

Trenco

818 Soundside Rd Edenton, NC 27932

Re: LGI\_Homes Avery Floor

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle, NC).

Pages or sheets covered by this seal: E10633996

thruE10634003

My license renewal date for the state of North Carolina is

December 31, 2017.

North Carolina COA: C-0844

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.



June 12,2017

Strzyzewski, Marvin

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdictions(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to Trenco. Any project specific information included is for Trenco's customer's file reference purpose only, and was not taken into account in the preparation of these designs. Trenco has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of the design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Avery Floor
LGI_HOMES	F1E	Floor Supported Gable	1	1	Lioh Reference (cotingel)

Builders FirstSource. Albemade NC 28001

7,540 s Apr 19 2016 MITak Industries, Inc. Mon Jun 12 12:12:38 2017 Page 1 ID:DO?ru4b2DX75jRW9yxkgi6znuDx-DXnch1W8JD?drsYCrlpEdg12fx6OGBDuSk2Wmnz72A9

8-14-0

0-148

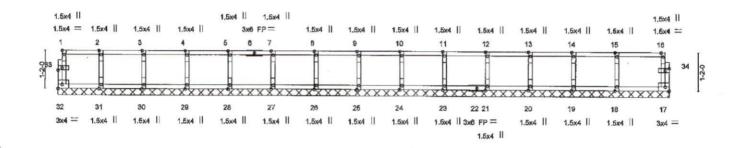


Plate Offsets (X,Y) [1:	Fdge 0-0-121 [33:0-1-	8 0-0-121 134	·0-1-8 0-0-1	01 .	18-11-8 18-11-8						
LOADING (psf) TCLL 40.0	SPACING- Plate Grip DOL	2-0-0 1.00	CSI.	0.10	DEFL.	in	(loc)	Vdefl	L/d	PLATES	GRIP
TCDL 10.0 BCLL 0.0	Lumber DOL Rep Stress Incr	1.00 1.00 YES	BC WB	0.02	Vert(LL) Vert(TL) Horz(TL)	n/a n/a 0.00	17	n/a n/a n/a	999 999	MT20	244/190
BCDL 5.0	Code IRC2009/TI		(Matr		11012(12)	0.00	1.6	Iva	n/a	Weight: 79 lb	FT = 20%F, 11%

TOP CHORD 2x4 SP No.2(flat) BOT CHORD 2x4 SP No.2(flat) 2x4 SP No 3/flaf WEBS OTHERS 2x4 SP No.3(flat)

BRACING-

TOP CHORD BOT CHORD

end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

REACTIONS. (lb/slze) 32=60/18-11-8, 17=72/18-11-8, 31=138/18-11-8, 30=149/18-11-8, 29=146/18-11-8, 28=147/18-11-8, 27=147/18-11-8, 26=147/18-11-8, 26=147/18-11-8, 26=147/18-11-8, 26=147/18-11-8, 28=147/18-1 24=147/18-11-8, 23=147/18-11-8, 21=146/18-11-8, 20=148/18-11-8, 19=142/18-11-8, 18=163/18-11-8

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 32-33=-53/0, 1-33=-52/0, 17-34=-67/0, 16-34=-66/0, 1-2=-14/0, 2-3=-14/0, 3-4=-14/0, 4-5=-14/0, 5-6=-14/0, 6-7=-14/0, 7-8=-14/0, 8-9=-14/0, 9-10=-14/0, 10-11=-14/0, 11-12=-14/0, 12-13=-14/0, 13-14=-14/0, 14-15=-14/0, 15-16=-14/0 
31-32=0/14, 30-31=0/14, 29-30=0/14, 28-29=0/14, 27-28=0/14, 26-27=0/14, 25-26=0/14, 24-25=0/14, 23-24=0/14, **BOT CHORD** 

22-23=0/14, 21-22=0/14, 20-21=0/14, 19-20=0/14, 18-19=0/14, 17-18=0/14 WEBS

2-31=-128/0, 3-30=-135/0, 4-29=-133/0, 5-28=-133/0, 7-27=-133/0, 8-26=-133/0, 9-25=-133/0, 10-24=-133/0,

11-23=-133/0, 12-21=-133/0, 13-20=-134/0, 14-19=-129/0, 15-18=-147/0

## NOTES.

1) Gable requires continuous bottom chord bearing.

- 2) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 3) Gable studs spaced at 1-4-0 oc.
- 4) "Semi-rigid pitchbreaks with fixed heets" Member end fixity model was used in the analysis and design of this truss.

  5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



🛕 WARNING - Verify design perameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/02/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must varify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual fuses web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual fuses web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucklings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of trusses and truss systems, see

ANDITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	Avery Floor
LGLHOMES	F2	Floor	1	1	Job Reference (options)

Builders FirstSource.

Albemarle , NC 28001

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 12 12:12:37 2017 Page 1 ID:DO?ru4b2DX75jRW9yxkgi6znuDx-hjL?uNXn4X7TT07OO?KTAua3MKHP?U41hOn4JDz72AB

HI 1-3-0

0-7-12 1-8-0 0-11-4

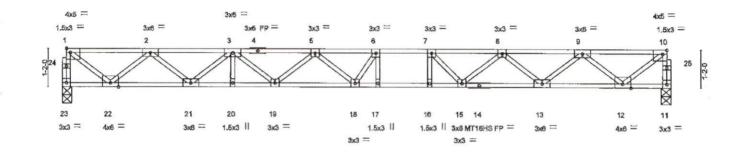


Plate Offsets (X,Y)	)- [1:Edge,0-1-8], [10:0-1-8,Edge]	•	18-11-8				
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING- 2-0-0 Plate Grlp DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES	CSI. TC 0.68 BC 0.74 WB 0.70		n (loc) l/defl 8 17-18 >591 9 17-18 >378 8 11 n/a	L/d 480 240 n/a	PLATES MT20 MT18HS	GRIP 244/190 244/190
BCDL 5.0	Code IRC2009/TPI2007	(Matrix)	TO SHOW THE PERSON OF THE PERS			Weight: 96 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat)

BOT CHORD 2x4 SP DSS(flat) "Except"

11-14: 2x4 SP No.1(flat)

WERS 2x4 SP No.3(flat)

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-7-14 oc purlins, except

end verticals.

Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS, (lb/size) 23=1023/0-3-8, 11=1023/0-3-8

FORCES. (Ib) - Maximum Compression/Maximum Tension

23-24=-1017/0, 1-24=-1016/0, 11-25=-1016/0, 10-25=-1015/0, 1-2=-1210/0, 2-3=-3001/0, 3-4=-4141/0, 4-5=-4141/0, TOP CHORD

5-6=-4570/0, 6-7=-4514/0, 7-8=-4114/0, 8-9=-3004/0, 9-10=-1210/0

BOT CHORD 22-23=0/61, 21-22=0/2279, 20-21=0/3735, 19-20=0/3735, 18-19=0/4512, 17-18=0/4514, 16-17=0/4514, 15-16=0/4514, 14-15=0/3686, 13-14=0/3686, 12-13=0/2283, 11-12=0/61

6-17=-390/149, 7-16=-104/336, 1-22=0/1467, 2-22=-1392/0, 2-21=0/940, 3-21=-937/0, 3-20=0/16, 3-19=0/519,

5-19=-483/0, 5-18=-127/332, 6-18=-335/406, 10-12=0/1467, 9-12=-1397/0, 9-13=0/940, 8-13=-888/0, 8-15=0/630, 7-15=-760/0

# NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nalls. Strongbacks to be attached to walls at their outer ends or restrained by other means.



A WARNING - Verify design paras eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTckée connectors. This days by this AND WILLIADED MITER MEFFERENCE PAGE M81-7473 FW. 1003/2015 BEFORE USE. Design valid for use only with MiTckée connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracking indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection are bracing of trusses and truss systems, see ANSI/TPI Laugh Critical Control of the Contr



818 Soundside Road Edenton, NC 27932

Job	Trues	Truss Type	Qty	Ply	Avery Floor
LGI_HOMES	F3	Floor	4	1	E10633988

Builders FirstSource. Albemaria NC 26001

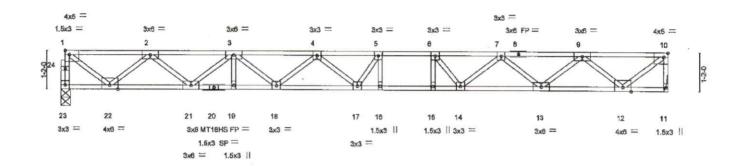
7.840 s Apr 19 2016 MITek Industries, Inc. Mon Jun 12 12:12:37 2017 Page 1 ID:DO?ru4b2DX75jRW9yxkgi6znuDx-hjl.?uNXn4X7TT07OO?KTAua3WKDL?U91hOn4JDz72A8

0-1-8

H 1-3-0

0-7-121 1-6-0 110-9-4

Scale = 1:32.4



	1-6-0	4-0-0 2-6-0	6-7-8 2-7-8	8-1-8 2-6-0	9-10-12 10-7-12 11-4-12   12-3-8 0-9-4 0-9-0 0-9-0 0-10-12		17-3-8 2-6-0	18-8-0
Plate Of	fisets (X,Y)-	1:Edge,0-1-8], [10:0-1-8,	,Edge]					
LOADIN TCLL TCDL BCLL	40.0 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 YES	CSI. TC 0.67 BC 1.00 WB 0.69	DEFL. In (loc) Vert(LL) -0.37 16-17 Vert(TL) -0.57 16-17 Horz(TL) 0.08 11	Vdefl L/d >606 480 >388 240 n/a n/a	PLATES MT20 MT18HS	GRIP 244/190 244/190
BCDL	5,0	Code IRC2009/TF	PI2007	(Matrix)			Weight: 94 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat)

BOT CHORD 2x4 SP No.2(flat) \*Except\*

11-20: 2x4 SP DSS(flat)

WERS 2x4 SP No.3(flat) BRACING.

