

Trenco

818 Soundside Rd
Edenton, NC 27932

Re: LGI_Homes
Alamance

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E10611241 thru E10611252

My license renewal date for the state of North Carolina is December 31, 2017.

North Carolina COA: C-0844

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.



June 5, 2017

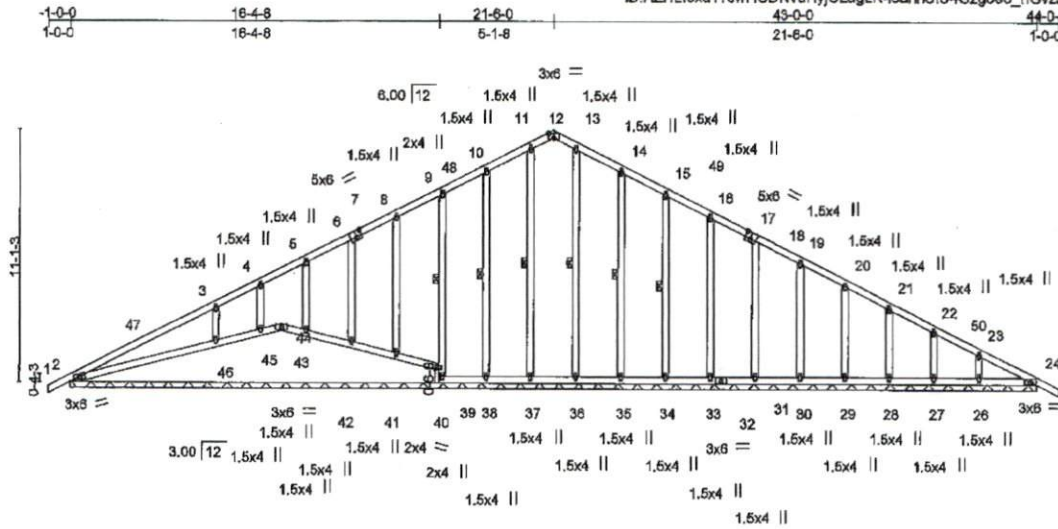
Strzyzewski, Marvin

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to Trenco. Any project specific information included is for Trenco's customer's file reference purpose only, and was not taken into account in the preparation of these designs. Trenco has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of the design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

| | | | | | | |
|-----------|-------|------------|-----|-----|----------|-----------|
| Job | Truss | Truss Type | Qty | Ply | Alumance | E10611241 |
| LGL_HOMES | A1E | GABLE | 1 | 1 | | |

Builders FirstSource, Albemarle, NC 28001

7.640 s Apr 18 2016 Mitek Industries, Inc. Mon Jun 05 13:01:28 2017 Page 1
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| | | |
|-------|--------|--------|
| 9-5-2 | 19-4-8 | 43-0-0 |
| 9-5-2 | 6-11-6 | 26-7-8 |

| | | | | | | | | | | |
|---------------|-----------|----------------------|-------|----------|------|----------|--------|-----|----------------|----------|
| LOADING (psf) | | SPACING- | 2-0-0 | CSL | DEFL | in (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL (roof) | 20.0 | Plate Grip DOL | 1.15 | TC | 0.52 | -0.00 | 24 | n/r | MT20 | 244/190 |
| Snow (Pf/Pg) | 15.4/20.0 | Lumber DOL | 1.15 | BC | 0.35 | Vert(TL) | 0.00 | 24 | n/r | 120 |
| TCDL | 10.0 | Rep Stress Incr | YES | WB | 0.12 | Horz(TL) | 0.01 | 24 | n/a | n/a |
| BCLL | 0.0 | Code IRC2009/TPI2007 | | (Matrix) | | | | | | |
| BCDL | 10.0 | | | | | | | | Weight: 269 lb | FT = 20% |

| | |
|-----------------------|---------------------------------------------------------------------------|
| LUMBER- | BRACING- |
| TOP CHORD 2x4 SP No.2 | TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. |
| BOT CHORD 2x4 SP No.2 | BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except: |
| OTHERS 2x4 SP No.3 | 6-0-0 oc bracing: 45-46. |

| | |
|------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| REACTIONS. (lb/size) | 2=278/43-0-0, 44=63/43-0-0, 40=133/43-0-0, 39=19/43-0-0, 37=153/43-0-0, 38=160/43-0-0, 41=167/43-0-0, 42=162/43-0-0, 43=187/43-0-0, 45=-110/43-0-0, 46=578/43-0-0, 36=153/43-0-0, 35=161/43-0-0, 34=160/43-0-0, 33=160/43-0-0, 31=160/43-0-0, 30=160/43-0-0, 29=159/43-0-0, 28=162/43-0-0, 27=150/43-0-0, 26=191/43-0-0, 24=163/43-0-0 |
| Max Horz 2=-119(LC 11) | |
| Max Uplift 2=-21(LC 11), 40=-55(LC 11), 38=-42(LC 10), 41=-8(LC 10), 42=-18(LC 10), 43=-33(LC 10), 45=-119(LC 5), 46=60(LC 10), 35=39(LC 11), 34=-16(LC 11), 33=-19(LC 11), 31=-18(LC 11), 30=-18(LC 11), 29=-18(LC 11), 28=-18(LC 11), 27=-21(LC 11), 26=-12(LC 11) | |
| Max Grav 2=278(LC 1), 44=91(LC 5), 40=146(LC 3), 39=35(LC 5), 37=187(LC 3), 38=210(LC 3), 41=167(LC 1), 42=162(LC 15), 43=187(LC 1), 46=578(LC 15), 36=186(LC 4), 35=211(LC 4), 34=173(LC 4), 33=160(LC 16), 31=160(LC 1), 30=160(LC 16), 29=159(LC 1), 28=162(LC 1), 27=150(LC 16), 26=191(LC 16), 24=163(LC 1) | |

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|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| FORCES. (lb) - Maximum Compression/Maximum Tension |
| TOP CHORD 1-2=0/22, 2-47=161/32, 3-47=133/90, 3-4=-97/75, 4-5=-53/85, 5-6=-44/106, 6-7=-40/114, 7-8=-36/124, 8-9=-41/155, 9-48=-48/179, 10-48=-14/189, 10-11=-51/232, 11-12=43/205, 12-13=-43/207, 13-14=-51/239, 14-49=-14/195, 15-49=-48/185, 15-16=-41/183, 16-17=-36/129, 17-18=-41/119, 18-19=-41/96, 19-20=-41/62, 20-21=-41/29, 21-22=-51/18, 22-50=-74/15, 23-50=-84/0, 23-24=-124/25, 24-25=0/25 |
| BOT CHORD 2-46=16/169, 45-46=30/149, 44-45=0/153, 43-44=-1/151, 42-43=-6/153, 41-42=-5/152, 40-41=-4/158, 39-40=0/0, 9-40=-133/55, 38-39=0/144, 37-38=0/144, 36-37=0/144, 35-36=0/144, 34-35=0/144, 33-34=0/144, 32-33=0/144, 31-32=0/144, 30-31=0/144, 29-30=0/144, 28-29=0/144, 27-28=0/144, 26-27=0/144, 24-26=0/144 |
| WEBS 11-37=-147/0, 10-38=-169/106, 8-41=-122/53, 6-42=-112/47, 5-43=-157/68, 4-45=-23/36, 3-46=-402/183, 13-36=-146/0, 14-35=-171/07, 15-34=-133/53, 16-33=-120/51, 18-31=-120/51, 19-30=-120/51, 20-29=-120/51, 21-28=-121/51, 22-27=-114/50, 23-26=-139/82 |

- NOTES-
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) 1-0-0 to 3-3-10, Interior(1) 3-3-10 to 21-6-0, Exterior(2) 21-6-0 to 25-9-10 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 4) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.



Continued on page 2

WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY
TRENCO
 A Mitek Affiliate

818 Soundside Road
 Edenonton, NC 27832

| Job | Truss Type | Qty | Alt | Job Reference (optional) |
|-----------|------------|-----|-----|--------------------------|
| LOHOMES | AIE | 1 | | |
| | | | | Altman |
| E10611241 | | | | |

Builders FirstSource, Albemarle, NC 29001
 7,640 3 Apr 19 2016 Mitek Industries, Inc. Mon Jun 05 13:01:28 2017 Page 2
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- NOTES-**
- 7) Gable requires continuous bottom chord bearing.
 - 8) Gable studs spaced at 2'-0" oc.
 - 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
 - 11) Bearing at joint(s) 40 considers parallel to grain value using ANSITRP1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 40, 8 lb uplift at joint 41, 18 lb uplift at joint 42, 33 lb uplift at joint 43, 119 lb uplift at joint 45 and 60 lb uplift at joint 46.
 - 13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at joint(s) 2, 39, 38, 35, 34, 33, 31, 30, 29, 28, 27, and 26. This connection is for uplift only and does not consider lateral forces.
 - 14) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 44, 41, 42, 43, 45, 46.
 - 15) *Semi-rigid pitchbreaks including heeels* Member end fixity model was used in the analysis and design of this truss.

