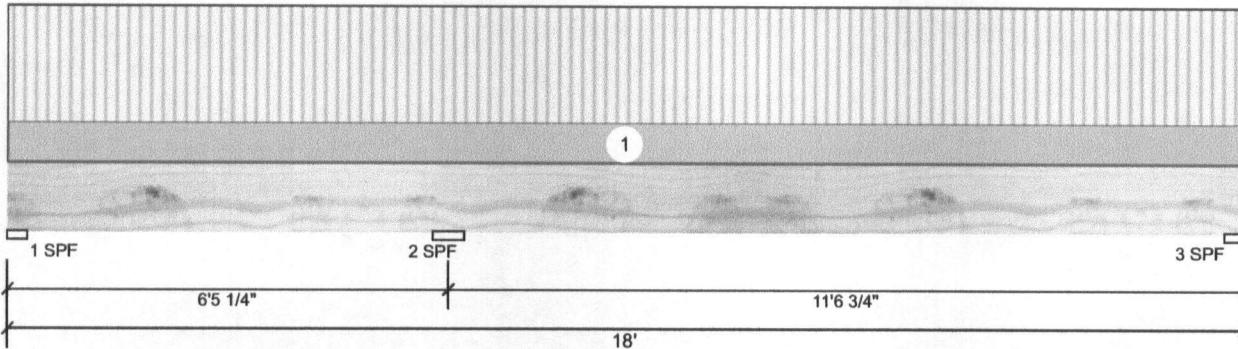




# BM1 Kerto-S LVL 1.750" X 11.875" 3-Ply - PASSED

Level: Level



### Member Information

|                     |               |
|---------------------|---------------|
| Type:               | Girder        |
| Plies:              | 3             |
| Moisture Condition: | Dry           |
| Deflection LL:      | 480           |
| Deflection TL:      | 360           |
| Importance:         | Normal        |
| Temperature:        | Temp <= 100°F |

|                |             |
|----------------|-------------|
| Application:   | Roof        |
| Slope:         | 0/12        |
| Design Method: | ASD         |
| Building Code: | IBC 2012    |
| Load Sharing:  | Yes         |
| Deck:          | Not Checked |

### Reactions UNPATTERNED lb (Uplift)

| Brg | Live | Dead | Snow | Wind | Const |
|-----|------|------|------|------|-------|
| 1   | 1162 | 409  | 0    | 0    | 0     |
| 2   | 8927 | 3140 | 0    | 0    | 0     |
| 3   | 3771 | 1326 | 0    | 0    | 0     |

### Bearings

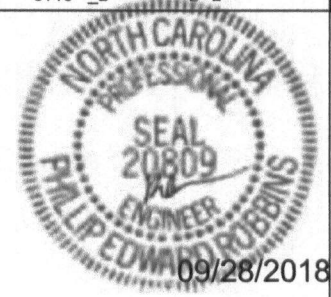
| Bearing | Length | Cap. | React D/L lb | Total       | Ld. Case | Ld. Comb. |
|---------|--------|------|--------------|-------------|----------|-----------|
| 1 - SPF | 3.500" | 35%  | 376 / 2355   | 2731 (-734) | L_       | D+L       |
| 2 - SPF | 5.500" | 100% | 3191 / 9072  | 12263       | LL       | D+L       |
| 3 - SPF | 3.500" | 66%  | 1308 / 3835  | 5143        | _L       | D+L       |

### Analysis Results

| Analysis     | Actual        | Location    | Allowed       | Capacity    | Comb. | Case |
|--------------|---------------|-------------|---------------|-------------|-------|------|
| Neg Moment   | -12572 ft-lb  | 6'5 1/4"    | 31060 ft-lb   | 0.405 (40%) | D+L   | LL   |
| Unbraced     | -11259 ft-lb  | 6'5 1/4"    | 20065 ft-lb   | 0.561 (56%) | D+L   | _L   |
| Pos Moment   | 11556 ft-lb   | 13' 11/16"  | 31060 ft-lb   | 0.372 (37%) | D+L   | _L   |
| Unbraced     | 11556 ft-lb   | 13' 11/16"  | 14626 ft-lb   | 0.790 (79%) | D+L   | _L   |
| Shear        | 5977 lb       | 7'5 1/8"    | 13300 lb      | 0.449 (45%) | D+L   | LL   |
| LL Defl inch | 0.148 (L/920) | 12'5 1/2"   | 0.283 (L/480) | 0.520 (52%) | L     | _L   |
| TL Defl inch | 0.196 (L/695) | 12'5 13/16" | 0.378 (L/360) | 0.520 (52%) | D+L   | _L   |

### Design Notes

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Tie-down connection required at bearing 1 for uplift 734 lb (Combination D+L, Load Case \_L).
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.



| ID | Load Type           | Location | Trib Width | Side | Dead 0.9          | Live 1  | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments |
|----|---------------------|----------|------------|------|-------------------|---------|-----------|----------|-------------|----------|
| 1  | Uniform Self Weight |          |            | Top  | 257 PLF<br>14 PLF | 770 PLF | 0 PLF     | 0 PLF    | 0 PLF       | F04      |

**Notes**  
Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**  
1. Dry service conditions, unless noted otherwise  
2. LVL not to be treated with fire retardant or corrosive chemicals

**Handling & Installation**

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Job# PER181912  
P. E. Robbins, P.E. - #309-240-6424  
1777 State Rt 167 Victoria IL 61485  
This design is valid until 7/10/2021

**Manufacturer Info**  
Metsä Wood  
301 Merritt 7 Building, 2nd Floor  
Norwalk, CT 06851  
(800) 622-5850  
www.metsawood.com/us  
ICC-ES: ESR-3633

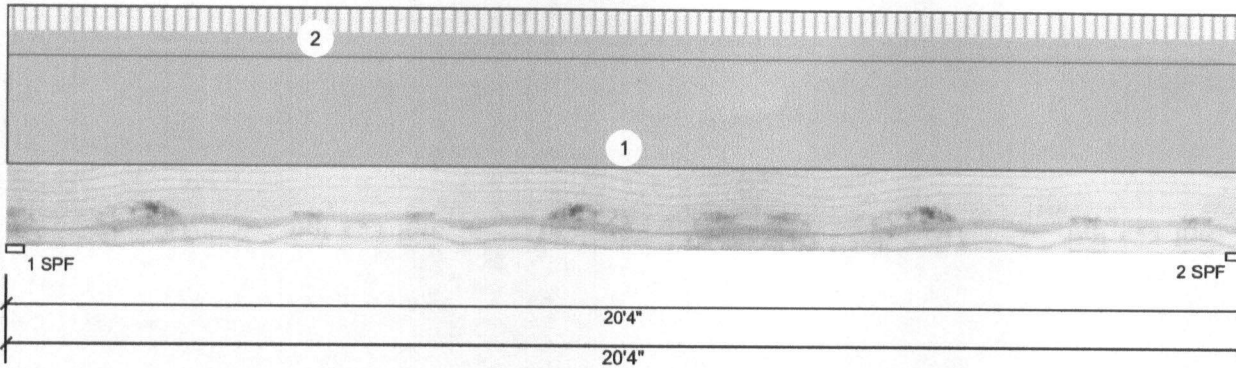
Comtech, Inc.  
1001 S. Reilly Road, Suite #639  
Fayetteville, NC  
USA  
28314  
910-864-TRUS





**BM3 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED**

Level: Level



**Member Information**

|                     |               |
|---------------------|---------------|
| Type:               | Girder        |
| Plies:              | 2             |
| Moisture Condition: | Dry           |
| Deflection LL:      | 480           |
| Deflection TL:      | 360           |
| Importance:         | Normal        |
| Temperature:        | Temp <= 100°F |

|                |             |
|----------------|-------------|
| Application:   | Roof        |
| Slope:         | 0/12        |
| Design Method: | ASD         |
| Building Code: | IBC 2012    |
| Load Sharing:  | No          |
| Deck:          | Not Checked |

**Reactions UNPATTERNED lb (Uplift)**

| Brg | Live | Dead | Snow | Wind | Const |
|-----|------|------|------|------|-------|
| 1   | 407  | 2312 | 0    | 0    | 0     |
| 2   | 407  | 2312 | 0    | 0    | 0     |

**Bearings**

| Bearing | Length | Cap. | React D/L  | lb   | Total | Ld. Case | Ld. Comb. |
|---------|--------|------|------------|------|-------|----------|-----------|
| 1 - SPF | 3.500" | 52%  | 2312 / 407 | 2719 | L     | D+L      |           |
| 2 - SPF | 3.500" | 52%  | 2312 / 407 | 2719 | L     | D+L      |           |

**Analysis Results**

| Analysis     | Actual         | Location   | Allowed       | Capacity     | Comb. | Case |
|--------------|----------------|------------|---------------|--------------|-------|------|
| Moment       | 13233 ft-lb    | 10'2"      | 34565 ft-lb   | 0.383 (38%)  | D+L   | L    |
| Unbraced     | 13233 ft-lb    | 10'2"      | 13250 ft-lb   | 0.999 (100%) | D+L   | L    |
| Shear        | 2304 lb        | 1'6 5/8"   | 11947 lb      | 0.193 (19%)  | D+L   | L    |
| LL Defl inch | 0.063 (L/3784) | 10'2 1/16" | 0.497 (L/480) | 0.130 (13%)  | L     | L    |
| TL Defl inch | 0.422 (L/566)  | 10'2 1/16" | 0.663 (L/360) | 0.640 (64%)  | D+L   | L    |



**Design Notes**

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at a maximum of 8'11 1/4" o.c.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

| ID | Load Type   | Location | Trib Width | Side | Dead 0.9 | Live 1 | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments |
|----|-------------|----------|------------|------|----------|--------|-----------|----------|-------------|----------|
| 1  | Uniform     |          |            | Top  | 175 PLF  | 0 PLF  | 0 PLF     | 0 PLF    | 0 PLF       | WALL     |
| 2  | Uniform     |          |            | Top  | 40 PLF   | 40 PLF | 0 PLF     | 0 PLF    | 0 PLF       | ROOF     |
|    | Self Weight |          |            |      | 12 PLF   |        |           |          |             |          |

**Notes**  
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**  
1. Dry service conditions, unless noted otherwise  
2. LVL not to be treated with fire retardant or corrosive chemicals

**Handling & Installation**  
1. LVL beams must not be cut or drilled  
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals  
3. Damaged Beams must not be used  
4. Design assumes top edge is laterally restrained  
5. Provide lateral support at bearing points to avoid lateral displacement and rotation  
6. For flat roofs provide proper drainage to prevent ponding

Job# PER181912  
P. E. Robbins, P.E. - #309-240-6424  
1777 State Rt 167 Victoria IL 61485  
This design is valid until 7/10/2021

**Manufacturer Info**  
Metsä Wood  
301 Merritt 7 Building, 2nd Floor  
Norwalk, CT 06851  
(800) 622-5850  
www.metsawood.com/us  
ICC-ES: ESR-3633

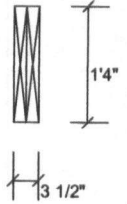
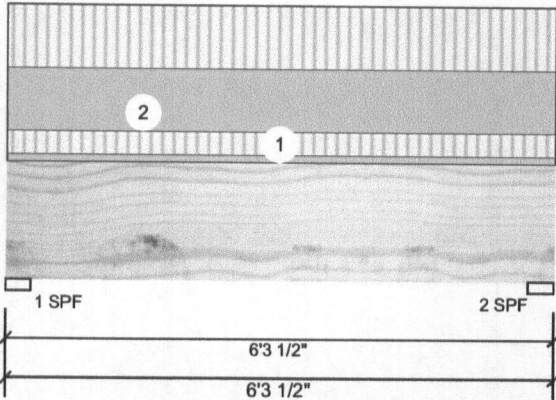
Comtech, Inc.  
1001 S. Reilly Road, Suite #639  
Fayetteville, NC  
USA  
28314  
910-864-TRUS





**BM4 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED**

Level: Level



**Member Information**

|                     |               |
|---------------------|---------------|
| Type:               | Girder        |
| Plies:              | 2             |
| Moisture Condition: | Dry           |
| Deflection LL:      | 480           |
| Deflection TL:      | 360           |
| Importance:         | Normal        |
| Temperature:        | Temp <= 100°F |

|                |             |
|----------------|-------------|
| Application:   | Roof        |
| Slope:         | 0/12        |
| Design Method: | ASD         |
| Building Code: | IBC 2012    |
| Load Sharing:  | No          |
| Deck:          | Not Checked |

**Reactions UNPATTERNED lb (Uplift)**

| Brg | Live | Dead | Snow | Wind | Const |
|-----|------|------|------|------|-------|
| 1   | 1746 | 1471 | 0    | 0    | 0     |
| 2   | 1746 | 1471 | 0    | 0    | 0     |

**Bearings**

| Bearing | Length | Cap. React | D/L lb      | Total | Ld. Case | Ld. Comb. |
|---------|--------|------------|-------------|-------|----------|-----------|
| 1 - SPF | 3.500" | 62%        | 1471 / 1746 | 3216  | L        | D+L       |
| 2 - SPF | 3.500" | 62%        | 1471 / 1746 | 3216  | L        | D+L       |

**Analysis Results**


| Analysis     | Actual         | Location | Allowed       | Capacity    | Comb. | Case |
|--------------|----------------|----------|---------------|-------------|-------|------|
| Moment       | 4380 ft-lb     | 3'1 3/4" | 34565 ft-lb   | 0.127 (13%) | D+L   | L    |
| Unbraced     | 4380 ft-lb     | 3'1 3/4" | 19579 ft-lb   | 0.224 (22%) | D+L   | L    |
| Shear        | 1630 lb        | 1'6 5/8" | 11947 lb      | 0.136 (14%) | D+L   | L    |
| LL Defl inch | 0.011 (L/6369) | 3'1 3/4" | 0.146 (L/480) | 0.080 (8%)  | L     | L    |
| TL Defl inch | 0.020 (L/3457) | 3'1 3/4" | 0.195 (L/360) | 0.100 (10%) | D+L   | L    |

**Design Notes**

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.



| ID | Load Type   | Location | Trib Width | Side | Dead 0.9 | Live 1  | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments |
|----|-------------|----------|------------|------|----------|---------|-----------|----------|-------------|----------|
| 1  | Uniform     |          |            | Top  | 50 PLF   | 150 PLF | 0 PLF     | 0 PLF    | 0 PLF       | FLOOR    |
| 2  | Uniform     |          |            | Top  | 405 PLF  | 405 PLF | 0 PLF     | 0 PLF    | 0 PLF       | J1       |
|    | Self Weight |          |            |      | 12 PLF   |         |           |          |             |          |

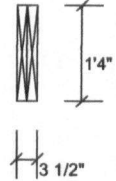
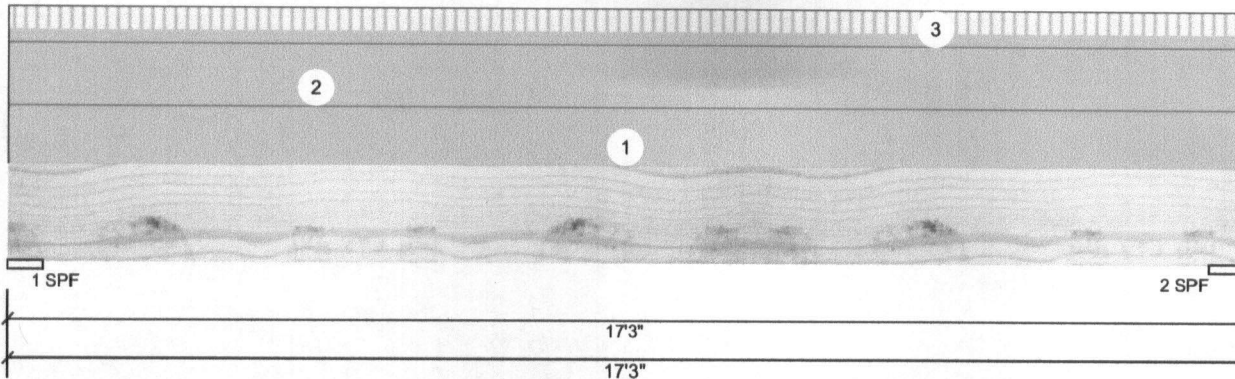
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|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| <p><b>Notes</b></p> <p>Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.</p> <p><b>Lumber</b></p> <ol style="list-style-type: none"> <li>Dry service conditions, unless noted otherwise</li> <li>LVL not to be treated with fire retardant or corrosive chemicals</li> </ol> | <p><b>Handling &amp; Installation</b></p> <ol style="list-style-type: none"> <li>LVL beams must not be cut or drilled</li> <li>Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals</li> <li>Damaged Beams must not be used</li> <li>Design assumes top edge is laterally restrained</li> <li>Provide lateral support at bearing points to avoid lateral displacement and rotation</li> </ol> | <p>6. For flat roofs provide proper drainage to prevent ponding</p> <p>Job# PER181912<br/>P. E. Robbins, P.E. - #309-240-6424<br/>1777 State Rt 167 Victoria IL 61485<br/>This design is valid until 7/10/2021</p> | <p><b>Manufacturer Info</b></p> <p>Metsä Wood<br/>301 Merritt 7 Building, 2nd Floor<br/>Norwalk, CT 06851<br/>(800) 622-5850<br/>www.metsawood.com/us<br/>ICC-ES: ESR-3633</p> | <p>Comtech, Inc.<br/>1001 S. Reilly Road, Suite #639<br/>Fayetteville, NC<br/>USA<br/>28314<br/>910-864-TRUS</p>  |
|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|





**GDH Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED**

Level: Level



**Member Information**

|                     |               |
|---------------------|---------------|
| Type:               | Girder        |
| Plies:              | 2             |
| Moisture Condition: | Dry           |
| Deflection LL:      | 480           |
| Deflection TL:      | 600           |
| Importance:         | Normal        |
| Temperature:        | Temp <= 100°F |

|                |             |
|----------------|-------------|
| Application:   | Roof        |
| Slope:         | 0/12        |
| Design Method: | ASD         |
| Building Code: | IBC 2012    |
| Load Sharing:  | No          |
| Deck:          | Not Checked |

**Reactions UNPATTERNED lb (Uplift)**

| Brg | Live | Dead | Snow | Wind | Const |
|-----|------|------|------|------|-------|
| 1   | 518  | 3083 | 0    | 0    | 0     |
| 2   | 518  | 3083 | 0    | 0    | 0     |

**Bearings**

| Bearing | Length | Cap. React | D/L lb     | Total | Ld. Case | Ld. Comb. |
|---------|--------|------------|------------|-------|----------|-----------|
| 1 - SPF | 6.000" | 40%        | 3083 / 518 | 3600  | L        | D+L       |
| 2 - SPF | 6.000" | 40%        | 3083 / 518 | 3600  | L        | D+L       |

**Analysis Results**

| Analysis     | Actual         | Location  | Allowed       | Capacity     | Comb. | Case |
|--------------|----------------|-----------|---------------|--------------|-------|------|
| Moment       | 14027 ft-lb    | 8'7 1/2"  | 34565 ft-lb   | 0.406 (41%)  | D+L   | L    |
| Unbraced     | 14027 ft-lb    | 8'7 1/2"  | 14045 ft-lb   | 0.999 (100%) | D+L   | L    |
| Shear        | 2866 lb        | 1'9 1/8"  | 11947 lb      | 0.240 (24%)  | D+L   | L    |
| LL Defl inch | 0.045 (L/4374) | 8'7 9/16" | 0.410 (L/480) | 0.110 (11%)  | L     | L    |
| TL Defl inch | 0.313 (L/629)  | 8'7 9/16" | 0.328 (L/600) | 0.950 (95%)  | D+L   | L    |