TOP CHORD

Structural wood sheathing directly applied or 4-8-15 oc purlins, except

end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

2-2-0 oc bracing: 19-21.

REACTIONS. (lb/slze) 23=1010/0-3-8, 11=1016/Mechanical

FORCES. (lb) - Maximum Compression/Maximum Tension

23-24=-1005/0, 1-24=-1003/0, 10-11=-1008/0, 1-2=-1194/0, 2-3=-2956/0, 3-4=-4063/0, 4-5=-4462/0, 5-6=-4387/0 TOP CHORD

6-7=-4026/0, 7-8=-2921/0, 8-9=-2921/0, 9-10=-1141/0

BOT CHORD 22-23=0/60, 21-22=0/2248, 20-21=0/3671, 19-20=0/3671, 18-19=0/3671, 17-18=0/4420, 16-17=0/4387, 15-16=0/4387

14-15=0/4387, 13-14=0/3594, 12-13=0/2208, 11-12=0/0

5-16=400/127, 6-15=97/380, 1-22=0/1447, 2-22=-1373/0, 2-21=0/922, 3-21=-912/0, 3-19=0/12, 3-18=0/501, 4-18=-465/0

, 4-17=-138/314, 5-17=-304/418, 10-12=0/1456, 9-12=-1389/0, 9-13=0/928, 7-13=-876/0, 7-14=0/642, 6-14=-763/0

# NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) The Fabrication Tolerance at joint 20 = 11%
- 4) Refer to girder(s) for truss to truss connections.
- 5) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nalls. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 10/09/2015 BEFORE USE. Design valid for use only with MiTricke connectors. This design is based only upon parameters share REFERENCE PAGE MIL-7473 FW. 100/370415 REFORE USE. Design valid for use only with MiTricke connectors. This design is based only upon parameters share and is for an Individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate that design into the overall building design. Bracing indicated is to prevent buckling of Infididual truss web and/or chard members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidence regarding the flabilitation, storage, delivery, erection and bracing of trusses and truss systems, see

AMSUTPI Guaratty Criteria, DSB-89 and BCSI Building Component
Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	Avery Floor	٦
LGLHOMES	F4	FLOOR	12	1	E10633996	

Builders FirstSource,

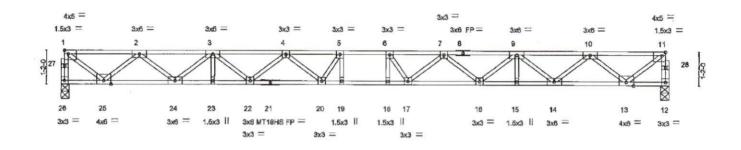
Albemarie , NC 28001

7.840 s Apr 19 2016 MITek Industries, Inc. Mon Jun 12 12:12:38 2017 Page 1 ID:DO?ru4b2DX75jRW9yxkgi6znuDx-9wvN6jXPrrFK49ksyjslj57EUkZCkylBw2Xdrfz72A7

0-1-8

H 1-3-0

9-7-12 1-6-0 9-7-12



						21-3-8					
		[1:Edge,0-1-8], [11:0-1-8	Edge	Ι		T					
LOADIN	41 1	SPACING-	1-7-3	CSI.		DEFL.	in (loc)	Vdefl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.65	Vert(LL)	-0.48 18-19	>525	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.96	Vert(TL)	-0.75 18-19	>335	240	MT18HS	244/190
BCLL	0.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.11 12	n/a	n/a	\$2000 (2000)	
BCDL	5.0	Code IRC2009/T	P12007	(Matr	ix)					Weight: 109 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat) BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat)

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-3-6 oc purlins, except

end verticals.

BOT CHORD Rigid celling directly applied or 10-0-0 oc bracing, Except:

2-2-0 oc bracing: 18-19.

REACTIONS. (lb/size) 26=920/0-3-8, 12=920/0-3-8

FORCES. (Ib) - Maximum Compression/Maximum Tension TOP CHORD

26-27=-916/0, 1-27=-914/0, 12-28=-916/0, 11-28=-914/0, 1-2=-1102/0, 2-3=-2781/0, 3-4=-3949/0, 4-5=-4545/0, 5-8=-4640/0, 6-7=-4545/0, 7-8=-3949/0, 8-9=-3949/0, 9-10=-2781/0, 10-11=-1102/0

25-26=0/55, 24-25=0/2080, 23-24=0/3495, 22-23=0/3495, 21-22=0/4360, 20-21=0/4360, 19-20=0/4640, 18-19=0/4640, BOT CHORD 17-18=0/4640, 16-17=0/4360, 15-16=0/3495, 14-15=0/3495, 13-14=0/2080, 12-13=0/55

5-19=-217/229, 6-18=-217/229, 1-25=0/1337, 2-25=-1272/0, 2-24=0/913, 3-24=-912/0, 3-23=0/9, 3-22=0/579, 4-22=-535/0

. 4-20=-13/418, 5-20=-462/185, 11-13=0/1337, 10-13=-1272/0, 10-14=0/913, 9-14=-912/0, 9-15=0/9, 9-16=0/579,

7-16=-535/0, 7-17=-13/418, 6-17=-462/185

# NOTES-

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



MARNENG - Verify design paremeters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2016 BEFORE USE. Design valid for use only with MiTekte connectors. This design is based only upon parameters have and is for an individual building component, not a truss system. Before use, the building designer must varify the applicability of design parameters and properly incorporate this design into the overall building designer must varify the applicability of design parameters and properly incorporate this design into the overall building design. Brading indicated is to prevent bucking of individual kusus web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to provent colleages with possible porsonal injury and properly demage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, are

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety information available from Truss Plate Institute, 218 N, Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	Avery Floor
LGI_HOMES	F5E	Floor Supported Gable	1	1	E10834000

Builders FirstSource, Albemarle , NC 28001

7.640 s Apr 19 2016 MiTek Industries, Inc. Mon Jun 12 12:12:39 2017 Page 1 ID:DO?ru4b2DX75jRW9yxkgi6znuDx-d6TlJ3Y1c8NBlJHmWQNxFJIYE87FTX0KBiGAN6z72A6

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0-48

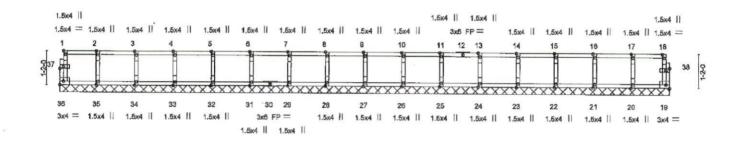


Plate Offsets (X,Y) J	[1:Edge,0-0-12], [37:0-1-8,0-0-12], [38	:0-1-8,0-0-12]	21-3-8 21-3-8						
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5,0	SPACING	CSI. TC 0.08 BC 0.01 WB 0.03 (Matrix)	DEFL. Vert(LL) Vert(TL) Horz(TL)	in n/a n/a 0.00	(loc) - - 19	I/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 88 lb	GRIP 244/190 FT = 20%F, 11%E

TOP CHORD 2x4 SP No.2(flat)

BOT CHORD 2x4 SP No.2(flat) 2x4 SP No 3(flat) WEBS

**OTHERS** 2x4 SP No.3(flat)

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 36=55/21-3-8, 19=54/21-3-8, 35=144/21-3-8, 34=148/21-3-8, 33=148/21-3-8, 32=147/21-3-8, 31=147/21-3-8, 29=147/21-3-8, 28=147/21-3-8, 21=147/21-3-8, 21=148/21-3-8, 21

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 36-37=-51/0, 1-37=-50/0, 19-38=-48/0, 18-38=-48/0, 1-2=-9/0, 2-3=-9/0, 3-4=-9/0, 4-5=-9/0, 5-6=-9/0, 6-7=-9/0, 7-8=-9/0

8-9=-9/0, 9-10=-9/0, 10-11=-9/0, 11-12=-9/0, 12-13=-9/0, 13-14=-9/0, 14-15=-9/0, 15-16=-9/0, 16-17=-9/0, 17-18=-9/0 35-36=0/9, 34-35=0/9, 33-34=0/9, 32-33=0/9, 31-32=0/9, 30-31=0/9, 29-30=0/9, 28-29=0/9, 27-28=0/9, 26-27=0/9,

BOT CHORD 25-26=0/9, 24-25=0/9, 23-24=0/9, 22-23=0/9, 21-22=0/9, 20-21=0/9, 19-20=0/9

WEBS

2-35=-131/0, 3-34=-134/0, 4-33=-133/0, 5-32=-133/0, 6-31=-133/0, 7-29=-133/0, 8-28=-133/0, 9-27=-133/0, 10-26=-133/0, 11-25=-133/0, 13-24=-133/0, 14-23=-133/0, 15-22=-133/0, 16-21=-135/0, 17-20=-128/0

## NOTES.

Gable requires continuous bottom chord bearing.

