



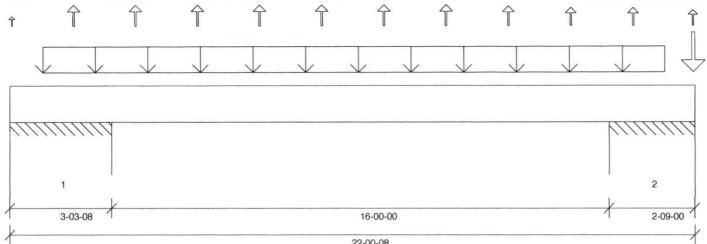
Member Type: Beam | Level: 1st Floor MiTek SAPPHIRE™ Structure Version 8.2.0.246.Update9 Designed by Single Member Design Engine

Member: 2 - 2.0 RigidLam LVL 1-3/4 x 14

Label: BM1-i40

Page: 1 of 6 Date: 02/25/2019 10:30:30

Status: Design Passed



22-00-08

Graphical Illustration - Not To Scale Member Cut Length - 22'- 1/2" MemberPitch - 0/12

Building Code: Design Methodology:	ASD		Floor Dead Load: Floor Live Load: Unbraced Length	10.0 lb/ft² 40.0 lb/ft² Top: 1'- 10 1/2"	Roof Li	ead Load: ve Load: : 16'	10.0 lb/ft² 20.0 lb/ft²	Ground S	Snow Load:	0.0 lb/ft²	
Design Resu	ults:										
		Location	De	sign	Co	ntrol		Result	LDF	Load Combination	
Critical Moment (Pos Critical Moment (Neg Critical Moment (Neg	g)	19'- 5"	0.0	.83 lb ft 0 lb ft 0 lb ft	0.00	0.06 lb ft 0 lb ft 0 lb ft		Passed - 34%	1.25	D + Lr	
Critical Shear		18'- 1 1/2"	3437	7.69 lb	1184	1.67 lb		Passed - 29%	1.25	D + Lr	
Live Load Deflection	(11'- 2 15/16	" 0'-	1/16"	N/A (L/480)		Passed - L/999	-	Lr	
Total Load Deflection	n	11'- 2 15/16	0'-	1/8"	N/A (L/240)		Passed - L/999	-	D+Lr	
Max. Reaction					Supported Mtl	Supporting	Mtl				
		0'- 1 1/2"	-263	4.24 lb	18375.00 lb			Passed - 15%	1.25	D+Lr	
		3'- 2"	6998	3.88 lb	18375.00 lb	17762.50) lb	Passed - 39%	1.25	D + Lr	
		19'- 5"	7675	5.04 lb	18375.00 lb	17762.50	lb	Passed - 43%	1.25	D + Lr	
		21'- 11"	-312	5.38 lb	18375.00 lb			Passed - 18%	1.25	D + Lr	

Design Notes:

Design Information:

Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

ading:							
					Maximum Loa	ad Magnitudes	
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	22'- 1/2"	Self Weight	13 lb/ft	-		
Uniform	1'- 3/4"	21'- 3/4"	Smoothed Load	206 lb/ft	140	222 lb/ft	2
Point	0'- 3/4"	0'- 3/4"	A1GE(c01)		(#3)	-1.00 lb	
Point	2'- 3/4"	2'- 3/4"	A1(c01)	*	-	-7.00 lb	2
Point	4'- 3/4"	4'- 3/4"	A1(c03)			-7.00 lb	
Point	6'- 3/4"	6'- 3/4"	A1(c08)	3	-	-7.00 lb	
Point	8'- 3/4"	8'- 3/4"	A1(c09)	*	(4)	-7.00 lb	
Point	10'- 3/4"	10'- 3/4"	A1(c02)			-7.00 lb	
Point	12'- 3/4"	12'- 3/4"	A1(c07)	2	(a)	-7.00 lb	2
Point	14'- 3/4"	14'- 3/4"	A1(c11)		26.0	-7.00 lb	
Point	16'- 3/4"	16'- 3/4"	A1(c10)		-	-7.00 lb	
Point	18'- 3/4"	18'- 3/4"	A1(c05)	*	-	-7.00 lb	
Point	20'- 3/4"	20'- 3/4"	A1(c04)	2	-	-7.00 lb	