**Design Notes**

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at a maximum of 8'4 7/8" o.c.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

| ID | Load Type   | Location | Trib Width | Side | Dead 0.9 | Live 1 | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments   |
|----|-------------|----------|------------|------|----------|--------|-----------|----------|-------------|------------|
| 1  | Uniform     |          |            | Top  | 150 PLF  | 0 PLF  | 0 PLF     | 0 PLF    | 0 PLF       | WALL       |
| 2  | Uniform     |          |            | Top  | 160 PLF  | 0 PLF  | 0 PLF     | 0 PLF    | 0 PLF       | BRICK      |
| 3  | Uniform     |          |            | Top  | 35 PLF   | 60 PLF | 0 PLF     | 0 PLF    | 0 PLF       | Roof/Floor |
|    | Self Weight |          |            |      | 12 PLF   |        |           |          |             |            |

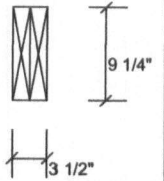
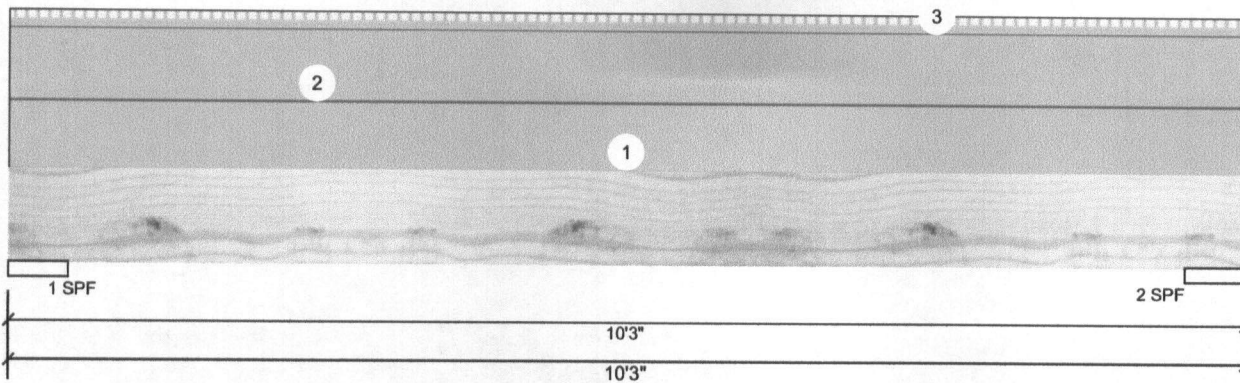
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|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------------------------------------------------------------------|
| <p><b>Notes</b></p> <p>Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.</p> <p><b>Lumber</b></p> <ol style="list-style-type: none"> <li>Dry service conditions, unless noted otherwise</li> <li>LVL not to be treated with fire retardant or corrosive chemicals</li> </ol> | <p><b>Handling &amp; Installation</b></p> <ol style="list-style-type: none"> <li>LVL beams must not be cut or drilled</li> <li>Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals</li> <li>Damaged Beams must not be used</li> <li>Design assumes top edge is laterally restrained</li> <li>Provide lateral support at bearing points to avoid lateral displacement and rotation</li> </ol> | <p>6. For flat roofs provide proper drainage to prevent ponding</p> <p>Job# PER181912<br/>P. E. Robbins, P.E. - #309-240-6424<br/>1777 State Rt 167 Victoria IL 61485<br/>This design is valid until 7/10/2021</p> | <p><b>Manufacturer Info</b></p> <p>Metsä Wood<br/>301 Merritt 7 Building, 2nd Floor<br/>Norwalk, CT 06851<br/>(800) 622-5850<br/>www.metsawood.com/us<br/>ICC-ES: ESR-3633</p> | <p>Comtech, Inc.<br/>1001 S. Reilly Road, Suite #639<br/>Fayetteville, NC<br/>USA<br/>28314<br/>910-884-TRUS</p> |
|                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                    |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                                                                                                                                                                                                                    |                                                                                                                                                                                |                                                                                                                  |





**GDH-9 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED**

Level: Level



**Member Information**

|                     |               |
|---------------------|---------------|
| Type:               | Girder        |
| Plies:              | 2             |
| Moisture Condition: | Dry           |
| Deflection LL:      | 480           |
| Deflection TL:      | 600           |
| Importance:         | Normal        |
| Temperature:        | Temp <= 100°F |

|                |             |
|----------------|-------------|
| Application:   | Roof        |
| Slope:         | 0/12        |
| Design Method: | ASD         |
| Building Code: | IBC 2012    |
| Load Sharing:  | No          |
| Deck:          | Not Checked |

**Reactions UNPATTERNED lb (Uplift)**

| Brg | Live | Dead | Snow | Wind | Const |
|-----|------|------|------|------|-------|
| 1   | 103  | 1728 | 0    | 0    | 0     |
| 2   | 103  | 1728 | 0    | 0    | 0     |

**Bearings**

| Bearing | Length | Cap. | React D/L lb | Total | Ld. Case | Ld. Comb. |
|---------|--------|------|--------------|-------|----------|-----------|
| 1 - SPF | 6.000" | 21%  | 1728 / 103   | 1831  | L        | D+L       |
| 2 - SPF | 6.000" | 21%  | 1728 / 103   | 1831  | L        | D+L       |

**Analysis Results**

| Analysis               | Actual     | Location | Allowed       | Capacity    | Comb. | Case    |
|------------------------|------------|----------|---------------|-------------|-------|---------|
| Moment                 | 3705 ft-lb | 5'1 1/2" | 11288 ft-lb   | 0.328 (33%) | D     | Uniform |
| Unbraced               | 3924 ft-lb | 5'1 1/2" | 7663 ft-lb    | 0.512 (51%) | D+L   | L       |
| Shear                  | 1321 lb    | 9' 1/2"  | 6216 lb       | 0.212 (21%) | D     | Uniform |
| LL Defl inch (L/13536) | 0.008      | 5'1 1/2" | 0.234 (L/480) | 0.040 (4%)  | L     | L       |
| TL Defl inch (L/758)   | 0.148      | 5'1 1/2" | 0.188 (L/600) | 0.790 (79%) | D+L   | L       |



**Design Notes**

- Girders are designed to be supported on the bottom edge only.
- Multiple plies must be fastened together as per manufacturer's details.
- Top loads must be supported equally by all plies.
- Top braced at bearings.
- Bottom braced at bearings.
- Lateral slenderness ratio based on single ply width.

| ID | Load Type   | Location | Trib Width | Side | Dead 0.9 | Live 1 | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments |
|----|-------------|----------|------------|------|----------|--------|-----------|----------|-------------|----------|
| 1  | Uniform     |          |            | Top  | 150 PLF  | 0 PLF  | 0 PLF     | 0 PLF    | 0 PLF       | WALL     |
| 2  | Uniform     |          |            | Top  | 160 PLF  | 0 PLF  | 0 PLF     | 0 PLF    | 0 PLF       | BRICK    |
| 3  | Uniform     |          |            | Top  | 20 PLF   | 20 PLF | 0 PLF     | 0 PLF    | 0 PLF       | Roof     |
|    | Self Weight |          |            |      | 7 PLF    |        |           |          |             |          |

**Notes**  
Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

**Lumber**  
1. Dry service conditions, unless noted otherwise  
2. LVL not to be treated with fire retardant or corrosive chemicals

**Handling & Installation**  
1. LVL beams must not be cut or drilled  
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals  
3. Damaged Beams must not be used  
4. Design assumes top edge is laterally restrained  
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Job# PER181912  
P. E. Robbins, P.E. - #309-240-6424  
1777 State Rt 167 Victoria IL 61485  
This design is valid until 7/10/2021

**Manufacturer Info**  
Metsä Wood  
301 Merritt 7 Building, 2nd Floor  
Norwalk, CT 06851  
(800) 622-5850  
www.metsawood.com/us  
ICC-ES: ESR-3633

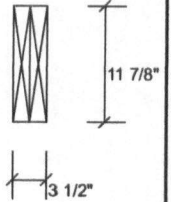
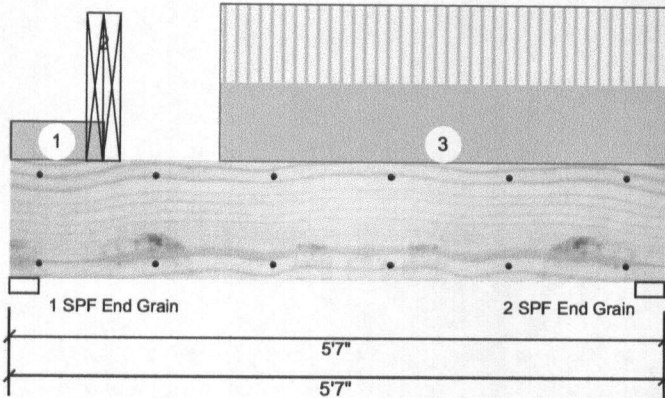
Comtech, Inc  
1001 S. Reilly Road, Suite #639  
Fayetteville, NC  
USA  
28314  
910-864-TRUS





**Sliding Door Header Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED**

Level: Level



**Member Information**

|                     |               |
|---------------------|---------------|
| Type:               | Girder        |
| Plies:              | 2             |
| Moisture Condition: | Dry           |
| Deflection LL:      | 360           |
| Deflection TL:      | 240           |
| Importance:         | Normal        |
| Temperature:        | Temp <= 100°F |

|                |             |
|----------------|-------------|
| Application:   | Floor       |
| Design Method: | ASD         |
| Building Code: | IBC 2012    |
| Load Sharing:  | No          |
| Deck:          | Not Checked |

**Reactions UNPATTERNED lb (Uplift)**

| Brg | Live | Dead | Snow | Wind | Const |
|-----|------|------|------|------|-------|
| 1   | 684  | 2489 | 0    | 0    | 0     |
| 2   | 709  | 960  | 0    | 0    | 0     |

**Bearings**

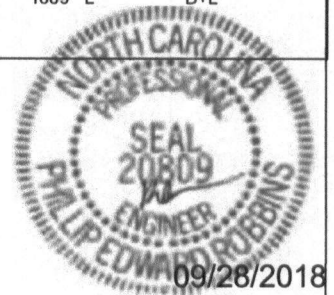
| Bearing           | Length | Cap. React | D/L lb     | Total | Ld. Case | Ld. Comb. |
|-------------------|--------|------------|------------|-------|----------|-----------|
| 1 - SPF End Grain | 3.000" | 35%        | 2489 / 684 | 3173  | L        | D+L       |
| 2 - SPF End Grain | 3.000" | 18%        | 960 / 709  | 1669  | L        | D+L       |

**Analysis Results**

| Analysis     | Actual         | Location  | Allowed       | Capacity    | Comb. | Case |
|--------------|----------------|-----------|---------------|-------------|-------|------|
| Moment       | 2327 ft-lb     | 2'5 3/16" | 19911 ft-lb   | 0.117 (12%) | D+L   | L    |
| Unbraced     | 2327 ft-lb     | 2'5 3/16" | 15061 ft-lb   | 0.155 (15%) | D+L   | L    |
| Shear        | 2004 lb        | 1'2 1/8"  | 8867 lb       | 0.226 (23%) | D+L   | L    |
| LL Defl inch | 0.007 (L/9597) | 2'9 7/8"  | 0.174 (L/360) | 0.040 (4%)  | L     | L    |
| TL Defl inch | 0.018 (L/3391) | 2'7 1/2"  | 0.260 (L/240) | 0.070 (7%)  | D+L   | L    |

**Design Notes**

- 1 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



| ID | Load Type                    | Location       | Trib Width | Side | Dead 0.9         | Live 1  | Snow 1.15 | Wind 1.6 | Const. 1.25 | Comments  |
|----|------------------------------|----------------|------------|------|------------------|---------|-----------|----------|-------------|-----------|
| 1  | Part. Uniform                | 0-0-0 to 0-9-8 |            | Top  | 125 PLF          | 0 PLF   | 0 PLF     | 0 PLF    | 0 PLF       | Wall Load |
| 2  | Point                        | 0-9-8          |            | Top  | 2312 lb          | 407 lb  | 0 lb      | 0 lb     | 0 lb        | BM3 Brg 1 |
| 3  | Part. Uniform<br>Self Weight | 1-9-8 to 5-7-0 |            | Top  | 260 PLF<br>9 PLF | 260 PLF | 0 PLF     | 0 PLF    | 0 PLF       | C2        |

|                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                          |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                |                                                                                                                                                                                                                    |                                                                                                                                                                                |                                                                                                                  |
|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------------------------------------------------------------------|
| <p><b>Notes</b></p> <p>Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.</p> <p><b>Lumber</b></p> <ol style="list-style-type: none"> <li>1. Dry service conditions, unless noted otherwise</li> <li>2. LVL not to be treated with fire retardant or corrosive chemicals</li> </ol> | <p><b>Handling &amp; Installation</b></p> <ol style="list-style-type: none"> <li>1. LVL beams must not be cut or drilled</li> <li>2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals</li> <li>3. Damaged Beams must not be used</li> <li>4. Design assumes top edge is laterally restrained</li> <li>5. Provide lateral support at bearing points to avoid lateral displacement and rotation</li> </ol> | <p>6. For flat roofs provide proper drainage to prevent ponding</p> <p>Job# PER181912<br/>P. E. Robbins, P.E. - #309-240-6424<br/>1777 State Rt 167 Victoria IL 61485<br/>This design is valid until 7/10/2021</p> | <p><b>Manufacturer Info</b></p> <p>Metsä Wood<br/>301 Merritt 7 Building, 2nd Floor<br/>Norwalk, CT 06851<br/>(800) 622-5850<br/>www.metsawood.com/us<br/>ICC-ES: ESR-3633</p> | <p>Comtech, Inc.<br/>1001 S. Reilly Road, Suite #639<br/>Fayetteville, NC<br/>USA<br/>28314<br/>910-864-TRUS</p> |
|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------------------------------------------------------------------|





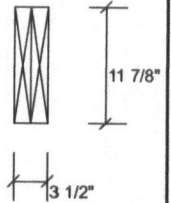
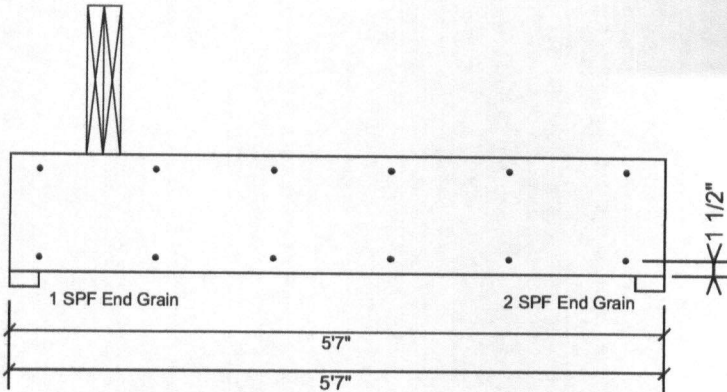
isDesign™

Client: Watermark Homes  
Project:  
Address:

Date: 9/27/2018  
Designer: Anthony Williams  
Job Name: Lot 38 South Creek  
Project #: J0918-4410

**Sliding Door Header Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED**

Level: Level



**Multi-Ply Analysis**

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

|                          |           |
|--------------------------|-----------|
| Capacity                 | 0.0 %     |
| Load                     | 0.0 PLF   |
| Yield Limit per Foot     | 163.7 PLF |
| Yield Limit per Fastener | 81.9 lb.  |
| Yield Mode               | IV        |
| Edge Distance            | 1 1/2"    |
| Min. End Distance        | 3"        |
| Load Combination         |           |
| Duration Factor          | 1.00      |



|                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                    |                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |                                                                                                                                                                                                                    |                                                                                                                                                                                |                                                                                                                 |
|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-----------------------------------------------------------------------------------------------------------------|
| <p><b>Notes</b></p> <p>Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.</p> <p><b>Lumber</b></p> <ol style="list-style-type: none"> <li>Dry service conditions, unless noted otherwise</li> <li>LVL not to be treated with fire retardant or corrosive chemicals</li> </ol> | <p><b>Handling &amp; Installation</b></p> <ol style="list-style-type: none"> <li>LVL beams must not be cut or drilled</li> <li>Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals</li> <li>Damaged Beams must not be used</li> <li>Design assumes top edge is laterally restrained</li> <li>Provide lateral support at bearing points to avoid lateral displacement and rotation</li> </ol> | <p>6. For flat roofs provide proper drainage to prevent ponding</p> <p>Job# PER181912<br/>P. E. Robbins, P.E. - #309-240-6424<br/>1777 State Rt 167 Victoria IL 61485<br/>This design is valid until 7/10/2021</p> | <p><b>Manufacturer Info</b></p> <p>Metsä Wood<br/>301 Merritt 7 Building, 2nd Floor<br/>Norwalk, CT 06851<br/>(800) 622-5850<br/>www.metsawood.com/us<br/>ICC-ES: ESR-3633</p> | <p>Comtech, Inc.<br/>1001 S. Rally Road, Suite #639<br/>Fayetteville, NC<br/>USA<br/>28314<br/>910-864-TRUS</p> |
|----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-----------------------------------------------------------------------------------------------------------------|



RE: J0918-4409  
 Lot 38 South Creek / Harnett County

**Trenco**  
 818 Soundside Rd  
 Edenton, NC 27932

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: IRC2009/TPI2007  
 Wind Code: ASCE 7-05 Wind Speed: 100 mph  
 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.1  
 Design Method: MWFRS(low-rise)/C-C hybrid Wind ASCE 7-05  
 Floor Load: N/A psf

This package includes 32 individual, dated Truss Design Drawings and 0 Additional Drawings.

| No. | Seal#     | Truss Name | Date    |
|-----|-----------|------------|---------|
| 1   | E11391720 | a1         | 1/25/18 |
| 2   | E11391721 | a1-ge      | 1/25/18 |
| 3   | E11391722 | a2         | 1/25/18 |
| 4   | E11391723 | b1         | 1/25/18 |
| 5   | E11391724 | b1-ge      | 1/25/18 |
| 6   | E11391725 | b2         | 1/25/18 |
| 7   | E11391726 | b2-ge      | 1/25/18 |
| 8   | E11391727 | c1         | 1/25/18 |
| 9   | E11391728 | c1-ge      | 1/25/18 |
| 10  | E11391729 | c2         | 1/25/18 |
| 11  | E11391730 | d1         | 1/25/18 |
| 12  | E11391731 | d1-ge      | 1/25/18 |
| 13  | E11391732 | d2         | 1/25/18 |
| 14  | E11391733 | d3-ge      | 1/25/18 |
| 15  | E11391734 | e1         | 1/25/18 |
| 16  | E11391735 | e1-ge      | 1/25/18 |
| 17  | E11391736 | e2         | 1/25/18 |
| 18  | E11391737 | g1         | 1/25/18 |
| 19  | E11391738 | g1-ge      | 1/25/18 |
| 20  | E11391739 | h1-ge      | 1/25/18 |
| 21  | E11391740 | h2         | 1/25/18 |
| 22  | E11391741 | j1         | 1/25/18 |
| 23  | E11391742 | j2         | 1/25/18 |
| 24  | E11391743 | j3         | 1/25/18 |
| 25  | E11391744 | k1-ge      | 1/25/18 |
| 26  | E11391745 | ve-1       | 1/25/18 |
| 27  | E11391746 | vh-1       | 1/25/18 |
| 28  | E11391747 | vh-2       | 1/25/18 |
| 29  | E11391748 | vh-3       | 1/25/18 |
| 30  | E11391749 | vh-4       | 1/25/18 |
| 31  | E11391750 | vh-5       | 1/25/18 |
| 32  | E11391751 | vh-6       | 1/25/18 |