- 2) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 3) Gable studs spaced at 1-4-0 oc.

4) "Semi-rigid pitchbreaks with fixed heets" Member end fixity model was used in the analysis and design of this truss.

5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nalls. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 18/03/2015 BEFORE USE.

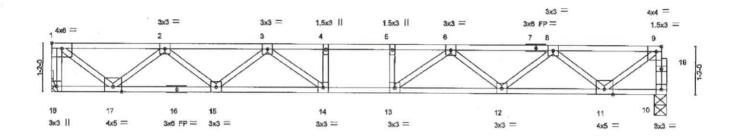
Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Setore use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designs. Brading indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is shreys required for stability and to prevent collapse with possible personal highry and property damage. For general guidance regarding the fabrication, storage, delivery, crection and brading of incuses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BGSI Building Component Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 2234 4.



Job	Truss	Trues Type	Qty	Ply	Avery Floor
LGLHOMES	F6	Floor	3	1	E10634001
Builders FirstSource, Alberr	arle , NC 28001		ID:D0?ru4	7.	Job Reference (optional) 840 s Apr 19 2016 MTek Industries, Inc. Mon Jun 12 12:12:39 2017 Page 1 Neyxkgi6znuDx-d6TIJ3Y1 c8NBIJHmWQNxFJITU8xuTPvK8 GAN6z72A6
1-3-0		1-4-4	<b>→</b> 1-4	-4	o-f1-8

Scale = 1:25,6



	***************************************		14-11-8					
Plate Offsets (X,Y)-	[1:Edge,0-1-8], [9:0-1-8,Edge]	,						
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	Vdefl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.44	Vert(LL)	-0.16 13-14	>999	480	MT20	244/190
CDL 10,0	Lumber DOL 1.00	BC 0.80	Vert(TL)	-0.25 13-14	>705	240	/0.00 miles	
BCLL 0.0	Rep Stress Incr YES	WB 0.55	Horz(TL)	0.05 10	n/a	n/a		
BCDL 5.0	Code IRC2009/TPI2007	(Matrix)			5,500		Weight: 76 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat) BOT CHORD 2x4 SP No.2(flat) WEBS 2x4 SP No.3(flat) BRACING-

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

end verticals.

BOT CHORD Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 18=809/Mechanical, 10=803/0-3-8

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD

BOT CHORD

WEBS

1-18=-802/0, 10-19=-797/0, 9-19=-796/0, 1-2=-920/0, 2-3=-2187/0, 3-4=-2826/0, 4-5=-2826/0, 6-6=-2826/0, 6-7=-2186/0, 7-8=-2186/0, 8-9=-922/0 17-18=-0/0, 16-17=0/1734, 15-16=0/1734, 14-15=0/2608, 13-14=0/2826, 12-13=0/2608, 11-12=0/1731, 10-11=0/48 4-14=-217/0, 5-13=-217/0, 1-17=0/1155, 2-17=-1059/0, 2-15=0/590, 3-15=-547/0, 3-14=-35/513, 9-11=0/1116, 8-11=-1053/0, 8-12=0/592, 6-12=-549/0, 11-12=0/1731, 10-11=0/1116, 8-11=-1053/0, 8-12=0/592, 6-12=-549/0, 11-12=0/1731, 10-11=0/1116, 8-11=-1053/0, 8-12=0/592, 6-12=-549/0, 11-12=0/1731, 11-12=0/

6-13=-36/513

# NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Refer to girder(s) for truss to truss connections.
- 3) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to
- be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2018 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building design must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracking indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible parameter layers and permanent bracing the fashication, storage, delivery, erection and bracking of trusses and truss systems, see ANSITTHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Piate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Type Truss Qty Avery Floo E10834002 GLHOMES F7 Floor

Builders FirstSource Albemarle , NC 28001 Job Reference (optional)
7.540 s Apr 19 2018 MiTak Industries, Inc. Mon Jun 12 12:12:40 2017 Page 1
ID:DO?ru4b2DX76jRW9yxkgi6znuDx-5I17XPZfNSV2KTsz48uAoWCZXYGuCpgUNL0kwYz72A5

HI 1-3-0

0-10-0 1-6-0 10-10-0

0-1<sub>1</sub>8 Scale = 1:33.2

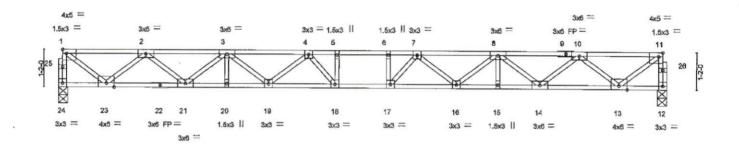


Plate Of	fsets (X,Y)-	[1:Edge,0-1-8], [11:0-1-8	B,Edge]			19-2-0					
LOADIN TCLL TCDL BCLL	G (psf) 40.0 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.00 1.00 YES	CSI. TC BC WB	0.75 0.88 0.71	DEFL. Verl(LL) Verl(TL) Horz(TL)	In (loc) -0.40 17-18 -0.62 17-18 0.10 12	Vdefl >570 >365 n/a	L/d 480 240 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code IRC2009/T	Pl2007	(Matr	rlx)					Weight: 98 lb	FT = 20%F, 11%E

LUMBER-

TOP CHORD 2x4 SP No.2(flat)

BOT CHORD 2x4 SP No.2(flat) \*Except\*

12-22: 2x4 SP No.1(flat)

WERS

2x4 SP No.3(flat)

BRACING.

TOP CHORD

Structural wood sheathing directly applied or 4-6-12 oc purlins, except

end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 24=1034/0-3-8, 12=1034/0-3-8

FORCES. (Ib) - Maximum Compression/Maximum Tension

24-25=-1029/0, 1-25=-1028/0, 12-26=-1029/0, 11-26=-1027/0, 1-2=-1225/0, 2-3=-3046/0, 3-4=-4210/0, 4-5=-4680/0, TOP CHORD

5-6=-4680/0, 6-7=-4680/0, 7-8=-4210/0, 8-9=-3046/0, 9-10=-3046/0, 10-11=-1225/0

BOT CHORD 23-24=0/62, 22-23=0/2308, 21-22=0/2308, 20-21=0/3794, 19-20=0/3794, 18-19=0/4584, 17-18=0/4680, 16-17=0/4584,

15-16=0/3794, 14-15=0/3794, 13-14=0/2308, 12-13=0/82

5-18=-284/91, 6-17=-284/91, 1-23=0/1486, 2-23=-1409/0, 2-21=0/961, 3-21=-955/0, 3-20=-13/28, 3-19=0/532,

4-19=-509/0, 4-18=-225/528, 11-13=0/1485, 10-13=-1409/0, 10-14=0/961, 8-14=-954/0, 8-15=-13/28, 8-16=0/532, 7-16=-509/0, 7-17=-225/528

# NOTES-

WEBS

1) Unbalanced floor live loads have been considered for this design.

2) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MM-7473 rev. 10/03/2016 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent occlipaes with possible paraonal injury and property demage. For general guidance regarding the fabrication, storage, delivery, serction and bracing of thusses and truss systems, see

ANSI/TPI1 Quality Oritoria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Stroot, Suite 312, Alexandric, VA 22314.



Job	Trues	Truss Type	Qty	Ply	Avery Floor		200000000000000000000000000000000000000
LGI_HOMES	F8	Floor	1	1			E10634003
Builders FirstSource, A	Ibemarie , NC 28001		1D:D0?ru4	7. 2DX76JRV	Job Reference (optio 640 s Apr 19 2016 MiTek V9yxkgl6znuDx-5117XF	Industries, Inc. Mon Jun 1	2 12:12:40 2017 Page 1 CjEYSkCz7UNL0kwYz72A5
	0-1-8  1   3x3 = 0-	8-4 2 1.5×3 <sub>1</sub> 11 1	-0-0 3 1.6x3	l	1-3-0	4 3×3 =	Scale = 1:8.7
1250	1.843	3x3 = 7	3x3	4		5	1.2-0
	3x3 =					1,5x3	
			3-6-12 3-6-12				
LOADING (psf) TCLL 40,0 TCDL 10,0 BCLL 0.0 BCDL 5.0	SPACING-         2-0-0           Plate Grip DOL         1.00           Lumber DOL         1.00           Rep Stress Incr         YES           Code IRC2009/TPI2007	TC 0.12 BC 0.12 WB 0.10	DEFL.     Vert(LL) -0.0 Vert(TL) -0.0 Horz(TL) 0.0	6	l/defi L/d >999 480 >999 240 n/a n/a	PLATES MT20 Weight: 21 lb	GRIP 244/190 FT = 20%F, 11%E
			BRACING- TOP CHORD BOT CHORD	end ver	ticals.		12 oc purlins, except

REACTIONS. (lb/size) 8=179/0-3-8, 5=186/Mechanical

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 8-9=-188/0, 1-9=-188/0, 4-5=-172/0, 1-2=-143/0, 2-3=-143/0, 3-4=-143/0

BOT CHORD 7-8=0/11, 6-7=0/143, 5-6=0/0 WEBS 4-6=0/180, 1-7=0/215, 2-7=-128/0, 3-6=-121/0

# NOTES-

1) Unbalanced floor live loads have been considered for this design.