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 Design valid for use only with MITEK connectors. This design is based on design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITRP1 Quality Criteria, DBB-89 and BCSI Building Component safety information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

TRENCO
 ENGINEERING BY
 A MITEK AFFILIATE
 818 Soundside Road
 Edenon, NC 27932

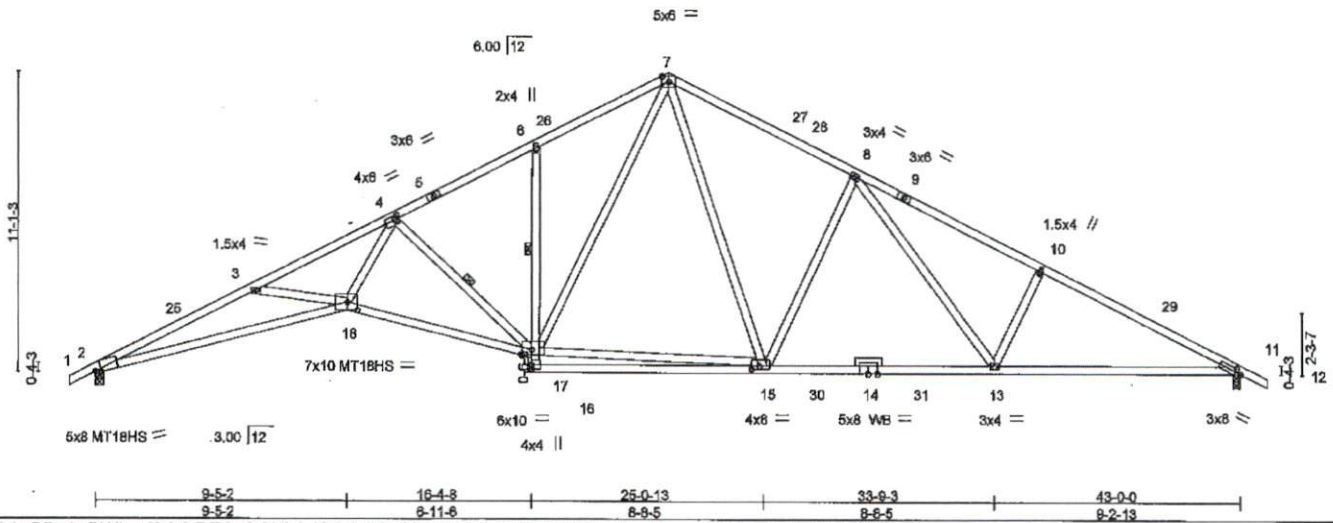
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|-----------|-------|--------------|-----|-----|-----------|-----------|
| Job | Truss | Truss Type | Qty | Ply | Allowance | E10811242 |
| LGI_HOMES | A2 | Roof Special | 5 | 1 | | |

Builders FirstSource, Albemarle, NC 28001

7.840 s Apr 19 2016 MTEK Industries, Inc. Mon Jun 05 13:01:29 2017 Page 1
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Scale = 1:78.1



| | |
|----------------------|-----------------------------------------------------------------------------------------------------------|
| Plate Offsets (X,Y)= | [2:0-2-7,Edge], [4:0-1-12,0-1-12], [11:0-2-9,0-1-8], [15:0-3-6,0-1-8], [17:0-4-4,0-2-8], [18:0-4-8,0-3-8] |
|----------------------|-----------------------------------------------------------------------------------------------------------|

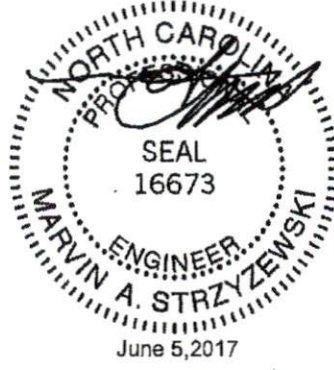
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|---------------|-----------|----------------------|-------|------------|------|----------|----------|--------|------|----------------|----------|
| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | | DEFL. | in (loc) | I/defl | L/d | PLATES | GRIP |
| TCLL (roof) | 20.0 | Plate Grip DOL | 1.15 | TC | 0.86 | Vert(LL) | -0.52 | 13-15 | >995 | MT20 | 244/190 |
| Snow (Pf/Pg) | 15.4/20.0 | Lumber DOL | 1.15 | BC | 0.84 | Vert(TL) | -1.16 | 17-18 | >446 | MT18HS | 244/190 |
| TCDL | 10.0 | Rep Stress Incr | YES | WB | 0.91 | Horz(TL) | 0.52 | 11 | n/a | | |
| BCLL | 0.0 | Code IRC2009/TP12007 | | (Matrix-S) | | Wind(LL) | 0.26 | 17-18 | >999 | | |
| BCDL | 10.0 | | | | | | | | | Weight: 250 lb | FT = 20% |

| | | | |
|-----------|-------------------------------------------------------------|-----------|----------------------------------------------------------------|
| LUMBER- | | BRACING- | |
| TOP CHORD | 2x4 SP No.2 *Except* 1-5,9-12: 2x4 SP No.1 | TOP CHORD | Structural wood sheathing directly applied. |
| BOT CHORD | 2x4 SP No.1 *Except* 2-18: 2x4 SP DSS, 6-16: 2x4 SP No.3 | BOT CHORD | Rigid ceiling directly applied. Except: 1 Row at midpt 6-17 |
| WEBS | 2x4 SP No.3 *Except* 4-18: 2x4 SP No.2 | WEBS | 1 Row at midpt 4-17 |
| OTHERS | 2x4 SP No.2 | | |

REACTIONS. (lb/size) 2=1825/0-3-8, 11=1892/0-3-8
 Max Horz 2=120(LC 10)
 Max Uplift 2=19(LC 10), 11=19(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/27, 2-25=-6226/563, 3-25=-6127/606, 3-4=-5835/487, 4-5=-2698/331, 5-6=-2626/356, 6-26=-2668/413, 7-26=-2548/438, 7-27=-2364/385, 27-28=-2393/362, 8-28=-2465/351, 8-9=-3071/389, 9-10=-3242/369, 10-29=-3318/347, 11-29=-3419/324, 11-12=0/27
 BOT CHORD 2-18=488/5661, 17-18=-224/3991, 16-17=0/164, 8-17=-347/159, 15-16=0/288, 15-30=-113/2415, 14-30=-113/2415, 14-31=-113/2415, 13-31=-113/2415, 11-13=-199/2968
 WEBS 3-18=343/185, 4-18=-109/2826, 4-17=-2094/206, 15-17=-20/1553, 7-17=-174/1144, 7-16=-86/921, 8-16=-733/209, 6-13=-54/702, 10-13=-354/162

- NOTES-
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph; TC DL=6.0psf; BC DL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 3-3-10, Interior(1) 3-3-10 to 21-6-0, Exterior(2) 21-6-0 to 25-9-10 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-05; P=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15; Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow); Lumber DOL=1.15 Plate DOL=1.15; Category II; Exp B; Partially Exp.; Ct=1.1
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Bearing at joint(s) 2 considers parallel to grain value using ANSITP1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Continued on page 2.
WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE MM-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MTEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-09 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.
 ENGINEERING BY **TRENCO** A MITEK Affiliates
 818 Soundside Road Edenton, NC 27832

| Job | Truss | Truss Type | Qty | Ply | Allowance | |
|-----------|-------|--------------|-----|-----|-----------|-----------|
| LGL_HOMES | A2 | Roof Special | 5 | 1 | | E10611242 |

Builders FirstSource, Albemarle, NC 28001

7.840 s Apr 19 2016 MITEK Industries, Inc. Mon Jun 05 13:01:29 2017 Page 2
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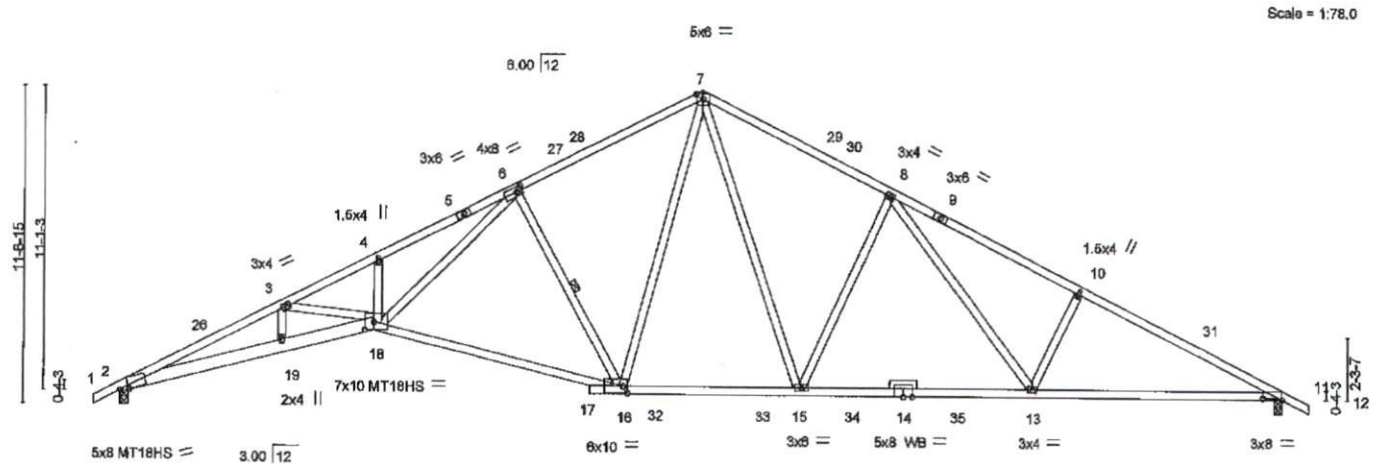
Job Reference (optional)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MS-7473 rev. 10/03/2016 BEFORE USE.
 Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIP/TPI Quality Criteria, DSB-89 and BCS Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY
TRENCO
 A MITEK Affiliate