Support Information:

21'- 11 3/4"

				Maximum Analysis Reactions						
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow			
1	0'	3'- 3 1/2"	E15(i39)	3471.00/-1216.00 lb		3711.00/-1473.00 lb	2			
==>	0'- 1 1/2"	0'- 1 1/2"	E15(i39)	-1216.00 lb	-	178.00/-1417.00 lb				
==>	3'- 2"	3'- 2"	E15(i39)	3471.00 lb		3533.00/-56.00 lb	2			
2	19'- 3 1/2"	22'- 1/2"	E14(i98)	3811.00/-1328.00 lb		4300.00/-1868.00 lb	*			
==>	19'- 5"	19'- 5"	E14(i98)	3811.00 lb		3881.00/-61.00 lb				
==>	21'- 11"	21'- 11"	E14(i98)	-1328.00 lb		419.00/-1807.00 lb				

330.00 lb

310.00/-8.00 lb

Errors, Warnings & Notes:

The dead loads used in the design of this member were applied to the structure as sloped dead loads.

21'- 11 3/4"

- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

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⁻ Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.



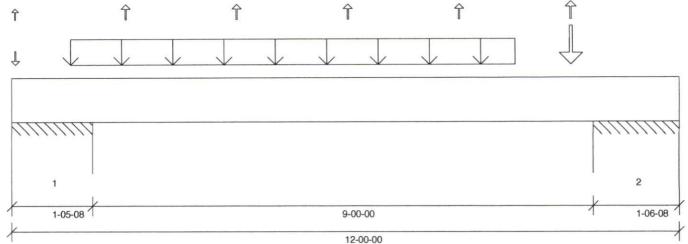
Member Type: Beam I Level: 1st Floor MiTek SAPPHIRE™ Structure Version 8.2.0.246.Update9 Designed by Single Member Design Engine

Member: 2 - 2.0 RigidLam LVL 1-3/4 x 9-1/2

Label: BM2-i42

Page: 2 of 6 Date: 02/25/2019 10:30:31

Status: Design Passed



Graphical Illustration - Not To Scale Member Cut Length - 12' MemberPitch - 0/12

Design into	IIIIauoii.	
Building Code:	IRC 2012	Floor De
Design Methodology	: ASD	Floor Liv
		Linkson

cian Information

ead Load: 10.0 lb/ft² 40.0 lb/ft² ve Load: Unbraced Length Top: 1'- 10 1/2" Roof Dead Load: 10.0 lb/ft² Roof Live Load: 20.0 lb/ft² Bottom: 9'

Ground Snow Load:

0.0 lb/ft²

D	esi	ign	Results:

	Location	Design	Cor	ntrol	Result	LDF	Load Combination
Critical Moment (Pos)	1'- 4"	4585.00 lb ft	16776	.02 lb ft	Passed - 27%	1.25	D+Lr
Critical Moment (Neg)		0.00 lb ft	0.00) lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00) lb ft			
Critical Shear	2'- 3"	2584.79 lb	8035	.42 lb	Passed - 32%	1.25	D+Lr
Live Load Deflection	6'- 6 3/4"	0'- 1/16"	N/A (L/480)	Passed - L/999	-	Lr
Total Load Deflection	6'- 6 5/8"	0'- 1/16"	N/A (L/240)	Passed - L/999	-	D+Lr
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 1 1/2"	-3703.87 lb	18375.00 lb		Passed - 21%	1.25	D+Lr
	1'- 4"	6607.72 lb	18375.00 lb	17762.50 lb	Passed - 37%	1.25	D+Lr
	10'- 7"	2290.17 lb	18375.00 lb	17762.50 lb	Passed - 13%	1.25	D+Lr
	10'- 7"	-446.92 lb	18375.00 lb		Passed - 3%	1.60	0.6D + 0.6W
	11'- 10 1/2"	520.28 lb	18375.00 lb	17762.50 lb	Passed - 3%	1.60	0.6D + 0.6W
	11'- 10 1/2"	-307.87 lb	18375.00 lb		Passed - 2%	1.60	D + 0.6W