2) Refer to girder(s) for truss to truss connections.

3) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to

be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITeke connectors. This design is based only upon parameters and round in and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Brading Individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal highly and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss expeters, see.

ANSITTY Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Piate Institute, 218 N, Lee Street, Suite 312, Alexandria, V2 22314.

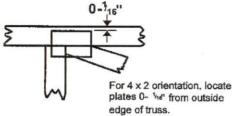


# **Symbols**

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, v offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MiTek 20/20 software or upon request.

#### PLATE SIZE

 $4 \times 4$ 

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

#### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

#### BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

#### **Industry Standards:**

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

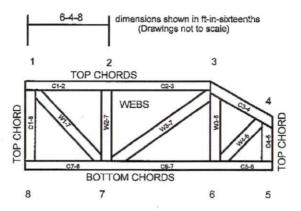
DSB-89: Design Standard for Bracing.

BCSI:

Building Component Safety Information. Guide to Good Practice for Handling.

Installing & Bracing of Metal Plate Connected Wood Trusses.

# **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

### PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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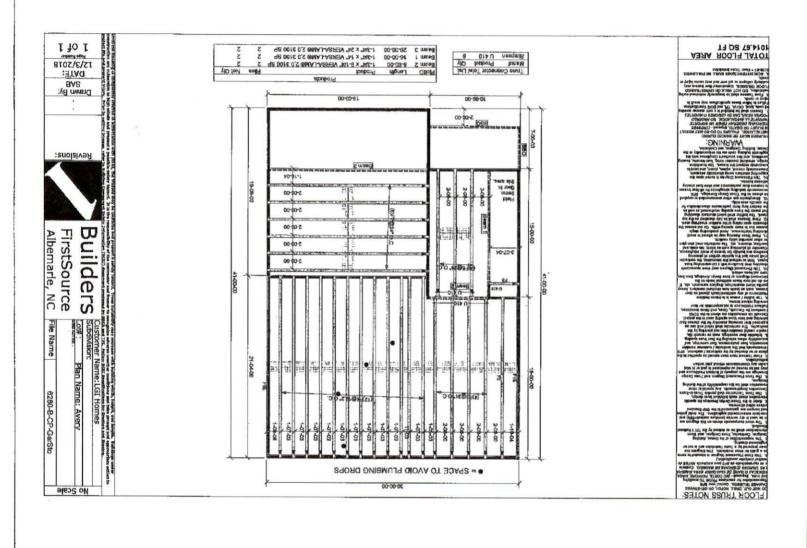
MiTek Engineering Reference Sheet MII-7473 rev. 10/03/2015



# General Safety Notes

# Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI,
- 2. Truss bracing must be designed by an engineer, For wide truss spacing, individual lateral braces themselves may require bracing, or atternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11, Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft, spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or after truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.





Trenco

818 Soundside Rd Edenton, NC 27932

Re: LGI Homes Avery Roof

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle, NC).

Pages or sheets covered by this seal: E10798395

thruE10798405

My license renewal date for the state of North Carolina is December 31, 2017.

North Carolina COA: C-0844

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

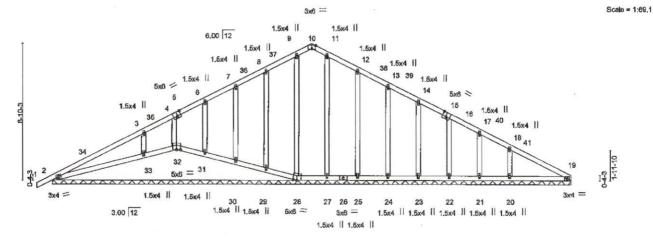


August 1,2017

Strzyzewski, Marvin

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdictions(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to Trenco. Any project specific information included is for Trenco's customer's file reference purpose only, and was not taken into account in the preparation of these designs. Trenco has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of the design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job Truss Truss Type Qty Avery Roof E10798395 LGI\_HOMES A1F GABLE Job Reference (optional)
7.640 e Apr 19 2016 MiTek Industries, Inc. Tue Aug 01 09:29:53 2017 Page 1
ID:W1XKcU07\$3cwWireuGga5gznu0U-iHEoPnfC75w2HjHfg7vvwYxyk0xXERFOaBx2iXysGml Builders FirstSource. Albemarle NC 28001 1-0-0 17-0-0 17-0-0 17-0-0



-	8-1-14 8-1-14		6-0-4 '-10-6			***	34-0- 17-11-			<del></del>	
Plate Offsets (X,Y)- [5  LOADING (psf)  TCLL (roof) 20.0  Snow (Pf/Pg) 15.4/20.0  TCDL 10.0  BCLL 0.0	SPACING-Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1,15 1,15 1,15 YES	-3-4], [32:0-3 CSI, TC BC WB	0.42 0.28 0.17	DEFL. Vert(LL) Vert(TL) Horz(TL)	in -0.00 0.04 0.01	(loc) 1 1 19	l/defl n/r n/r n/r	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2009/T	P12007	(Matr	IX)						Weight: 198 [b	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

BRACING-

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 32-33.

REACTIONS. (lb/size) 2=264/34-0-0, 32=-13/34-0-0, 28=143/34-0-0, 29=166/34-0-0, 30=150/34-0-0, 31=201/34-0-0, 33=507/34-0-0, 27=154/34-0-0, 25=160/34-0-0, 24=161/34-0-0, 23=156/34-0-0, 22=178/34-0-0, 21=88/34-0-0, 20=327/34-0-0, 19=138/34-0-0

Max Horz 2=111(LC 9)

Max Uplift 2=57(LC 10), 32=-16(LC 15), 29=-34(LC 11), 30=-15(LC 11), 31=-31(LC 10), 33=-144(LC 10), 25=-36(LC 11), 24=-16(LC 10), 23=-18(LC 10), 22=-24(LC 11), 21=-22(LC 11), 20=-99(LC 11)

Max Grav 2=264(LC 1), 32=82(LC 10), 28=168(LC 3), 29=198(LC 3), 30=150(LC 1), 31=201(LC 15), 33=507(LC 15),

27=180(LC 4), 25=197(LC 4), 24=161(LC 1), 23=156(LC 16), 22=178(LC 1), 21=88(LC 16), 20=327(LC 16), 19=138(LC 1)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-34=-128/9, 3-34=-111/79, 3-35=-88/41, 4-35=-66/50, 4-5=-39/63, 5-6=-39/64, 6-7=-46/79, 7-36=-43/100,

36-37=-17/108, 8-37=-5/110, 8-9=-50/151, 9-10=-42/139, 10-11=-43/140, 11-12=-51/151, 12-38=-4/111, 38-39=-16/108,

13-39=-43/101, 13-14=-43/78, 14-15=-40/45, 15-16=-44/35, 16-40=-23/14, 17-40=-40/5, 17-18=-59/1, 18-41=-93/48,

19-41=-113/0

**BOT CHORD** 2-33=-21/137, 32-33=-21/130, 31-32=-7/132, 30-31=-3/133, 29-30=-4/133, 28-29=-3/134, 27-28=0/126, 26-27=0/126,

25-26=0/126, 24-25=0/126, 23-24=0/126, 22-23=0/126, 21-22=0/126, 20-21=0/126, 19-20=0/126

9-28=-136/0, 8-29=-154/92, 7-30=-113/45, 8-31=-150/62, 4-32=-18/17, 3-33=-354/190, 11-27=-138/0, 12-25=-157/92, 13-24=-120/48, 14-23=-118/51, 16-22=-130/57, 17-21=-77/45, 18-20=-226/127 WEBS

# NOTES.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind; ASCE 7-05; 100mph; TCDL=6,0psf; BCDL=6,0psf; h=25ft; Cat, II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-4-13, Interior(1) 2-4-13 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1,60 plate grip DOL=1,60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry
- Gable End Detaits as applicable, or consult qualified building designer as per ANSI/TPI 1.

  4) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL≃1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

5) Unbalanced snow loads have been considered for this design.

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEPORE U.S.E. Design valid for use only with MiTexte connectors. This design is based only upon parameters shown, and is for an individual building component, not a trues system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent culcapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and russ systems, see

ANSITPH Quality Criteria, DSB-88 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N, Lee Street, Suite 312, Alexandria, VA 22214.