818 Soundside Road
 Edenton, NC 27832

| | | | | | | |
|----------------------------------------------------------------------------|-------------|----------------------------|----------|----------|----------|---------------------------------------------------------------------------|
| Job LGL_HOMES | Truss A3 | Truss Type Roof Special | Qty 1 | Ply 1 | Alamance | E10611243 |
| Builders FirstSource, Albemarle, NC 28001 | | | | | | Job Reference (optional) |
| 7.840 s Nov 10 2016 Mitek Industries, Inc. Mon Jun 06 13:11:42 2017 Page 1 | | | | | | ID:AZHLI0xa1?swHCDNvuHyjOzagLR-lymzjdURNyo_vKY1Qb1UowwloquSbuZJTvnoNz8Kyf |
| 1-0-0 | 8-0-2 | 9-5-2 | 11-2-5 | 14-7-5 | 21-8-0 | 28-5-13 |
| 1-0-0 | 6-0-2 | 3-5-0 | 1-9-3 | 3-5-0 | 6-10-11 | 6-11-13 |
| | | | | | | 35-5-11 |
| | | | | | | 6-11-13 |
| | | | | | | 43-0-0 |
| | | | | | | 44-0-0 |
| | | | | | | 1-0-0 |



| | |
|---------------------|--------------------------------------------------------------------------------------------|
| Plate Offsets (X,Y) | [2:0-3-16,0-0-10], [6:0-2-8,0-2-0], [11:0-8-4,0-0-10], [17:0-1-12,0-3-0], [18:0-4-0,0-3-8] |
|---------------------|--------------------------------------------------------------------------------------------|

| | | | | | |
|------------------------|----------------------|-------------|-------------------------------|----------------|-------------|
| LOADING (psf) | SPACING- | CSI. | DEFL. | PLATES | GRIP |
| TCLL (roof) 20.0 | 2-0-0 | TC 0.39 | in (loc) l/defl L/d | MT20 | 244/190 |
| Snow (Pf/Pg) 15.4/20.0 | Plate Grip DOL 1.15 | BC 0.67 | Vert(LL) -0.42 17-18 >999 360 | MT18HS | 244/190 |
| TCDL 10.0 | Lumber DOL 1.15 | WB 0.91 | Vert(TL) -1.12 17-18 >459 240 | | |
| BCLL 0.0 | Rep Stress Incr YES | (Matrix-S) | Horz(TL) 0.45 11 n/a n/a | | |
| BCDL 10.0 | Code IRC2009/TPI2007 | | Wind(LL) 0.21 17-18 >999 240 | | |
| | | | | Weight: 247 lb | FT = 20% |

LUMBER-
TOP CHORD 2x4 SP DSS
BOT CHORD 2x4 SP DSS *Except*
2-18: 2x6 SP DSS, 16-18: 2x4 SP No.1
WEBS 2x4 SP No.3 *Except*
6-18: 2x4 SP No.2
OTHERS 2x4 SP No.2

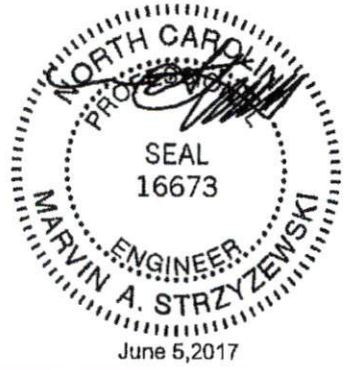
BRACING-
TOP CHORD Sheathed.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 6-16

REACTIONS. (lb/size) 2=1918/0-3-8 (min. 0-1-14), 11=1959/0-3-8 (min. 0-2-0)
Max Horz 2=-120(LC 11)
Max Uplift 2=-20(LC 10), 11=-20(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/27, 2-3=-6271/535, 3-4=-8012/465, 4-6=-6016/540, 6-7=-2481/373, 7-8=-2616/384, 8-10=-3389/385, 10-11=-3567/343, 11-12=0/27
BOT CHORD 2-19=-394/5691, 18-19=-385/5722, 17-18=-120/2931, 16-17=-125/2892, 15-16=0/1917, 13-15=-111/2551, 11-13=-196/3101
WEBS 6-16=-1450/230, 7-16=-84/844, 7-15=-104/1063, 8-15=-729/204, 8-13=-49/693, 10-13=-354/164, 6-18=-206/3532, 4-16=-207/96, 3-19=-72/27, 3-18=-259/127

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=8.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 3-3-10, Interior(1) 3-3-10 to 21-6-0, Exterior(2) 21-6-0 to 25-9-10 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Bearing at joint(s) 2 considers parallel to grain value using ANSITPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at j(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S)



Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 rev. 10/03/2016 BEFORE USE.

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ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

| Job | Truss | Truss Type | Qty | Ply | Allowance | |
|-------------------------------------------|-------|--------------|-----|-----|-----------|--------------------------|
| LGL_HOMES | AS | Roof Special | 1 | 1 | | E10611243 |
| Builders FirstSource, Albemarle, NC 28601 | | | | | | Job Reference (optional) |

7.840 s Nov 10 2015 Mitek Industries, Inc. Mon Jun 05 13:11:42 2017 Page 2
 ID:AZHLIOxa1?swHCDNvuHyjOzagLR-tyzmJdU6RNYo_vKY1Qb1UowwloquSbuZJTvNoNz9Ky

LOAD CASE(S)

- 1) Dead + Roof Live (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-7=60, 7-12=60, 18-20=20, 17-18=20, 16-17=20, 16-32=20, 32-33=60, 33-34=20, 34-35=60, 23-35=20
- 2) Dead + Snow (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-7=51, 7-12=51, 18-20=20, 17-18=20, 16-17=20, 16-32=20, 32-33=60, 33-34=20, 34-35=60, 23-35=20
- 3) Dead + Snow (Unbal. Left) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-27=51, 7-27=82, 7-12=29, 18-20=20, 17-18=20, 16-17=20, 16-32=20, 32-33=60, 33-34=20, 34-35=60, 23-35=20
- 4) Dead + Snow (Unbal. Right) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-7=29, 7-30=82, 12-30=51, 18-20=20, 17-18=20, 16-17=20, 16-32=20, 32-33=60, 33-34=20, 34-35=60, 23-35=20
- 5) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-7=20, 7-12=20, 18-20=40, 17-18=40, 16-17=40, 16-23=40
- 6) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=55, 2-28=30, 7-28=18, 7-29=30, 11-29=18, 11-12=12, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=67, 2-26=42, 7-26=30, 7-29=42, 11-29=30, 11-12=24
- 7) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=12, 2-28=18, 7-28=30, 7-31=18, 11-31=30, 11-12=55, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=24, 2-28=30, 7-28=42, 7-31=30, 11-31=42, 11-12=67
- 8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=12, 2-7=3, 7-11=7, 11-12=2, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=24, 2-7=9, 7-11=19, 11-12=14
- 9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=2, 2-7=7, 7-11=3, 11-12=12, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=14, 2-7=19, 7-11=9, 11-12=24
- 10) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=30, 2-7=15, 7-11=5, 11-12=1, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=42, 2-7=27, 7-11=17, 11-12=11
- 11) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=1, 2-7=5, 7-11=15, 11-12=30, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=11, 2-7=17, 7-11=27, 11-12=42
- 12) Dead + 0.6 MWFRS Wind (Pos. Internal) 3rd Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=30, 2-7=15, 7-11=5, 11-12=1, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=42, 2-7=27, 7-11=17, 11-12=11
- 13) Dead + 0.6 MWFRS Wind (Pos. Internal) 4th Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-2=1, 2-7=5, 7-11=15, 11-12=30, 18-20=12, 17-18=12, 16-17=12, 16-23=12
 Horz: 1-2=11, 2-7=17, 7-11=27, 11-12=42
- 14) Dead + Snow on Overhangs: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-2=51, 2-7=20, 7-11=20, 11-12=51, 18-20=20, 17-18=20, 16-17=20, 16-23=20
- 15) 1st Dead + Roof Live (unbalanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-7=60, 7-12=20, 18-20=20, 17-18=20, 16-17=20, 16-32=20, 32-33=60, 33-34=20, 34-35=60, 23-35=20
- 16) 2nd Dead + Roof Live (unbalanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-7=20, 7-12=60, 18-20=20, 17-18=20, 16-17=20, 16-32=20, 32-33=60, 33-34=20, 34-35=60, 23-35=20

