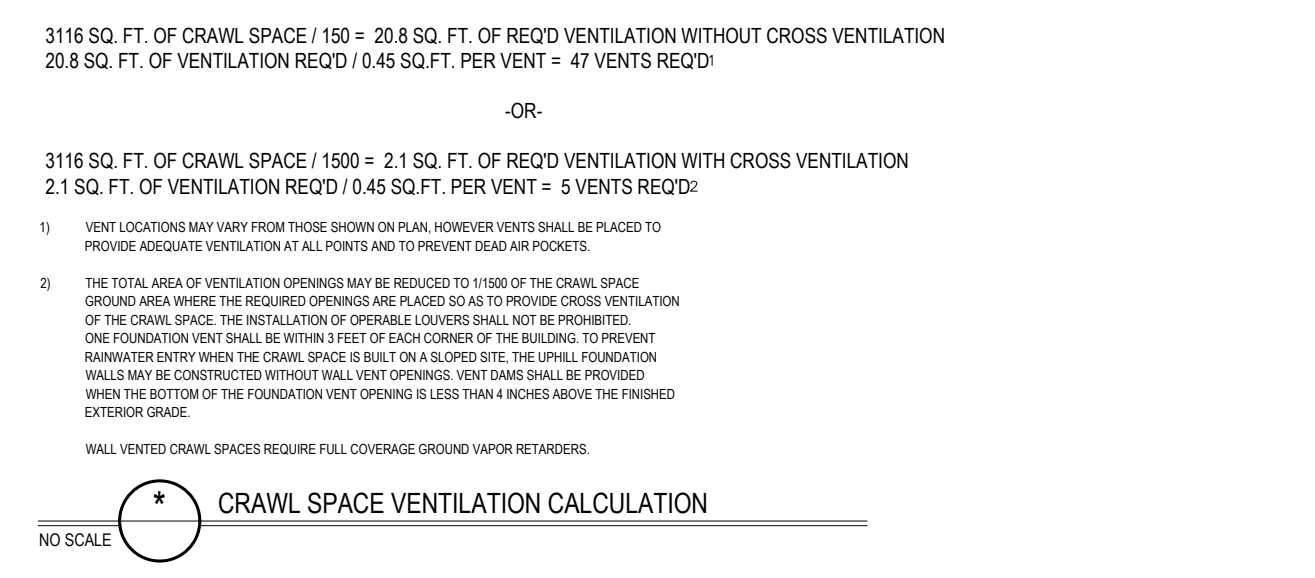


TABLE N1102.1 CLIMATE ZONES 3-5

CLIMATE ZONES	FENESTRATION U-FACTOR ^a	SKYLIGHT U-FACTOR ^b	GLAZED FENESTRATION SHGC ^{c,d,e}	CEILING R-VALUE ^f	WOOD FRAMED WALL R-VALUE ^g	MASS WALL R-VALUE ^h	FLOOR R-VALUE ⁱ	BASEMENT WALL R-VALUE ^j	SLAB R-VALUE AND DEPTH ^k	CRAWL SPACE WALL R-VALUE ^l
3	0.35	0.55	0.30	38 or 30 cont.	15 or 13 + 2.5	5/13 or 5/10 cont.	19	5/13	0	5/13
4	0.35	0.55	0.30	38 or 30 cont.	15 or 13 + 2.5	5/13 or 5/10 cont.	19	10/15	10	10/15
5	0.35	0.55	NR	38 or 30 cont.	19, or 13 + 5 or 15 + 3	13/17 or 13/12.5 cont.	30 ^g	10/15	10	10/19

- NO SCALE**
- * TABLE N1102.1 CLIMATE ZONES 3-5
- ^a R-VALUES ARE MINIMUM U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.
- ^b THE FENESTRATION U-FACTOR COLUMN EXCLUDES SKYLIGHTS. THE SQUARE-HEAT-GAIN COEFFICIENT (SHGC) COLUMN APPLIES TO ALL GLAZED FENESTRATION.
- ^c 100% MEANS R-5 CONTINUOUS INSULATION (INCLUDING ON THE INTERIOR OR EXTERIOR OF THE HOME OR IN A CAVITY) INSULATION AT THE INTERIOR OF THE BASEMENT WALL OR CRAWL SPACE WALL.
- ^d FOR MONOLITHIC SLAB INSULATION SHALL BE APPLIED FROM THE INSULATION GAP COMMAND TO THE BOTTOM OF THE FOOTING OR MINIMUM 2" BELOW SPACE INCHES IN SLAB. FOR CONCRETE SLAB INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 2" INCHES BELOW SLAB. R-4 SHALL BE ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS.
- ^e SEE LIST.
- ^f BASEMENT WALL INSULATION IS NOT REQUIRED IN WINDWARD LOCATIONS AS DEFINED BY FIGURE N1102.7 AND TABLE N1102.7.
- ^g OR INSULATION EQUIVALENT TO FILL THE FRAMING CAVITY. 10" MINIMUM.
- ^h THE FIRST VALUE IS CAVITY INSULATION. THE SECOND VALUE IS CONTINUOUS INSULATION. 10" 13" 15" MEANS R-13 CAVITY INSULATION PLUS R-13 INSULATED SHEATHING. 15" 13" 15" MEANS R-13 CAVITY INSULATION, PLUS R-13 INSULATED SHEATHING. STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR. INSULATION BRACING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. STRUCTURAL SHEATHING COVERS MORE THAN 25% PRESENT OF THE EXTERIOR SHALL BE SUBSTITUTED WITH INSULATION BRACING AT LEAST 16" x 16" MEANS R-13 CAVITY INSULATION PLUS R-13 SHEATHING.
- ⁱ FOR MASS WALLS THE SECOND R-VALUE APPLIES MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL.
- ^j IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MINIMUM OF 7" OF GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- ^k IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MINIMUM OF 7" OF GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- ^l R-4 SHALL BE ADDED TO THE REQUIRED INSULATION REQUIREMENT WHERE THE FULL HEIGHT OF AN INSULATED INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE LEVELS. OTHERWISE, INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION TABLE OR WITHIN 1/8" OF THE TOP EDGE OF THE BRICK.
- ^m TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF. THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BARREL.
- ⁿ IN 10' PERIODS OF TIME COMPRESSED AND NOTED IN A MINIMUM 1.5" FRAMING CAVITY IS DEEMED TO COMPLY. INSULATED BATTES SPACED AT 16" OR HIGHER COMPRESSED AND RE-INSTALLED IN 24" WALLS IS NOT PERMITTED TO COMPLY.
- ^o BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.



Engineers and designers shall not include construction means, methods, techniques, sequences, procedures or safety precautions. Any deviation or discrepancy on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering & Design, P.A. liability.

Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.