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2018

North Carolina COA:C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



January 25, 2018



|                   |             |                            |          |          |                                     |           |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>A1 | Truss Type<br>Roof Special | Qty<br>7 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391720 |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:26 2018 Page 1

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5x5 =

Scale = 1:94.1

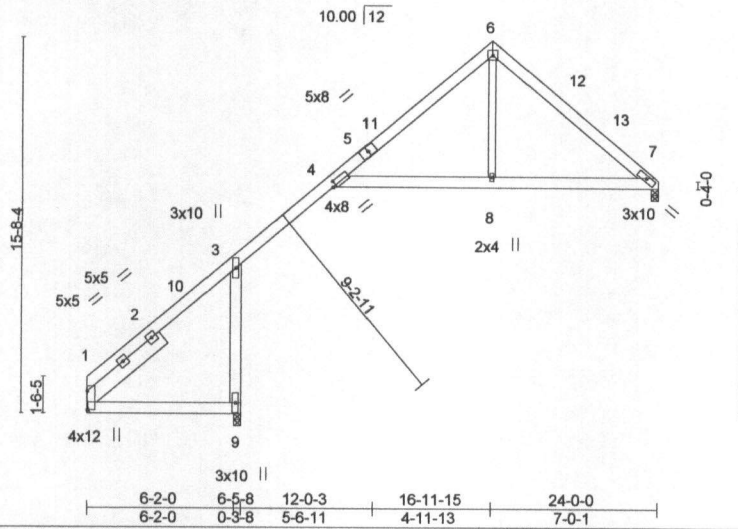


Plate Offsets (X,Y)- [1:Edge,0-0-0], [4:0-1-13,0-2-8]

| LOADING (psf) | SPACING-             | CSI.     | DEFL.    | in (loc) | l/defl | L/d  | PLATES         | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|------|----------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.58  | Vert(LL) | -0.25    | 4-8    | >845 | MT20           | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.50  | Vert(TL) | -0.59    | 4-8    | >360 |                |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.15  | Horz(TL) | 0.46     | 7      | n/a  |                |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-S | Wind(LL) | 0.24     | 4-8    | >864 |                |          |
|               | Code IRC2009/TP12007 |          |          |          |        |      | Weight: 155 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x6 SP No.1 \*Except\*  
1-5: 2x6 SP 2400F 2.0E  
BOT CHORD 2x6 SP No.1  
WEBS 2x4 SP No.3  
SLIDER Left 2x8 SP No.1 4-2-8

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.**

(lb/size) 7=622/0-3-8, 1=41/Mechanical, 9=1164/0-3-8  
Max Horz 1=409(LC 6)  
Max Uplift 7=-22(LC 7), 1=-163(LC 5), 9=-451(LC 7)  
Max Grav 7=622(LC 1), 1=372(LC 6), 9=1164(LC 1)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
TOP CHORD 1-3=-518/327, 3-4=-447/83, 4-6=-694/166, 6-7=-776/188  
BOT CHORD 3-9=-1128/568, 4-8=-10/512, 7-8=-10/512  
WEBS 6-8=0/398

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 16-11-15, Exterior(2) 16-11-15 to 21-4-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7 except (jt=lb) 1=163, 9=451.



January 25, 2018

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSIP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

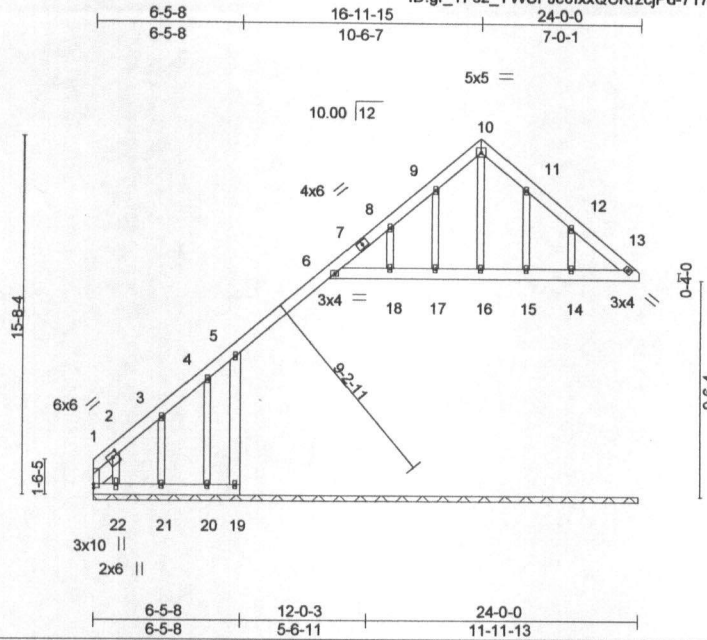


818 Soundside Road  
Edenton, NC 27932

|                   |                |                     |          |          |                                                 |
|-------------------|----------------|---------------------|----------|----------|-------------------------------------------------|
| Job<br>J0918-4409 | Truss<br>A1-GE | Truss Type<br>GABLE | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Hamett County<br>E11391721 |
|-------------------|----------------|---------------------|----------|----------|-------------------------------------------------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:26 2018 Page 1  
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Scale = 1:98.5

Plate Offsets (X,Y)-- [1:Edge,0-0-0], [2:0-2-12,0-2-4]

| LOADING (psf) | SPACING-             | 2-0-0 | CSI.     | DEFL.    | in (loc) | l/defl | L/d | PLATES         | GRIP     |
|---------------|----------------------|-------|----------|----------|----------|--------|-----|----------------|----------|
| TCLL 20.0     | Plate Grip DOL       | 1.15  | TC 0.07  | Vert(LL) | n/a      | -      | n/a | MT20           | 244/190  |
| TCDL 10.0     | Lumber DOL           | 1.15  | BC 0.03  | Vert(TL) | n/a      | -      | n/a |                |          |
| BCLL 0.0 *    | Rep Stress Incr      | YES   | WB 0.14  | Horz(TL) | -0.01    | 13     | n/a |                |          |
| BCDL 10.0     | Code IRC2009/TP12007 |       | Matrix-S |          |          |        |     | Weight: 176 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
BOT CHORD 2x6 SP No.1  
OTHERS 2x4 SP No.3  
SLIDER Left 2x8 SP No.1 1-4-15

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purfins.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.**

All bearings 24-0-0.  
(lb) - Max Horz 1=519(LC 7)  
Max Uplift All uplift 100 lb or less at joint(s) 20, 15 except 1=399(LC 5), 19=207(LC 7), 6=178(LC 7), 17=104(LC 7), 18=103(LC 7), 21=140(LC 7), 22=536(LC 7), 14=160(LC 8)  
Max Grav All reactions 250 lb or less at joint(s) 13, 19, 16, 17, 18, 20, 21, 15, 14 except 1=845(LC 7), 6=281(LC 1), 22=257(LC 5)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=921/446, 2-3=555/307, 3-4=465/286, 4-5=428/286, 5-6=338/279  
WEBS 2-22=201/474

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 15 except (t=lb) 1=399, 19=207, 6=178, 17=104, 18=103, 21=140, 22=536, 14=160.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 13, 6, 16, 17, 18, 15, 14.



January 25, 2018

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



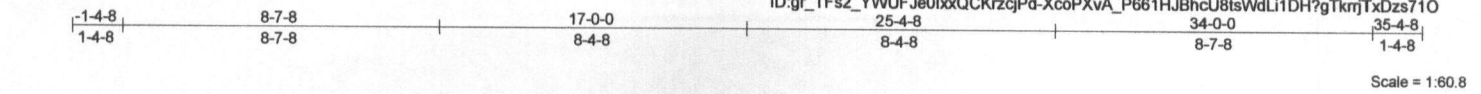
818 Soundside Road  
Edenton, NC 27932



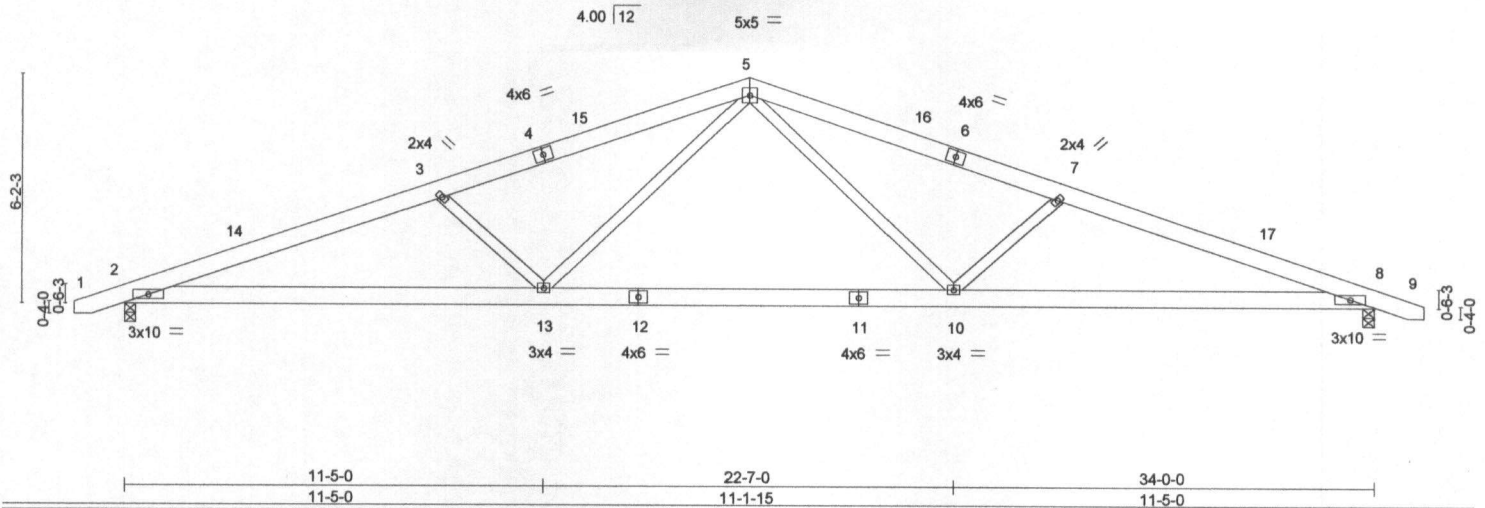
|                   |             |                      |          |          |                                     |           |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>B1 | Truss Type<br>COMMON | Qty<br>4 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391723 |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:29 2018 Page 1  
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Scale = 1:60.8



|                      |                      |             |                               |                |             |
|----------------------|----------------------|-------------|-------------------------------|----------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>                  | <b>PLATES</b>  | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.31     | in (loc) l/defl L/d           | MT20           | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.55     | Vert(LL) -0.15 10-13 >999 360 |                |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.38     | Vert(TL) -0.41 10-13 >993 240 |                |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-S    | Horz(TL) 0.10 8 n/a n/a       |                |             |
|                      | Code IRC2009/TPI2007 |             | Wind(LL) 0.10 10-13 >999 240  | Weight: 204 lb | FT = 20%    |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 3-11-15 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.**

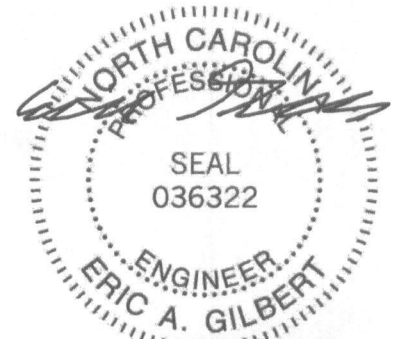
(lb/size) 2=1426/0-3-8, 8=1426/0-3-8  
 Max Horz 2=80(LC 7)  
 Max Uplift 2=150(LC 5), 8=150(LC 6)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-3258/675, 3-5=-2849/571, 5-7=-2849/571, 7-8=-3258/675  
 BOT CHORD 2-13=-554/3031, 10-13=-289/1998, 8-10=-565/3031  
 WEBS 5-10=-89/911, 7-10=-592/295, 5-13=-89/911, 3-13=-592/295

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-1-13 to 3-3-0, Interior(1) 3-3-0 to 17-0-0, Exterior(2) 17-0-0 to 21-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=150, 8=150.



January 25, 2018

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI1-7473 rev. 10/03/2015 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



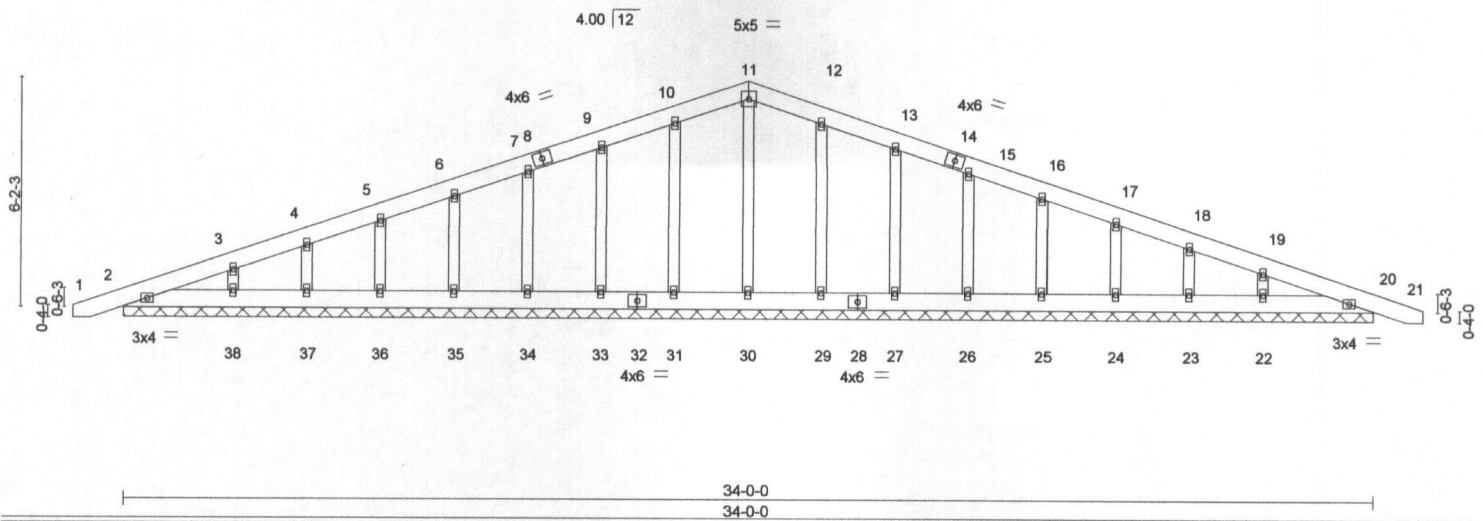
818 Soundside Road  
 Edenton, NC 27932

|                   |                |                                    |          |          |                                     |           |
|-------------------|----------------|------------------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>B1-GE | Truss Type<br>COMMON SUPPORTED GAB | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391724 |
|-------------------|----------------|------------------------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:30 2018 Page 1  
 ID:gr\_TFs2\_YWUFJe0IxxQCKrzCjPd-?pMnkEBcAPEuuTtACf6PjAauQhncCdu3VSOtfs71N  
 34-0-0 35-4-8  
 17-0-0 1-4-8

Scale = 1:60.8



|                      |                       |             |                                  |                |             |
|----------------------|-----------------------|-------------|----------------------------------|----------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b> 2-0-0 | <b>CSI.</b> | <b>DEFL.</b> in (loc) l/defl L/d | <b>PLATES</b>  | <b>GRIP</b> |
| TCLL 20.0            | Plate Grip DOL 1.15   | TC 0.03     | Vert(LL) -0.00 20 n/r 120        | MT20           | 244/190     |
| TCDL 10.0            | Lumber DOL 1.15       | BC 0.02     | Vert(TL) 0.00 20 n/r 120         |                |             |
| BCLL 0.0 *           | Rep Stress Incr YES   | WB 0.06     | Horz(TL) 0.00 20 n/a n/a         |                |             |
| BCDL 10.0            | Code IRC2009/TPI2007  | Matrix-S    |                                  | Weight: 233 lb | FT = 20%    |

**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 OTHERS 2x4 SP No.3

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 34-0-0.  
 (lb) - Max Horz 2=97(LC 5)  
 Max Uplift All uplift 100 lb or less at joint(s) 2, 31, 33, 34, 35, 36, 37, 38, 29, 27, 26, 25, 24, 23, 22, 20  
 Max Grav All reactions 250 lb or less at joint(s) 2, 30, 31, 33, 34, 35, 36, 37, 38, 29, 27, 26, 25, 24, 23, 22, 20

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - All plates are 2x4 MT20 unless otherwise indicated.
  - Gable requires continuous bottom chord bearing.
  - Gable studs spaced at 2-0-0 oc.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 31, 33, 34, 35, 36, 37, 38, 29, 27, 26, 25, 24, 23, 22, 20.