Design Notes:

Loading:

					Maximum Loa	d Magnitudes		
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow	-
Self Weight	0'	12'	Self Weight	9 lb/ft	(*)			
Uniform	1'- 3/4"	9'- 3/4"	Smoothed Load	237 lb/ft	Part	226 lb/ft	2	
Point	0'- 3/4"	0'- 3/4"	T1GE(c01)	80.00 lb	0.00	106.00 lb		
Point	2'- 3/4"	2'- 3/4"	T1(c05)		•	8		
Point	4'- 3/4"	4'- 3/4"	T1(c01)		(¥)	-	×	
Point	6'- 3/4"	6'- 3/4"	T1(c02)		0.00			
Point	8'- 3/4"	8'- 3/4"	T1(c03)	(4)	-	9	9	
Point	10'- 3/4"	10'- 3/4"	T1(c04)	602.00 lb		665.00 lb	-	

Support Information:

					Maximum An	alysis Reactions	
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	1'- 5 1/2"	E3(i41)	3435.00/-1880.00 lb	(10)	3313.00/-1850.00 lb	œ
==>	0'- 1 1/2"	0'- 1 1/2"	E3(i41)	-1880.00 lb	-	118.00/-1838.00 lb	-
==>	1'- 4"	1'- 4"	E3(i41)	3435.00 lb		3195.00/-12.00 lb	
2	10'- 5 1/2"	12'	E2(i21)	1129.00 lb	-	1117.00 lb	
==>	10'- 7"	10'- 7"	E2(i21)	1129.00 lb		1117.00 lb	i e
=>	11'- 10 1/2"	11'- 10 1/2"	E2(i21)	-	12		•

- CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member. * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

^{*} Member design assumed proper ply to ply connection. Verify connection between plies according to code specification



Member Type: Beam | Level: 1st Floor

MiTek SAPPHIRE™ Structure Version 8.2.0.246.Update9 Designed by Single Member Design Engine

Member: 3 - 2.0 RigidLam LVL 1-3/4 x 14

Label: BM3-i104

Page: 3 of 6 Date: 02/25/2019 10:30:31

Status: Design Passed



Graphical Illustration - Not To Scale Member Cut Length - 23'- 6 1/4" MemberPitch - 0/12

Design infor	matio	11.									
Building Code: Design Methodology:	IRC 2012 ASD	2	Floor Dead Load: Floor Live Load: Unbraced Length	10.0 lb/ft² 40.0 lb/ft² Top: 14'- 7 1/4	Roof Liv		10.0 lb/ft² 20.0 lb/ft²	Ground S	Snow Load:	0.0 lb/ft²	
Design Resu	ults:										
		Location	n <u>De</u>	sign	Co	ntrol		Result	LDF	Load Combination	
Critical Moment (Pos	s)	7'- 3/4"	7560	.76 lb ft	52958	1.15 lb ft		Passed - 14%	1.25	D + 0.75(L + Lr)	
Critical Moment (Neg	g)		0.0	0 lb ft	0.00	0 lb ft					
Critical Moment (Neg	g)		0.0	0 lb ft	0.00	0 lb ft					
Critical Shear		8'- 4 1/2"	671	3.97 lb	1776	2.50 lb		Passed - 38%	1.25	D + Lr	
Live Load Deflection	1	14'- 5/8"		0'	N/A (L/480)		Passed - L/999	×	0.75(L + Lr + 0.6W)	
Total Load Deflection	n	14'- 4"	0'-	1/16"	N/A (L/240)		Passed - L/999		D + 0.75(L + Lr + 0.6W)	
Max. Reaction					Supported Mtl	Supporting	MtI				
		0'- 2 1/2"	135	5.73 lb	15257.79 lb	7809.37 II	b	Passed - 17%	1.00	D+L	
		0'- 2 1/2"	-457	7.27 lb	15257.79 lb	-		Passed - 6%	1.25	D + Lr	
		7'- 3/4"	7718	8.15 lb	15257.78 lb	7809.36 II	b	Passed - 99%	1.00	D+L	
		23'- 3 3/4"	389	.94 lb	15257.96 lb	7809.46 II	b	Passed - 5%	1.25	D+Lr	

