

RE: 1110004_OFA - H&H-NC/Topsail/ Site Information: Project Customer: H and H Project Name: 11100 Lot/Block: A Subdivity Model: Address: All City: Fayetteville State: I General Truss Engineering Criteria & Design Load Drawings Show Special Loading Conditions): Design Code: IRC2009/TPI2007 Wind Code: ASCE 7-05 Wind Speed: 130 mph Roof Load: 40.0 psf Mean Roof Height (feet): 25	Trenco 818 Soundside Rd Edenton, NC 27932 O4 ion: IC ds (Individual Truss Design) Design Program: MiTek 20/20 7.6 Design Method: MWFRS(low-rise)/C-C hybrid Wind ASCE 7-05 Floor Load: N/A psf Exposure Category: C
No.Seal#Truss NameDateNo.Seala1 130128612 A01 $6/12/17$ 35 13012 2 130128613 A02 $6/12/17$ 36 13012 3 130128614 A03 $6/12/17$ 37 13012 4 130128615 A04 $6/12/17$ 37 13012 5 130128616 A05 $6/12/17$ 39 13012 6 130128617 A06 $6/12/17$ 39 13012 6 130128619 A08 $6/12/17$ 39 13012 7 130128620 A09 $6/12/17$ 39 13012 9 130128621 A10 $6/12/17$ $6/12/17$ 10 130128623 A12 $6/12/17$ 7 11 130128624 A13 $6/12/17$ 13 130128625 A14 $6/12/17$ 14 130128627 B02 $6/12/17$ 15 130128628 B03 $6/12/17$ 16 130128629 B04 $6/12/17$ 17 130128632 C02 $6/12/17$ 18 130128632 C02 $6/12/17$ 20 130128633 C03 $6/12/17$ 21 130128634 CP01 $6/12/17$ 22 130128637 J02 $6/12/17$ 23 130128639 J04 $6/12/17$ 24 130128636 J01 $6/12/17$ 25 130128639 J04 $6/12/17$ 26 130128644 J05 $6/12/17$ 27	Truss Name Date 646 J11 6/12/17 648 V01 6/12/17 649 V02 6/12/17 650 V03 6/12/17
The truss drawing(s) referenced above have been prepare MiTek USA, Inc. under my direct supervision based on provided by Builders FirstSource-Sumter,SC. Truss Design Engineer's Name: Komnick, Chad My license renewal date for the state of North Carolina i IMPORTANT NOTE: The seal on these truss component desi that the engineer named is licensed in the jurisdiction(s) identified an designs comply with ANSI/TPI 1. These designs are based upon pa shown (e.g., loads, supports, dimensions, shapes and design codes) given to MiTek or TRENCO. Any project specific information include TRENCO's customers file reference purpose only, and was not taker preparation of these designs. MiTek or TRENCO has not independe applicability of the design parameters or the designs for any particular	d by ne parameters December 31, 2017 ns is a certification that the ameters which were d is for MITek's or into account in the ty verified the building. Before use,

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 12,2017

Komnick, Chad



besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

A MiTek Affiliate 818 Soundside Road Edenton, NC 27932







June 12,2017

ENGINEERING BY EREPACED A MITek Atfiliate 818 Soundside Road Edenton, NC 27932



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TRENCIO AMITERATINATION

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Topsail/
1110004_OFA	A05	Roof Special	8	1	130128616
					Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s	Sep 29 2015 MiTek Industries, Inc. Mon Jun 12 13:03:25 2017 Page 2
		ID:X	87q8Dmm	5MnjRaL0	GAgDs0yzEGbe-Nchyfcd?RcOigt3n81Tu5Jw8mBC1eA1ZOhBreuz70YG

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: A-E=-60, E-I=-60, M-R=-20, L-M=-20, L-U=-20



Job	Truss	Truss Type	Qty	Ply	H&H-NC/Topsail/
1110004 OFA	A06	ROOF SPECIAL	8	1	130128617
			-		Job Reference (optional)
Builders FirstSource	Sumter SC 29153			7 640 s	Sep 29 2015 MiTek Industries Inc. Mon Jun 12 13:03:26 2017 Page 1