Job	Truss	Truss Type	Qly	Pily	Avery Roof	
LGI_HOMES	A1E	GABLE	1	1	Job Reference (optional)	E10798395

Builders FirstSource, Albemarte NC 28001

Too reterence (opinotal)
7-540 s April 2016 Miffel Industries, Inc. Tue Aug 01 09:29:53 2017 Page 2
ID:W1XKcU07S3cwWireuGga5gznu.0U-IHEoPnfC75w2HjHfg7vvwYxyk0xXERFOaBx2lXysGml

#### NOTES-

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 2, 16 lb uplift at joint 32, 34 lb uplift at joint 29, 15 lb uplift at joint 30, 31 lb uplift at joint 31, 144 lb uplift at joint 33, 36 lb uplift at joint 25, 16 lb uplift at joint 24, 18 lb uplift at joint 23, 24 lb uplift at joint 22, 22 lb uplift at joint 21 and 99 lb uplift at joint 24, 18 lb uplift at joint 23, 24 lb uplift at joint 27, 27 lb uplift at joint 28, 28 lb uplift at joint 29, 29 lb uplift at joint 29, 20 lb uplift at joint 20, 20 lb uplift at joint 2 Joint 20.
- 12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 32, 29, 30, 31, 33, 13) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an Individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Endeling indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent cotlapse with possible personal injury and properly demage. For general guidance regarding the flabrication, storage, delivery, erection and bracing of trusses and futures systems, see

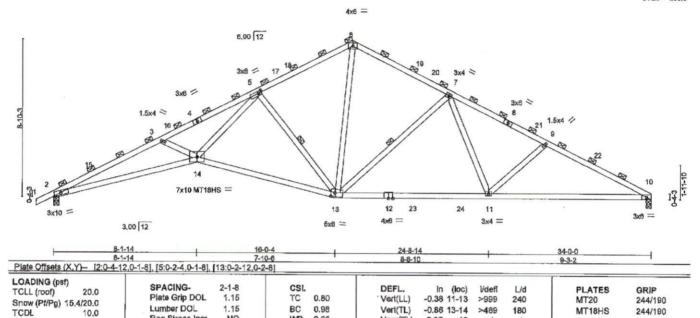
AMBUTPI1 Quality Criteria, DSB-89 and BCBI Building Component Safety information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandrin, VA 22314.



818 Soundside Road Edenton, NC 27932

Job		Truss		Truss Type		Q	ty F	Ply	Avery Roof	
LGI_HOMES		A2		ROOF SPECIAL		2		1		E10798396
Builders FirstSource,	Alben	narie , NC 28001						7.6	Job Reference (optional) 40 s Apr 19 2016 MTek Industri	es, Inc. Tue Aug 01 09:29:54 2017 Page 1
1-0-0	6-2-	10	11-7	<b>4</b> 5	17-0-0		XK¢U07 2-5-13	S3cwWin	euGga5gznu0U-BToAc7gqti 27-11-11	P3vvtsrEqQ8SIU1QQ6rzmuXprhbEzysGmh
1-0-0	6-2-	10	5-4-	11 '	5-4-11	1	5-5-13		5-5-13	6-0-5

Scale = 1:60.0



Horz(TL)

BRACING-

TOP CHORD

BOT CHORD

WEBS

0.37

1 Row at midpt

10

2-0-0 oc purlins (2-5-11 max.)

n/a

n/a

5-13, 7-13

(Switched from sheeted: Spacing > 2-0-0). Rigid ceiling directly applied or 10-0-0 oc bracing.

BCDL LUMBER-

BCLL

TOP CHORD 2x4 SP No.1 \*Except\*

0.0 \*

10.0

1-4: 2x4 SP DSS, 8-10: 2x4 SP No.2

BOT CHORD 2x4 SP No.2 \*Except\*

2-14: 2x4 SP DSS, 10-12: 2x4 SP No.1 2x4 SP No.3 \*Except\*

WEBS

5-14: 2x4 SP No.2

REACTIONS. (lb/size) 2=1547/0-3-8, 10=1504/0-3-8

Max Horz 2=118(LC 9)

Max Uplift 2=168(LC 10), 10=-117(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-15=-5090/439, 3-15=-5006/471, 3-16=-4750/387, 4-16=-4690/396, 4-5=-4676/416, 5-17=-1850/273,

17-18=-1793/281, 6-18=-1767/300, 6-19=-1615/291, 19-20=-1639/272, 7-20=-1698/263, 7-8=-2484/299, 8-21=-2501/273, 9-21=-2565/271, 9-22=-2754/325, 10-22=-2837/307 2-14=-374/4593, 13-14=-151/2486, 12-13=-100/2013, 12-23=-100/2013, 23-24=-100/2013, 11-24=-100/2013,

WB 0,66

(Matrix)

Rep Stress Incr NO Code IRC2009/TPI2007

NO

BOT CHORD

10-11=-208/2468

3-14=-305/195, 5-14=-125/2687, 5-13=-1350/209, 6-13=-124/1199, 7-13=-776/149, 7-11=0/580, 9-11=-336/178

# WEBS NOTES-

Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6,0psf; BCDL=6,0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-4-13, Interior(1) 2-4-13 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof live load: Lumber DOL=1.15 Plate DOL=1.15

snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

All plates are MT20 plates unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Bearing at Joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity

applied by grain of bearing surfaces.
 of bearing

12) Graphical puriln representation does not depict the size or the orientation of the puriln along the top and/or bottom chord.

SEAL 16673

VGINEEN
A. STRI August 1,2017

Weight: 171 b

FT = 20%

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design with for use only with MiTakle connectors. This design be based only upon parameters shown, and is for an includidual building component, not a truss system. Sefore use, the building designer must verify the applicability of design parameters end properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent building of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal highly and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and future systems, see — ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	Avery Roof		
LGI_HOMES	A3	Roof Special		4	1			E10798397
Builders FirstSource.	Albemarie , NC 28001					Job Reference (option		
Distribution,	Application, No 2000			ID:W1XKcU078			dustries, Inc. Tue Aug 01 09:29 SeiBmW1R2oYxN?z0BJqV	
71-0-9	8-2-10	11-7-5	17-0-0	22-5-13		27-11-11	34-0-0	t was a count to count
1-0-0	8-2-10	5-4-11	5-4-11	5-5-13		5-5-13	8-0-5	

Scale: 3/16"=1"

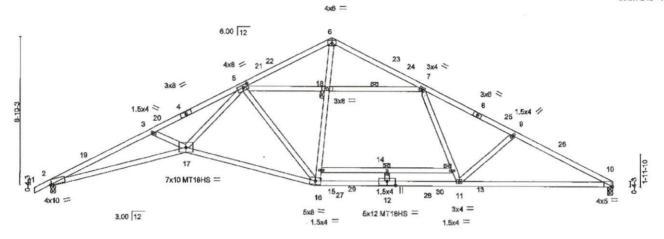


Plate Offsets (X,Y) [2:0-0-14	8-1-14 4,Edge), [5:0-3-10,0-1-12], [10:0-1-0,8	7-10-8 [dge], [12:0-6-0,0-3-0]	4-4-5	4-4-5		9-3-2	22.000	1
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.86 BC 0.83 WB 0.95 (Matrix)	DEFL. Vert(LL) Vert(TL) Horz(TL)	In (loc) -0.60 14-15 -1.11 12-16 0.40 10	l/defi >679 >365 n/a	L/d 240 180 n/a	PLATES MT20 MT18HS Weight: 187 lb	GRIP 244/190 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

JOINTS

24-8-14

6-0-0 oc bracing: 13-15

1 Brace at Jt(s): 18, 13

1 Row at midpt

Structural wood sheathing directly applied or 2-0-6 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

LUMBER-

TOP CHORD 2x4 SP No.2 \*Except\*

1-4: 2x4 SP DSS

BOT CHORD 2x4 SP DSS "Except"

16-17: 2x4 SP No.1, 13-15: 2x4 SP No.2

8-1-14

WERS 2x4 SP No.3 \*Except\*

5-17: 2x4 SP No.2

REACTIONS. (lb/size) 2=1687/0-3-8, 10=1729/0-3-8

Max Horz 2=111(LC 9)

Max Uplift 2=-119(LC 10), 10=-50(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/24, 2-19=-5710/253, 3-19=-5629/282, 3-20=-5413/203, 4-20=-5366/211, 4-5=-5348/229, 5-21=-1036/24,

21-22=979/33, 6-22=-952/51, 6-23=-882/72, 23-24=-893/54, 7-24=-917/46, 7-8=-3050/149, 8-25=-3065/123,

16-0-4

9-25=-3125/121, 9-26=-3294/174, 10-26=-3388/157

2-17=-206/5155, 16-17=-46/2968, 16-27=0/2460, 12-27=0/2460, 12-28=0/2460, 11-28=0/2460, 10-11=-80/2941, 15-29=-17/0, 14-29=-17/0, 14-30=-17/0, 13-30=-17/0 BOT CHORD

WEBS 3-17=-249/187, 5-17=-42/2916, 5-16=-811/125, 15-16=-118/398, 15-18=-2/562, 6-18=0/625, 7-13=0/780, 11-13=0/653,

9-11=-293/172, 7-18=-1778/199, 5-18=-1763/195, 12-14=-115/0

# NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Whid: ASCE 7-05; 100mph; TCDL=6,0psf; BCDL=6,0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-4-13, Interior(1) 2-4-13 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1,60 plate grip DOL=1,60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) All plates are MT20 plates unless otherwise indicated.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads,
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 11) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

SEAL 16673

A. STRZ

August 1,2017 August 1,2017

leters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 19/03/2015 BEFORE USE. Design valid for use only with MTTekle connectors. This design is based only upon parameters shown, and is for an incividual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of incividual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of incividual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with passible personal nijury and property demage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and futures systems, see

ANSITTY Quality Criteria, DSB-89 and BCSI Building Component Safety Information, available from Truss Plate Institute, 218 N, Lee Street, Suite 312, Alexandria, VA 22314.