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2016 BEFORE USE.
 Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road
 Edenton, NC 27932

| | | | | | | |
|-------------------------------------------|-------|--------------|-----|-----|-----------|--------------------------|
| Job | Truss | Truss Type | Qty | Ply | Allowance | E10611244 |
| LGL_HOMES | A4 | Roof Special | 3 | 1 | | |
| Builders FirstSource, Albemarle, NC 28001 | | | | | | Job Reference (optional) |

7.640 x Apr 19 2016 Mitek Industries, Inc. Mon Jun 06 13:01:30 2017 Page 1
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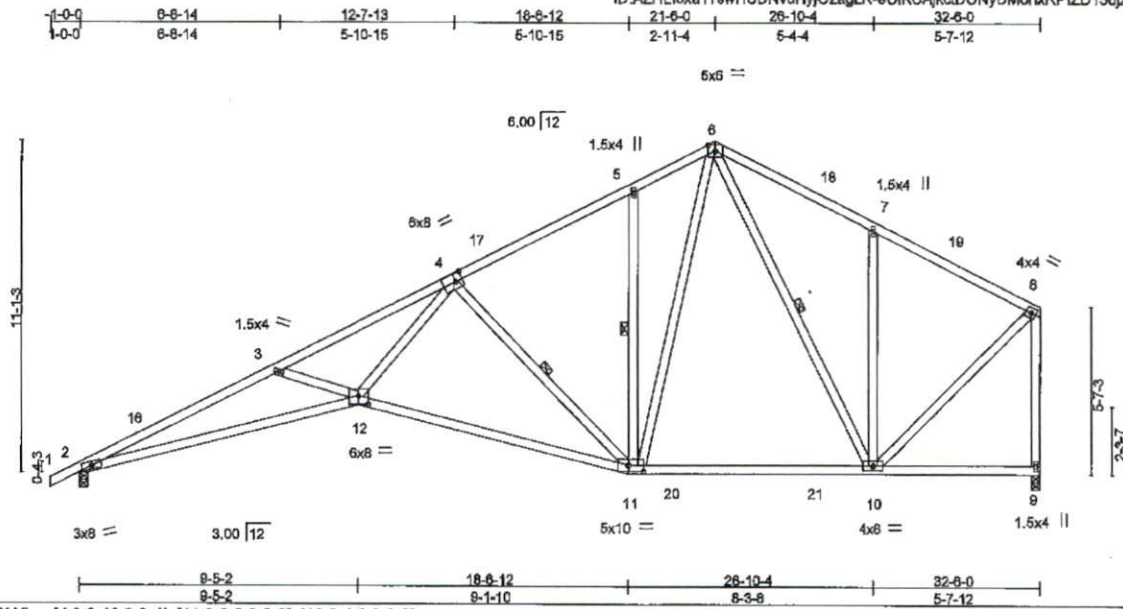


Plate Offsets (X,Y) - [4,0-2-12,0-3-4], [11,0-6-0,0-2-8], [12,0-4-0,0-3-8]

| | | | | | |
|------------------------|----------------------|-------------|-------------------------------|----------------|-------------|
| LOADING (psf) | SPACING- | CSI. | DEFL. | PLATES | GRIP |
| TCLL (roof) 20.0 | 2-0-0 | TC 0.92 | In (loc) l/def l/d | MT20 | 244/190 |
| Snow (Pf/Pg) 15.4/20.0 | Plate Grip DOL 1.15 | BC 0.92 | Vert(LL) -0.40 10-11 >970 360 | | |
| TCDL 10.0 | Lumber DOL 1.15 | WB 0.97 | Vert(TL) -0.92 11-12 >424 240 | | |
| BCLL 0.0 * | Rep Stress Incr YES | (Matrix-S) | Horz(TL) 0.37 9 n/a n/a | | |
| BCDL 10.0 | Code IRC2009/TPI2007 | | Wind(LL) 0.19 11-12 >999 240 | | |
| | | | | Weight: 207 lb | FT = 20% |

LUMBER-

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2 *Except*
 2-12: 2x4 SP No.1
 WEBS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied.
 WEBS 1 Row at midpt 4-11, 5-11, 6-10

REACTIONS. (lb/size) 2=1420/0-3-8, 9=1423/0-3-8
 Max Horz 2=215(LC 10)
 Max Uplift 2=28(LC 10)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/27, 2-16=-4564/488, 3-16=-4476/515, 3-4=-4147/429, 4-17=-1475/222, 5-17=-1312/224, 5-8=-1431/289,
 6-18=-969/259, 7-18=-1050/233, 7-19=-945/152, 8-19=-1032/135, 8-9=-1410/194
 BOT CHORD 2-12=-578/4145, 11-12=-289/2284, 11-20=-64/962, 20-21=-64/962, 10-21=-64/962, 9-10=-6/10
 WEBS 3-12=-389/174, 4-12=-195/2332, 4-11=-1411/241, 5-11=-276/144, 6-11=-156/1087, 6-10=-297/26, 7-10=-408/178,
 8-10=-92/1197

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 2-3-0, Interior(1) 2-3-0 to 21-8-0, Exterior(2) 21-8-0 to 24-9-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at J(s) 2. This connection is for uplift only and does not consider lateral forces.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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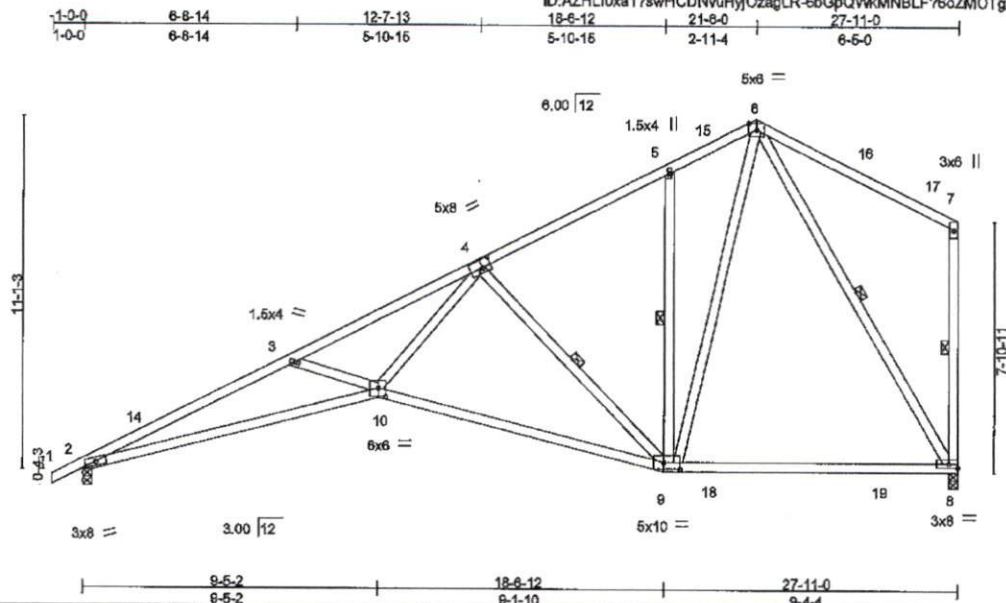
ENGINEERING BY
TRENCO
 A Mitek Affiliate

818 Soundside Road
 Edenton, NC 27932

| | | | | | | |
|-----------|-------|--------------|-----|-----|----------|-----------|
| Job | Truss | Truss Type | Qty | Ply | Alamance | E10611245 |
| LGI_HOMES | A5 | Roof Special | 3 | 1 | | |

Builder: FirstSource, Albemarle, NC 28001

Job Reference (optional)
 7.840 s Apr 15 2016 MITek Industries, Inc. Mon Jun 05 13:01:31 2017 Page 1
 ID:AZ:HLlOxa17swHCDNvUHyjOzagLR-6bGpQWkMNBf76oZMOTgxW8TRVCPrilFdpv_zrHz9L6l