TYNDALL ENGINEERING & DESIGN P.A.
1000 W. 10th Street, Suite 100
Wilmington, NC 28403
919.778.3400 • 919.778.3444
www.tyndallengineering.com

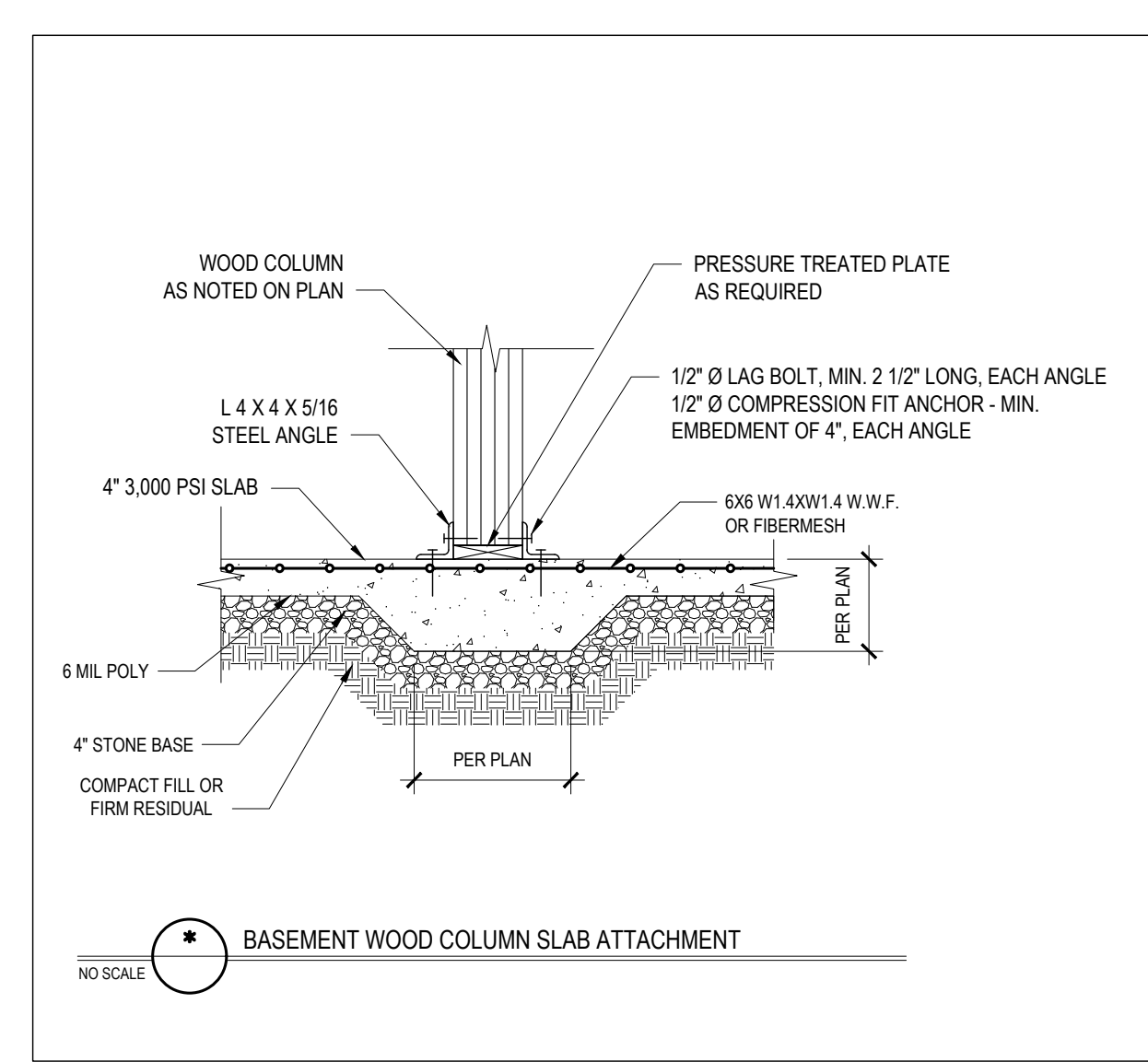
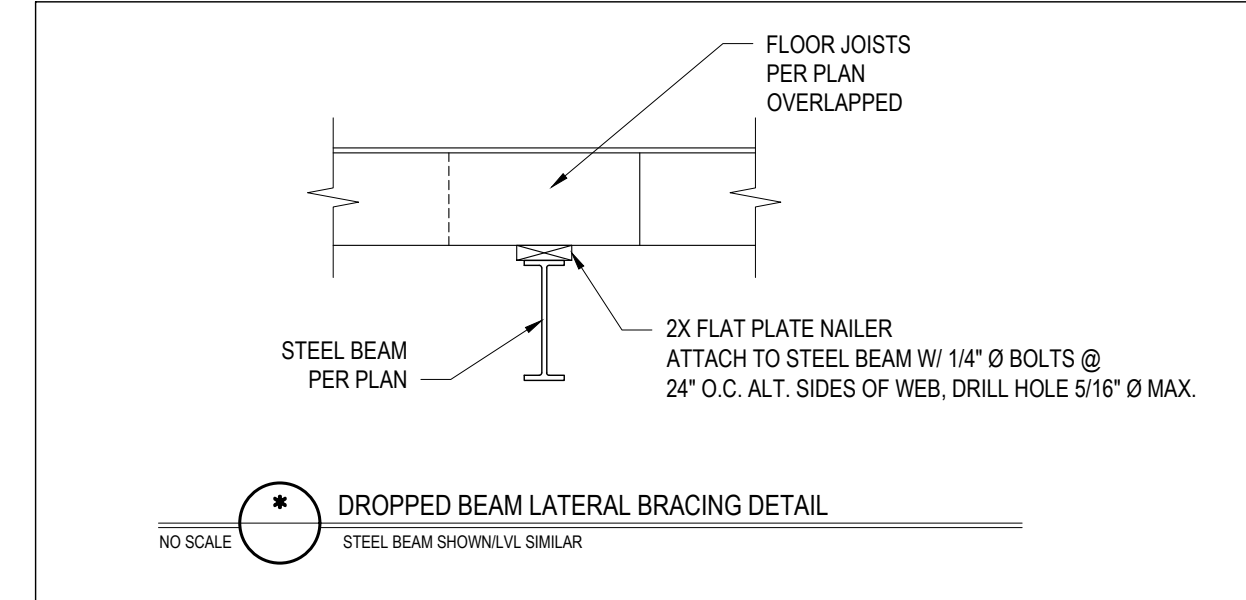
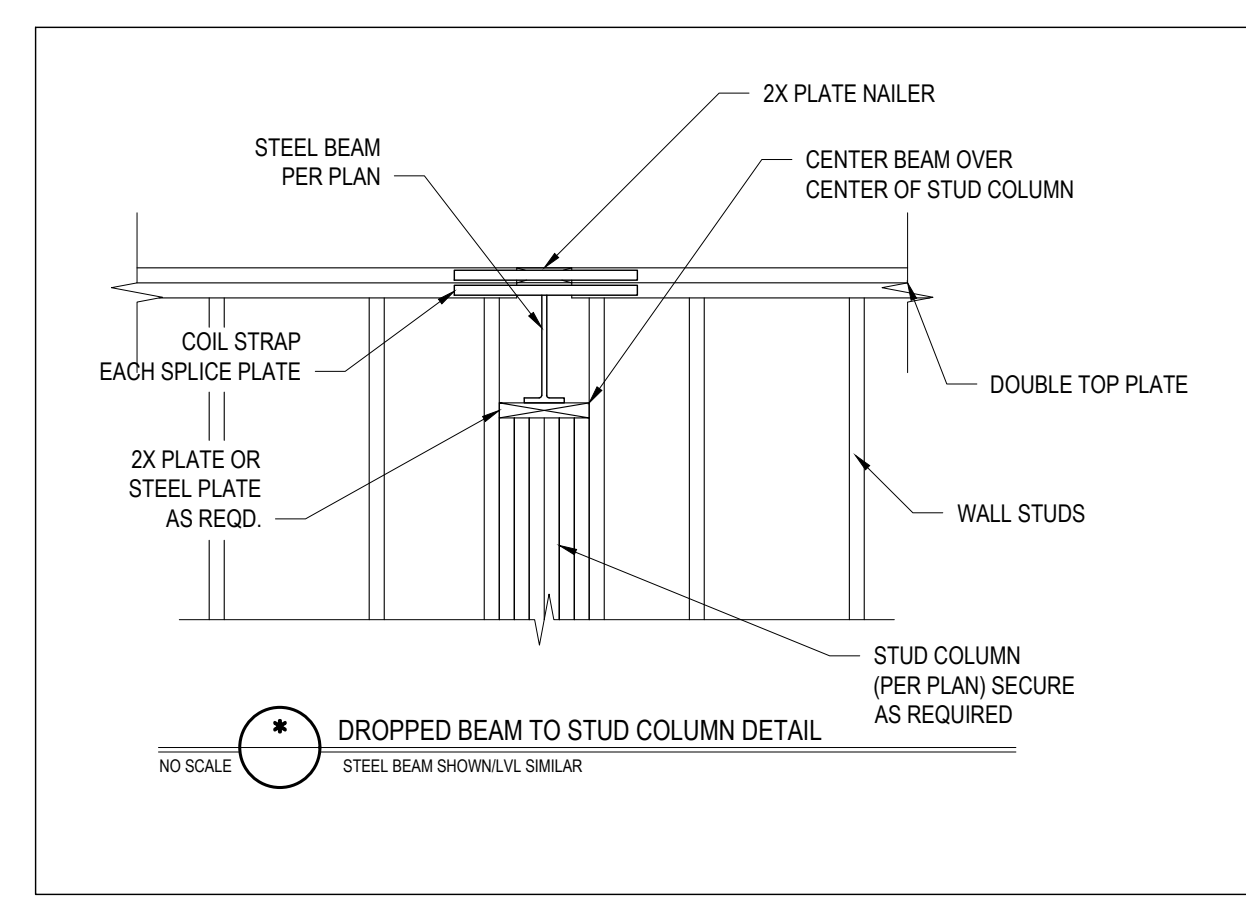