Job		Truss	Truss Type		Qty	Ply	Avery Roof			
LGI_HOMES		A4	Common		2	1				E10798398
D 414 - E1 40	4.5						Job Reference (	optional)		
Builders FirstSource,	Alber	narie , NC 28001			In this series	7.6	340 s Apr 19 2016 M	liTek Industries, Inc. Tue	e Aug 01 09:29:58	2017 Page 1
						753cwVVIII	euGga5gznu0U-7	sww1ph4P0Jd8B0E	LFScYAZO?Dt4	RitqG9AiJrysGmf
	11-0-0	5-10-14	10-8-6	17-0-0	23-3-10		28-1-2	34-0-0	35-0-0	
	1-0-0	5-10-14	4-9-9	6-3-10	8-3-10		4-9-9	5-10-14	1-0-0	

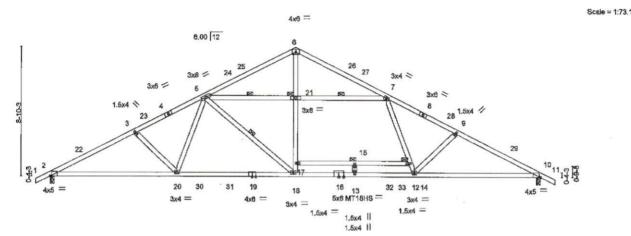


Plate Offsets (X,Y) [2:0-1-0,	8-9-4 8-9-4 Edge], [10:0-14,Edge]	8-2-12		21-1-6 4-1-6	25-2-12 4-1-6			34-0-0 8-9-4		****
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE Code IRC2009/TPI200	15 TC 15 BC S WB	0.74 0.68 0.57 ix)	DEFL. Vert(LL) Vert(TL) Horz(TL)	in -0.41 -0.74 0.13	(loc) 15 15 10	I/def  >975 >546 n/a	L/d 240 180 n/a	PLATES MT20 MT18HS Weight: 193 lb	GRIP 244/190 244/190 FT = 20%

BRACING-

WEBS

JOINTS

TOP CHORD

BOT CHORD

1 Row at midpt

1 Brace at Jt(s): 21, 14

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP DSS \*Except\*

14-17: 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=1743/0-3-8, 10=1851/0-3-8 Max Horz 2=-99(LC 8)

Max Uplift 2=-122(LC 10), 10=-99(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/26, 2-22=-3213/169, 3-22=-3141/198, 3-23=-2978/164, 4-23=-2917/173, 4-5=-2860/187, 5-24=-1232/30,

24-25=-1156/35, 6-25=-1136/53, 6-26=-1135/67, 26-27=-1136/49, 7-27=-1174/44, 7-8=-3138/131, 8-28=-3196/117, 9-28=-3257/107, 9-29=-3409/143, 10-29=-3487/114, 10-11=0/26

**BOT CHORD** 

2-20=84/2787, 20-30=9/2472, 30-31=-9/2472, 19-31=-9/2472, 18-19=-9/2472, 16-18=0/2619, 13-16=0/2619, 13-32=0/2619, 12-32=0/2619, 10-12=-42/3025, 15-17=-7/14, 15-33=-7/14, 14-33=-7/14 
17-18=0/545, 17-21=0/712, 6-21=0/762, 7-14=0/693, 12-14=0/589, 9-12=-240/152, 5-18=-146/562, 5-20=-7/437, WEBS

3-20=253/148, 5-21=-1711/204, 7-21=-1719/207, 13-15=-141/0

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. It; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-4-13, interior(1) 2-4-13 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load; Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pg=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pg=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pg=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pg=15.4 psf (flat roof load); Pg=15.4 psf (flat Pol-1-1.15); Pg=20.0 psf (ground snow); Pg=15.4 psf (flat roof load); Pg=15.4 psf (fla
- snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12,0 psf or 1,00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads

- 6) All plates are MT20 plates unless otherwise indicated.
  7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- 10) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss



Structural wood sheathing directly applied or 2-4-8 oc purlins.

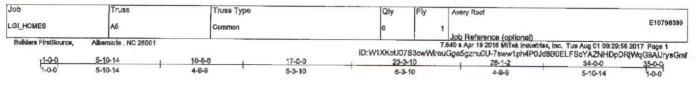
5-18, 5-21, 7-21

Rigid celling directly applied or 10-0-0 oc bracing. Except: 6-0-0 oc bracing: 14-17

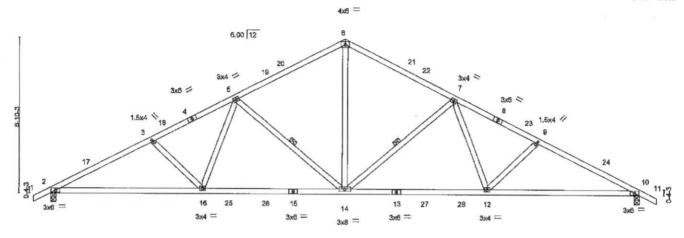
Hers and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 19/03/2015 BEFORE USE. Design valid for use only with MITelde connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Braining indicated is to prevent building designer must verify the uses well and/or chord members only. Additional temperary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general guidence regarding the fabilities of storage, delivery, erection and bracing of trusses and truss systems, see

AMSITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Piale Institute, 218 N, Lee Street, Suke 312, Alexandria, VA 22314.





Scale = 1:60.9



<u> </u>	8-9-4 8-9-4	17-0-0 8-2-12	25-2-12 8-2-12	34-0-0 8-9-4	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSL TC 0.78 BC 0.92 WB 0.46 (Matrix)	Vert(LL) -0.21 14-16 >8 Vert(TL) -0.47 14-16 >8	defi L/d PLATES 999 240 MT20 364 180 n/a n/a Welght: 175 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/stze) 2=1502/0-3-8, 10=1502/0-3-8

Max Horz 2=99(LC 9)

Max Uplift 2=-158(LC 10), 10=-158(LC 11)

FORCES. (ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/26, 2-17=-2705/247, 3-17=-2634/275, 3-18=-2466/242, 4-18=-2403/251, 4-5=-2347/266, 5-19=-1734/244,

19-20-1858/249, 6-20-1646/267, 6-21-1646/267, 21-22-1656/249, 7-22-1734/244, 7-8-2347/266, 8-23-2403/251,

9-23=-2466/242, 9-24=-2634/275, 10-24=-2705/247, 10-11=0/26 BOT CHORD 2-16=-153/2337, 16-25=-80/1994, 25-26=-80/1994, 15-26=-80/19

PRD 2-16=-153/2337, 16-25=-80/1994, 25-26=-80/1994, 15-26=-80/1994, 14-15=-80/1994, 13-14=-94/1994, 13-27=-94/1994,

27-28-94/1994, 12-28-94/1994, 10-12-159/2337

WEBS 6-14=-81/1109, 7-14=-700/146, 7-12=-2/484, 9-12=-267/147, 5-14=-700/146, 5-16=-2/464, 3-16=-267/147

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6,0psf; BCDL=6,0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-4-13, interior(1) 2-4-13 to 17-0-0, Exterior(2) 17-0-0 to 20-4-13 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (fiat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10,0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will
  fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

9) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

7-14, 5-14

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 rev. 10/03/2018 BEFORE USE.

Dealgn valid for use only with MTI-64® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must variety the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and parament bracing is always required for stability and to prevent collepse with possible personal righty and property damage. For general guidance regarding the fabrication, storage, delivery, section and tracing of trusses and frust systems, see ANSI/TPH Quality Criteria, DSB-89 and BC8I Building Component Sefety Information available from Truss Plate Institute, 218 N, Lee Street, Suits 312, Alexandria, VA 22314.