Scale = 1:87.3

Plate Offsets (X,Y)-- [4:0-3-4,0-3-0], [9:0-6-0,0-2-8], [10:0-2-12,0-3-4]

| | | | | | | | | | | | |
|----------------------|-----------|----------------------|-------|------------|------|--------------|----------|--------|------|----------------|-------------|
| LOADING (psf) | | SPACING- | 2-0-0 | CSL | | DEFL. | in (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL (roof) | 20.0 | Plate Grip DOL | 1.15 | TC | 0.62 | Vert(LL) | -0.44 | 8-9 | >758 | MT20 | 244/190 |
| Snow (Pf/Pg) | 15.4/20.0 | Lumber DOL | 1.15 | BC | 0.99 | Vert(TL) | -0.79 | 8-9 | >423 | | |
| TCDL | 10.0 | Rep Stress Incr | YES | WB | 0.80 | Horz(TL) | 0.31 | 8 | n/a | | |
| BCLL | 0.0 | Code IRC2009/TPI2007 | | (Matrix-S) | | WInd(LL) | 0.18 | 10 | >999 | | |
| BCDL | 10.0 | | | | | | | | | Weight: 174 lb | FT = 20% |

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2 *Except*
 8-9: 2x4 SP No.1
 WEBS 2x4 SP No.3 *Except*
 7-8: 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied.
 WEBS 1 Row at midpt 4-9, 5-9, 7-8, 8-8

REACTIONS. (lb/size) 2=1216/0-3-8, 8=1283/0-3-8
 Max Horz 2=253(LC 10)
 Max Uplift 2=20(LC 10), 8=19(LC 10)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/27, 2-14=3730/411, 3-14=3650/439, 3-4=3296/352, 4-5=-1028/141, 5-15=-975/193, 6-15=-907/207, 6-16=-43/84, 16-17=48/65, 7-17=146/57, 7-8=241/111
 BOT CHORD 2-10=-575/3384, 9-10=-284/1747, 9-18=-68/557, 18-19=-68/557, 8-19=-68/557
 WEBS 3-10=410/174, 4-10=-197/1940, 4-9=-1233/243, 5-9=-274/116, 6-9=-116/1125, 6-8=-1030/133

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCCL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 21-6-0, Exterior(2) 21-6-0 to 24-6-0 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-05; Pr=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15; Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow); Lumber DOL=1.15 Plate DOL=1.15; Category II; Exp B; Partially Exp.; Ct=1.1
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 10.0psf.
 - Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at J(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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ENGINEERING BY
TRENCO
 A MITek Alliance
 818 Soundside Road
 Edenton, NC 27932

| | | | | | | |
|-----------|-------|--------------|-----|-----|----------|-----------|
| Job | Truss | Truss Type | Qty | Ply | Alumance | E10811246 |
| LGI_HOMES | A6 | Roof Special | 7 | 1 | | |

Builders FirstSource, Albemarle, NC 28001

7.640 # Apr 19 2016 MITEK Industries, Inc. Mon Jun 05 13:01:31 2017 Page 1

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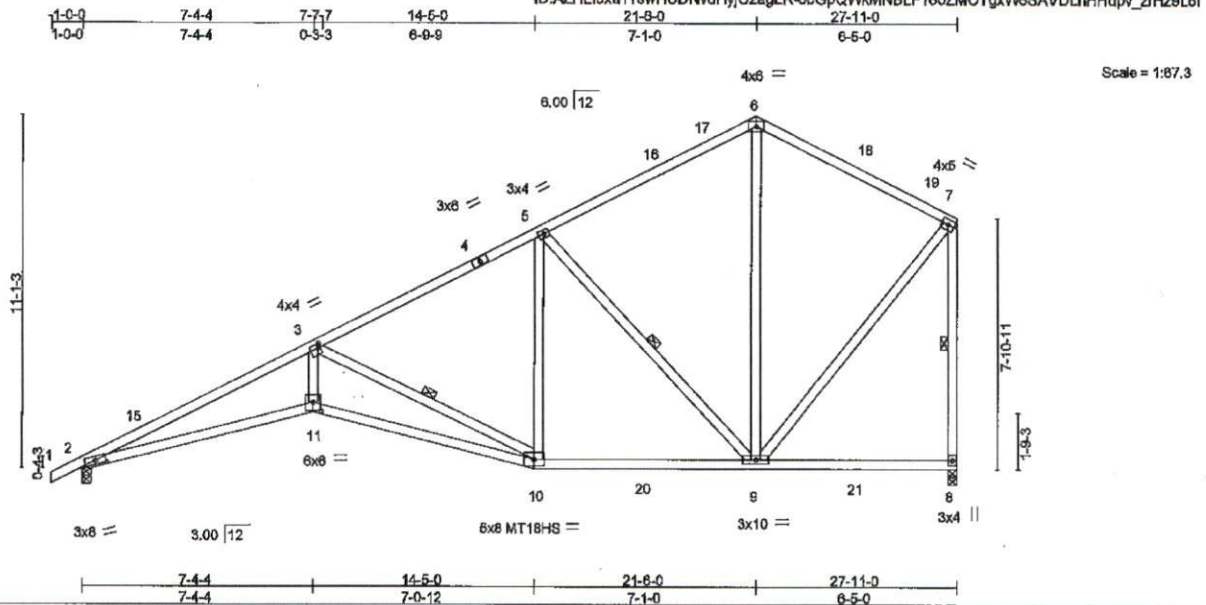


Plate Offsets (X,Y)- [2:0-4-0,0-1-9], [3:0-1-0,0-1-8], [11:0-3-0,0-3-4]

| LOADING (psf) | SPACING- | CSL | DEFL. | PLATES | GRIP |
|------------------------|----------------------|------------|-------------------------------|----------------|----------|
| TCLL (roof) 20.0 | 2-0-0 | TC 0.70 | in (loc) l/defl L/d | MT20 | 244/190 |
| Snow (Pf/Pg) 15.4/20.0 | Plate Grip DOL 1.15 | BC 0.96 | Vert(TL) -0.27 11 >999 360 | MT18HS | 244/190 |
| TGDL 10.0 | Lumber DOL 1.15 | WB 0.87 | Vert(TL) -0.87 10-11 >500 240 | | |
| BCLL 0.0 * | Rep Stress Incr YES | (Matrix-S) | Horz(TL) 0.32 8 n/a n/a | | |
| BCDL 10.0 | Code IRC2009/TPI2007 | | Wind(L.L) 0.18 11 >999 240 | | |
| | | | | Weight: 171 lb | FT = 20% |

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

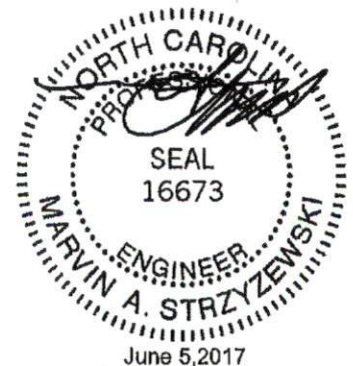
BRACING-
 TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied.
 WEBS 1 Row at midpt 3-10, 5-9, 7-8

REACTIONS. (lb/size) 2=1242/0-3-8, 8=1303/0-3-8
 Max Horz 2=253(LC 10)
 Max Uplift 2=20(LC 10), 8=19(LC 10)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/27, 2-15=3816/359, 3-15=3725/391, 3-4=1465/117, 4-5=1291/137, 5-16=758/126, 16-17=652/129,
 6-17=590/145, 6-18=646/149, 18-19=856/130, 7-19=734/122, 7-8=1162/206
 BOT CHORD 2-11=525/3443, 10-11=525/3444, 10-20=169/1222, 9-20=169/1222, 9-21=4/23, 8-21=4/23
 WEBS 3-11=169/1772, 3-10=2371/375, 5-10=0/527, 5-9=834/157, 6-9=0/272, 7-9=89/897

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TGDL=8.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 21-6-0, Exterior(2) 21-6-0 to 24-6-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2016 BEFORE USE.

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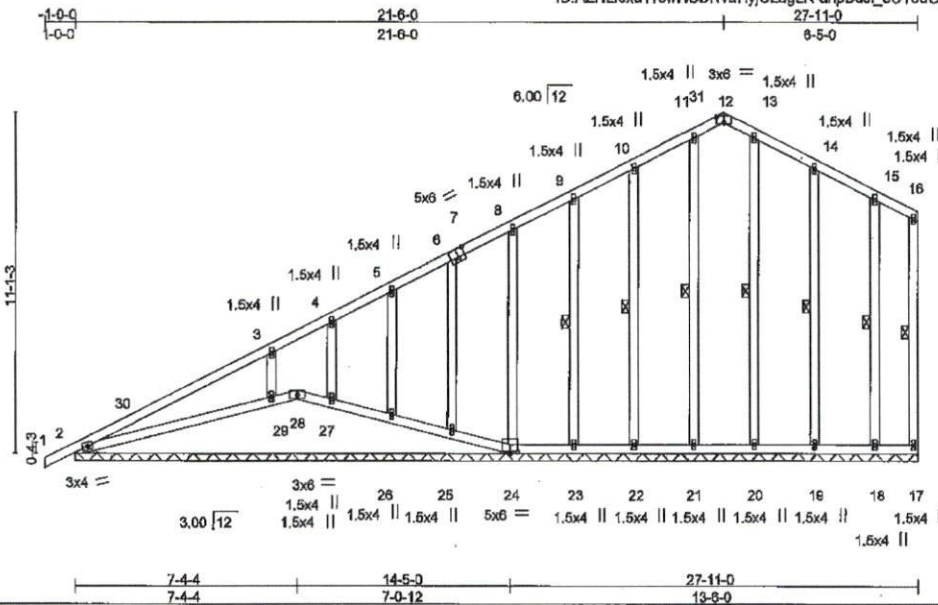
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 A MITEK Alliance