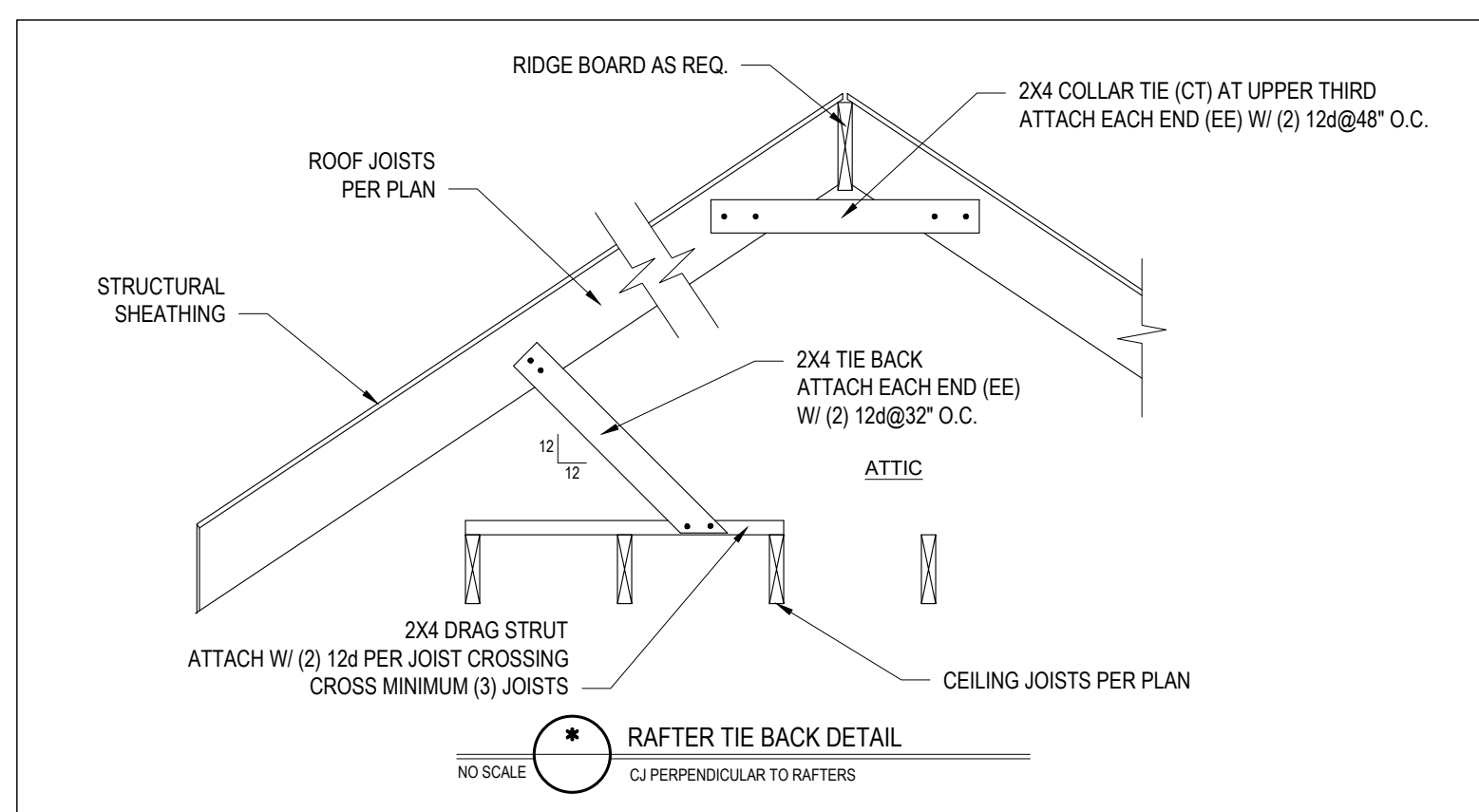
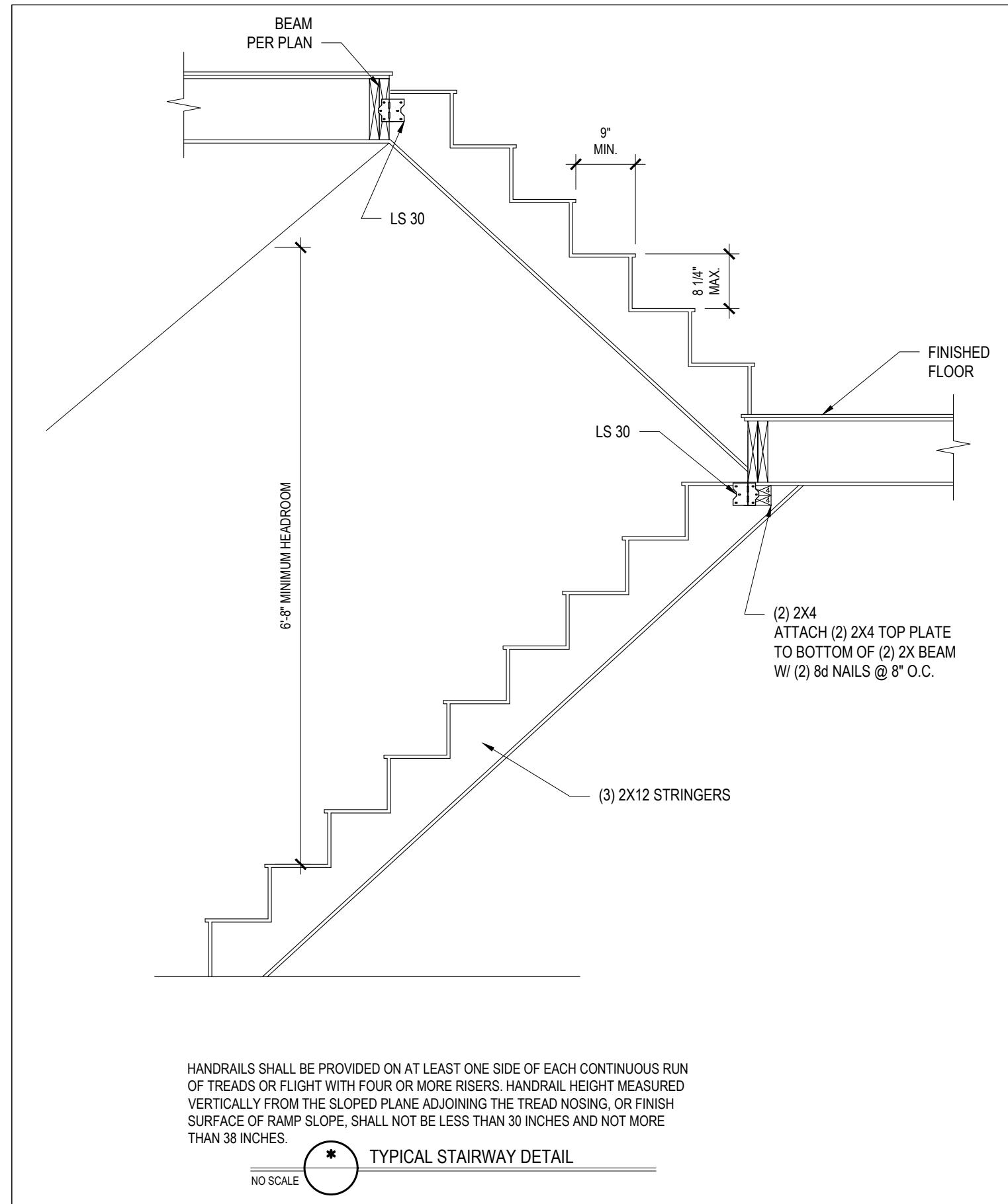
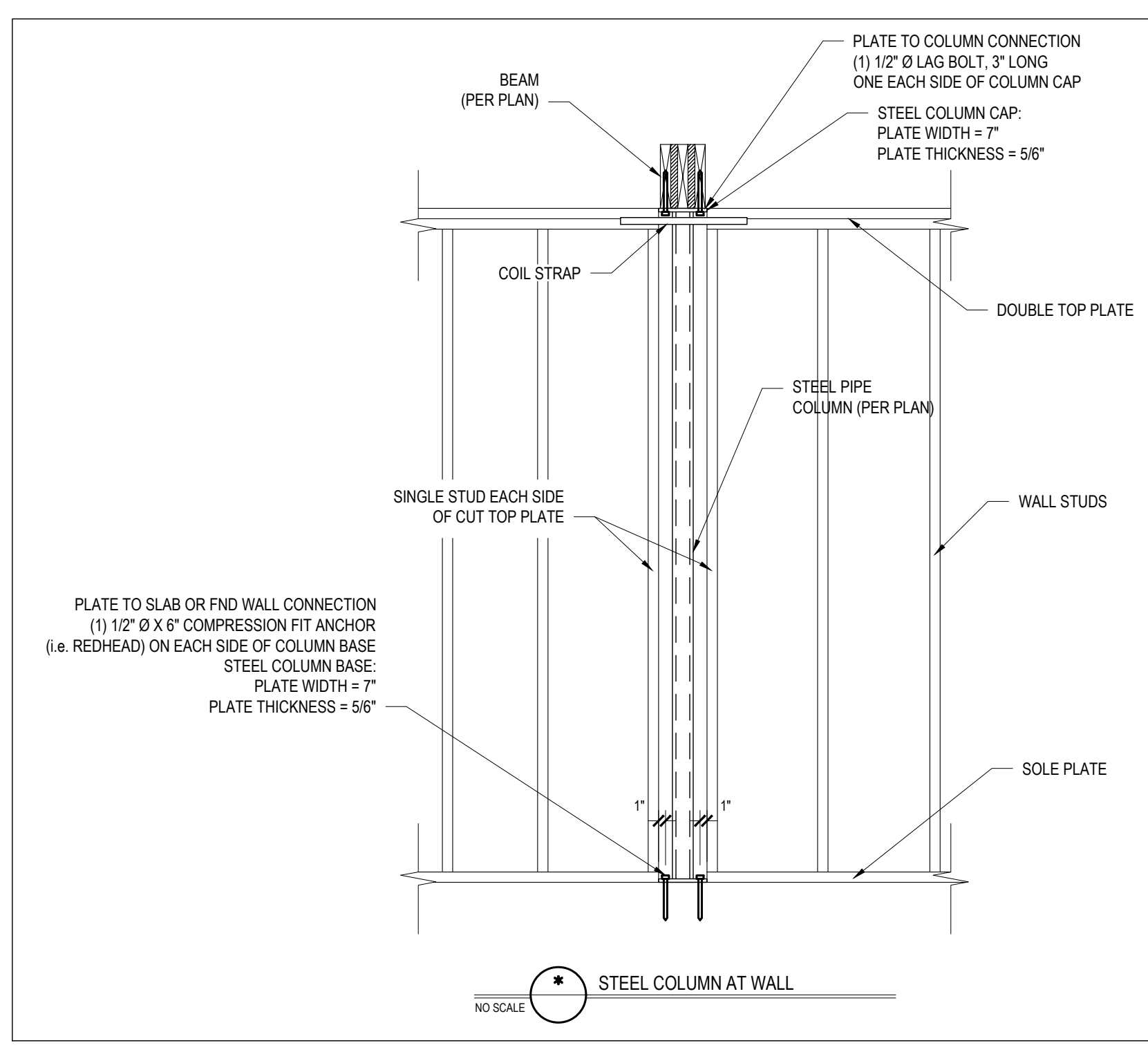