January 25, 2018

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 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

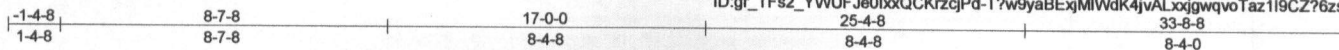
ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate  
 818 Soundside Road  
 Edenton, NC 27932

|                   |             |                      |          |          |                                     |           |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>B2 | Truss Type<br>COMMON | Qty<br>5 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391725 |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:31 2018 Page 1  
ID:gr\_TFs2\_YWUFJe0lxQCKRzCjPd-T?w9yaBExiMIWdK4jvALxjgwqvoTaz119CZ?6zs71M

Job Reference (optional)



Scale = 1:58.8

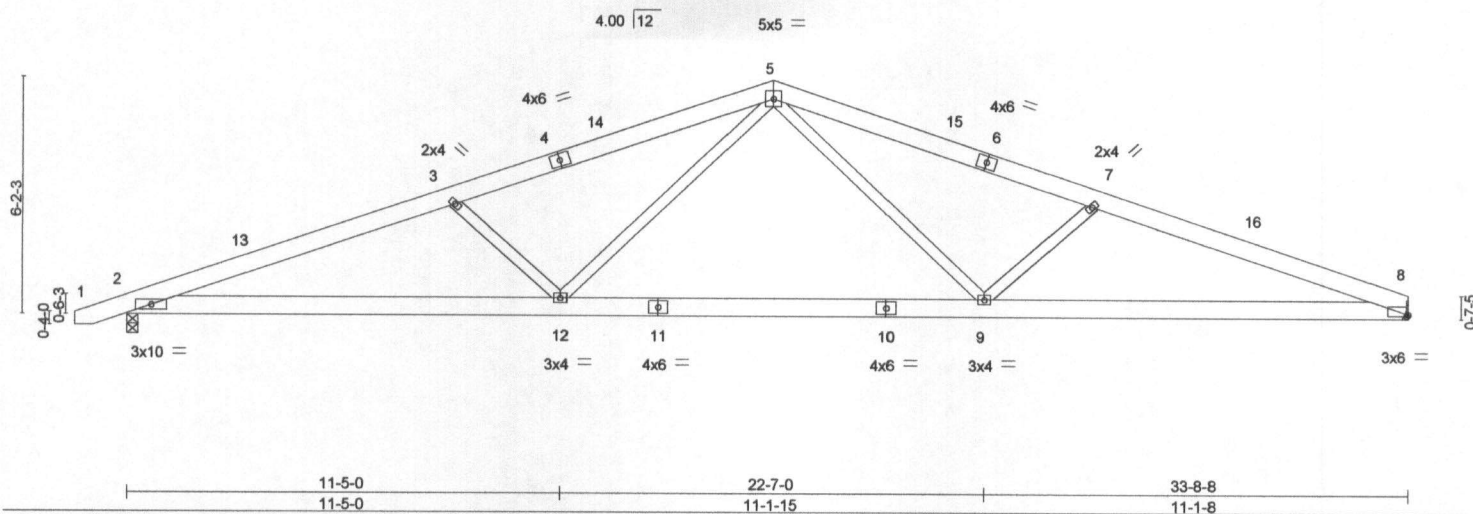


Plate Offsets (X, Y) - [8:0-0-7, 0-0-9]

| LOADING (psf) | SPACING-             | CSL      | DEFL.    | in (loc) | l/defl | L/d  | PLATES         | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|------|----------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.33  | Vert(LL) | -0.14    | 9-12   | >999 | MT20           | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.55  | Vert(TL) | -0.40    | 9-12   | >999 |                |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.38  | Horz(TL) | 0.10     | 8      | n/a  |                |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-S | Wind(LL) | 0.10     | 9-12   | >999 |                |          |
|               | Code IRC2009/TPI2007 |          |          |          |        |      | Weight: 200 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
BOT CHORD 2x6 SP No.1  
WEBS 2x4 SP No.3

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=1418/0-3-8, 8=1338/Mechanical  
Max Horz 2=85(LC 7)  
Max Uplift 2=150(LC 5), 8=97(LC 6)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3235/683, 3-5=-2825/579, 5-7=-2800/594, 7-8=-3193/693  
BOT CHORD 2-12=-582/3009, 9-12=-306/1975, 8-9=-575/2963  
WEBS 5-9=-87/883, 7-9=-565/292, 5-12=-89/911, 3-12=-592/296

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-1-13 to 3-3-0, Interior(1) 3-3-0 to 17-0-0, Exterior(2) 17-0-0 to 21-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 2=150.



January 25, 2018

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**

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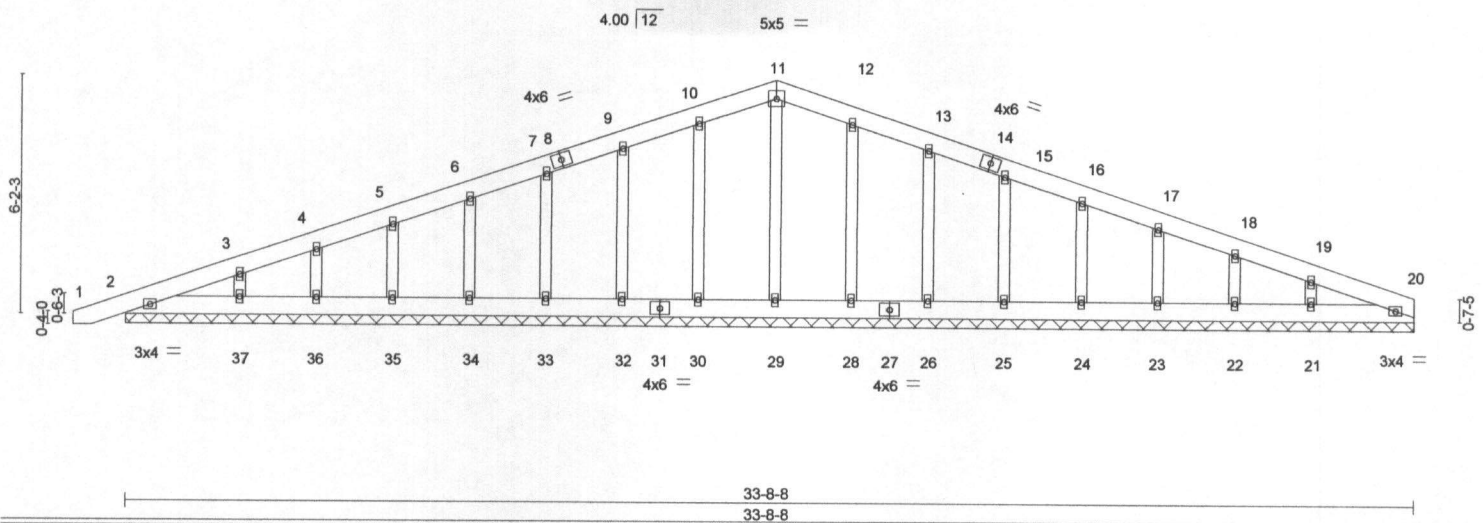
818 Soundside Road  
Edenton, NC 27932

|                   |                |                                    |          |          |                                                  |
|-------------------|----------------|------------------------------------|----------|----------|--------------------------------------------------|
| Job<br>J0918-4409 | Truss<br>B2-GE | Truss Type<br>COMMON SUPPORTED GAB | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County<br>E11391726 |
|-------------------|----------------|------------------------------------|----------|----------|--------------------------------------------------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:32 2018 Page 1  
 ID:gr\_TFs2\_YWUFJe0lxxQCKrzcjPd-yBUJ9wCti1Uc8mvGHchaU8FwNEMFC66Bxpx7XYzs71L  
 33-8-8  
 16-8-8

Scale = 1:58.5



|                      |                      |             |                          |                |             |
|----------------------|----------------------|-------------|--------------------------|----------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>             | <b>PLATES</b>  | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.03     | in (loc) l/defl L/d      | MT20           | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.02     | Vert(LL) -0.00 1 n/r 120 |                |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.06     | Vert(TL) 0.00 1 n/r 120  |                |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-S    | Horz(TL) 0.00 20 n/a n/a |                |             |
|                      | Code IRC2009/TPI2007 |             |                          | Weight: 228 lb | FT = 20%    |

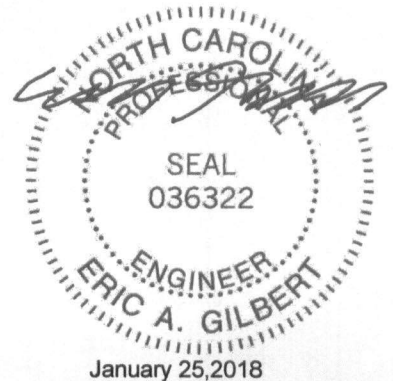
**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 OTHERS 2x4 SP No.3

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 33-8-8.  
 (lb) - Max Horz 2=106(LC 5)  
 Max Uplift All uplift 100 lb or less at joint(s) 2, 30, 32, 33, 34, 35, 36, 37, 28, 26, 25, 24, 23, 22, 21  
 Max Grav All reactions 250 lb or less at joint(s) 2, 29, 30, 32, 33, 34, 35, 36, 37, 28, 26, 25, 24, 23, 22, 21, 20

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - All plates are 2x4 MT20 unless otherwise indicated.
  - Gable requires continuous bottom chord bearing.
  - Gable studs spaced at 2-0-0 oc.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 30, 32, 33, 34, 35, 36, 37, 28, 26, 25, 24, 23, 22, 21.



|                   |             |                      |          |          |                                    |           |
|-------------------|-------------|----------------------|----------|----------|------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>C1 | Truss Type<br>COMMON | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Hamett County | E11391727 |
|-------------------|-------------|----------------------|----------|----------|------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

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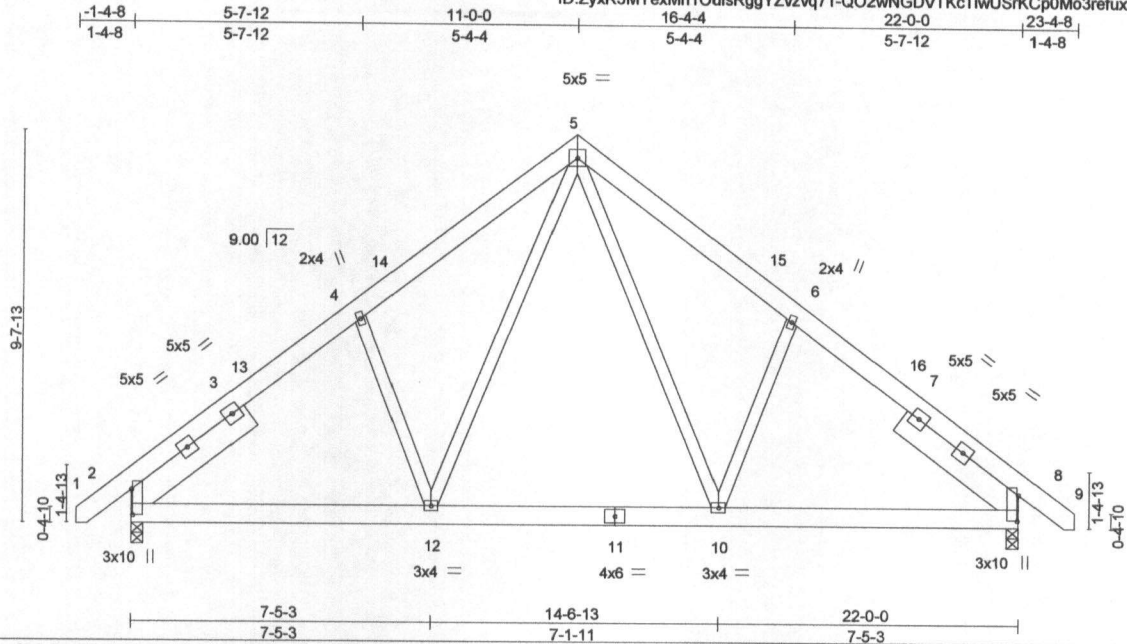


Plate Offsets (X,Y) - [2:0-7-11,0-0-5], [8:0-7-11,0-0-5]

|                      |                      |             |                               |                |             |
|----------------------|----------------------|-------------|-------------------------------|----------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>                  | <b>PLATES</b>  | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.18     | in (loc) l/defl L/d           | MT20           | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.25     | Vert(LL) -0.06 10-12 >999 360 |                |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.24     | Vert(TL) -0.10 10-12 >999 240 |                |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-S    | Horz(TL) 0.02 8 n/a n/a       |                |             |
|                      | Code IRC2009/TPI2007 |             | Wind(LL) 0.01 12 >999 240     | Weight: 190 lb | FT = 20%    |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3  
 SLIDER Left 2x8 SP No.1 3-8-2, Right 2x8 SP No.1 3-8-2

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.**

(lb/size) 2=1098/0-3-8, 8=1098/0-3-8  
 Max Horz 2=254(LC 6)  
 Max Uplift 2=-84(LC 7), 8=-84(LC 8)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-4=-1307/241, 4-5=-1109/341, 5-6=-1109/341, 6-8=-1307/241  
 BOT CHORD 2-12=-89/885, 10-12=0/660, 8-10=-57/885  
 WEBS 5-10=-137/504, 5-12=-137/504

**NOTES-**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



January 25, 2018

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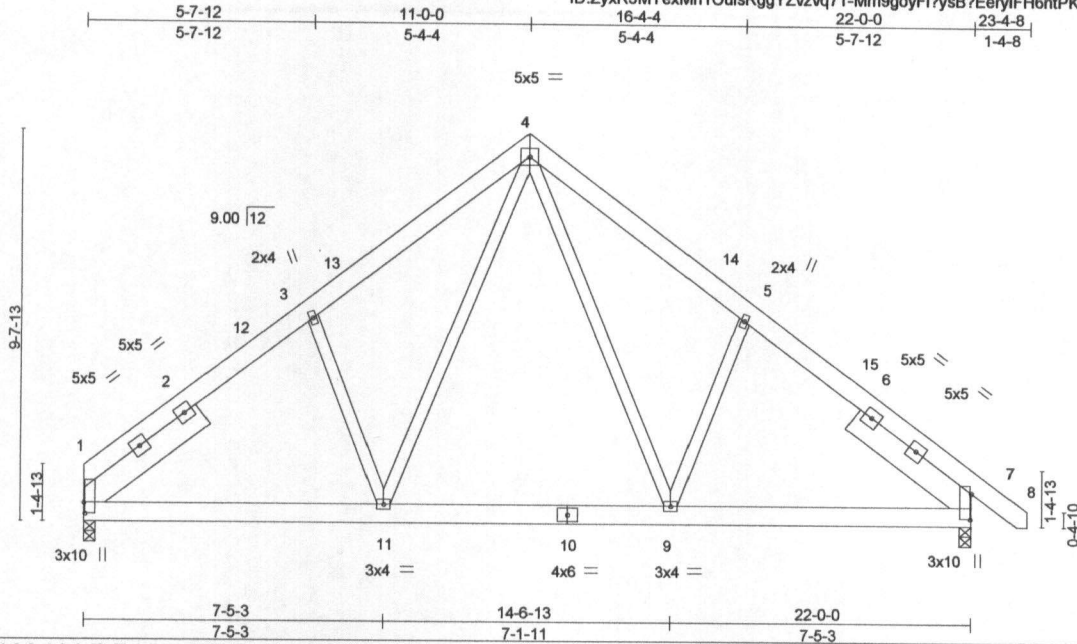


|                   |             |                      |          |          |                                     |           |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>C2 | Truss Type<br>COMMON | Qty<br>3 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391729 |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

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ID:ZyxR5MYexMn1OulsRggYZvzvq71-Mm9goyFI?ysB?EerylFH6ntPKRLNPPzdDmAn8tzs711



Scale = 1:55.5

Plate Offsets (X,Y)-- [1:0-3-4,0-0-5], [7:0-7-11,0-0-5]

| LOADING (psf) | SPACING-             | 2-0-0 | CSI.     | DEFL.    | in (loc)   | l/defl | L/d | PLATES         | GRIP     |
|---------------|----------------------|-------|----------|----------|------------|--------|-----|----------------|----------|
| TCLL 20.0     | Plate Grip DOL       | 1.15  | TC 0.18  | Vert(LL) | -0.06 9-11 | >999   | 360 | MT20           | 244/190  |
| TCDL 10.0     | Lumber DOL           | 1.15  | BC 0.25  | Vert(TL) | -0.09 9-11 | >999   | 240 |                |          |
| BCLL 0.0 *    | Rep Stress Incr      | YES   | WB 0.25  | Horz(TL) | 0.02 7     | n/a    | n/a |                |          |
| BCDL 10.0     | Code IRC2009/TPI2007 |       | Matrix-S | Wind(LL) | 0.01 11    | >999   | 240 |                |          |
|               |                      |       |          |          |            |        |     | Weight: 186 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3  
 SLIDER Left 2x8 SP No.1 3-8-2, Right 2x8 SP No.1 3-8-2

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.**

(lb/size) 1=1021/0-3-8, 7=1100/0-3-8  
 Max Horz 1=-255(LC 5)  
 Max Uplift 1=-35(LC 7), 7=-84(LC 8)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 1-3=-1315/252, 3-4=-1117/357, 4-5=-1112/342, 5-7=-1310/241  
 BOT CHORD 1-11=-90/893, 9-11=0/663, 7-9=-62/887  
 WEBS 4-9=-136/503, 4-11=-139/512

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.



January 25, 2018

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

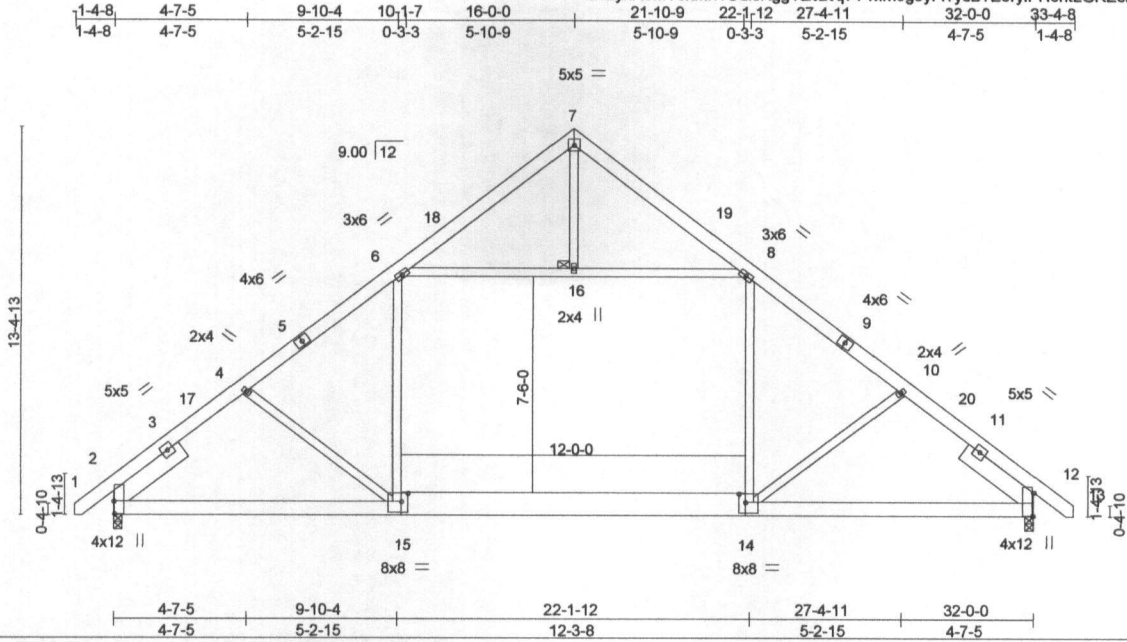


818 Soundside Road  
 Edenton, NC 27932

|                   |             |                          |          |          |                                     |           |
|-------------------|-------------|--------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>D1 | Truss Type<br>ROOF TRUSS | Qty<br>2 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391730 |
|-------------------|-------------|--------------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

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 ID:ZyxR5MYexMn1OulsRggYZvzvq71-Mm9goyFI?ysB?EerylFH6ntLGREcPHGdMAn8tzs711



Scale = 1:77.9

Plate Offsets (X,Y) - [2-0-5-4,0-0-5], [12-0-9-11,0-0-5], [14-0-2-12,0-3-8], [15-0-2-12,0-3-8]

| LOADING (psf) | SPACING-             | CSI.     | DEFL.    | in (loc) | l/defl | L/d  | PLATES         | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|------|----------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.44  | Vert(LL) | 0.29     | 2-15   | >999 | MT20           | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.68  | Horz(TL) | -0.34    | 14-15  | >999 |                |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.74  | Wind(LL) | 0.34     | 2-15   | >999 |                |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-S |          |          |        |      | Weight: 282 lb | FT = 20% |
|               | Code IRC2009/TPI2007 |          |          |          |        |      |                |          |

| LUMBER-                                                | BRACING-                                                                  |
|--------------------------------------------------------|---------------------------------------------------------------------------|
| TOP CHORD 2x6 SP No.1                                  | TOP CHORD Structural wood sheathing directly applied or 4-7-5 oc purlins. |
| BOT CHORD 2x6 SP No.1 *Except*                         | BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.            |
| WEBS 2x4 SP No.1 *Except*                              | JOINTS 1 Brace at Jt(s): 16                                               |
| SLIDER Left 2x8 SP No.1 3-0-5, Right 2x8 SP No.1 3-0-5 |                                                                           |

**REACTIONS.** (lb/size) 2=1715/0-3-8, 12=1715/0-3-8  
 Max Horz 2=358(LC 6)  
 Max Uplift 2=-101(LC 7), 12=-101(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-4=-2164/350, 4-6=-1998/348, 6-7=-428/169, 7-8=-428/169, 8-10=-1998/348, 10-12=-2163/350  
 BOT CHORD 2-15=-172/1506, 14-15=-20/1576, 12-14=-137/1506  
 WEBS 6-16=-1333/317, 8-16=-1333/317, 6-15=0/593, 8-14=0/593, 4-15=-147/259, 10-14=-149/261

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=101, 12=101.