818 Soundside Road
 Eden, NC 27522

| | | | | | | |
|------------------|--------------|---------------------|----------|----------|----------|-----------|
| Job LGI_HOMES | Truss ATE | Truss Type GABLE | Qty 1 | Ply 1 | Alamance | E10611247 |
|------------------|--------------|---------------------|----------|----------|----------|-----------|

Builders FirstSource, Albemarle, NC 28001

Job Reference (optional)
7.640 s Apr 19 2016 Mitek Industries, Inc. Mon Jun 05 13:01:32 2017 Page 1
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Scale = 1:89.7

| | | | | | |
|--------------------------------------------------------------------------|----------------------|-------------|--------------------------|----------------|-------------|
| Plate Offsets (X,Y) - [7:0-2-12,0-3-4], [12:0-3-0,Edge], [24:0-3-0,0-12] | | | | | |
| LOADING (psf) | SPACING- | CSI. | DEFL | PLATES | GRIP |
| TCLL (roof) 20.0 | 2-0-0 | TC 0.52 | in (loc) l/defl L/d | MT20 | 244/190 |
| Snow (P/Pg) 15.4/20.0 | Plate Grip DOL 1.15 | BC 0.37 | Vert(LL) -0.00 1 n/r 120 | | |
| TCDL 10.0 | Lumber DOL 1.15 | WB 0.13 | Vert(TL) 0.05 1 n/r 120 | | |
| BCLL 0.0 | Rep Stress Incr YES | (Matrix) | Horz(TL) 0.00 29 n/a n/a | | |
| BCDL 10.0 | Code IRC2009/TPI2007 | | | Weight: 214 lb | FT = 20% |

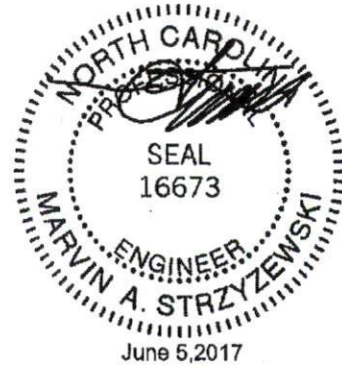
| | |
|-----------------------|-------------------------------------------------------------------------------------------------|
| LUMBER- | BRACING- |
| TOP CHORD 2x4 SP No.2 | TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. |
| BOT CHORD 2x4 SP No.2 | BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. |
| WEBS 2x4 SP No.3 | WEBS 1 Row at midpt 16-17, 11-21, 10-22, 9-23, 13-20, 14-19, 15-18 |
| OTHERS 2x4 SP No.3 | |

REACTIONS. (lb/size) 2=272/27-11-0, 28=153/27-11-0, 17=38/27-11-0, 21=161/27-11-0, 22=158/27-11-0, 23=164/27-11-0, 24=158/27-11-0, 25=154/27-11-0, 26=194/27-11-0, 27=18/27-11-0, 29=654/27-11-0, 20=160/27-11-0, 19=164/27-11-0, 18=140/27-11-0
 Max Horz 2=252(LC 10)
 Max Uplift 28=224(LC 5), 17=4(LC 11), 22=26(LC 10), 23=15(LC 10), 24=21(LC 10), 25=16(LC 10), 26=29(LC 10), 29=38(LC 10), 19=29(LC 11), 18=15(LC 11)
 Max Grav 2=272(LC 1), 17=38(LC 1), 21=161(LC 1), 22=161(LC 15), 23=164(LC 1), 24=158(LC 15), 25=154(LC 1), 26=194(LC 15), 27=72(LC 5), 29=654(LC 15), 20=192(LC 4), 19=214(LC 4), 18=157(LC 4)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/22, 2-30=234/0, 3-30=216/101, 3-4=-138/2, 4-5=-128/41, 5-6=-94/25, 6-7=-68/23, 7-8=-65/34, 8-9=-40/44, 9-10=-27/63, 10-31=-28/99, 11-31=0/101, 11-12=-27/101, 12-13=-33/103, 13-14=-38/104, 14-15=-35/56, 15-16=-13/21, 16-17=-29/19
 BOT CHORD 2-29=20/37, 28-29=61/0, 27-28=-13/1, 26-27=-10/10, 25-26=-10/9, 24-25=-9/10, 23-24=0/0, 22-23=0/0, 21-22=0/0, 20-21=0/0, 19-20=0/0, 18-19=0/0, 17-18=0/0
 WEBS 11-21=-121/11, 10-22=-122/60, 9-23=-120/49, 8-24=-122/52, 6-25=-113/48, 5-26=-156/65, 4-27=-11/34, 3-28=-401/167, 13-20=-152/12, 14-19=-173/82, 15-18=-121/71

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 21-6-0, Exterior(2) 21-6-0 to 24-6-0 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; C=1.1
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Continued on page 2



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with Mitek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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| Job | Truss | Truss Type | Qty | Ply | Allowance |
|-----------|-------|------------|-----|-----|-----------|
| LGI_HOMES | A7E | GABLE | 1 | 1 | E10611247 |

Builders FirstSource, Albemarle, NC 28001

7,640 s Apr 19 2016 MITek Industries, Inc. Mon Jun 05 13:01:32 2017 Page 2

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NOTES-

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 28, 21 lb uplift at joint 24, 16 lb uplift at joint 25, 29 lb uplift at joint 26 and 38 lb uplift at joint 29.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 17, 22, 23, 19, and 18. This connection is for uplift only and does not consider lateral forces.
- 13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 28, 25, 26, 27, 29.
- 14) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPP1 Quality Criteria, D8B-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.

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| | | | | | | |
|--------------------------------------------------------------------------|--------------|--------------------------------------------------------------------------------------------------------|----------|----------|-----------|-----------|
| Job LGI_HOMES | Truss B1E | Truss Type Common Supported Gable | Qty 1 | Ply 1 | Allowance | E10811248 |
| Builders FirstSource, Albemarle, NC 28001 | | Job Reference (optional) 7.640 s Apr 18 2018 MITek Industries, Inc. Mon Jun 05 13:01:33 2017 Page 1 | | | | |
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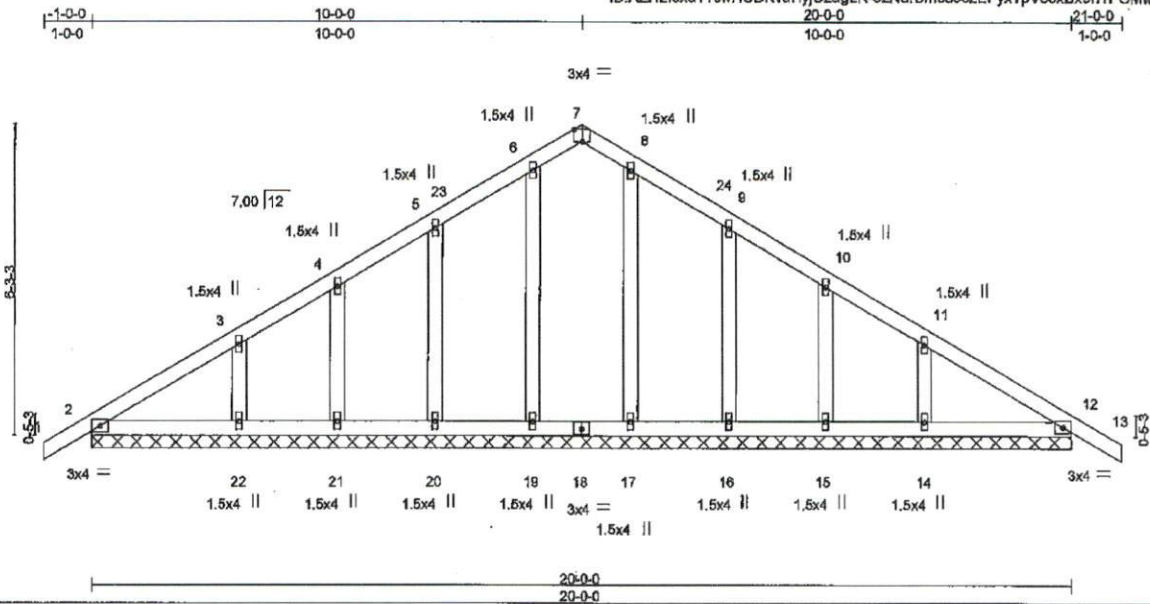


Plate Offsets (X,Y) = [7'-0"-2'-0", Edge]

| | | | | | |
|------------------------|----------------------|------------|---------------|----------------|-------------|
| LOADING (psf) | SPACING- | CSL | DEFL. | PLATES | GRIP |
| TCLL (roof) 20.0 | 2'-0"-0" | TC 0.09 | In (loc) 12 | MT20 | 244/190 |
| Snow (Pf/Pg) 15.4/20.0 | Plate Grip DOL 1.15 | BC 0.06 | Vert(TL) 0.00 | | |
| TCDL 10.0 | Lumber DOL 1.15 | WB 0.06 | Horz(TL) 0.00 | | |
| BCLL 0.0 * | Rep Stress Incr YES | (Matrix) | | | |
| BCDL 10.0 | Code IRC2009/TPI2007 | | | | |
| | | | | Weight: 108 lb | FT = 20% |

LUMBER-

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 OTHERS 2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6'-0" oc bracing.