ID:X87q8Dmm5MnjRaLGAgDs0yzEGbe-roFKsyedCwWZI1dzil_7eXSM7bbnNZRjdLwPAKz70YF 19-11-8 -0<u>-10+8</u> 0-10-8 7-9-0 15-2-8 7-9-0 7-5-8 4-9-0 8.00 12 2x4 || 3x8 MT20HS 🚧 3x5 💋 D С 10-8-12 7-6-12 1-11-0 Μ 2x4 =2x4 = 5x6 = ^H 2x4 || Т G 4x5 || 5x6 = 2x4 || 6x8 = 5x6 ⋍ 4.00 122x4

Scale: 1/8"=1'



Plate Off	fsets (X,Y)	[<u>B:0-1-5,0-1-8], [J:0-4-0,Edge]</u>			
LOADIN	G (psf)	SPACING- 2-0-12	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.63	Vert(LL) -0.21 G-H >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.62	Vert(TL) -0.35 G-H >679 240 MT20HS 187/143	
BCLL	0.0 *	Rep Stress Incr NO	WB 0.87	Horz(TL) -0.09 G n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL) 0.13 J >999 240 Weight: 148 lb FT = 20%	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1 *Except*	TOP CHORD	Structural wood sheathing directly applied or 4-8-0 oc purlins, except
	E-F: 2x4 SP No.2, A-D: 2x4 SP SS		end verticals.
BOT CHORD	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 5-1-9 oc bracing.
	B-J: 2x6 SP No.2	WEBS	1 Row at midpt E-I, E-G
WEBS	2x4 SP No.3 *Except*		

WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) B=873/0-5-8, G=816/0-3-8 Max Horz B=790(LC 8) Max Uplift B=-568(LC 8), G=-691(LC 8) Max Grav B=873(LC 1), G=827(LC 2)

F-G,L-M: 2x4 SP No.2

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

B-C=-1817/1416, C-D=-683/547, D-E=-540/593 TOP CHORD

BOT CHORD B-J=-1748/1487, I-J=-1333/1090, H-I=-296/320, G-H=-296/320

WEBS C-J=-982/963, C-I=-934/1333, I-L=-581/549, E-L=-575/650, E-M=-664/650, G-M=-700/654

NOTES-(10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads. 4) All plates are MT20 plates unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Bearing at joint(s) B considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=568, G=691.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: A-E=-62, E-F=-62, J-O=-21, I-J=-21, G-I=-21



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Job	Truss	Truss Type	Qty	Ply	H&H-NC/Topsail/
1110004 OFA	A07	Roof Special	8	1	130128618
			°		Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s	Sep 29 2015 MiTek Industries, Inc. Mon Jun 12 13:03:26 2017 Page

ID:X87q8Dmm5MnjRaLGAgDs0yzEGbe-roFKsyedCwWZI1dzil_7eXSIzbZ0Na5jdLwPAKz70YF -0<u>-10+8</u> 0-10-8 7-9-0 15-2-8 19-11-8 7-9-0 4-9-0 8.00 12 2x4 II 3x8 MT20HS 🚧 3x5 1 D С 10-8-12 7-6-12 1-11-0 0 H N 5x6 = G 4x5 || 5x6 = 6x8 = 5x6 ⋍ 4.00 122x4

6-2-8 11-11-8 19-11-8 5-9-0 8-0-0

Plate Of	<u>ISEIS (X,Y)</u>	[B:0-1-5,0-1-8], [I:0-4-0,Edge]			
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.90	Vert(LL) -0.22 G-H >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.73	Vert(TL) -0.42 G-H >562 240 MT20HS 187/143	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.83	Horz(TL) -0.09 G n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.14 H-I >999 240 Weight: 137 lb FT = 20%	

BRACING-

WEBS

TOP CHORD BOT CHORD

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2 *Except
	B-I: 2x6 SP No.2
WEBS	2x4 SP No.3 *Except
	F-G: 2x4 SP No.2

WEDGE

Left: 2x4 SP No.3

REACTIONS. (lb/size) B=846/0-5-8, G=791/0-3-8

Max Horz B=766(LC 8) Max Uplift B=-551(LC 8), G=-670(LC 8)

Max Grav B=846(LC 1), G=805(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD B-C=-1752/1360, C-D=-664/524, D-E=-530/568

BOT CHORD B-I=-1682/1431, H-I=-1282/1049, H-N=-263/276, N-O=-263/276, G-O=-263/276

WEBS C-I=-946/922, C-H=-890/1274, E-H=-542/587, E-G=-651/651

NOTES-(10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) All plates are MT20 plates unless otherwise indicated.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Bearing at joint(s) B considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=551, G=670.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

E-H. E-G

Rigid ceiling directly applied.