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 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

**ENGINEERING BY**  
**TRENCO**  
 A MITEK AFFILIATE

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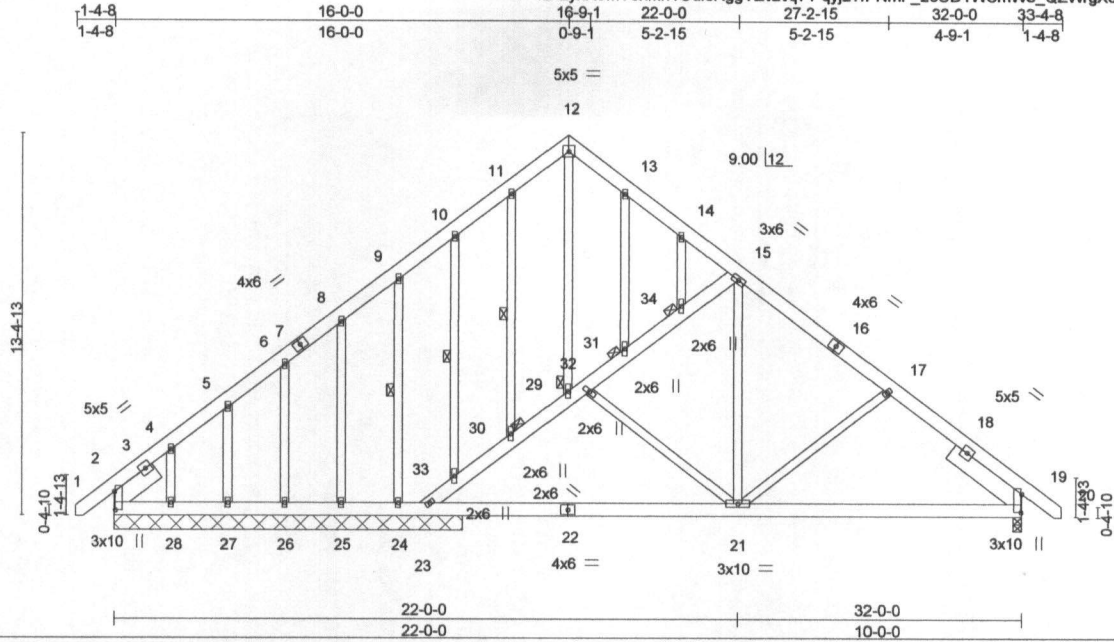
|                   |                |                         |          |          |                                    |           |
|-------------------|----------------|-------------------------|----------|----------|------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>D1-GE | Truss Type<br>QUEENPOST | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Hamett County | E11391731 |
|-------------------|----------------|-------------------------|----------|----------|------------------------------------|-----------|

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ID:ZyxR5MYexMn1OulsRggYZvzvq71-qyj2?IFNmF\_2cOD1WSmWe\_QZWrgX8somSQwKgJzs71H

Job Reference (optional)



Scale = 1:78.9

Plate Offsets (X,Y)-- [2:0-7-11,0-0-5], [19:0-7-11,0-0-5]

| LOADING (psf) | SPACING-             | CSI.     | DEFL.    | in (loc) | l/defl | L/d  | PLATES         | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|------|----------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.15  | Vert(LL) | -0.05    | 21-23  | >999 | MT20           | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.32  | Vert(TL) | -0.13    | 21-23  | >999 |                |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.21  | Horz(TL) | -0.02    | 2      | n/a  |                |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-S | Wind(LL) | 0.01     | 21     | >999 |                |          |
|               | Code IRC2009/TPI2007 |          |          |          |        |      | Weight: 343 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3 \*Except\*  
 15-23: 2x6 SP No.1  
 SLIDER Left 2x8 SP No.1 1-10-6, Right 2x8 SP No.1 3-1-6

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpt 11-30, 10-33, 9-24  
 JOINTS 1 Brace at Jt(s): 29, 30, 31, 34

**REACTIONS.**

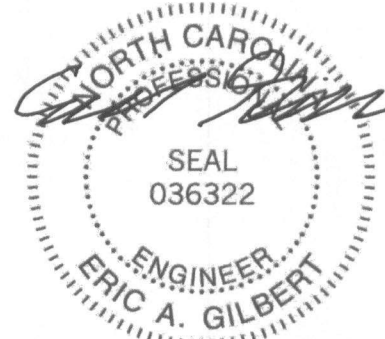
All bearings 12-3-8 except (jt=length) 19=0-3-8.  
 (lb) - Max Horz 19=448(LC 6)  
 Max Uplift All uplift 100 lb or less at joint(s) 2, 25, 27 except 24=473(LC 2), 26=102(LC 7), 28=260(LC 7), 19=216(LC 8)  
 Max Grav All reactions 250 lb or less at joint(s) 25, 26, 27, 28 except 2=376(LC 1), 23=1093(LC 2), 19=985(LC 1)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 14-15=315/185, 15-17=870/231, 17-19=1104/247, 2-4=459/90, 4-5=273/64, 9-10=277/134, 10-11=255/200  
 BOT CHORD 2-28=58/364, 27-28=58/364, 26-27=58/364, 25-26=58/364, 24-25=58/364, 23-24=58/364, 21-23=226/709, 19-21=329/740  
 WEBS 23-33=722/329, 30-33=664/295, 29-30=642/277, 29-32=663/342, 31-32=643/342, 31-34=589/300, 15-34=591/293, 15-21=10/436, 4-28=67/261

**NOTES-**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 25, 27 except (jt=lb) 24=473, 26=102, 28=260, 19=216.



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ENGINEERING BY  
**TRENCO**  
 A MiTek Affiliate  
 818 Soundside Road  
 Edenton, NC 27932

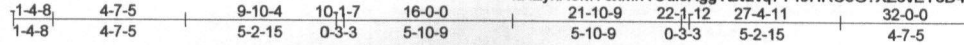
|                   |             |                          |          |          |                                     |           |
|-------------------|-------------|--------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>D2 | Truss Type<br>ROOF TRUSS | Qty<br>8 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391732 |
|-------------------|-------------|--------------------------|----------|----------|-------------------------------------|-----------|

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ID:ZyxR5MYexMn1OulsRggYZvzvq71-19HRCeG7XZ6vEYoD4AHIBCzg0FsNtC\_wg4fuDzsz71G

Job Reference (optional)



Scale = 1:77.9

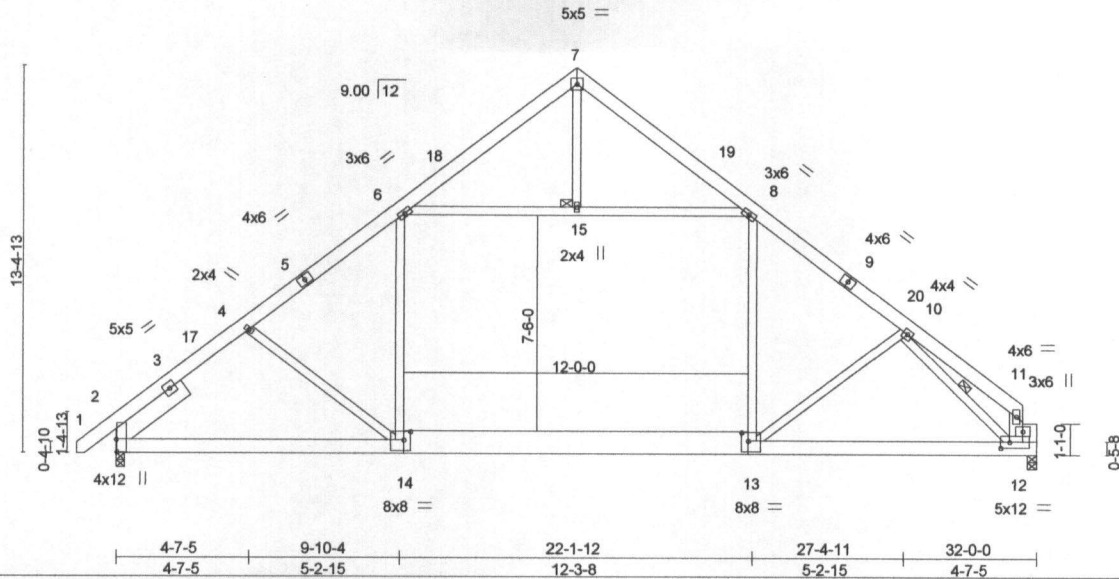


Plate Offsets (X,Y)- [2:0-5-4,0-0-5], [12:0-3-12,0-2-8], [13:0-2-12,0-3-8], [14:0-2-12,0-3-8]

|                      |                      |             |                              |                |             |
|----------------------|----------------------|-------------|------------------------------|----------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>                 | <b>PLATES</b>  | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.42     | in (loc) l/defl L/d          | MT20           | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.92     | Vert(LL) -0.32 14 >999 360   |                |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.66     | Vert(TL) -0.42 2-14 >901 240 |                |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-S    | Horz(TL) 0.04 12 n/a n/a     |                |             |
|                      | Code IRC2009/TP12007 |             | Wind(LL) 0.35 2-14 >999 240  | Weight: 279 lb | FT = 20%    |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1 \*Except\*  
 13-14: 2x10 SP No.1  
 WEBS 2x4 SP No.3 \*Except\*  
 6-8,6-14,8-13: 2x4 SP No.1  
 OTHERS 2x6 SP No.1  
 SLIDER Left 2x8 SP No.1 3-0-5

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.  
 WEBS 1 Row at midpt 10-12  
 JOINTS 1 Brace at Jt(s): 15

**REACTIONS.**

(lb/size) 2=1681/0-3-8, 12=1619/0-3-8  
 Max Horz 2=358(LC 6)  
 Max Uplift 2=-100(LC 7), 12=-49(LC 8)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-4=-2120/342, 4-6=-1927/338, 6-7=-423/168, 7-8=-438/170, 8-10=-1919/345,  
 10-11=-276/86  
 BOT CHORD 2-14=-187/1470, 13-14=-68/1508, 12-13=-177/1321  
 WEBS 6-15=-1260/313, 8-15=-1260/313, 6-14=0/581, 8-13=0/558, 4-14=-150/252,  
 10-13=-126/350, 10-12=-1802/263

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-3-0 to 3-1-13, Interior(1) 3-1-13 to 16-0-0, Exterior(2) 16-0-0 to 20-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12 except (jt=lb) 2=100.



January 25, 2018

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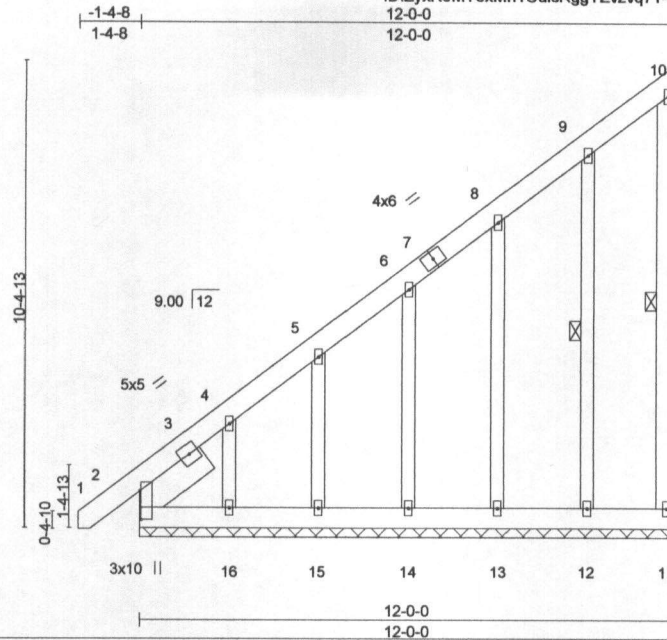
|                   |                |                                   |          |          |                                     |           |
|-------------------|----------------|-----------------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>D3-GE | Truss Type<br>MONOPITCH SUPPORTED | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391733 |
|-------------------|----------------|-----------------------------------|----------|----------|-------------------------------------|-----------|

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Job Reference (optional)



Scale = 1:50.0

Plate Offsets (X,Y)- [2-0-7-11,0-0-5]

| LOADING (psf) | SPACING-             | CSL.     | DEFL.    | in (loc) | l/defl | L/d | PLATES         | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|-----|----------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.07  | Vert(LL) | 0.00     | 1      | n/r | MT20           | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.02  | Vert(TL) | -0.00    | 1      | n/r |                |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.11  | Horz(TL) | -0.00    | 11     | n/a |                |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-S |          |          |        |     | Weight: 132 lb | FT = 20% |
|               | Code IRC2009/TP12007 |          |          |          |        |     |                |          |

**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x6 SP No.1  
 OTHERS 2x4 SP No.3  
 SLIDER Left 2x8 SP No.1 1-10-6

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpt 10-11, 9-12

**REACTIONS.** All bearings 12-0-0.  
 (lb) - Max Horz 2=452(LC 7)  
 Max Uplift All uplift 100 lb or less at joint(s) 11, 2, 12, 14, 15 except 13=102(LC 7), 16=306(LC 7)  
 Max Grav All reactions 250 lb or less at joint(s) 11, 12, 13, 14, 15, 16 except 2=383(LC 7)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-4=598/59, 4-5=379/38, 5-6=299/35  
 WEBS 4-16=103/310

- NOTES-**
- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - 3) All plates are 2x4 MT20 unless otherwise indicated.
  - 4) Gable requires continuous bottom chord bearing.
  - 5) Gable studs spaced at 2-0-0 oc.
  - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
  - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 2, 12, 14, 15 except (jt=lb) 13=102, 16=306.



January 25, 2018

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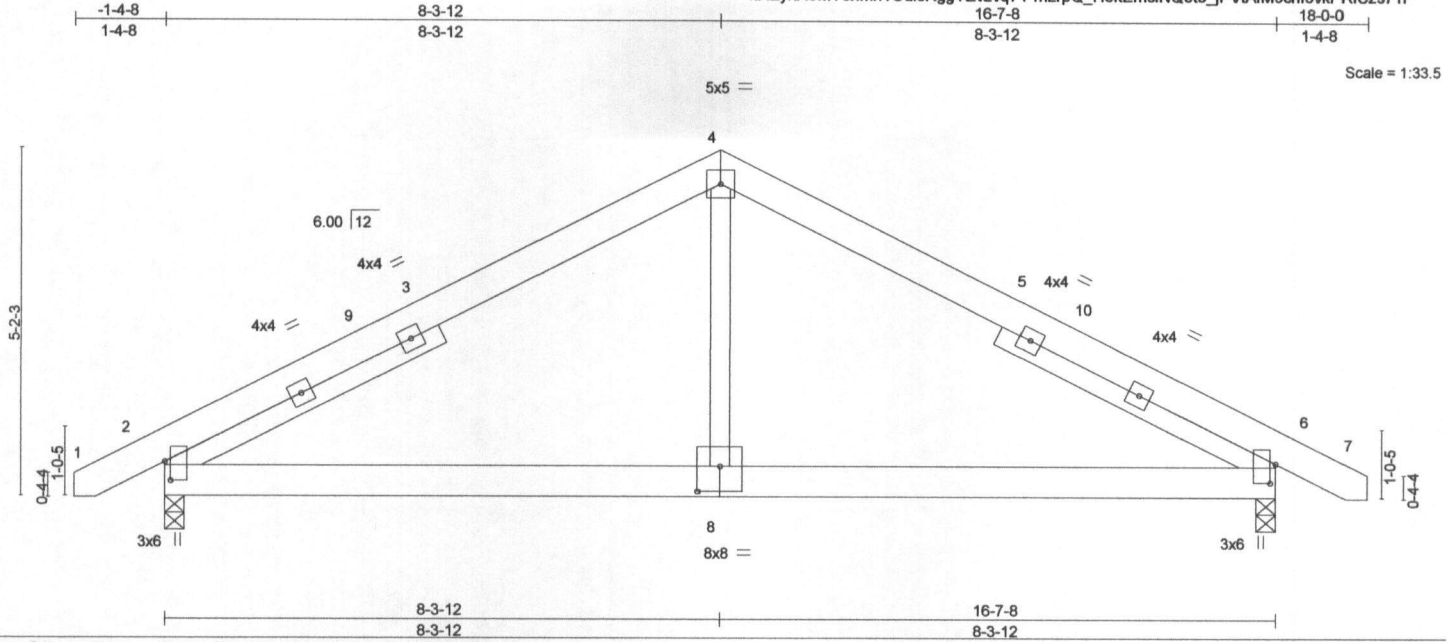


818 Soundside Road  
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|                   |             |                      |          |          |                                     |           |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>E1 | Truss Type<br>COMMON | Qty<br>4 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391734 |
|-------------------|-------------|----------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:38 2018 Page 1  
ID:ZyxR5MYexMn1OulsRggYZvzvq71-mLrpQ\_HeltEmsinQeto\_jPVIAfM9cni3vkPRICzs71F



Scale = 1:33.5

Plate Offsets (X,Y)-- [2:0-3-7,0-1-0], [6:0-3-7,0-1-0], [8:0-4-0,0-4-8]

| LOADING (psf) | SPACING-             | CSI.     | DEFL.    | in (loc) | l/defl | L/d  | PLATES         | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|------|----------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.27  | Vert(LL) | -0.02    | 2-8    | >999 | MT20           | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.24  | Vert(TL) | -0.07    | 2-8    | >999 |                |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.15  | Horz(TL) | 0.01     | 6      | n/a  |                |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-S | Wind(LL) | 0.01     | 2-8    | >999 |                |          |
|               | Code IRC2009/TPI2007 |          |          |          |        |      | Weight: 110 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3  
 SLIDER Left 2x4 SP No.3 4-7-1, Right 2x4 SP No.3 4-7-1

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.**

(lb/size) 2=738/0-3-8, 6=738/0-3-8  
 Max Horz 2=-59(LC 5)  
 Max Uplift 2=-84(LC 7), 6=-84(LC 8)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-4=-880/225, 4-6=-880/225  
 BOT CHORD 2-8=-53/655, 6-8=-53/655  
 WEBS 4-8=0/384

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-2-10 to 3-2-3, Interior(1) 3-2-3 to 8-3-12, Exterior(2) 8-3-12 to 12-8-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