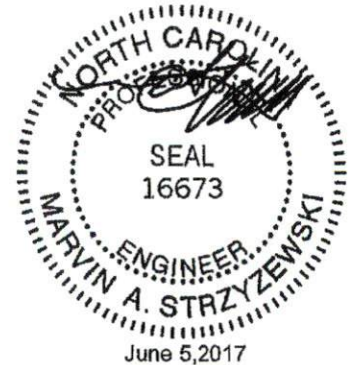
REACTIONS. (lb/size) 7=51/20'-0"-0", 2=171/20'-0"-0", 19=128/20'-0"-0", 20=172/20'-0"-0", 21=135/20'-0"-0", 22=228/20'-0"-0", 17=126/20'-0"-0", 16=172/20'-0"-0", 15=136/20'-0"-0", 14=228/20'-0"-0", 12=171/20'-0"-0"
 Max Horz 2=-136(LC 8)
 Max Uplift 2=-24(LC 8), 19=-11(LC 10), 20=-25(LC 10), 21=-23(LC 10), 22=-27(LC 10), 17=-11(LC 11), 16=-25(LC 11), 15=-23(LC 11), 14=-26(LC 11), 12=-23(LC 11)
 Max Grav 7=110(LC 11), 2=171(LC 15), 19=146(LC 3), 20=172(LC 1), 21=135(LC 15), 22=228(LC 1), 17=146(LC 4), 16=172(LC 1), 15=135(LC 16), 14=228(LC 1), 12=171(LC 16)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/25, 2-3=-116/103, 3-4=-71/92, 4-5=-43/93, 5-23=-34/131, 6-23=-6/142, 6-7=-21/175, 7-8=-21/175, 8-24=-0/142, 9-24=-34/131, 9-10=-28/91, 10-11=-36/48, 11-12=-54/49, 12-13=0/25
 BOT CHORD 2-22=-18/82, 21-22=-18/82, 20-21=-18/82, 19-20=-18/82, 18-19=-18/82, 17-18=-18/82, 16-17=-18/82, 15-16=-18/82, 14-15=-18/82, 12-14=-18/82
 WEBS 6-19=-106/52, 5-20=-130/78, 4-21=-104/65, 3-22=-164/100, 8-17=-106/52, 9-16=-130/78, 10-15=-104/65, 11-14=-164/100

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Comer(3)-1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 10-0-0, Comer(3) 10-0-0 to 13-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2'-0" oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at j(s) 2, 19, 20, 21, 22, 17, 16, 15, 14, and 12. This connection is for uplift only and does not consider lateral forces.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 12.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 rev. 10/03/2015 BEFORE USE.

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| | | | | | | |
|------------------|-------------|----------------------|----------|----------|----------|-----------|
| Job LGI_HOMES | Truss B2 | Truss Type Common | Qty 2 | Ply 1 | Alamance | E10611249 |
|------------------|-------------|----------------------|----------|----------|----------|-----------|

Builders FirstSource, Albemarle, NC 28001

7.640 s Apr 19 2016 MTEK Industries, Inc. Mon Jun 05 13:01:33 2017 Page 1
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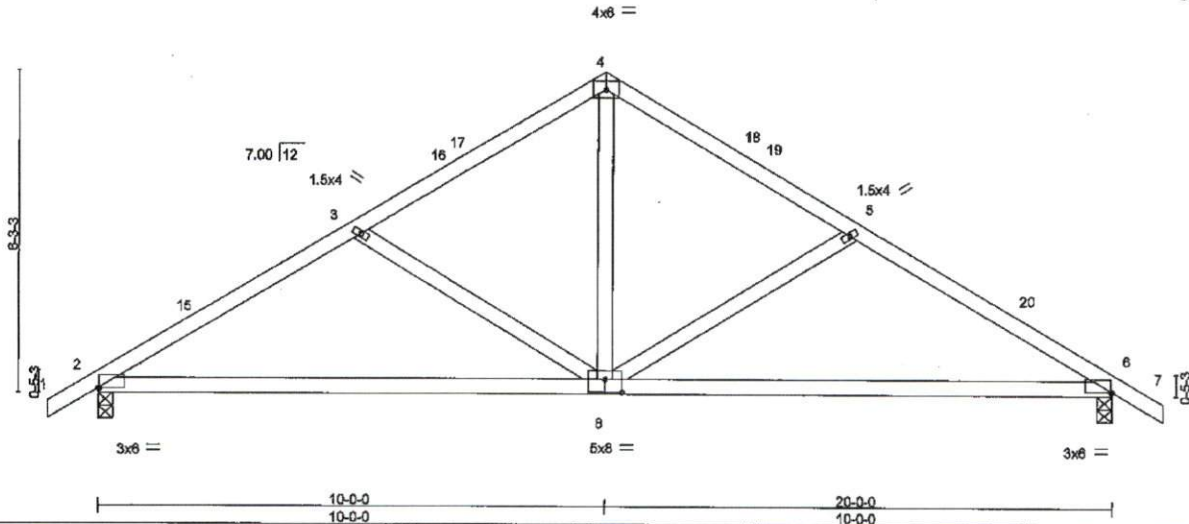


Plate Offsets (X,Y) = [2:0-0-4,0-0-1], [6:0-0-4,0-0-1], [6:0-4-0,0-3-0]

| | | | | | | | | | | |
|----------------------|-----------|----------------------|-------|-------------|--------------|----------|--------|------|---------------|-------------|
| LOADING (psf) | | SPACING- | 2-0-0 | CSI. | DEFL. | in (loc) | l/defl | L/d | PLATES | GRIP |
| TCLL (roof) | 20.0 | Plate Grip DOL | 1.15 | TC | 0.27 | -0.14 | 8-11 | >999 | MT20 | 244/190 |
| Snow (Pf/Pg) | 15.4/20.0 | Lumber DOL | 1.15 | BC | 0.84 | -0.37 | 8-11 | >645 | | |
| TCDL | 10.0 | Rep Stress Incr | YES | WB | 0.22 | 0.03 | 6 | n/a | | |
| BCLL | 0.0 * | Code IRC2009/TPI2007 | | (Matrix-S) | | Wind(LL) | 0.02 | 8-11 | >999 | 240 |
| BCDL | 10.0 | | | | | | | | | |

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 2=860/0-3-8, 6=860/0-3-8
Max Horz 2=-133(LC 8)
Max Uplift 2=-23(LC 10), 6=-23(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/30, 2-15=-1178/141, 3-16=-1118/167, 3-16=-887/119, 16-17=-815/121, 4-17=-807/137, 4-18=-807/137,
18-19=-815/121, 5-19=-887/119, 5-20=-1118/167, 6-20=-1178/142, 6-7=0/30
BOT CHORD 2-8=-58/966, 6-8=-67/966
WEBS 4-8=-17/558, 5-8=-329/138, 3-8=-329/138

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCCL=6.0psf; h=26ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 10-0-0, Exterior(2) 10-0-0 to 13-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load); Lumber DOL=1.15 Plate DOL=1.15; Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow); Lumber DOL=1.15 Plate DOL=1.15; Category II; Exp B; Partially Exp.; Ct=1.1
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(a) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

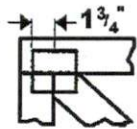
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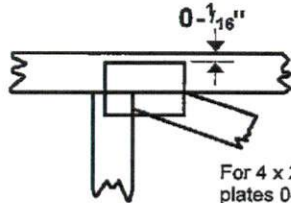
818 Soundside Road
Edenton, NC 27832

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

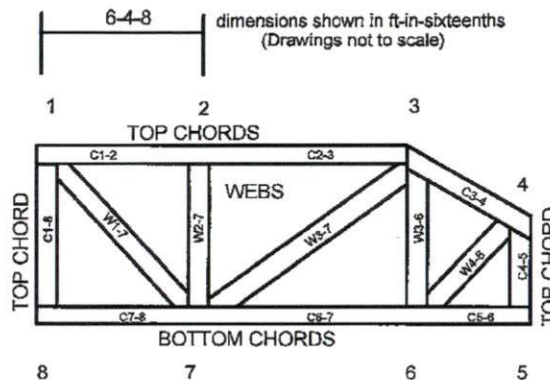


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor 1 bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