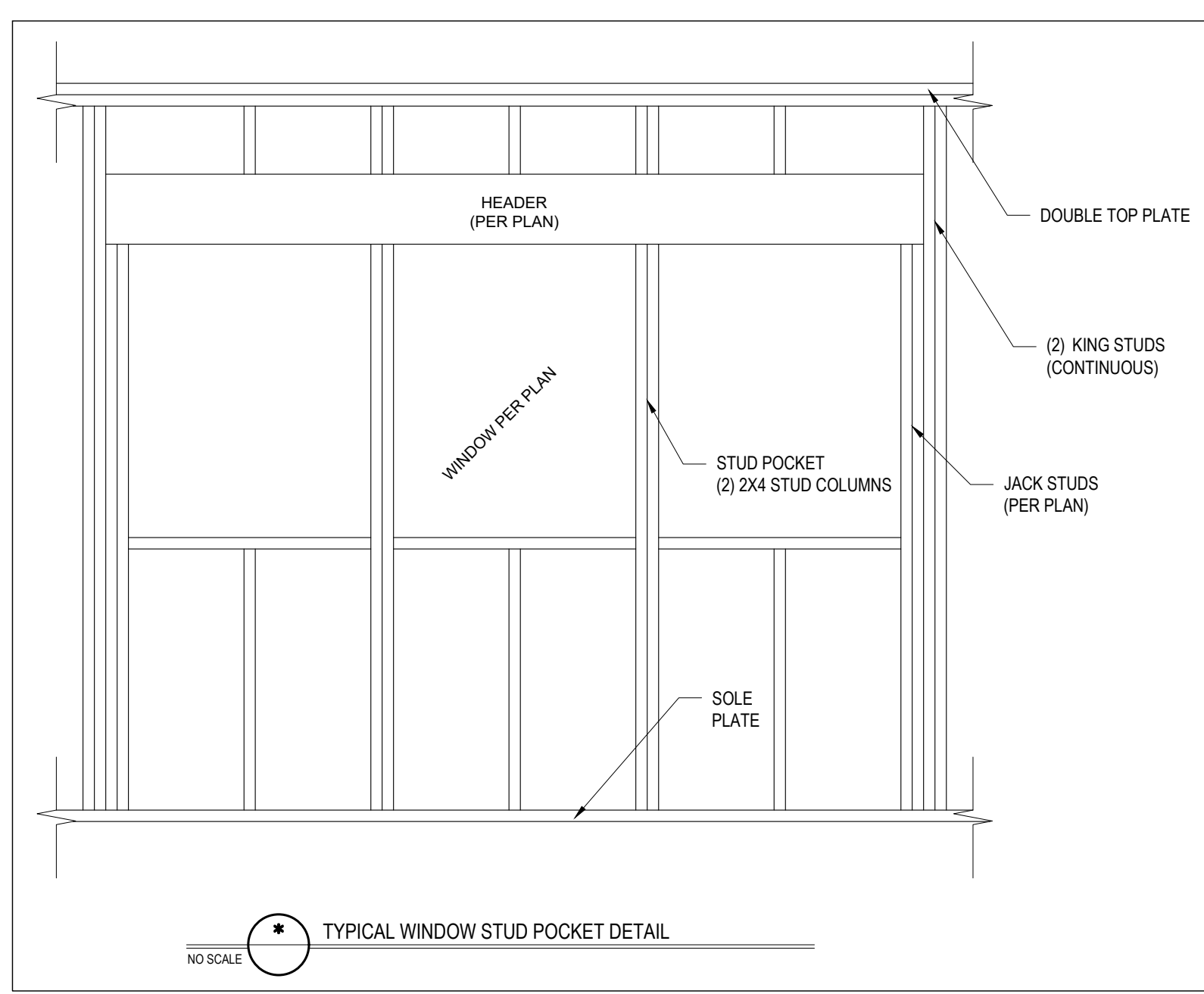
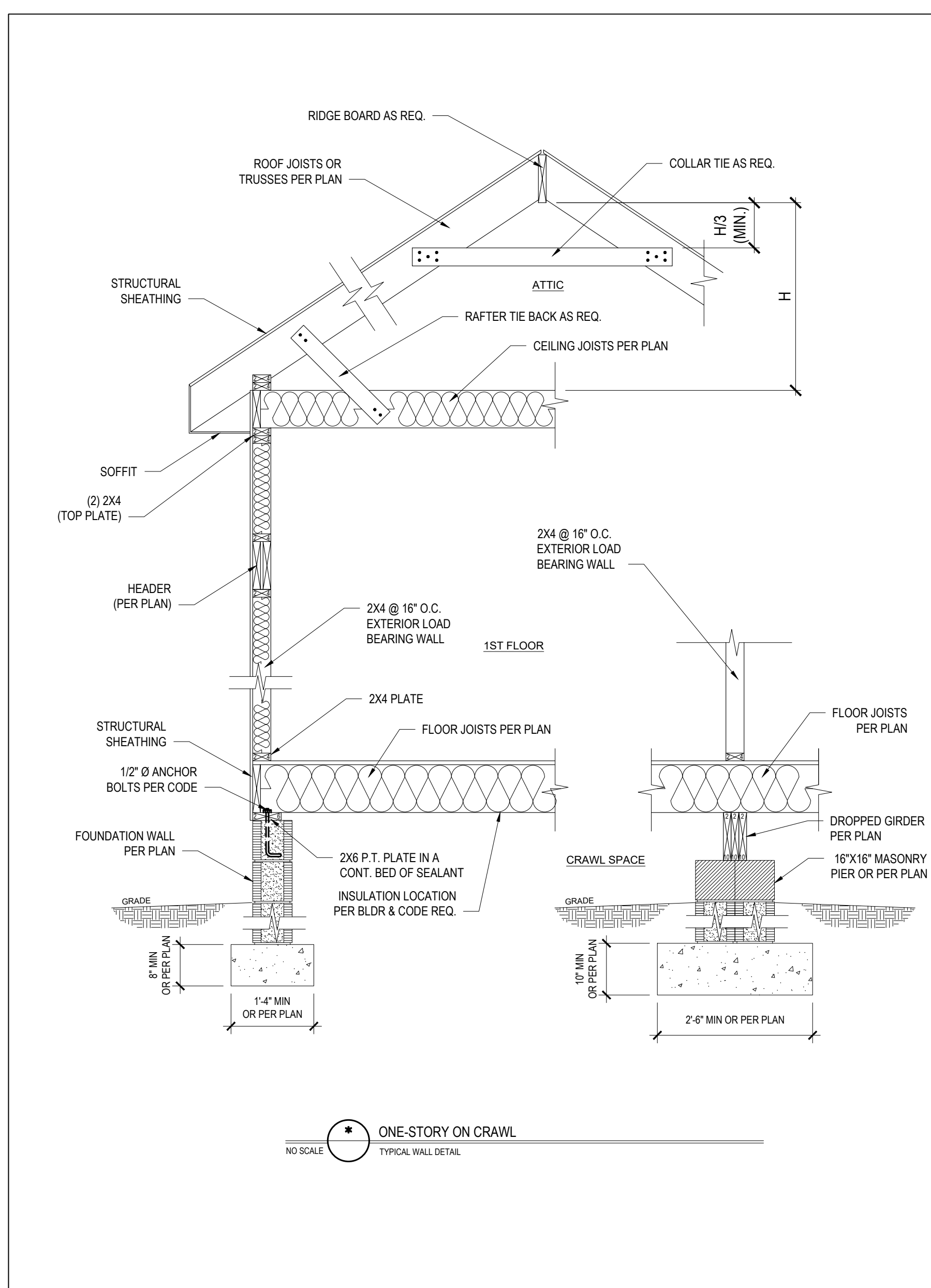
MITCH WOODWARD
WOODWARD RESIDENCE