Design Notes:

Decian Information

^{*} Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

L				

					Maximum Lo	ad Magnitudes	
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	23'- 6 1/4"	Self Weight	19 lb/ft			
Point	0'- 10"	0'- 10"	F1(c02)	295.00 lb	801.00 lb	2	2
Point	2'- 10"	2'- 10"	F1(c04)	295.00 lb	801.00 lb	*	
Point	4'- 10"	4'- 10"	F1(c01)	295.00 lb	801.00 lb		
Point	6'- 10"	6'- 10"	F1(c03)	299.00 lb	817.00 lb		
Point	8'- 8 3/8"	8'- 8 3/8"	BM4(i105)	3983.00 lb	413.00 lb	2818.00/-3.00 lb	6

Support Information:

			_	Maximum Analysis Reactions					
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow		
1	0'	0'- 3 1/2"	5(i36)	-82.00 lb	1389.00/-61.00 lb	*			
2	6'- 11"	7'- 2 1/2"	10(i44)	5427.00 lb	2327.00 lb	2523.00/-3.00 lb	2		
3	23'- 2 3/4"	23'- 6 1/4"	1(i25)	287.00 lb	18.00/-39.00 lb	296.00 lb			

- * The dead loads used in the design of this member were applied to the structure as sloped dead loads.
 * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

⁻ This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.



Member Type: Beam I Level: 1st Floor

MiTek SAPPHIRE™ Structure Version 8.2.0.246.Update9

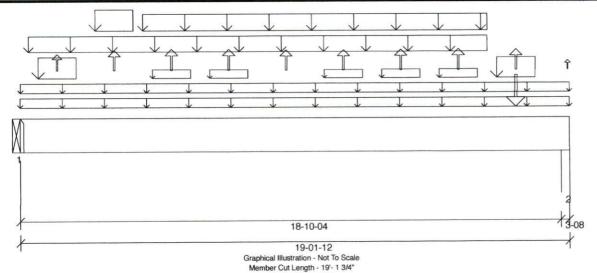
Designed by Single Member Design Engine

Member: 3 - 2.0 RigidLam LVL 1-3/4 x 14

Label: BM4-i105

Page: 4 of 6 Date: 02/25/2019 10:30:31

Status: Design Passed



Design Information:

IRC 2012 Building Code: IRC 2
Design Methodology: ASD

Floor Dead Load: Floor Live Load:

40.0 lb/ft² Unbraced Length Top: 0'

10.0 lb/ft²

Roof Dead Load: Roof Live Load: Bottom: 1'- 10 1/2"

MemberPitch - 0/12

10.0 lb/ft²

20.0 lb/ft²

Ground Snow Load:

0.0 lb/ft²

Design Results:							
	Location	Design	Contr	rol	Result	LDF	Load Combination
Critical Moment (Pos)	9'- 3 1/2"	33047.09 lb ft	54322.76 lb ft		Passed - 61%	1.25	D + Lr
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb	oft .			
Critical Shear	1'- 2"	6403.55 lb	17762.5	i0 lb	Passed - 36%	1.25	D+Lr
Live Load Deflection	9'- 5 1/2"	0'- 3/8"	N/A (L/4	180)	Passed - L/596		Lr
Total Load Deflection	9'- 5 9/16"	0'- 15/16"	N/A (L/2	240)	Passed - L/248		D+Lr
Max. Reaction			Supported Mtl S	Supporting Mtl			
	0'	6800.64 lb	6800.64 lb	0.00 lb	Passed - 100%	1.25	D+Lr
	18'- 11 1/4"	6701.38 lb	15257.96 lb	7809.46 lb	Passed - 86%	1.25	D+Lr