818 Soundate Road Edenton NC 27932 Job Truss Truss Type Avery Roof E10798400 LGI\_HOMES ARE Common Supported Gable | Job Reference (optional) 7.640 s Apr 19 2018 MT+sk Industries, Inc. Tue Aug 01 09:29:57 2017 Page 1 |ID:W1XKcU07S3cwWreuGga5gznu0U-b2T1F8llAKRUmLbQvzzzr4O6ledM7AFDzVpvGrlysGme Builders FirstSource ade NC 28001 1-0-0 34-0-0 35-0-0 17-0-0

Scale = 1:69.7

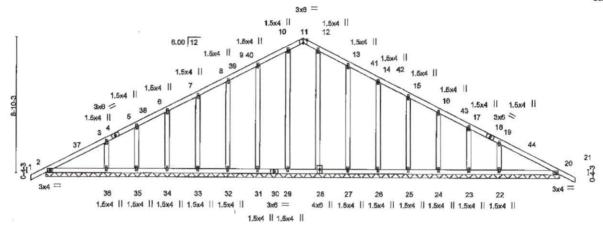


Plate Offsets (X,Y) [11:0-3-0.Edge]												
LOADING (psf TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL	20.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.17 0.11 0.17	DEFL. Vert(LL) Vert(TL) Horz(TL)	in 0.00 0.01 0.01	(loc) 21 21 20	I/defI n/r n/r n/a	⊥/d 120 120 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code IRC2009/TI	P12007	(Matr	ix)						Weight: 211 lb	FT = 20%

34-0-0

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=208/34-0-0, 29=153/34-0-0, 31=160/34-0-0, 32=161/34-0-0, 33=156/34-0-0, 34=176/34-0-0, 35=96/34-0-0, 36=310/34-0-0, 28=152/34-0-0, 27=159/34-0-0, 26=161/34-0-0, 25=156/34-0-0, 24=176/34-0-0, 23=96/34-0-0,

22=310/34-0-0, 20=208/34-0-0

Max Horz 2=99(LC 9)

Max Uplift 2=-19(LC 10), 31=-37(LC 11), 32=-16(LC 10), 33=-19(LC 11), 34=-22(LC 10), 35=-29(LC 10), 36=-84(LC 10), 27=-38(LC 11), 26=-15(LC 10), 25=-19(LC 10), 24=-22(LC 11), 23=-29(LC 11), 22=-84(LC 11), 20=-28(LC 11) Max Grav 2=208(LC 1), 29=180(LC 3), 31=197(LC 3), 32=161(LC 1), 33=156(LC 15), 34=176(LC 1), 35=06(LC 15), 35=06(LC 15) 36=310(LC 15), 28=178(LC 4), 27=195(LC 4), 26=161(LC 1), 25=156(LC 16), 24=176(LC 1), 23=96(LC 16), 22=310(LC 16), 20=208(LC 1)

FORCES. (ib) - Maximum Compression/Maximum Tension
TOP CHORD
1-2=0/25, 2-37=-152/8, 3-37=-139/46, 3-4=-83/30, 4-5=-80/39, 5-38=-61/43, 6-38=-56/52, 6-7=-45/74, 7-8=-44/121,
8-39=-44/157, 39-40=-18/164, 9-40=-6/166, 9-10=-51/224, 10-11=-44/208, 11-12=-44/209, 12-13=-50/227, 13-41=-4/169, 41-42--16/167, 14-42--42/159, 14-15--42/124, 16-16--43/77, 16-43--27/27, 17-43--39/20, 17-18--66/0, 18-19--69/0, 19-44=-141/47, 20-44=-154/0, 20-21=0/25

BOT CHORD

2-36=0/193, 35-36=0/193, 34-35=0/193, 33-34=0/193, 32-33=0/193, 31-32=0/193, 30-31=0/193, 29-30=0/193 28-29=0/193, 27-28=0/192, 26-27=0/192, 25-26=0/192, 24-25=0/192, 23-24=0/192, 22-23=0/192, 20-22=0/192 10-29=-138/0, 9-31=-167/153, 8-32=-120/77, 7-33=-118/81, 6-34=-129/89, 5-35=-80/58, 3-36=-218/167, 12-28=-138/0, 13-27=-157/153, 14-26=-120/77, 15-25=-118/81, 16-24=-129/89, 17-23=-80/58, 19-22=-218/167

# NOTES-

WERS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) -1-0-0 to 2-4-13, Exterior(2) 2-4-13 to 17-0-0, Corner(3) 17-0-0 to 20-4-13 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

  4) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1 5) Unbalanced snow loads have been considered for this design.

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.

8) Gable studs spaced at 2-0-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

WARNING - Varify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE Mil-1473 rev. 10/83/2016 BEFORE USE. ANAMONIC - Varily costin parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MB-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MilTelde connectors. This design is based only upon parameters shown, and is for an includual building component, not a luss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design is the overall building design. Bracing facilizated is to prevent bucking of individual truss web and/or chord members only. Additional emperary and permanent bracing is always required for stability and to prevent occlapse with possible personal righty and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITTEI Quality Criteria, DSB-89 and BGSI Building Component Safety Information available from Truss Piste Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Job	Truss	Truss Type	Qty	Ply	Avery Roof	
LGLHOMES	A6E	Common Supported Gable	1	1	E	10798400
Builders First Source Albert	20de NC 28001				Job Reference (optional)	

7.840 s Apr 19 2016 MilTak Industries, Inc. Tue Aug 01 09:29:58 2017 Page 2
ID:W1XKcU07S3cwWreuGga5gznu0U-3F1hSUjKxdZLNUAcTgU4dbelO1hMviT7kTfpNkysGrnd

NOTES.

NOTES11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 ib uplift at joint 2, 37 ib uplift at joint 31, 16 ib uplift at joint 32, 19 ib uplift at joint 33, 22 ib uplift at joint 34, 29 ib uplift at joint 35, 84 ib uplift at joint 27, 15 ib uplift at joint 26, 19 ib uplift at joint 25, 22 ib uplift at joint 24, 29 ib uplift at joint 23, 84 ib uplift at joint 22 and 28 ib uplift at joint 20.

12) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-7473 rev. 14/02/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an inclvidual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bradein includated is to prevent bucking of inclvidual truss web and/or chord members only. Additional temperary and permanent bracing is always required for stability and to prevent occlapse with possible personal injury and proper demand. For general guidance regarding the fabrication, storage, delivery, erection and bracing of inclusees and truss systems, see

ANSITIPI Quality Criteria, DSB-85 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandris, VA 22314.



Job	Truss	Truss Type	Qty	Ply	Avery Roof	
LGI_HOMES	G1	Monopitch Supported Gable	11	1	E10	0798401
Builders FirstSource, A	Ibemarle , NC 28001		ID:W1XKcU	7.6	Job Reference (optional) 40 s Apr 19 2018 MiTek Industries, Inc. Tue Aug 01 09:29:58 2017 Pag uGga5gznu0U-3F1h8UjKxdZLNUAcTgU4dbeiT1ZzykA7kTtpN	je 1
<u> </u>	-1-0-0		6-11-8		The state of the special state	hyooniu
	1-0-0		6-11-8			

Scale = 1:15,7

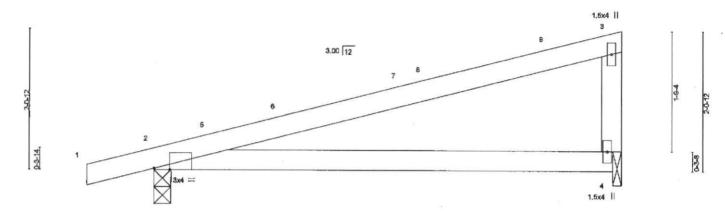


Plate Offsets (X,Y) [2:0-2-12,Edge] 6-11-8											
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.87 BC 0.58 WB 0.00 (Matrix)	DEFL. Vert(LL) Vert(TL) Horz(TL)	in -0.12 -0.30 0.00	(loc) 2-4 2-4	l/defl >669 >268 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 25 lb	GRIP 244/190 FT = 20%		

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

2x4 SP No.2

BRACING-

6-11-8

TOP CHORD

Structural wood sheathing directly applied or 2-2-1 oc purlins, except

end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=341/0-3-0, 4=262/0-1-8 Max Horz 2=78(LC 8)

Max Uplift 2=-89(LC 10), 4=-54(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/13, 2-5=-72/0, 5-6=-68/0, 6-7=-63/0, 7-8=-63/4, 8-9=-62/9, 3-9=-56/39, 3-4=-195/216

BOT CHORD 2-4=0/0

# NOTES-

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) -1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 6-9-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

  3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.16); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1; Min. flat roof snow load governs.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 12) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev., 10/03/2018 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracking inclosed is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, acvage, delivery, eredition and bracking of increases and inters systems, see \_\_ANSITHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Insitute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Edenton, NC 27932

Truss Truss Type Qty Avery Roof E10798402 LGI\_HOMES Monopitch Supported Gable Job Reference (optional)
7.840 s Apr 19 2016 MTiek Industries, Inc. Tue Aug 01 09:29:58 2017 Page 1
ID:WIXKcU07S3cwWfreuGga5gznu0U-3F1hSUfikxdZLNUAcTgU4dbepz1eov/kA7kTfpNkyeGmd Builders FirstSource arla NC 28001 1-0-0 5-0-0 1-0-0 5-0-0 Scale: 1"=1" 1.5x4 3,00 12 5 2 0-3-14 63-8 1.5x4 5-0-0 Piate Offsets (X,Y)- [2:0-2-12,Edge] LOADING (psf) SPACING-2-0-0 DEFL in (loc) Vdef! L/d PLATES GRIP TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.39 Vert(LL) -0.03 2-4 >999 240 Snow (Pf/Pg) 15.4/20.0 MT20 244/190 Lumber DOL 1.15 BC 0.28 Vert(TL) -0.07 2-4 >757 180 TCDL 10.0 Rep Stress Incr YES WR 0.00 Horz(TL) 0.00 n/a n/a BCII 0.0 Code IRC2009/TPI2007 (Matrix) Weight: 18 lb FT = 20% BCD 10.0 I LIMBER. BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except BOT CHORD 2x4 SP No.2 end verticals. 2x4 SP No.2 WEBS **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. REACTIONS. (lb/size) 2=265/0-3-0, 4=181/0-1-8