STANDARD DETAILS

Project #: 2001-010503
Date: 11/11/2020
Drawn/Design By: PSE
DWG. Checked By: PAT
Scale: SEE PLAN

No.	Date	Remarks

Sheet Number **D1**
of 8



ALLOWABLE SPANS FOR LINTELS SUPPORTING MASONRY VENEER

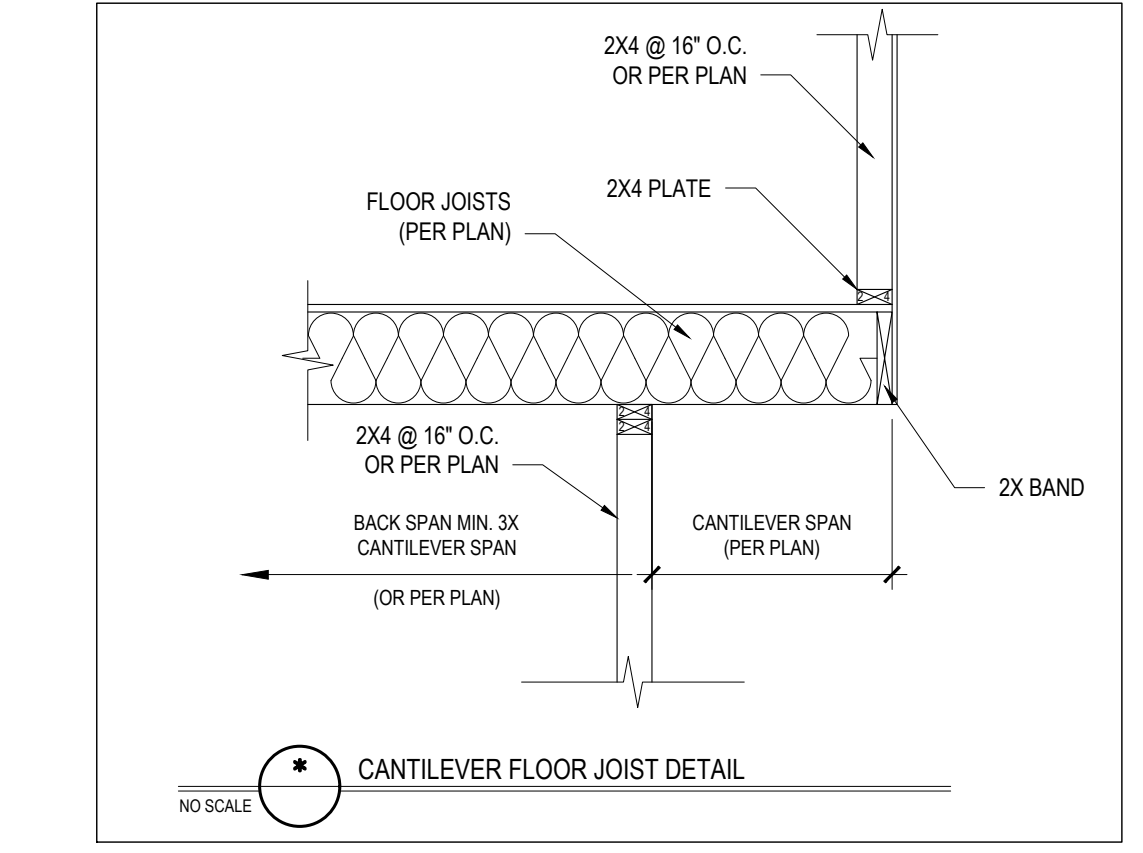
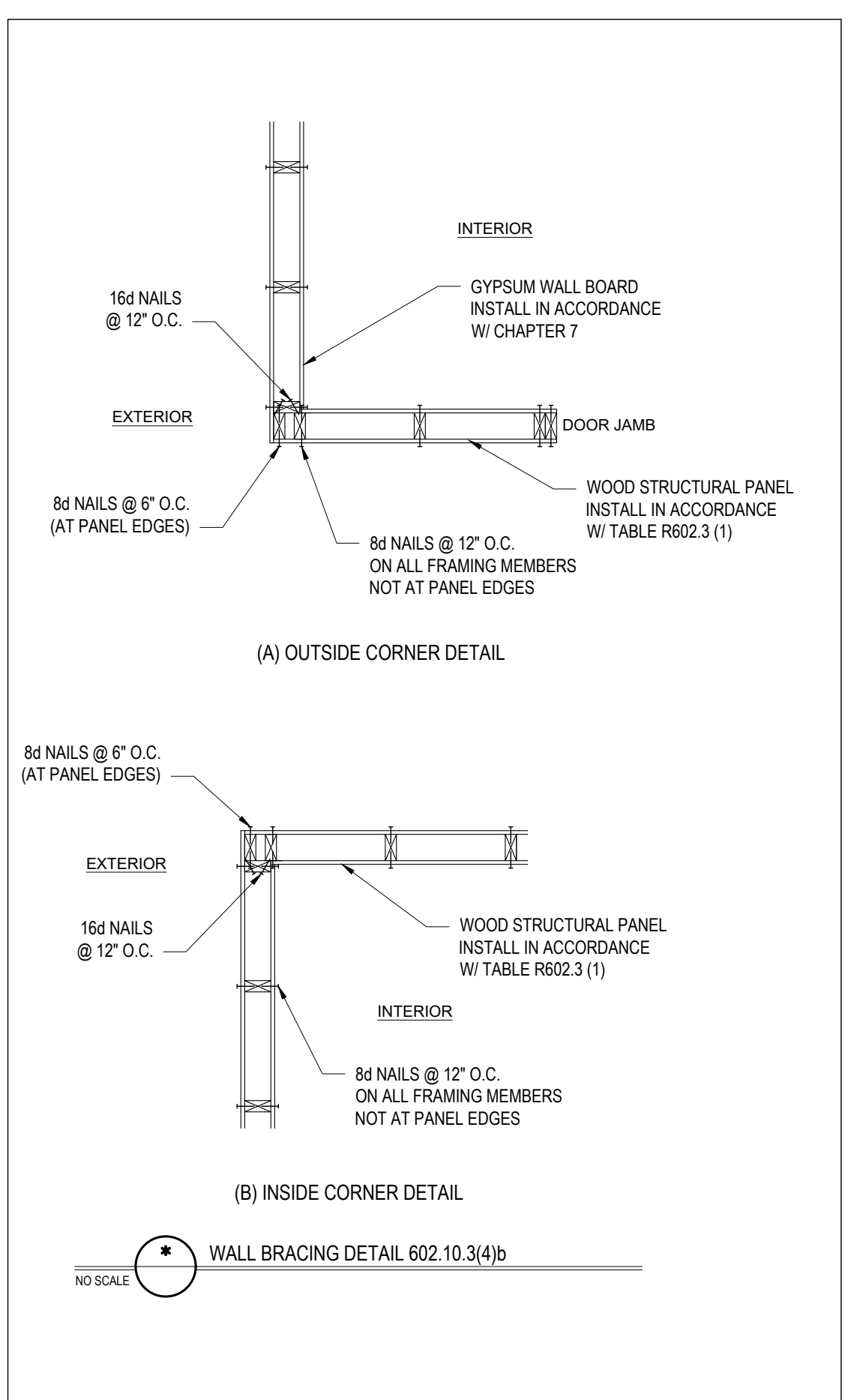
SIZE OF ANGLE (1,3)	NO STORY ABOVE (5)	1 STORY ABOVE (5)	2 STORIES ABOVE (5)	# OF 1/2" (OR EQUIV.) REINFORCING BARS IN REINFORCED LINTEL (2,4,5)
L 3 x 3 x 1/2	6'-0"	4'-6"	3'-0"	1
L 4 x 3 x 1/2	8'-0"	6'-0"	4'-6"	1
L 5 x 3 1/2 x 5/16	10'-0"	8'-0"	6'-0"	2
L 6 x 3 1/2 x 5/16	14'-0"	9'-6"	7'-0"	2
2L 5 x 3 1/2 x 5/16	20'-0"	12'-0"	9'-6"	4