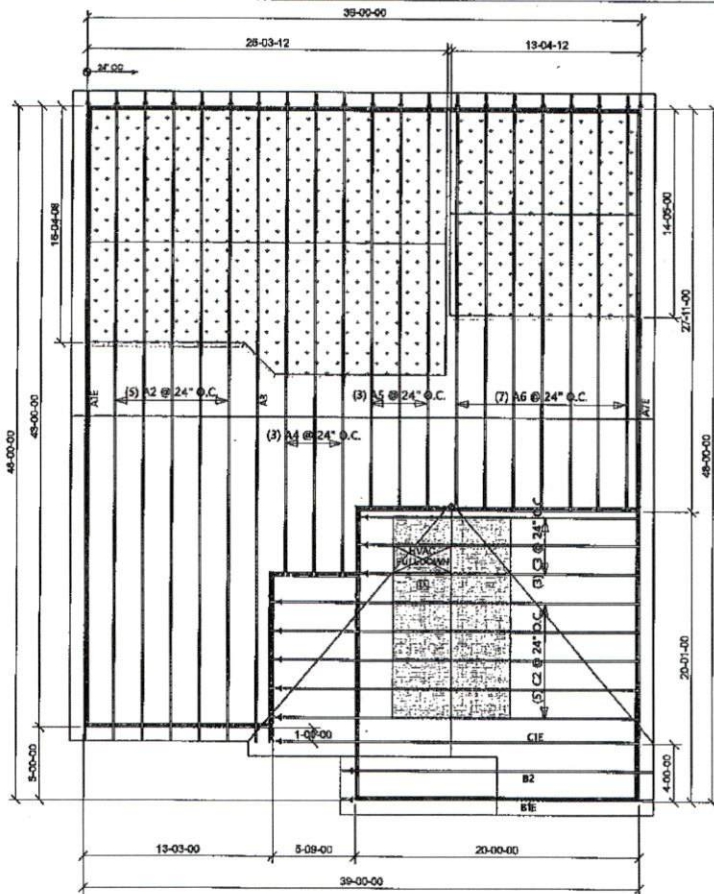
ROOF TRUSS NOTES:

- DO NOT CUT, SPLIT, NOTCH, OR OTHERWISE DAMAGE TRUSSES. Consult your DFR Representative for assistance. PRIOR TO modifying any beam.
- REASSEMBLY AND CORRECTION: HAGA REARMSAL O DANIEL DE CALAJUMBE OTRA MATERIAL LAS TRUSSES CORRECCION DE REARMSAL. Contacta a tu representante de DFR para asistencia ANTES de volver a colocar.
- The Truss Placement Diagram is provided to serve as a guide for truss installation. This diagram has been prepared by a Truss Fabricator and is not an engineered drawing.
- The responsibilities of the Owner, Building Designer, Contractor, Truss Designer, and Truss Fabricator shall be as defined by the TPI 1 National Standard.
- The third components shown on this diagram are to be used in any truss (members coded R1) and members structural applications. The third plates and angles are submitted to the OIG Standard safety requirements.
- Refer to the Truss Design Drawings for specific information about each individual beam design.
- The Truss Fabricator shall provide Truss-Beam Connection Requirements. Any special or other connections shall be the responsibility of the Building Designer.
- The Truss Placement Diagram and Truss Design Drawings are the property of Builders Structures and may not be copied or reproduced in part or in total under any circumstances without prior written authorization.
- In some cases, field bracing may be required to maintain the load capacities shown within Connection Drawings.
- Field bracing, including valley rafters, installed over roof beams shall have a knee brace from the ridge to the beam top chord at intervals of 40' or smaller (O.C.) or less. Major knee braces shall be placed within 10' of the knee in addition to the knee brace and shall be connected at the beam or ridge as appropriate.
- Truss Top Chords shall be fully sheathed in Deck Shear Blocking (unless noted as 2" O.C.) or 2" Truss Blocking (unless noted as 1" or less) and shall be installed in accordance with the Truss Design Drawing. Field bracing shall be installed in accordance with the Truss Design Drawing. Truss Blocking shall be installed in accordance with the Truss Design Drawing. Truss Blocking shall be installed in accordance with the Truss Design Drawing.
- The Placement Diagram is based upon the following conditions being structurally adequate: (1) Truss Design Drawing, (2) Truss Design Drawing, (3) Truss Design Drawing, (4) Truss Design Drawing, (5) Truss Design Drawing, (6) Truss Design Drawing, (7) Truss Design Drawing, (8) Truss Design Drawing, (9) Truss Design Drawing, (10) Truss Design Drawing, (11) Truss Design Drawing, (12) Truss Design Drawing, (13) Truss Design Drawing, (14) Truss Design Drawing, (15) Truss Design Drawing, (16) Truss Design Drawing, (17) Truss Design Drawing, (18) Truss Design Drawing, (19) Truss Design Drawing, (20) Truss Design Drawing, (21) Truss Design Drawing, (22) Truss Design Drawing, (23) Truss Design Drawing, (24) Truss Design Drawing, (25) Truss Design Drawing, (26) Truss Design Drawing, (27) Truss Design Drawing, (28) Truss Design Drawing, (29) Truss Design Drawing, (30) Truss Design Drawing, (31) Truss Design Drawing, (32) Truss Design Drawing, (33) Truss Design Drawing, (34) Truss Design Drawing, (35) Truss Design Drawing, (36) Truss Design Drawing, (37) Truss Design Drawing, (38) Truss Design Drawing, (39) Truss Design Drawing, (40) Truss Design Drawing, (41) Truss Design Drawing, (42) Truss Design Drawing, (43) Truss Design Drawing, (44) Truss Design Drawing, (45) Truss Design Drawing, (46) Truss Design Drawing, (47) Truss Design Drawing, (48) Truss Design Drawing, (49) Truss Design Drawing, (50) Truss Design Drawing, (51) Truss Design Drawing, (52) Truss Design Drawing, (53) Truss Design Drawing, (54) Truss Design Drawing, (55) Truss Design Drawing, (56) Truss Design Drawing, (57) Truss Design Drawing, (58) Truss Design Drawing, (59) Truss Design Drawing, (60) Truss Design Drawing, (61) Truss Design Drawing, (62) Truss Design Drawing, (63) Truss Design Drawing, (64) Truss Design Drawing, (65) Truss Design Drawing, (66) Truss Design Drawing, (67) Truss Design Drawing, (68) Truss Design Drawing, (69) Truss Design Drawing, (70) Truss Design Drawing, (71) Truss Design Drawing, (72) Truss Design Drawing, (73) Truss Design Drawing, (74) Truss Design Drawing, (75) Truss Design Drawing, (76) Truss Design Drawing, (77) Truss Design Drawing, (78) Truss Design Drawing, (79) Truss Design Drawing, (80) Truss Design Drawing, (81) Truss Design Drawing, (82) Truss Design Drawing, (83) Truss Design Drawing, (84) Truss Design Drawing, (85) Truss Design Drawing, (86) Truss Design Drawing, (87) Truss Design Drawing, (88) Truss Design Drawing, (89) Truss Design Drawing, (90) Truss Design Drawing, (91) Truss Design Drawing, (92) Truss Design Drawing, (93) Truss Design Drawing, (94) Truss Design Drawing, (95) Truss Design Drawing, (96) Truss Design Drawing, (97) Truss Design Drawing, (98) Truss Design Drawing, (99) Truss Design Drawing, (100) Truss Design Drawing.

WARNING:

- TRUSSES MUST BE INSTALLED IN ACCORDANCE WITH THE TRUSS DESIGN DRAWING. FAILURE TO DO SO MAY RESULT IN INJURY OR DEATH.
- REPAROS: TRUSSES DEBEN SER INSTALADOS DE ACORDO CON EL DISEÑO DE TRUSSES. EL INCUMPLIMIENTO PUEDE RESULTAR EN LESIONES O MUERTE.
- Trusses shall be installed in a safe manner meeting all applicable codes, OSHA, TPI and other applicable codes. Failure to follow these specifications may result in injury or death.
- Buildings under construction are vulnerable to high winds and should be protected as possible when vacant. The Contractor is responsible for protecting against weather conditions and shall take appropriate action to prevent injury or death.
- CONTRACTOR SHALL FOLLOW THE FOLLOWING:
- OSHA 1910 - Fall Protection and Safety
 - OSHA 1915 - Fall Protection and Safety
 - OSHA 1917 - Fall Protection and Safety
 - OSHA 1918 - Fall Protection and Safety
 - OSHA 1919 - Fall Protection and Safety
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 - OSHA 1997 - Fall Protection and Safety
 - OSHA 1998 - Fall Protection and Safety
 - OSHA 1999 - Fall Protection and Safety
 - OSHA 2000 - Fall Protection and Safety

TOTAL ROOF AREA
2206.97 SQ FT



No Scale

Customer Name: L&L Homes
Subdivision:
Plan Name: Alamance
File Name

Builders
FirstSource
Albemarle, NC

Revisions:

Drawn By:
JEFF BURRIS
DATE:
12/3/2018
Page Number:
1 of 1