Design Notes:

Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

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					Maximum Loa	nd Magnitudes	
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	19'- 1 3/4"	Self Weight	19 lb/ft		2	
Uniform	0'	19'- 1 3/4"	4(i31)	65 lb/ft		(*)	95
Uniform	0,	19'- 1 3/4"	FC1 Floor Material	10 lb/ft	42 lb/ft	-	-
Uniform	0'- 3 1/2"	16'- 3 1/2"	Smoothed Load	157 lb/ft	9	142 lb/ft	*
Uniform	0'- 7 1/2"	1'- 11 1/2"	4(i31)	271 lb/ft		250 lb/ft	*
Uniform	2'- 7 1/2"	3'- 11 1/2"	4(i31)	268 lb/ft		246 lb/ft	*
Uniform	4'- 3 1/2"	16'- 3 1/2"	-	178 lb/ft		164 lb/ft	51
Uniform	4'- 7 1/2"	5'- 11 1/2"	4(i31)				2
Uniform	6'- 7 1/2"	7'- 11 1/2"	4(i31)		-	3 m	•
Uniform	10'- 7 1/2"	11'- 11 1/2"	4(i31)	-		-	
Uniform	12'- 7 1/2"	13'- 11 1/2"	4(i31)	-	4	-	
Uniform	14'- 7 1/2"	15'- 11 1/2"	4(i31)		(*)		
Uniform	16'- 7 1/2"	17'- 11 1/2"	4(i31)	260 lb/ft		232 lb/ft	2
Point	1'- 3 1/2"	1'- 3 1/2"		2		-1.00 lb	*
Point	3'- 3 1/2"	3'- 3 1/2"	(-)	-		-1.00 lb	₽
Point	5'- 3 1/2"	5'- 3 1/2"	3	-	-	-2.00 lb	*
Point	7'- 3 1/2"	7'- 3 1/2"			272	1.50	
Point	9'- 3 1/2"	9'- 3 1/2"	-		(#)		
Point	11'- 3 1/2"	11'- 3 1/2"	-		(*)	270	
Point	13'- 3 1/2"	13'- 3 1/2"				123	*
Point	15'- 3 1/2"	15'- 3 1/2"				Per sea	
Point	17'- 3 1/2"	17'- 3 1/2"		306.00 lb	•	271.00 lb	
Point	10'- 1"	19'- 1"	4(i31)	(4)	141		

Support Information:

1-1					Maximum Ana	alysis Reactions	
Support	Start o'	End o'	Source BM3(i104)	Dead 3983.00 lb	Floor Live 413.00 lb	Roof Live 2818.00/-3.00 lb	Snow
2	18'- 10 1/4"	19'- 1 3/4"	E12(i18)	3974.00 lb	403.00 lb	2728.00/-1.00 lb	4



Member Type: Beam I Level: 1st Floor MiTek SAPPHIRE™ Structure Version 8.2.0.246.Update9 Designed by Single Member Design Engine

Member: 3 - 2.0 RigidLam LVL 1-3/4 x 14

Label: BM4-i105

Page: 5 of 6 Date: 02/25/2019 10:30:31

Status: Design Passed

Connector Information:

				Vailing Requirement	S		
Support	Manufacturer	Model	Тор	Face	Member	Length	Other Information Connector has not been
1	N/A	N/A	N/A	N/A	N/A	N/A	designed. No information is available.