1. LONG LEG OF THE ANGLE SHALL BE PLACED IN A VERTICAL POSITION.
 2. DEPTH OF REINFORCING LINTELS SHALL NOT BE LESS THAN 8" AND ALL CELLS OF HOLLOW MASONRY LINTELS SHALL BE GROUTED. REINFORCING BARS SHALL EXTEND NOT LESS THAN 8" INTO THE SUPPORT.
 3. STEEL MEMBERS INDICATED ARE ADEQUATE TYPICAL EXAMPLES; OTHER STEEL MEMBERS MEETING STRUCTURAL DESIGN REQUIREMENTS SHALL BE PERMITTED TO BE USED.
 4. EITHER STEEL ANGLE OR REINFORCED LINTEL SHALL SPAN OPENING.
 5. SPANS OVER 4'-0" SHALL BE SHORED UP UNTIL CURED.

MASONRY VENEER SUPPORT FIG 703.8.3.1

HARDWARE CROSS-REFERENCE CHART

SIMPSON STRONG-TIE PRODUCT NUMBER	USP STRUCTURAL CONNECTORS PRODUCT NUMBER
A35	MPA1
ABE	PAE
CBSQ	CBSQ
CCQ	KCCQ
CMSTC16	CMSTC16
CS	RS
H1	RT15
H2.5A	RT7A
H10	RT16
HDO8-SDS3	UPH08
HDU2-SDS2.5	PHD2
HDU5-SDS2.5	PHD5
HETA	HTA
HGAM10KTA	HGAM
HHO14-SDS2.5	UPHD14
HTS	HTW
HTT	HTT
HUS	HUS
LTA1	LPTA
LTHA26	HUC26
LTP4	MP4F
LUS	JUS
MAS	FA3
MSTAM	MSTAM
PC	PCM
PHD-SDS3	PHD
SSP	RSP7B
STC	TR1
STD	STD



Project #: 2001-010503
 Date: 11/11/2020
 Drawn/Design By: PSE
 DWG. Checked By: PAT
 Scale: SEE PLAN

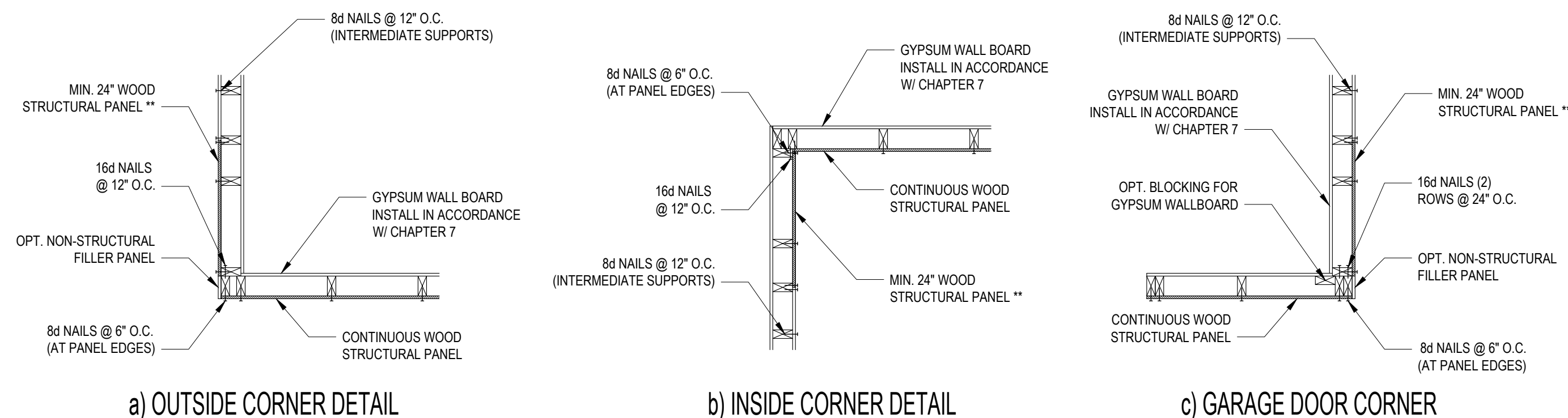
REVISIONS

No.	Date	Remarks

Client: MITCH WOODWARD
 Project: WOODWARD RESIDENCE

Sheet Number
D2
 of 8

FILENAME: Z:_RESIDENTIAL_EMS\2020_STRUCTURAL_PROJECTS\2001-010503 - WOODWARD RESIDENCE - HIGH SHEELS BUILDERS FROM PAKRICK\2001-010503\DWG 06: HOLLEY (LAST PLOT DATE): 11/12/2020 1:54 PM