1 Row at midpt

Scale: 1/8"=1'

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Structural wood sheathing directly applied, except end verticals.

E-H. E-G

Rigid ceiling directly applied.

1 Row at midpt

<u>10-1-15</u> 10-1-15 <u>19-11-8</u> 9-9-9

Plate Of	tsets (X,Y)	[<u>B:0-6-0,0-0-9], [G:Edge,0-4-0], [H:0</u>	-4-0,0-4-8]		
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.85	Vert(LL) -0.12 G-H >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.51	Vert(TL) -0.21 G-H >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.44	Horz(TL) -0.02 G n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.13 H-K >999 240 Weight: 136 lb FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x6 SP No.2

 WEBS
 2x4 SP No.3 *Except*

F-G: 2x4 SP No.2

WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) G=791/0-3-8, B=846/0-5-8 Max Horz B=766(LC 8) Max UpliftG=-665(LC 8), B=-556(LC 8) Max Grav G=852(LC 2), B=846(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD B-C=-1037/574, C-D=-881/673, D-E=-764/719 BOT CHORD B-L=-910/790, L-M=-910/790, H-M=-910/790, H-N=-268/282, N-O=-268/282, G-O=-268/282
- WEBS C-H=-422/783, E-H=-673/812, E-G=-653/655

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) G=665, B=556.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.









19-11-8 19-11-8

Plate Offsets (X,Y)-- [K:0-3-0,Edge], [O:Edge,0-2-0], [T:0-4-0,0-4-8] LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 тс 0.74 Vert(LL) -0.00 n/r 120 MT20 244/190 Α TCDL 10.0 Lumber DOL 1.15 BC 0.19 Vert(TL) -0.00 A n/r 120 BCLL 0.0 * Rep Stress Incr YES WB 0.15 Horz(TL) 0.00 0 n/a n/a BCDL 10.0 Code IRC2009/TPI2007 (Matrix) Weight: 176 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2x6 SP No.2 end verticals. 2x4 SP No.2 BOT CHORD WEBS

WEBS

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt J-R. I-S. L-Q. M-P

REACTIONS. All bearings 19-11-8

2x4 SP No.3

(lb) - Max Horz B=942(LC 7) Max Uplift All uplift 100 lb or less at joint(s) except O=-120(LC 9), B=-327(LC 6), R=-230(LC 7), S=-259(LC 8). T=-195(LC 8), U=-205(LC 8), V=-203(LC 8), W=-200(LC 8), X=-257(LC 8), Q=-105(LC 7), P=-318(LC 9) Max Grav All reactions 250 lb or less at joint(s) O, S, T, U, V, W, X, Q, P except B=482(LC 7), R=339(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD B-C=-907/556, C-D=-768/519, D-E=-676/507, E-F=-583/496, F-G=-490/472, G-H=-483/484, H-I=-394/466, I-J=-313/535, J-K=-155/428, K-L=-139/431, L-M=-208/526, M-N=-216/424 N-O=-171/350 BOT CHORD B-X=-160/283, W-X=-160/283, V-W=-160/283, U-V=-160/283, T-U=-160/283, S-T=-162/281,
- R-S=-162/281, Q-R=-162/281, P-Q=-162/281, O-P=-162/281 WEBS J-R=-324/261, I-S=-122/298, C-X=-127/285, M-P=-118/298

NOTES-(12)

OTHERS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint O, 327 lb uplift at joint B, 230 lb uplift at joint R, 259 lb uplift at joint S, 195 lb uplift at joint T, 205 lb uplift at joint U, 203 lb uplift at joint V, 200 lb uplift at joint W, 257 lb uplift at joint X, 105 lb uplift at joint Q and 318 lb uplift at joint P.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Scale = 1:84.0

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> 818 Soundside Road Edenton, NC 27932





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818 Soundside Road Edenton, NC 27932