Max Horz 2=59(LC 8)

Max Uplift 2=-80(LC 8), 4=-37(LC 11)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/13, 2-5=-56/0, 5-6=-49/0, 3-6=-48/27, 3-4=-134/169

BOT CHORD 2-4=0/0

# NOTES-

- 1) Wind: ASCE 7-05; 100mph; TCDL=6,0psf; BCDL=6,0psf; h=25ff; Cat. II; Exp B; enclosed; MWFRS (low-rise) automatic zone and C-C Corner(3) -1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 4-10-4 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1; Min. flat roof snow load governs.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) Gable studs spaced at 2-0-0 oc.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 9) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 12) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MR-7473 rev. 10/03/2016 BEFORE USE. A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE AM-747.1 rev. TAWAZZO16 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing incleated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage, For general guidance regarding the fabrication, storage, delivery, arection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Roa Edenton, NC 27932

Job Truss Truss Type Avery Roof E10798403 LG1\_HOMES GABLE Job Reference (optional)
7.840 s Apr 19 2016 MITek Industries, Inc. Tue Aug 01 09:29:59 2017 Page 1 Builders FirstSource de NC 28001 ID:W1XKcU07S3cwWfreuGga5gznu0U-XRb3gqkyixhC?elp10?J9pB4lR2eeBzGy7OMwAysGmc 11-10-14 5-11-7

3x4 =

Scale = 1:24.6

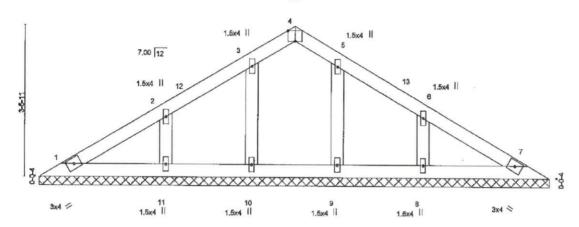


Plate Offsets (X,Y)-- [4:0-2-0,Edge], [5:0-0-0,0-0-0], [6:0-0-0,0-0-0] LOADING (psf) SPACING-CSI. DEFL in (loc) l/defl 1/4 PLATES GRIP TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.06 Vert(LL) n/a n/a 999 MT20 244/190 Snow (Pf/Pa) 15.4/20.0 Lumber DOL 1.15 0.04 BC Vert(TL) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.03 Horz(TL) 0.00 n/a n/a BCLL. 0.0 Code IRC2009/TPI2007 (Matrix) Weight: 47 lb FT = 20%BCD 10.0

11-10-14

LUMBER-

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS, (lb/size) 1=91/11-10-14, 7=91/11-10-14, 10=134/11-10-14, 11=208/11-10-14, 9=134/11-10-14, 8=208/11-10-14

Max Horz 1=91(LC 8)

Max Uplift 10=-19(LC 10), 11=-76(LC 10), 9=-14(LC 11), 8=-77(LC 11)

Max Grav 1=91(LC 1), 7=91(LC 1), 10=134(LC 1), 11=209(LC 14), 9=134(LC 1), 8=209(LC 15)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-76/41, 2-12=-61/22, 3-12=-27/33, 3-4=-50/42, 4-5=-50/41, 5-13=-26/21, 6-13=-61/12, 6-7=-63/23

BOT CHORD WEBS

1-11=14/74, 10-11=14/74, 9-10=-14/74, 8-9=-14/74, 7-8=-14/74 3-10=-99/44, 2-11=-149/97, 5-9=-99/42, 6-8=-149/98

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 5-11-7, Exterior(2) 5-11-7 to 8-11-7 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct≈1.1
- 4) Unbalanced snow loads have been considered for this design,

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing waits due to UPLIFT at Jt(s) 10, 11, 9, and 8. This connection is for uplift only and does not consider lateral forces.

  9) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



MARNING - Verify design paremeters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damages. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITTPI Quality Criteris, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N, Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type		Qty	Ply	Avery	Roof			
LGI_HOMES	V2	Valley		1	1					E10798404
Builders FirstSource,	Albemarle , NC 28001				1 7	Job R 640 s Ar	eference or 19 2016	(optional) MITek Industria	s, Inc. Tue Aug 01 09:29:59 ??elp10?J9pB0iR0AeB	9 2017 Page 1
		4-2-14	I	D:W1XKcU07	S3cwWin	euGga5	gznuOU-)	(Rb3gqkyixh(	?elp10?J9pB0iR0AeB	oGy7OMwAysGr
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	1								<b>**</b>	_
1 \$	$\bigotimes$	XXXXXXXXX	XXXXXXXXXXXX	XXXX	XXX	XX		XXXX	XXXXX	ģ.
			4							
	2x4 //		1.6x4					2x4 >		
	0-9-7		8-5-11						147	
	C-9-7 D-0-7		8-5-5							
LOADING (psf) TCLL (roof) 20	SPACIN		CSI.	DEFL.	In	(loc)	l/defi	L/ď	PLATES	GRIP
Snow (Pf/Pg) 15.4/20			TC 0.26 BC 0.14	Vert(LL)	n/a	-	n/a	999	MT20	244/190
	).0 Pen Str		WB 0.04	Vert(TL) Horz(TL)	n/a 0. <b>0</b> 0	3	n/a n/a	n/a		
	0.0 Code IR	C2009/TPI2007	(Matrix)	,			1.0.25		Weight: 28 lb	FT = 20%
LUMBER-				CING-					-	
TOP CHORD 2x4 SI				CHORD	Structur	al wood	sheath	ing directly a	pplied or 6-0-0 oc pur	tins.

**OTHERS** 

2x4 SP No.3

BOT CHORD

Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=155/8-4-14, 3=155/8-4-14, 4=282/8-4-14

Max Horz 1=-62(LC 8)
Max Uplift 1=-43(LC 10), 3=-46(LC 11), 4=-11(LC 10)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-5=-76/27, 5-6=-32/34, 6-7=-30/34, 2-7=-19/43, 2-8=-9/40, 8-9=-30/32, 9-10=-32/31, 3-10=-76/24

BOT CHORD 1-4=-12/29, 3-4=-12/29

WEBS 2-4=-190/68

## NOTES-

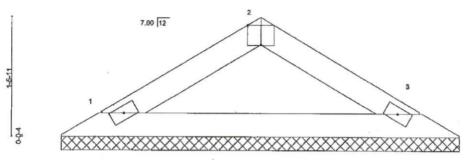
- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-6-8 to 3-6-8, interior(1) 3-6-8 to 4-2-14, Exterior(2) 4-2-14 to 7-2-14 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grlp DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1
- Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at II(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at ii(s) 4. This connection is for uplift only and does not consider lateral forces.
- 10) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



tions and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 19/03/2015 BEFORE USE. Design valid for use only with MiTakte connectors. This design is based only upon parameters shown, and is for an individual busining component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall busining design. Braining indicated is to prevent busining of individual truss web and/or chard members only. Additional temporary and permanent bracing is always required for stability and to prevent collegee with possible personal injury and proper only designer on the property of th



300	Truss	Truss Type	Qty	Ply	Avery Roof
LGLHOMES	V3	Valley	1	1	E10798406
Builders FirstSource,	Albemarle , NC 28001			7	Job Reference (optional) .840 s Apr 19 2016 MITek Industries, Inc. Tue Aug 01 09:30:00 2017 Page 1
		2-6-5	ID:W1XKc	U07S3cwWfrei	uGge5gznu0U-?d9RtAkbTFp3doK?a5XYt0jEQrLWNegQBn8wSdyaGmb 6-0-9
		2-8-5	1		2-8-5
			3x4 =		Scale = 1:13.2
			2		



2x4 =

2x4 >

	0-0-7 0-0-7	5-0-9 5-0-2							
Plate Offsets (X,Y)- [2:0-2-0,	Edgel								
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 *	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES	CSI, TC 0.07 BC 0.20 WB 0.00	DEFL. Vert(LL) Vert(TL) Horz(TL)	In n/a n/a 0.00	(loc) - - 3	Vdefi n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code IRC2009/TPI2007	(Matrix)						Weight: 14 lb	FT = 209

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-9 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/slze) 1=159/4-11-11, 3=159/4-11-11 Max Horz 1=-33(LC 6)

Max Uplift 1=-28(LC 8), 3=-28(LC 8)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-142/64, 2-3=-142/64

BOT CHORD 1-3=-27/102

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) automatic zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1,60 plate grip DOL=1.60
- TGLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1; Min. flat roof snow load governs.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) One MTS12 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

  8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.



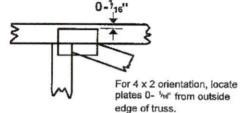


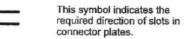
# **Symbols**

# PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.





\* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

#### BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

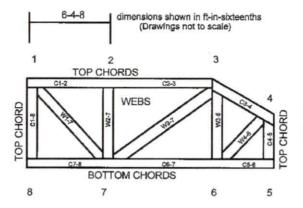
DSB-89: Design Standard for Bracing.

DSB-89 BCSI:

Building Component Safety Information,

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: Mil-7473 rev. 10/03/2015



# **General Safety Notes**

# Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use, Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