January 25, 2018

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**

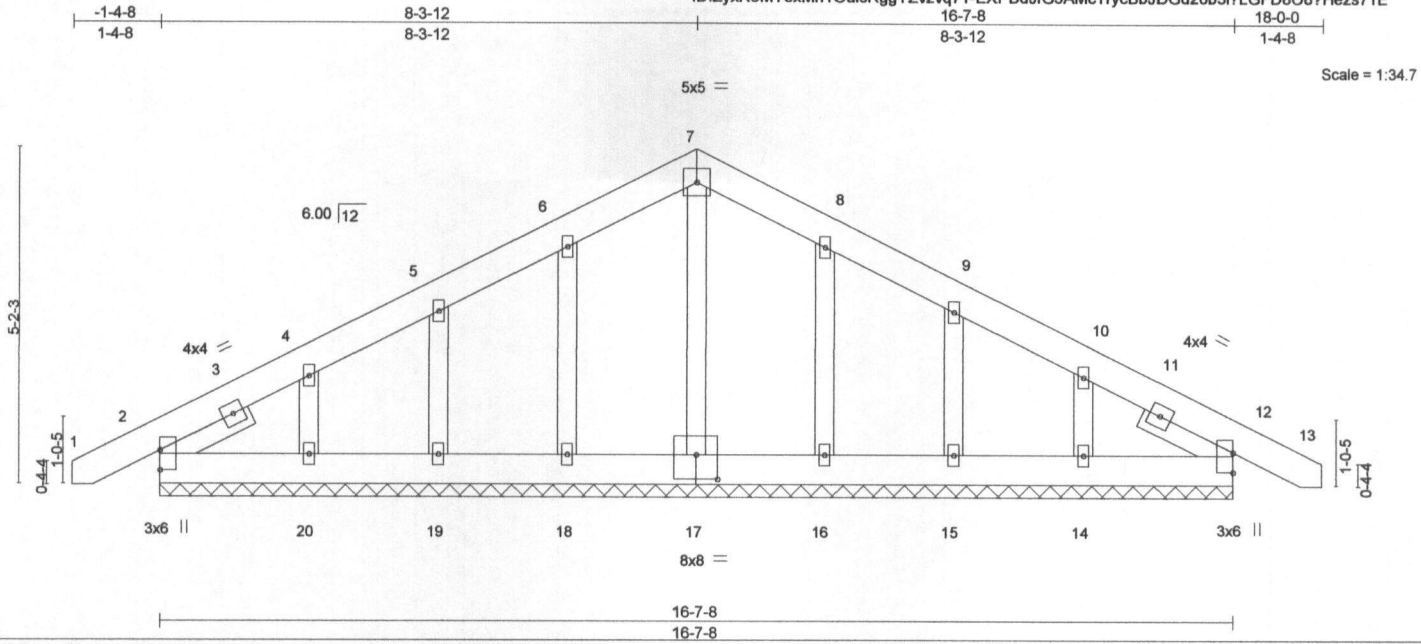
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



818 Soundside Road  
 Edenton, NC 27932

|                                       |                |                                    |          |          |                                    |           |
|---------------------------------------|----------------|------------------------------------|----------|----------|------------------------------------|-----------|
| Job<br>J0918-4409                     | Truss<br>E1-GE | Truss Type<br>COMMON SUPPORTED GAB | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Hamett County | E11391735 |
| Comtech, Inc., Fayetteville, NC 28309 |                |                                    |          |          | Job Reference (optional)           |           |

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:39 2018 Page 1  
 ID:ZyxR5MYexMn1OulsRggYZvzvq71-EXPBdJIG3AMcTrycBbJDGd26b3l?LGFD8O8?Hezs71E



Scale = 1:34.7

Plate Offsets (X,Y) - [17:0-4-0,0-4-8]

|                      |                       |             |                |          |        |     |                |             |
|----------------------|-----------------------|-------------|----------------|----------|--------|-----|----------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b> 2-0-0 | <b>CSI.</b> | <b>DEFL.</b>   | in (loc) | l/defl | L/d | <b>PLATES</b>  | <b>GRIP</b> |
| TCLL 20.0            | Plate Grip DOL 1.15   | TC 0.03     | Vert(LL) -0.00 | 12       | n/r    | 120 | MT20           | 244/190     |
| TCDL 10.0            | Lumber DOL 1.15       | BC 0.01     | Vert(TL) -0.00 | 13       | n/r    | 120 |                |             |
| BCLL 0.0 *           | Rep Stress Incr YES   | WB 0.04     | Horz(TL) 0.00  | 12       | n/a    | n/a |                |             |
| BCDL 10.0            | Code IRC2009/TPI2007  | Matrix-S    |                |          |        |     | Weight: 121 lb | FT = 20%    |

**LUMBER-**

- TOP CHORD 2x6 SP No.1
- BOT CHORD 2x6 SP No.1
- OTHERS 2x4 SP No.3
- SLIDER Left 2x4 SP No.3 1-6-8, Right 2x4 SP No.3 1-6-8

**BRACING-**

- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
- BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

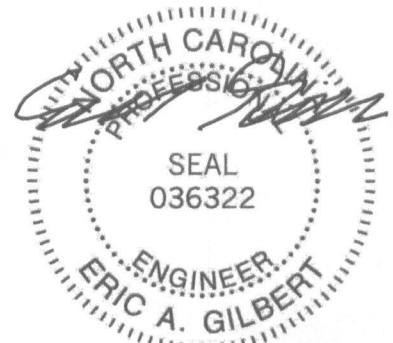
**REACTIONS.**

- All bearings 16-7-8.
- (lb) - Max Horz 2=-69(LC 8)
- Max Uplift All uplift 100 lb or less at joint(s) 12, 18, 19, 20, 16, 15, 14, 2
- Max Grav All reactions 250 lb or less at joint(s) 12, 17, 18, 19, 20, 16, 15, 14, 2

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 12, 18, 19, 20, 16, 15, 14, 2.



January 25, 2018

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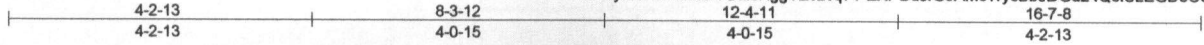


|                   |             |                             |          |          |                                    |           |
|-------------------|-------------|-----------------------------|----------|----------|------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>E2 | Truss Type<br>COMMON GIRDER | Qty<br>1 | Ply<br>2 | Lot 38 South Creek / Hamett County | E11391736 |
|-------------------|-------------|-----------------------------|----------|----------|------------------------------------|-----------|

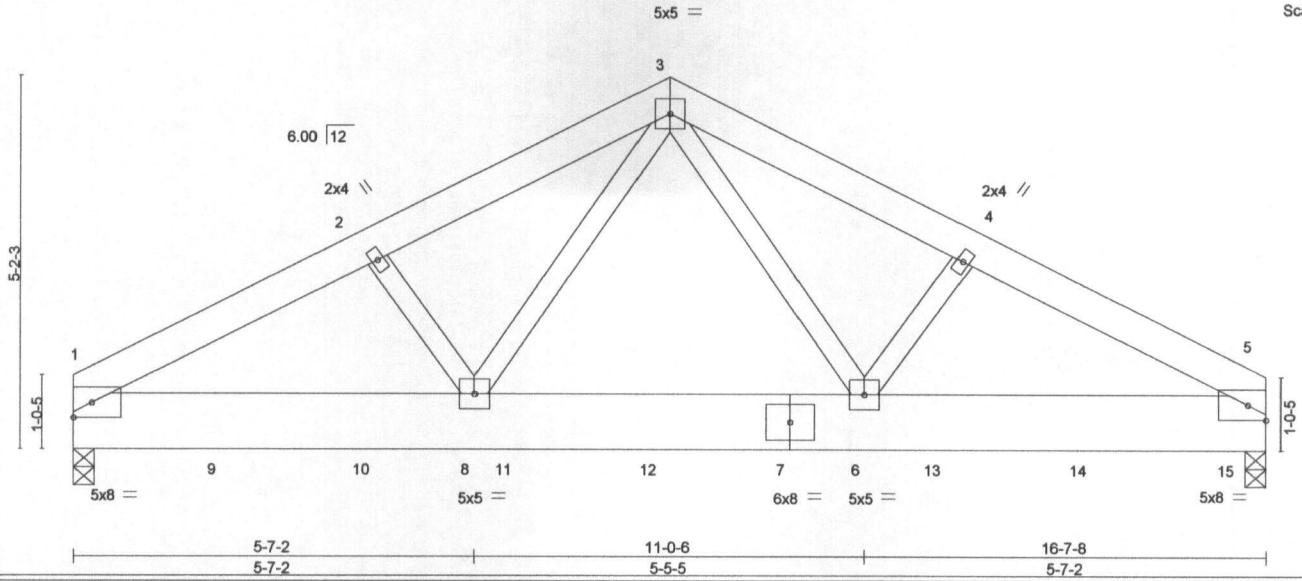
Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:39 2018 Page 1

ID:ZyxR5MYexMn1OuisRggYZvzvq71-EXPBdJIG3AMcTrycBbJDGd24Q3iSLEGD8087Hezs71E



Scale = 1:31.2



|                      |                      |       |             |              |       |       |        |     |                |             |
|----------------------|----------------------|-------|-------------|--------------|-------|-------|--------|-----|----------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | 2-0-0 | <b>CSI.</b> | <b>DEFL.</b> | in    | (loc) | l/defl | L/d | <b>PLATES</b>  | <b>GRIP</b> |
| TCLL 20.0            | Plate Grip DOL       | 1.15  | TC 0.17     | Vert(LL)     | -0.01 | 6-8   | >999   | 360 | MT20           | 244/190     |
| TCDL 10.0            | Lumber DOL           | 1.15  | BC 0.24     | Vert(TL)     | -0.02 | 6-8   | >999   | 240 |                |             |
| BCLL 0.0 *           | Rep Stress Incr      | NO    | WB 0.16     | Horz(TL)     | 0.01  | 5     | n/a    | n/a |                |             |
| BCDL 10.0            | Code IRC2009/TPI2007 |       | Matrix-S    | Wind(LL)     | -0.03 | 6-8   | >999   | 240 | Weight: 262 lb | FT = 20%    |

**LUMBER-**  
TOP CHORD 2x6 SP No.1  
BOT CHORD 2x10 SP No.1  
WEBS 2x4 SP No.3

**BRACING-**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=794/0-3-8, 5=874/0-3-8  
Max Horz 1=58(LC 11)  
Max Uplift 1=675(LC 5), 5=962(LC 6)  
Max Grav 1=1392(LC 11), 5=2010(LC 12)

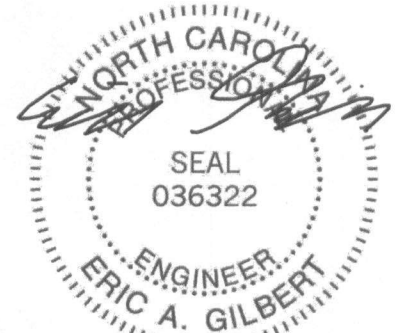
**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
TOP CHORD 1-2=-2057/1023, 2-3=-2044/1045, 3-4=-2071/1057, 4-5=-2086/1035  
BOT CHORD 1-8=-877/1810, 6-8=-603/1378, 5-6=-834/1776  
WEBS 3-6=-534/1102, 4-6=-150/313, 3-8=-514/1058, 2-8=-150/315

**NOTES-**

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.  
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-7-0 oc.  
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=675, 5=962.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 360 lb down and 175 lb up at 2-0-12, 360 lb down and 175 lb up at 4-0-12, 360 lb down and 175 lb up at 6-0-12, 360 lb down and 175 lb up at 8-0-12, 360 lb down and 175 lb up at 10-0-12, 360 lb down and 175 lb up at 12-0-12, and 360 lb down and 175 lb up at 14-0-12, and 730 lb down and 340 lb up at 16-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard

- Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15  
Uniform Loads (plf)  
Vert: 1-3=-60, 3-5=-60, 1-5=-20  
Concentrated Loads (lb)  
Vert: 7=-38(F) 9=-38(F) 10=-38(F) 11=-38(F) 12=-38(F) 13=-38(F) 14=-38(F) 15=-93(F)



January 25, 2018

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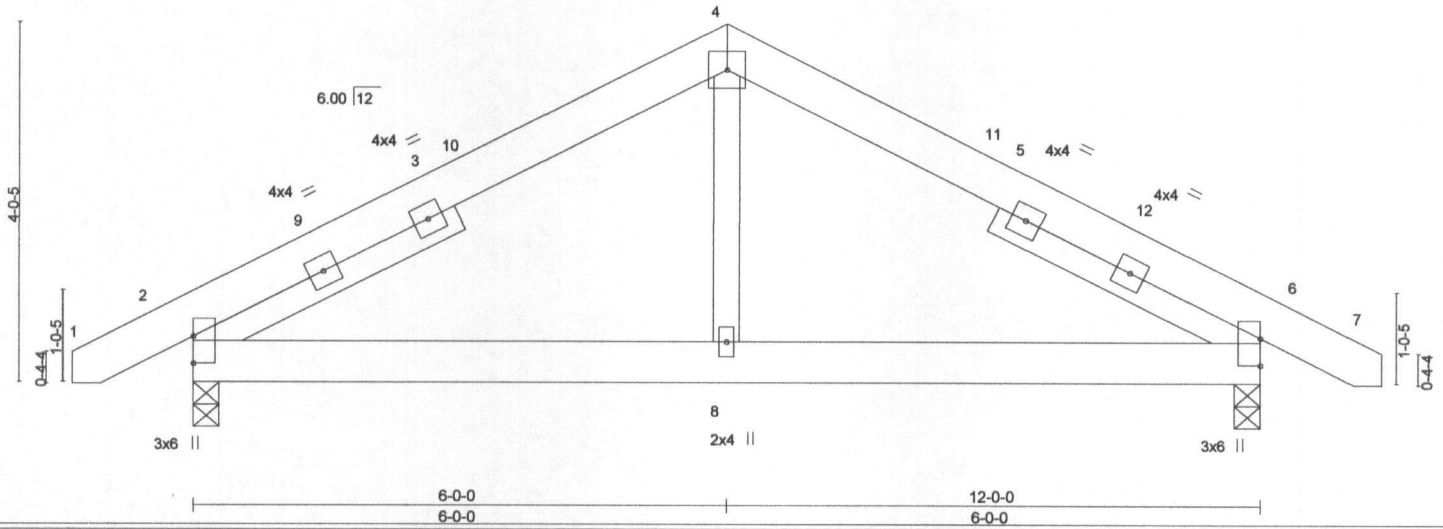
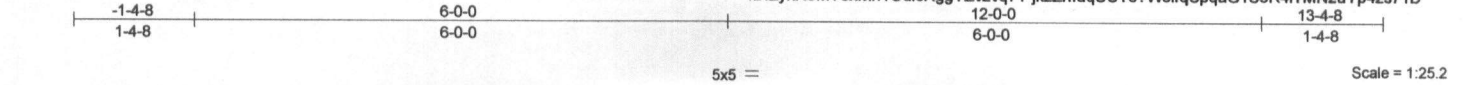
818 Soundside Road  
Edenton, NC 27932

|                   |             |                      |          |          |                                    |           |
|-------------------|-------------|----------------------|----------|----------|------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>G1 | Truss Type<br>COMMON | Qty<br>5 | Ply<br>1 | Lot 38 South Creek / Hamett County | E11391737 |
|-------------------|-------------|----------------------|----------|----------|------------------------------------|-----------|

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8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:40 2018 Page 1  
 ID:ZyxR5MYexMn1OulsRggYZzvq71-jkzZrfluqUUT5?WollqSpqaG1S3R4iTMN2uYp4zs71D

Job Reference (optional)



|                      |                      |             |                             |               |             |
|----------------------|----------------------|-------------|-----------------------------|---------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>                | <b>PLATES</b> | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.11     | in (loc) l/defl L/d         | MT20          | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.13     | Vert(LL) -0.01 6-8 >999 360 |               |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.10     | Vert(TL) -0.02 6-8 >999 240 |               |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-S    | Horz(TL) 0.01 6 n/a n/a     |               |             |
|                      | Code IRC2009/TPI2007 |             | Wind(LL) 0.00 2-8 >999 240  | Weight: 82 lb | FT = 20%    |

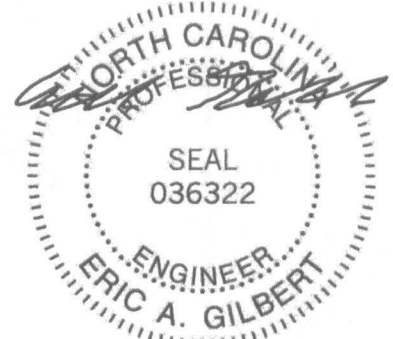
**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3  
 SLIDER Left 2x4 SP No.3 3-3-9, Right 2x4 SP No.3 3-3-9

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 2=553/0-3-8, 6=553/0-3-8  
 Max Horz 2=45(LC 6)  
 Max Uplift 2=-74(LC 7), 6=-74(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-4=609/203, 4-6=609/203  
 BOT CHORD 2-8=48/439, 6-8=48/439  
 WEBS 4-8=0/271

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-2-10 to 3-2-3, Interior(1) 3-2-3 to 6-0-0, Exterior(2) 6-0-0 to 10-4-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



January 25, 2018

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**TRENCO**  
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 A MITEK AFFILIATE  
 818 Soundside Road  
 Edenton, NC 27932

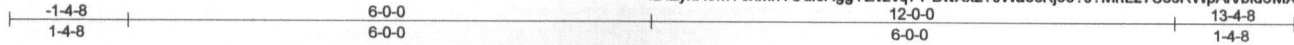
|                   |                |                     |          |          |                                     |           |
|-------------------|----------------|---------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>G1-GE | Truss Type<br>GABLE | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391738 |
|-------------------|----------------|---------------------|----------|----------|-------------------------------------|-----------|

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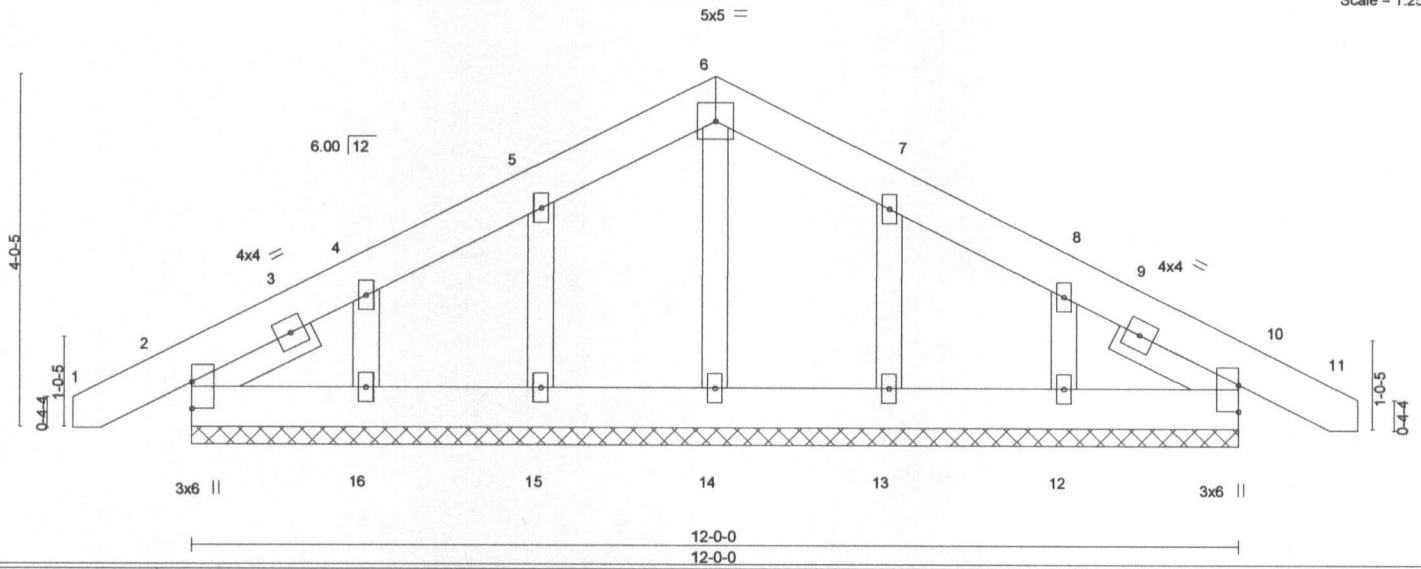
8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:41 2018 Page 1

ID:ZyxR5MYexMn1OulsRggYZvzq71-BwXx2?JWaoCKj95?J?MhL27S5sRWpAIVbid5MXzs71C

Job Reference (optional)



Scale = 1:25.7



|                      |                      |       |             |              |       |       |        |     |               |             |
|----------------------|----------------------|-------|-------------|--------------|-------|-------|--------|-----|---------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | 2-0-0 | <b>CSI.</b> | <b>DEFL.</b> | in    | (loc) | l/defl | L/d | <b>PLATES</b> | <b>GRIP</b> |
| TCLL 20.0            | Plate Grip DOL       | 1.15  | TC 0.03     | Vert(LL)     | -0.00 | 10    | n/r    | 120 | MT20          | 244/190     |
| TCDL 10.0            | Lumber DOL           | 1.15  | BC 0.01     | Vert(TL)     | -0.00 | 11    | n/r    | 120 |               |             |
| BCLL 0.0 *           | Rep Stress Incr      | YES   | WB 0.04     | Horz(TL)     | 0.00  | 10    | n/a    | n/a |               |             |
| BCDL 10.0            | Code IRC2009/TPI2007 |       | Matrix-S    |              |       |       |        |     | Weight: 86 lb | FT = 20%    |

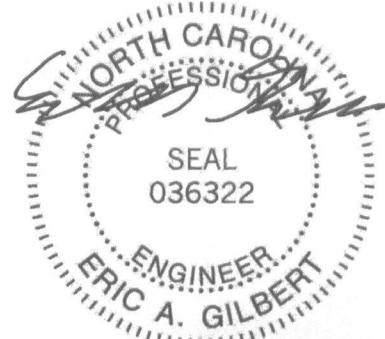
**LUMBER-**  
TOP CHORD 2x6 SP No.1  
BOT CHORD 2x6 SP No.1  
OTHERS 2x4 SP No.3  
SLIDER Left 2x4 SP No.3 1-6-8, Right 2x4 SP No.3 1-6-8

**BRACING-**  
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 12-0-0.  
(lb) - Max Horz 2=52(LC 7)  
Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 15, 16, 13, 12  
Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 16, 13, 12

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - All plates are 2x4 MT20 unless otherwise indicated.
  - Gable requires continuous bottom chord bearing.
  - Gable studs spaced at 2-0-0 oc.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 15, 16, 13, 12.