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 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoIVIT11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Plate Offsets (X.Y)-- [B:0-6-0.0-0-9]. [E:0-3-12.0-2-0]. [F:0-3-12.0-2-0]. [H:Edge.0-4-4]. [I:0-3-12.0-4-8]

			o reto e olt li treadoto i		
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.74	Vert(LL) -0.11 H-I >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.50	Vert(TL) -0.19 H-I >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.37	Horz(TL) -0.02 H n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.13 I-L >999 240 Weight: 148 lb FT = 20%	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied, except end verticals, and
BOT CHORD	2x6 SP No.2		2-0-0 oc purlins (6-0-0 max.): E-F.
WEBS	2x4 SP No.3 *Except*	BOT CHORD	Rigid ceiling directly applied.
	G-H: 2x4 SP No.2	WEBS	1 Row at midpt F-I, F-H

WEDGE Left: 2x4 SP No.3

REACTIONS. (Ib/size) B=846/0-5-8, H=791/0-3-8 Max Horz B=889(LC 7) Max UpliftB=-616(LC 8), H=-573(LC 8) Max Grav B=846(LC 1), H=846(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD B-C=-1017/752, C-D=-857/842, D-E=-754/882, E-F=-544/815, F-G=-227/485, G-H=-180/425 BOT CHORD B-M=-1056/770, I-N=-1056/770, I-O=-432/297, O-P=-432/297
- BOT CHORD B-M=-1056/770, M-N=-1056/770, I-N=-1056/770, I-O=-432/297, O-P=-432/297, H-P=-432/297 WEBS C-I=-387/720, E-I=-144/260, F-I=-583/599, F-H=-646/800

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 20.0psf on the botton chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=616, H=573.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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10-0-8 Plate Offsets (X,Y)-- [B:0-2-6,0-0-7], [D:0-2-6,0-0-7] LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) l/defl L/d PLATES GRIP 244/190 TCLL 20.0 Plate Grip DOL 1.15 тс 0.70 Vert(LL) -0.07 F-X >999 360 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.65 Vert(TL) -0.20 F-X >999 240 BCLL 0.0 Rep Stress Incr YES WB 0.44 Horz(TL) 0.12 D n/a

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.21

F-U

n/a

Rigid ceiling directly applied.

240

Structural wood sheathing directly applied.

Weight: 151 lb

FT = 20%

>999

BCDL	10.0	

TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS WEDGE

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) B=848/0-5-8, D=848/0-5-8 Max Horz B=554(LC 7) Max Uplift B=-597(LC 8), D=-597(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD B-C=-1471/735, C-D=-1471/826 BOT CHORD B-F=-415/1118, D-F=-404/1118

Code IRC2009/TPI2007

WFBS C-F=-287/1055

NOTES-(12)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

(Matrix-S)

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

All plates are 2x4 MT20 unless otherwise indicated.

Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Bearing at joint(s) B, D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=597, D=597.

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





<u>10-0-8</u> 10-0-8 <u>10-0-8</u> <u>10-0-8</u>

Plate Of	fsets (X,Y)	[B:0-2-6,0-0-7], [D:0-2-6,0-0-7]		
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.70	Vert(LL) -0.07 F-N >999 360 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.65	Vert(TL) -0.20 F-N >999 240
BCLL	0.0 *	Rep Stress Incr YES	WB 0.44	Horz(TL) 0.12 D n/a n/a
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.21 F-K >999 240 Weight: 134 lb FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

 TOP CHORD
 2x6 SP No.2

 BOT CHORD
 2x6 SP No.2 *Except*

 GH: 2x4 SP No.2
 2x6 SP No.3

 WEBS
 2x4 SP No.3

 WEDGE
 Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (Ib/size) B=848/0-5-8, D=848/0-5-8 Max Horz B=554(LC 7) Max UpliftB=-597(LC 8), D=-597(LC 9)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 B-C=-1471/735, C-D=-1471/826

 BOT CHORD
 B-F=-415/1118, D-F=-404/1118

 WFBS
 C-F=-287/1055

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) B, D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=597, D=597.

- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







<u>10-0-8</u> 10-0-8 10-0-8

Plate Of	<u>fsets (X,Y)</u>	[A:0-1-12,0-1-1], [C:0-1-12,0-1-1]			
LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.71	Vert(LL) -0.07 D-L >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.64	Vert(TL) -0.20 D-L >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.44	Horz(TĽ) 0.12 C n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.21 D-I >999 240 Weight: 129 lb FT = 20%	

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

 TOP CHORD
 2x6 SP No.2

 BOT CHORD
 2x6 SP No.2 *Except*

 E-F: 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE

WEDGE Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) A=803/0-5-8, C=803/0-5-8 Max Horz A=-548(LC 6) Max UpliftA=-515(LC 8), C=-515(LC 9)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 A-B=-1477/790, B-C=-1477/865

 BOT CHORD
 A-D=-480/1123, C-D=-468/1123

 WEBS
 B-D=-336/1060

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) A, C considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) A=515, C=515.

- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







		L		10-0	1-0				20-	1-0			
		I		10-0)-8		1		10-	0-8		I	
Plate Offs	sets (X,Y)	[<u>B:0-1-12,0-1-1], [</u> (<u>):0-3-8,0-2-</u>	2], [D:0-3-8,0)-2-12],	[E:0-1-12,0-	1-1], [G:0-6-0,0-1	-13]					
LOADING	G (psf)	SPACING-	2-0-)	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip I	DOL 1.1	5	TC	0.45	Vert(LL)	-0.06	G-W	>999	360	MT20	244/190
TCDL	10.0	Lumber DC	L 1.1	5	BC	0.37	Vert(TL)	-0.19	G-W	>999	240		
BCLL	0.0 *	Rep Stress	Incr YES	6	WB	0.33	Horz(TL)	0.12	E	n/a	n/a		
BCDL	10.0	Code IRC2	009/TPI200	,	(Matri	ix-S)	Wind(LL)	0.11	G-W	>999	240	Weight: 150 lb	FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied, except

2-0-0 oc purlins (5-8-1 max.): C-D.

Rigid ceiling directly applied.

LUMBER-

 TOP CHORD
 2x6 SP No.2

 BOT CHORD
 2x6 SP No.2

 WEBS
 2x4 SP No.3

 OTHERS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (lb/size) B=848/0-5-8, E=848/0-5-8 Max Horz B=-413(LC 6) Max UpliftB=-579(LC 8), E=-579(LC 9)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 B-C=-1538/918, C-D=-1653/1060, D-E=-1538/919

 BOT CHORD
 B-G=-931/1191, E-G=-571/1191

WEBS C-G=-189/797, D-G=-535/797

NOTES- (14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

5) Provide adequate drainage to prevent water ponding.

6) All plates are 2x4 MT20 unless otherwise indicated.

7) Gable studs spaced at 2-0-0 oc.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Bearing at joint(s) B, E considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=579, E=579.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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					20-1-0				10-0-8		L		
					10-0-8		1		10-0-8				
)-0-11]	12], [E:0-2-6,(0-4-12,0-2-1	12,0-2-12], [D:	[B:0-2-6,0-0-11], [C:0-4-	sets (X,Y)	Plate Off
							-				1 .	, .	
	GRIP	PLATES	L/d	l/defl	(loc)	in	DEFL.	0.50	CSI.	2-0-0	SPACING-	G(psf)	
	244/190	M120	360	>999	G-L	-0.06	Vert(LL)	0.59	IC	1.15	Plate Grip DOL	20.0	TCLL
-	GRIP 244/190	PLATES MT20	L/d 360	l/defl >999	(loc) G-L	in -0.06	DEFL. Vert(LL)	0.59	CSI. TC	2-0-0 1.15	SPACING- Plate Grip DOL	3 (psf) 20.0	

BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL)	0.15	G-L	>999	240	Weight: 136 lb	FT = 20%	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.25	Horz(TL)	0.08	E	n/a	n/a			
TCDL	10.0	Lumber DOL 1.15	BC 0.50	Vert(TL)	-0.18	G-L	>999	240			
TOLL	20.0	Plate Grip DOL 1.15	10 0.59		-0.00	G-L	~999	300	101120	244/190	

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied, except

2-0-0 oc purlins (6-0-0 max.): C-D.

Rigid ceiling directly applied.