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
- * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Member Type: Beam I Level: 1st Floor MiTek SAPPHIRE™ Structure Version 8.2.0.246.Update9

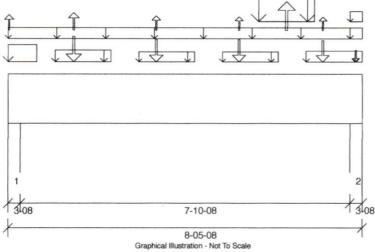
Designed by Single Member Design Engine

Member: 2 - 2.0 RigidLam LVL 1-3/4 x 14

Label: BM5-i106

Page: 6 of 6 Date: 02/25/2019 10:30:32

Status: Design Passed



Member Cut Length - 8'- 5 1/2" MemberPitch - 0/12

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Building Code: IRC 2012 Floor Dead Load: 10.0 lb/ft² Roof Dead Load: 10.0 lb/ft² Ground Snow Load: 0.0 lb/ft2 Design Methodology: ASD Floor Live Load: 40.0 lb/ft² Roof Live Load: 20.0 lb/ft² Unbraced Length Top: 0' Bottom: 1'- 10 1/2"

Design Results:

	Location	Design	Control	Result	LDE	Load Combination
				ricsuit	LDF	Load Combination
Critical Moment (Pos)	5'- 3 1/8"	2381.21 lb ft	36215.18 lb ft	Passed - 7%	1.25	D+Lr
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	7"	949.71 lb	11841.67 lb	Passed - 8%	1.25	D+Lr
Live Load Deflection	4'- 4 7/16"	0'	N/A (L/480)	Passed - L/999	-	Lr
Total Load Deflection	4'- 4 1/8"	0'	N/A (L/240)	Passed - L/999	-	D+Lr
Max. Reaction			Supported Mtl Supporting Mtl			
	0'- 2 1/2"	1221.35 lb	10171.87 lb 5206.25 lb	Passed - 23%	1.25	D+Lr
	8'- 3"	1578.24 lb	10171.88 lb 5206.26 lb	Passed - 30%	1.25	D + Lr

Design Notes:

Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes					
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow		
Self Weight	0'	8'- 5 1/2"	Self Weight	13 lb/ft		-	·		
Uniform	-0'	8'- 5 1/2"	E18(i30)	65 lb/ft					
Uniform	-0'	0'- 8 1/4"	E18(i30)	105 lb/ft		164 lb/ft	2		
Uniform	1'- 1 1/2"	2'- 5 1/2"	E18(i30)	52 lb/ft		42 lb/ft	-		
Uniform	3'- 1 1/2"	4'- 5 1/2"	E18(i30)	25 lb/ft		U=0			
Uniform	5'- 1 1/2"	6'- 5 1/2"	E18(i30)	35 lb/ft	121	120	-		
Uniform	5'- 11 13/16"	7'- 3 13/16"	E18(i30)	235 lb/ft		318 lb/ft			
Uniform	7'- 1 1/2"	8'- 5 1/2"	E18(i30)	67 lb/ft			2		
Uniform	8'- 2"	8'- 5 1/2"	FC1 Floor Material	-	43 lb/ft	-	-		
Point	0'- 1/4"	0'- 1/4"	E18(i30)	-	-	-	-		
Point	1'- 6 13/16"	1'- 6 13/16"		78.00 lb		178.00 lb	-		
Point	3'- 6 1/2"	3'- 6 1/2"		60.00 lb	-	146.00/-14.00 lb			
Point	5'- 6 1/4"	5'- 6 1/4"	T3(c02)	60.00 lb	-	146.00 lb	¥		
Point	6'- 7 13/16"	6'- 7 13/16"	E18(i30)	1*		· ·	-		
Point	7'- 6 1/4"	7'- 6 1/4"	T3(c04)	56.00 lb		138.00 lb	2		
Point	8'- 3 3/4"	8'- 3 3/4"	FC1 Floor Material	48.00 lb	-		<u></u>		

Support Information:

				Maximum Analysis Reactions				
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow	
1	0"	0'- 3 1/2"	6(i35)	685.00 lb	· ·	543.00/-8.00 lb		
2	8'- 2"	8'- 5 1/2"	7(i37)	903.00 lb	13.00 lb	669.00/-6.00 lb		

- The dead loads used in the design of this member were applied to the structure as sloped dead loads.
- * The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.

 * Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

⁻ Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Actual field conditions may differ from those shown. These results should be reviewed by a qualified design professional.