January 25, 2018

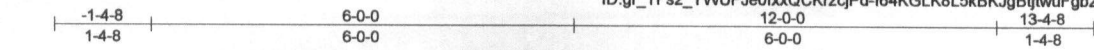
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818 Soundside Road  
Edenton, NC 27932

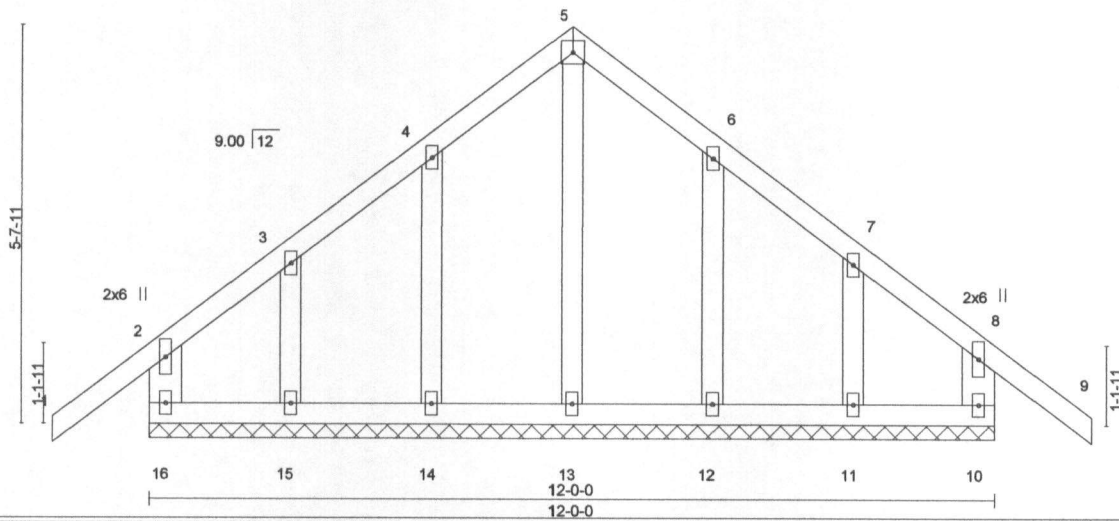
|                   |                |                                    |          |          |                                     |           |
|-------------------|----------------|------------------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>H1-GE | Truss Type<br>COMMON SUPPORTED GAB | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391739 |
|-------------------|----------------|------------------------------------|----------|----------|-------------------------------------|-----------|

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8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:42 2018 Page 1  
ID:gr\_TFs2\_YWUFJe0bxxQCKrzcjPd-f64KGLK8L5kBKJgBjtjwuFgb2Gn2YcHfqMNFuzzs71B



Scale: 3/8"=1'



|                      |                      |             |                          |               |             |
|----------------------|----------------------|-------------|--------------------------|---------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>             | <b>PLATES</b> | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.15     | in (loc) l/defl L/d      | MT20          | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.06     | Vert(LL) -0.01 9 n/r 120 |               |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.08     | Vert(TL) -0.02 9 n/r 120 |               |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-R    | Horz(TL) 0.00 10 n/a n/a |               |             |
|                      | Code IRC2009/TPI2007 |             |                          | Weight: 74 lb | FT = 20%    |

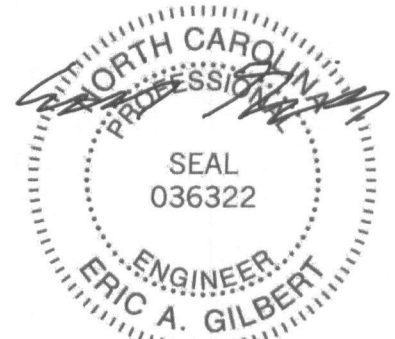
**LUMBER-**  
TOP CHORD 2x4 SP No.1  
BOT CHORD 2x4 SP No.1  
WEBS 2x6 SP No.1  
OTHERS 2x4 SP No.3

**BRACING-**  
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.  
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS.** All bearings 12-0-0.  
(lb) - Max Horz 16=157(LC 6)  
Max Uplift All uplift 100 lb or less at joint(s) 16, 10, 14, 12 except 15=118(LC 6), 11=110(LC 5)  
Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - All plates are 2x4 MT20 unless otherwise indicated.
  - Gable requires continuous bottom chord bearing.
  - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
  - Gable studs spaced at 2-0-0 oc.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 12 except (jt=lb) 15=118, 11=110.



January 25, 2018

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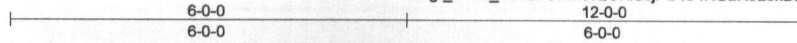
818 Soundside Road  
Edenton, NC 27932

|                   |             |                             |          |          |                                                  |
|-------------------|-------------|-----------------------------|----------|----------|--------------------------------------------------|
| Job<br>J0918-4409 | Truss<br>H2 | Truss Type<br>Common Girder | Qty<br>1 | Ply<br>2 | Lot 38 South Creek / Harnett County<br>E11391740 |
|-------------------|-------------|-----------------------------|----------|----------|--------------------------------------------------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:42 2018 Page 1

ID:gr\_TFs2\_YWUFJe0bxxQCKrzciPd-f64KGLK8L5kBKJgBtjtwuFgWyGdMYOSfqMNFuzzs71B



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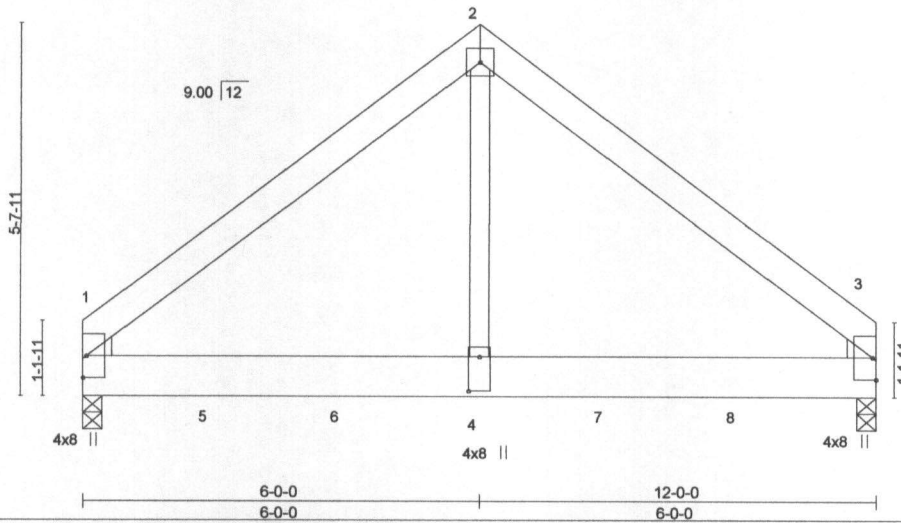


Plate Offsets (X,Y)-- [1:0-0-4,0-0-5], [1:0-0-7,0-3-11], [3:0-0-7,0-3-11], [3:0-0-4,0-0-5], [4:0-6-4,0-2-0]

| LOADING (psf) | SPACING-             | CSI.     | DEFL.          | in (loc) | l/defl | L/d | PLATES         | GRIP     |
|---------------|----------------------|----------|----------------|----------|--------|-----|----------------|----------|
| TCLL 20.0     | Plate Grip DOL 1.15  | TC 0.47  | Vert(LL) -0.04 | 3-4      | >999   | 360 | MT20           | 244/190  |
| TCDL 10.0     | Lumber DOL 1.15      | BC 0.68  | Vert(TL) -0.10 | 3-4      | >999   | 240 |                |          |
| BCLL 0.0 *    | Rep Stress Incr NO   | WB 0.97  | Horz(TL) 0.02  | 3        | n/a    | n/a |                |          |
| BCDL 10.0     | Code IRC2009/TPI2007 | Matrix-S | Wind(LL) 0.03  | 3-4      | >999   | 240 |                |          |
|               |                      |          |                |          |        |     | Weight: 163 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x8 SP No.1  
 WEBS 2x4 SP No.3  
 WEDGE  
 Left: 2x4 SP No.3, Right: 2x4 SP No.3

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.**

(lb/size) 1=3791/0-3-8, 3=3734/0-3-8  
 Max Horz 1=140(LC 4)  
 Max Uplift 1=-293(LC 5), 3=-289(LC 6)

**FORCES.**

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 1-2=-3994/351, 2-3=-3994/351  
 BOT CHORD 1-4=-218/2984, 3-4=-218/2984  
 WEBS 2-4=-314/4677

**NOTES-**

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
 Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.  
 Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-7-0 oc.  
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCCL=6.0psf, BCCL=6.0psf, h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (j=lb) 1=293, 3=289.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1318 lb down and 109 lb up at 1-11-4, 1318 lb down and 109 lb up at 3-11-4, 1318 lb down and 109 lb up at 5-11-4, and 1318 lb down and 109 lb up at 7-11-4, and 1318 lb down and 109 lb up at 9-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard

- Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15  
 Uniform Loads (plf)  
 Vert: 1-2=-60, 2-3=-60, 1-3=-20

Continued on page 2



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY  
**TRENCO**  
 A MiTek® Brand  
 818 Soundside Road  
 Edenton, NC 27932

|                   |             |                             |          |          |                                                                 |           |
|-------------------|-------------|-----------------------------|----------|----------|-----------------------------------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>H2 | Truss Type<br>Common Girder | Qty<br>1 | Ply<br>2 | Lot 38 South Creek / Harnett County<br>Job Reference (optional) | E11391740 |
|-------------------|-------------|-----------------------------|----------|----------|-----------------------------------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:42 2018 Page 2  
ID:gr\_TFs2\_YWUFJe0lxxQCKrzcjPd-f64KGLK8L5kBKJgBjtWuFgWYgDmYOSfqMNFuzzs71B

**LOAD CASE(S)** Standard

Concentrated Loads (lb)

Vert: 4=-1318(B) 5=-1318(B) 6=-1318(B) 7=-1318(B) 8=-1318(B)

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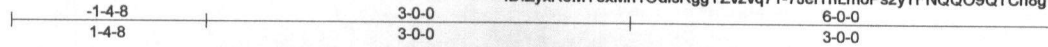
818 Soundside Road  
Edenton, NC 27932

|                   |             |                            |          |          |                                     |           |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>J1 | Truss Type<br>ROOF SPECIAL | Qty<br>3 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391741 |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|

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ID:ZyxR5MYexMn1OulsRggYZvzvq71-7JeiThLm6Ps2yTFNQO9QTCh8g?KH7o306CQPzs71A



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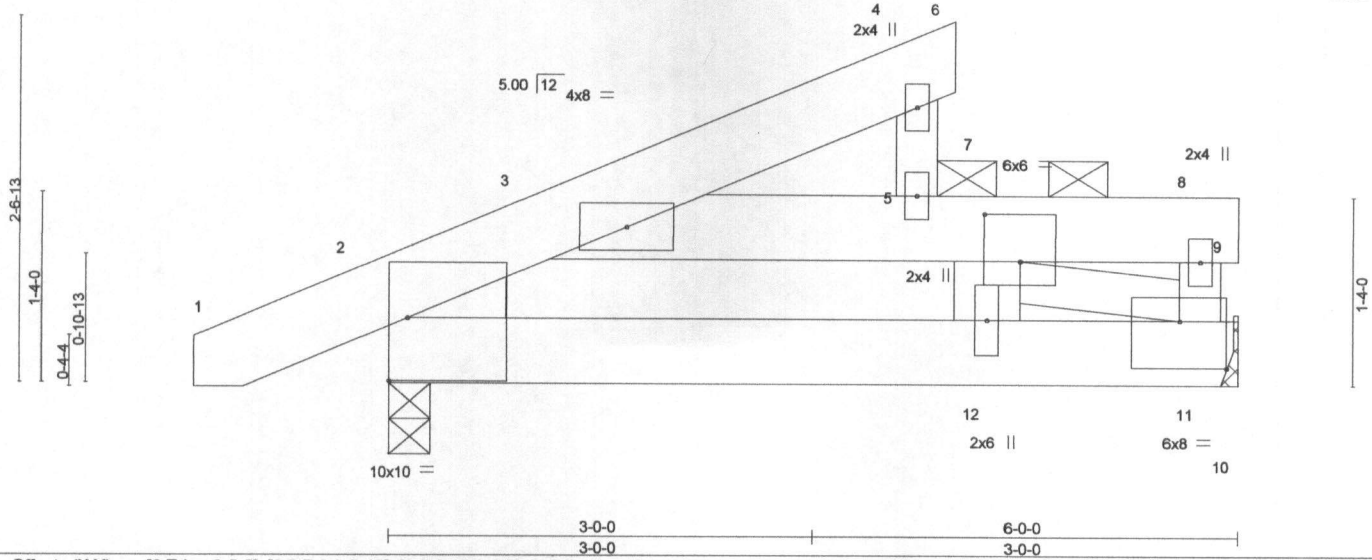


Plate Offsets (X,Y)-- [2:Edge,0-5-5], [2:0-7-2,0-0-10], [2:0-0-12,0-0-5], [7:0-3-0,0-4-0], [11:0-4-0,0-4-0]

| LOADING (psf) | SPACING-             | CSL.     | DEFL.                        | PLATES        | GRIP     |
|---------------|----------------------|----------|------------------------------|---------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.51  | in (loc) l/defl L/d          | MT20          | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.56  | Vert(LL) -0.02 2-12 >999 360 |               |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.56  | Vert(TL) -0.06 2-12 >999 240 |               |          |
| BCDL 10.0     | Rep Stress Incr NO   | Matrix-S | Horz(TL) 0.01 11 n/a n/a     |               |          |
|               | Code IRC2009/TPI2007 |          | Wind(LL) 0.05 2-12 >999 240  | Weight: 45 lb | FT = 20% |

**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3 \*Except\*  
 7-12: 2x6 SP No.1  
 WEDGE  
 Left: 2x4 SP No.3

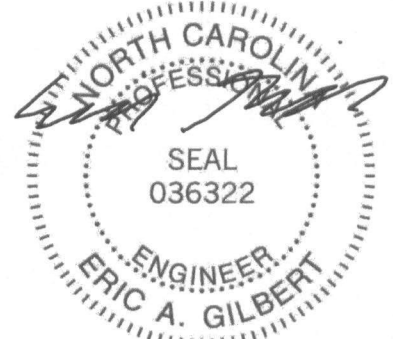
**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 7-12, 3-9.  
 BOT CHORD Rigid ceiling directly applied or 5-10-4 oc bracing.  
 JOINTS 1 Brace at Jt(s): 5

**REACTIONS.** (lb/size) 2=826/0-3-8, 11=1618/Mechanical  
 Max Horz 2=70(LC 7)  
 Max Uplift 2=122(LC 7), 11=170(LC 7)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-1420/1293, 7-12=-382/475, 3-5=-1429/1434, 5-7=-1429/1434  
 BOT CHORD 2-12=-1434/1429, 11-12=-1617/1643  
 WEBS 8-11=-376/361, 7-11=-1902/1872

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Corner(3) -1-2-7 to 4-0-0, Exterior(2) 3-8-12 to 6-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) Provide adequate drainage to prevent water ponding.
  - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - 6) Refer to girder(s) for truss to truss connections.
  - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=122, 11=170.
  - 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
  - 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1900 lb down and 1869 lb up at 4-4-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard  
 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15  
 Uniform Loads (plf)  
 Vert: 1-3=-60, 3-4=-60, 4-6=-20, 2-10=-20, 5-7=-60, 7-8=-60, 8-9=-20  
 Concentrated Loads (lb)  
 Vert: 7=-1900



January 25, 2018

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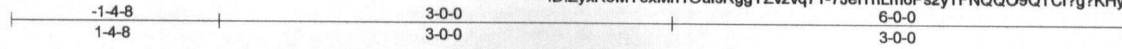
**ENGINEERING BY**  
**TRENCO**  
 A MITEK AFFILIATE  
 818 Soundside Road  
 Edenton, NC 27832

|                   |             |                            |          |          |                                     |           |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>J2 | Truss Type<br>ROOF SPECIAL | Qty<br>1 | Ply<br>2 | Lot 38 South Creek / Harnett County | E11391742 |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:43 2018 Page 1