LUMBER-

 TOP CHORD
 2x6 SP No.2

 BOT CHORD
 2x6 SP No.2 *Except*

 H-I: 2x4 SP No.2
 *H-I: 2x4 SP No.3

 WEBS
 2x4 SP No.3

Left: 2x4 SP No.3, Right: 2x4 SP No.3

REACTIONS. (Ib/size) B=848/0-5-8, E=848/0-5-8 Max Horz B=-499(LC 6) Max UpliftB=-591(LC 8), E=-591(LC 9)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 B-C=-1482/785, C-D=-1158/905, D-E=-1482/785

 BOT CHORD
 B-G=-681/1130, E-G=-380/1130

 WEBS
 C-G=-225/610, D-G=-611/701

NOTES- (11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Bearing at joint(s) B, E considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=591, E=591.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purifin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.











	⊢		9-11-8							19-11-0		
Plate Offse	ets (X Y) 1	[F·0-3-8 0-3-4]	9-11-8							9-11-8		
	<u>, , , , , , , , , , , , , , , , , , , </u>											
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.58	Vert(LL)	-0.12	È F-I	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.84	Vert(TL)	-0.36	F-I	>666	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.03	D	n/a	n/a		
BCDL	10.0	Code IRC2009/TF	PI2007	(Matr	ix-S)	Wind(LL)	0.20	F-I	>999	240	Weight: 91 lb	FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD 2x6 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

REACTIONS. (lb/size) B=836/0-5-8, D=836/0-5-8 Max Horz B=-130(LC 9) Max Uplift B=-622(LC 8), D=-622(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD B-C=-1227/1093, C-D=-1227/1093 BOT CHORD B-F=-776/1072, D-F=-776/1072

WEBS C-F=0/403

NOTES-(8)

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=622, D=622.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932







Design valid for use only using in particle connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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Plate Offsets (X,Y)	[B:0-3-1,Edge]			
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.88	DEFL. in (loc) l/defl L/d PLATES GRIP Vert(LL) 0.00 A n/r 120 MT20 244/190	
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.15 Rep Stress Incr YES	BC 0.52 WB 0.00	Vert(TL) 0.02 A n/r 120 Horz(TL) 0.00 D n/a n/a	
BCDL 10.0	Code IRC2009/TPI2007	(Matrix)	Weight: 22 lb FT = 20%	

TOP CHORD

BOT CHORD

end verticals.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2

REACTIONS. (lb/size) D=254/6-6-0, B=277/6-6-0 Max Horz B=152(LC 7) Max UpliftD=-208(LC 8), B=-234(LC 6)

Max Opin(D = 200(20.0), D = 204(20.0)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD C-D=-190/398

NOTES- (10)

- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) D=208, B=234.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 2-2-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.





LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc)	l/defl	L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.96	Vert(LL) -0.04	D-I	>999	360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.74	Vert(TL) -0.11	D-I	>665	240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.01	В	n/a	n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.23	D-I	>336	240	Weight: 22 lb FT = 20%

<u>6-6-0</u> 5-11-8

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. (lb/size) B=319/0-3-0, D=212/0-1-8

Max Horz B=149(LC 6) Max UpliftB=-432(LC 6), D=-310(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD C-D=-149/326

0-6-8

NOTES- (9

- Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=432, D=310.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

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 NoIVIT11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Gable studs spaced at 2-0-0 oc.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) B=132.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular

building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







0-6-8 6-4-8 5-10-0 Plate Offsets (X,Y)-- [B:0-4-5,0-0-5] LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 тс 0.45 Vert(LL) -0.04 E-I >999 360 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.51 Vert(TL) -0.10 E-I >757 240 BCLL 0.0 Rep Stress Incr NO WB 0.00 Horz(TL) -0.00 E n/a n/a Code IRC2009/TPI2007 BCDL 10.0 (Matrix-M) Wind(LL) 0.14 E-I >547 240 Weight: 27 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2x6 SP No.2 end verticals 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS

REACTIONS. (lb/size) E=487/Mechanical, B=477/0-3-0 Max Horz B=153(LC 4) Max Uplift E=-713(LC 4), B=-645(LC 4)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left exposed; end vertical left exposed; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) E=713, B=645.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: A-C=-60, C-D=-20, B-E=-20

Concentrated Loads (lb)

Vert: E=-138(F) J=-155(F) K=-144(F)