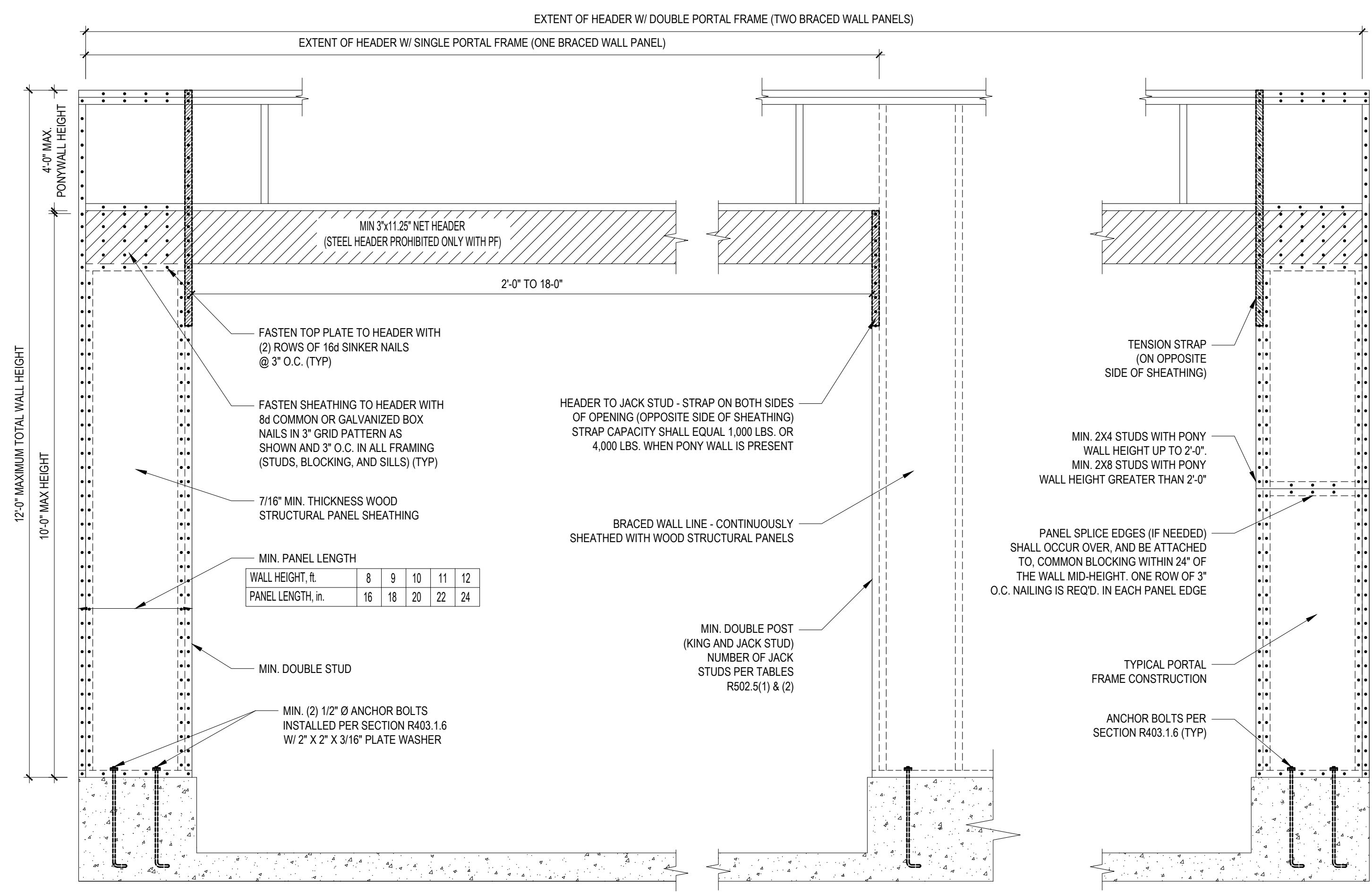
B1: TYPICAL EXTERIOR CORNER FRAMING FOR CONTINUOUS SHEATHING
NO SCALE

STRUCTURAL SHEATHING NOTES

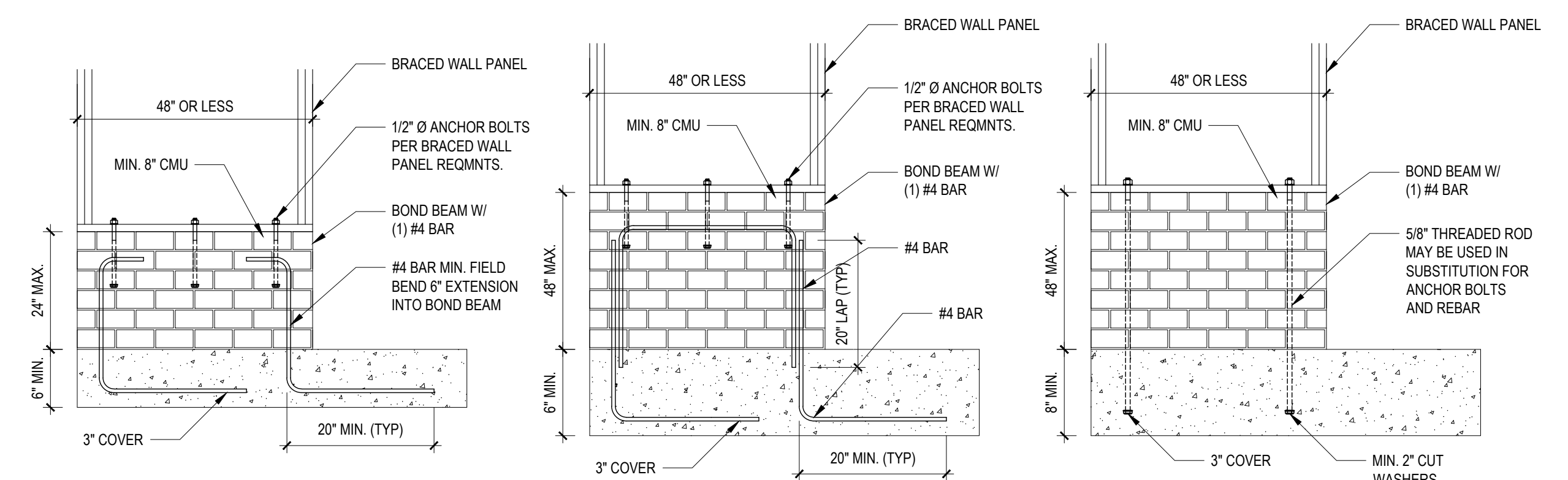
- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
- WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10.3 OF THE 2018 NRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- REFERENCE FIGURE R602.10.4.3 OF THE 2018 NRC.
- INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO).
- 12\"/>

REQUIRED BRACED WALL PANEL CONNECTIONS				
METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION	
			@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6\"/>	

B3: BRACE WALL PANEL CONNECTIONS
NO SCALE

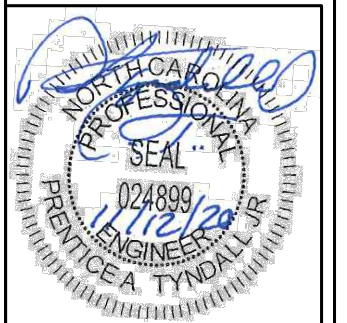


B2: METHOD CS-PF: CONTINUOUSLY SHEATHED PORTAL FRAME
FIGURE R602.10.1



B4: MASONRY STEM WALL SUPPORTING BRACED WALL PANELS
FIGURE R602.10.4.3 OF THE 2018 NRC
NOTE: GROUT BOND BEAMS AND ALL CELLS WHICH CONTAIN REBAR, THREADED RODS AND ANCHOR BOLTS

Engineers and architects do not include construction means, methods, techniques, sequences, procedures or safety precautions. Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyn dall Engineering & Design, P.A. Failure to do so will void Tyn dall Engineering & Design, P.A. liability. Please review these documents carefully. Tyn dall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.



TYNDALL
ENGINEERING & DESIGN, P.A.
1101 W. WINDY HILL DRIVE, SUITE 200
BOCA RATON, FLORIDA 33432
TEL: 561-733-0100 • FAX: 561-733-0444
WWW.TYNDALLENGINEERING.COM

Client: MITCH WOODWARD
Project: WOODWARD RESIDENCE

SHEATHING DETAILS

Project #: 2001-010503
Date: 11/11/2020
Drawn/Design By: PSE
DWG. Checked By: PAT
Scale: SEE PLAN

REVISIONS		
No.	Date	Remarks

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