ENGINEERING BY TREENCO A Mi Tek Affiliate 818 Soundside Road Edenton, NC 27932

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0-6-8

LOADING	(psf)	SPACING- 2-0-0	CSI.	DEFL.	n (loc)	l/defl	L/d	PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.15	TC 0.29	Vert(LL) -0.0	0 D-H	>999 3	360	MT20 244/190	
TCDL	10.0	Lumber DOL 1.15	BC 0.26	Vert(TL) -0.0	1 D-H	>999 2	240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.0	0 D	n/a	n/a		
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.0)3 D-H	>999 2	240	Weight: 22 lb FT = 20%	

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.2WEBS2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) B=252/0-3-0, D=159/0-1-8 Max Horz B=148(LC 6) Max UpliftB=-335(LC 6), D=-241(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (9)

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;

- end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=335, D=241.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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<u>| 0-6-8 | 5-2-0</u> 0-6-8 4-7-8

Plate Of	fsets (X,Y)	[B:0-0-7,Edge]										
	G (nof)		0.0	681	DEEL	in	(loc)	l/dofl	L /d		CPIP	
TCLL	20.0	Plate Grip DOL	1.15	TC 0.40	Vert(LL)	-0.01	(IOC) D-H	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC 0.37	Vert(TL)	-0.03	D-H	>999	240			
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	В	n/a	n/a			
BCDL	10.0	Code IRC2009/TPI2	007	(Matrix-S)	Wind(LL)	0.06	D-H	>940	240	Weight: 18 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (Ib/size) D=150/Mechanical, B=275/0-3-0 Max Horz B=153(LC 6) Max Uplift D=-227(LC 6), B=-368(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD C-D=-110/252

NOTES- (8)

 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) D=227, B=368.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.



⁷⁾ This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



			0-6-8	+		<u>5-2-0</u> 4-7-8							
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	-0.01	È-I	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.32	Vert(TL)	-0.03	E-I	>999	240			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.11	Horz(TL)	-0.00	E	n/a	n/a			
BCDL	10.0	Code IRC2009/T	PI2007	(Matri	x-S)	Wind(LL)	0.05	E-I	>999	240	Weight: 20 lb	FT = 20%	

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-

 TOP CHORD
 Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins: C-D.

 BOT CHORD
 Rigid ceiling directly applied.

REACTIONS. (Ib/size) B=261/0-3-0, E=164/Mechanical Max Horz B=99(LC 6) Max UpliftB=-363(LC 6), E=-232(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD B-C=-170/286

BOT CHORD B-E=-316/134

WEBS C-E=-152/358

NOTES- (10)

 Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=363, E=232.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.







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NOTES-(8)

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ;

- end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C, D except (jt=lb) B=198

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

- 8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- MUMMERT Summer and SEAL 039032 W. KON 11 minut June 12,2017

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ate Offsets (X,Y) [E	ا 3:0-3-1,Edge], [C:0-0-5,0-1-8									
DADING (psf) CLL 20.0 CDL 10.0 CLL 0.0 *	SPACING- 2-0- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE	0 CSI . 5 TC 5 BC S WB	0.82 0.42 0.00	DEFL. Vert(LL) Vert(TL) Horz(TL)	in 0.01 0.02 0.00	(loc) A A D	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20	GRIP 244/190
CDL 10.0	Code IRC2009/TPI200	7 (Matr	rix)						Weight: 25 lb	FT = 20%

TOP CHORD

BOT CHORD

end verticals.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x6 SP No.2 WEBS

REACTIONS. (Ib/size) D=267/6-11-0, B=291/6-11-0 Max Horz B=155(LC 6)

Max Uplift D=-228(LC 6), B=-233(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD C-D=-178/363

NOTES-(10)

- 1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOI = 1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) D=228, B=233

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







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REACTIONS. (lb/size) D=57/0-1-8, B=158/0-3-0 Max Horz B=59(LC 6) Max Uplift D=-74(LC 6), B=-213(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) B=213

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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REACTIONS. All bearings 11-3-13.

(Ib) - Max Horz A=-287(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) A, E except H=-419(LC 8), F=-418(LC 9) Max Grav All reactions 250 lb or less at joint(s) A, E, G except H=275(LC 1), F=275(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS B-H=-197/431, D-F=-197/431

NOTES- (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A, E except (jt=lb) H=419, F=418.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS B-N=-186/389, H-J=-186/384

NOTES- (12)

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7-05; 130mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) A except (jt=lb) L=187 , M=111, N=377, K=120, J=372.

10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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