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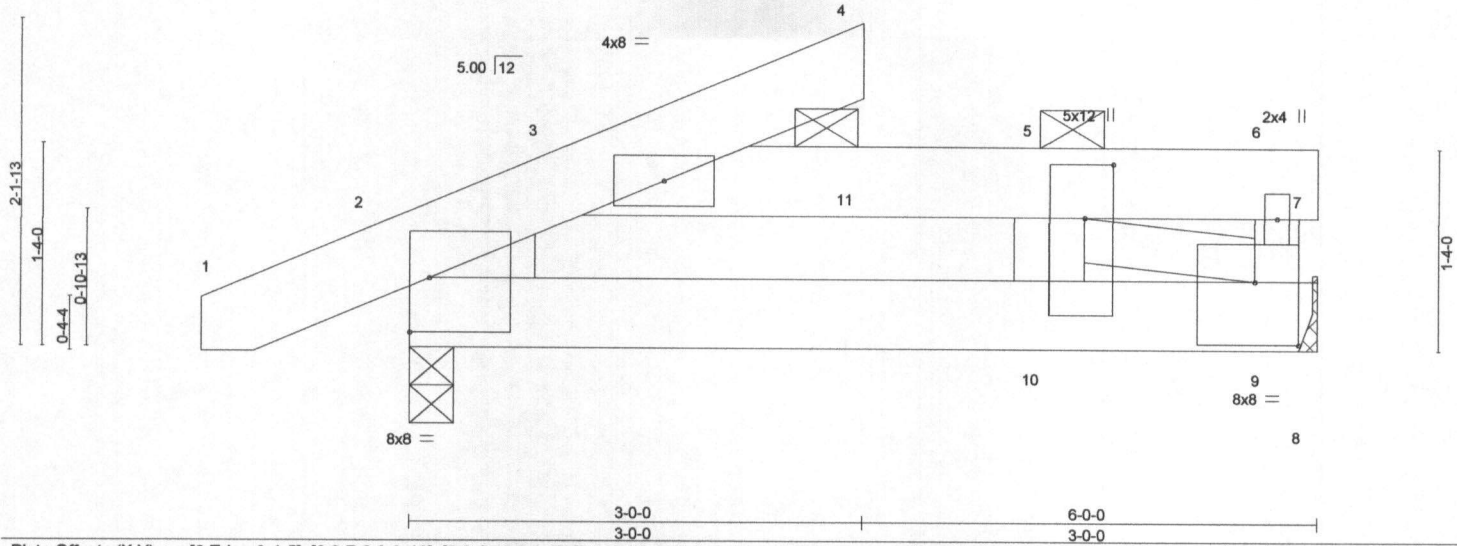


Plate Offsets (X, Y) - [2:Edge,0-4-5], [2:0-7-2,0-0-10], [2:0-0-12,0-0-5], [5:0-4-4,0-2-4], [9:0-3-8,0-5-0], [10:0-0-0,0-2-12]

| LOADING (psf) | SPACING-             | CSL      | DEFL.                        | PLATES        | GRIP     |
|---------------|----------------------|----------|------------------------------|---------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.58  | in (loc) l/defl L/d          | MT20          | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.56  | Vert(LL) -0.02 2-10 >999 360 |               |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.57  | Vert(TL) -0.06 2-10 >999 240 |               |          |
| BCDL 10.0     | Rep Stress Incr NO   | Matrix-S | Horz(TL) -0.01 9 n/a n/a     |               |          |
|               | Code IRC2009/TPI2007 |          | Wind(LL) 0.06 2-10 >999 240  | Weight: 82 lb | FT = 20% |

**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 WEBS 2x4 SP No.3 \*Except\*  
 5-10: 2x6 SP No.1

**WEDGE**  
 Left: 2x4 SP No.3

**REACTIONS.** (lb/size) 2=1357/0-3-8, 9=3011/Mechanical  
 Max Horz 2=56(LC 6)  
 Max Uplift 2=172(LC 7), 9=333(LC 6)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-3=-2397/2651, 5-10=-827/1004, 3-5=-2433/2837  
 BOT CHORD 2-10=-2837/2433, 9-10=-3316/3024  
 WEBS 6-9=-713/753, 5-9=-3501/3839

**NOTES-**

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:  
 Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.  
 Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.  
 Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Corner(3) -1-2-7 to 3-0-0, Exterior(2) 1-1-9 to 6-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=172, 9=333.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3792 lb down and 4081 lb up at 4-3-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard

Continued on page 2



January 25, 2018

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818 Soundside Road  
 Edenton, NC 27932



|                   |             |                            |          |          |                                                                 |           |
|-------------------|-------------|----------------------------|----------|----------|-----------------------------------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>J2 | Truss Type<br>ROOF SPECIAL | Qty<br>1 | Ply<br>2 | Lot 38 South Creek / Harnett County<br>Job Reference (optional) | E11391742 |
|-------------------|-------------|----------------------------|----------|----------|-----------------------------------------------------------------|-----------|

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8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:43 2018 Page 2  
ID:ZyxR5MYexMn1OulsRggYZvzvq71-7JeiThLm6Ps2yTFNQQO9QTCf?g?KHyo306CQPzs71A

**LOAD CASE(S)** Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 2-8=-20, 3-11=-20, 5-11=-60, 5-6=-60, 6-7=-20

Concentrated Loads (lb)

Vert: 5=-3792

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**

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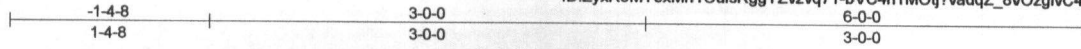
818 Soundside Road  
Edenton, NC 27932

|                   |             |                            |          |          |                                     |           |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>J3 | Truss Type<br>ROOF SPECIAL | Qty<br>6 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391743 |
|-------------------|-------------|----------------------------|----------|----------|-------------------------------------|-----------|

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8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:44 2018 Page 1  
ID:ZyxR5MYexMn1OulsRggYZzvq71-bVC4h1MOtj?vadqZ\_8vOzglvC4QS0SMYHgsmyrzs719

Job Reference (optional)



Scale = 1:15.4

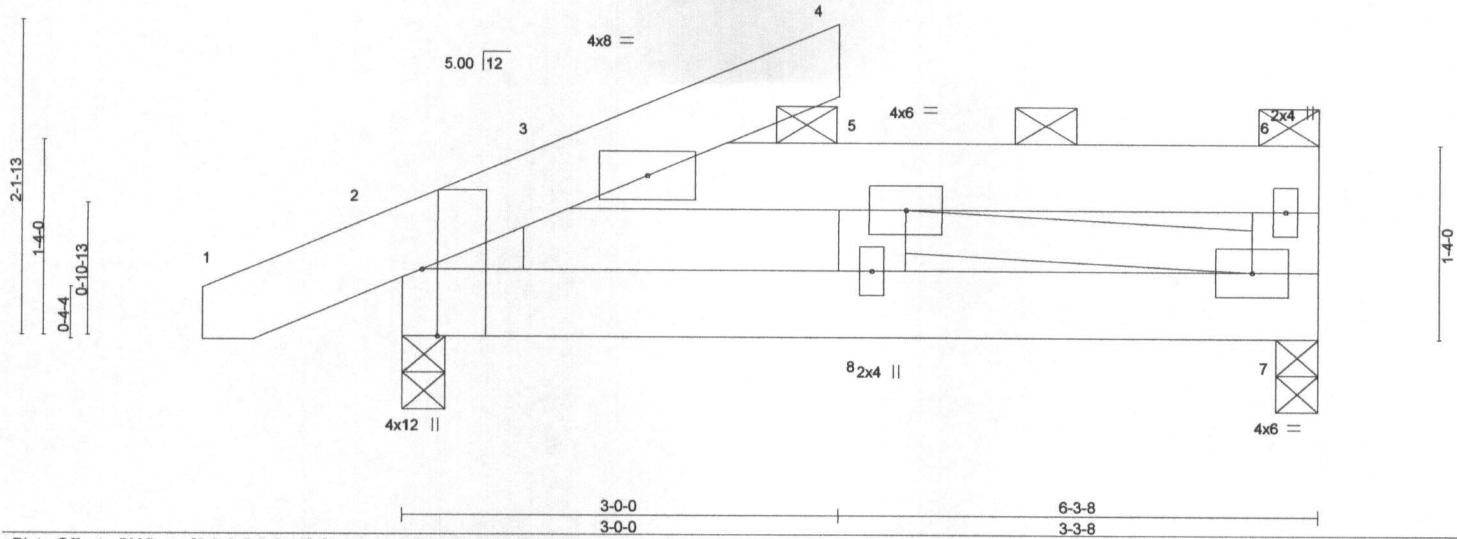


Plate Offsets (X,Y) - [2:0-0-5,0-0-12], [2:0-0-10,0-7-2], [2:0-5-8,Edge]

| LOADING (psf) | SPACING-             | CSI.     | DEFL.    | in (loc) | l/defl | L/d  | PLATES        | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|------|---------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.29  | Vert(LL) | -0.01    | 8      | >999 | MT20          | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.25  | Vert(TL) | -0.03    | 8      | >999 |               |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.37  | Horz(TL) | -0.01    | 7      | n/a  |               |          |
| BCDL 10.0     | Rep Stress Incr NO   | Matrix-S | Wind(LL) | 0.03     | 8      | >999 |               |          |
|               | Code IRC2009/TPI2007 |          |          |          |        |      | Weight: 45 lb | FT = 20% |

**LUMBER-**  
TOP CHORD 2x6 SP No.1  
BOT CHORD 2x6 SP No.1  
WEBS 2x6 SP No.1 \*Except\*  
5-7: 2x4 SP No.3  
**WEDGE**  
Left: 2x4 SP No.3

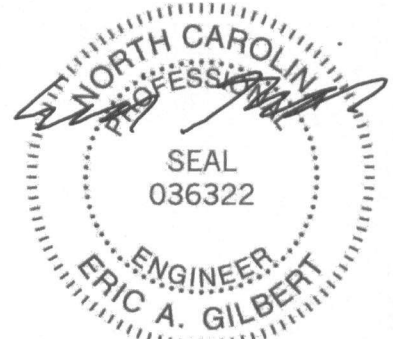
**BRACING-**  
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 5-8, 3-6.  
BOT CHORD Rigid ceiling directly applied or 6-11-9 oc bracing.

**REACTIONS.** (lb/size) 2=632/0-3-8, 7=595/0-3-8  
Max Horz 2=56(LC 6)  
Max Uplift 2=-97(LC 7), 7=-87(LC 6)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
TOP CHORD 2-3=-839/1132, 3-5=-749/1224  
BOT CHORD 2-8=-1224/749, 7-8=-1175/913  
WEBS 6-7=-229/287, 5-7=-956/1230

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Corner(3) -1-2-7 to 3-0-0, Exterior(2) 1-1-9 to 6-0-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Provide adequate drainage to prevent water ponding.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.
  - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
  - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 250 lb down and 269 lb up at 3-4-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard  
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15  
Uniform Loads (plf)  
Vert: 1-3=-60, 3-4=-60, 2-7=-20, 3-5=-100, 5-6=-140  
Concentrated Loads (lb)  
Vert: 5=-250



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**TRENCO**  
ENGINEERING BY  
A MiTek Affiliate  
818 Soundside Road  
Edenton, NC 27932

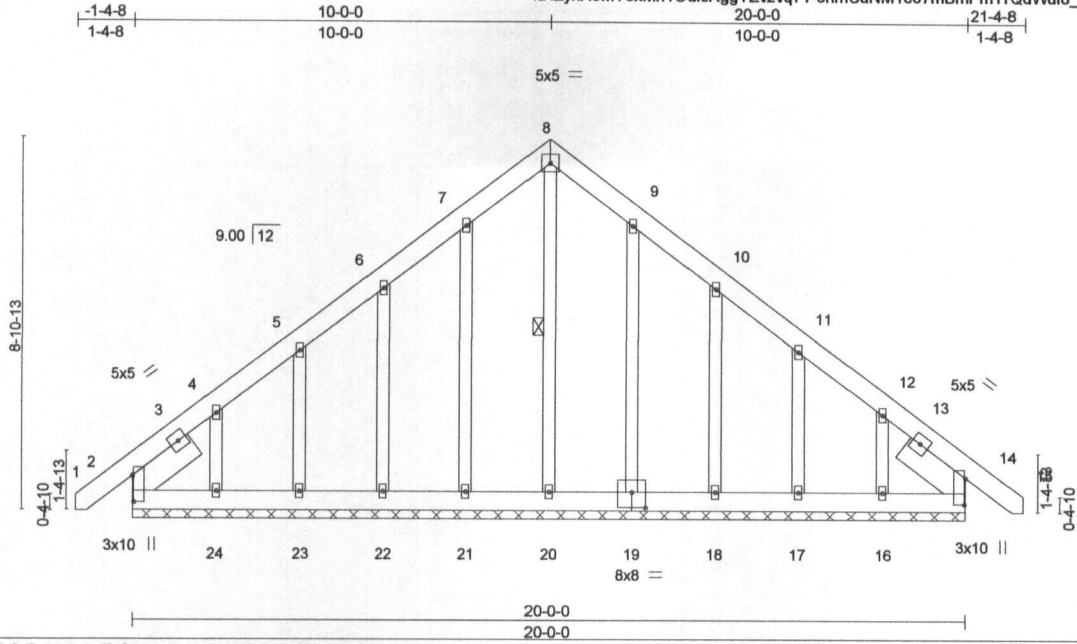
|                   |                |                                    |          |          |                                     |           |
|-------------------|----------------|------------------------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>K1-GE | Truss Type<br>COMMON SUPPORTED GAB | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391744 |
|-------------------|----------------|------------------------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

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ID:ZyxR5MYexMn1OulsRggYZvzq71-3hmSuNM1e07mBmPmYrQdWul8\_ToEiZh5WKbJVizs718

Job Reference (optional)



Scale = 1:53.8

Plate Offsets (X,Y)-- [2:0-7-11,0-0-5], [14:0-7-11,0-0-5], [19:0-4-0,0-4-8]

| LOADING (psf) | SPACING-             | CSI.     | DEFL.    | in (loc) | l/defl | L/d | PLATES         | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|-----|----------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.04  | Vert(LL) | -0.00    | 14     | n/r | MT20           | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.02  | Vert(TL) | -0.00    | 15     | n/r |                |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.10  | Horz(TL) | 0.00     | 14     | n/a |                |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-S |          |          |        |     |                |          |
|               | Code IRC2009/TPI2007 |          |          |          |        |     | Weight: 188 lb | FT = 20% |

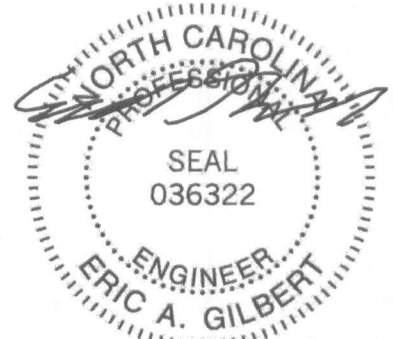
**LUMBER-**  
 TOP CHORD 2x6 SP No.1  
 BOT CHORD 2x6 SP No.1  
 OTHERS 2x4 SP No.3  
 SLIDER Left 2x8 SP No.1 1-10-6, Right 2x8 SP No.1 1-10-6

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpt 8-20

**REACTIONS.** All bearings 20-0-0.  
 (lb) - Max Horz 2=292(LC 6)  
 Max Uplift All uplift 100 lb or less at joint(s) 14, 21, 23, 19, 17 except 22=108(LC 7), 24=185(LC 7), 18=110(LC 8), 16=171(LC 8), 2=130(LC 5)  
 Max Grav All reactions 250 lb or less at joint(s) 14, 20, 21, 22, 23, 24, 19, 18, 17, 16, 2

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD 2-4=303/183

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
  - 4) All plates are 2x4 MT20 unless otherwise indicated.
  - 5) Gable requires continuous bottom chord bearing.
  - 6) Gable studs spaced at 2-0-0 oc.
  - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members, with BCDL = 10.0psf.
  - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 21, 23, 19, 17 except (jt=lb) 22=108, 24=185, 18=110, 16=171, 2=130.
  - 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 14.

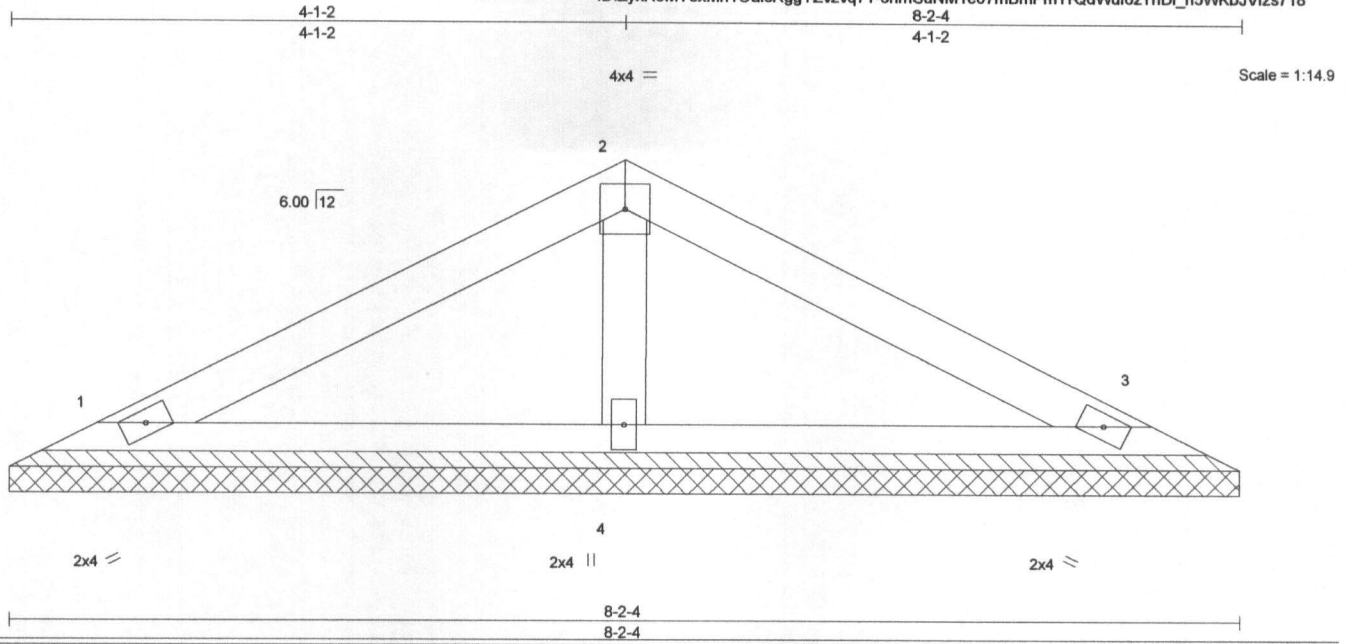


January 25, 2018

|            |       |            |     |     |                                     |           |
|------------|-------|------------|-----|-----|-------------------------------------|-----------|
| Job        | Truss | Truss Type | Qty | Ply | Lot 38 South Creek / Harnett County | E11391745 |
| J0918-4409 | VE-1  | VALLEY     | 1   | 1   |                                     |           |
|            |       |            |     |     | Job Reference (optional)            |           |

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8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:45 2018 Page 1  
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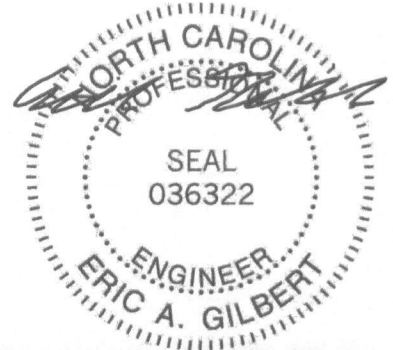
| LOADING (psf) | SPACING-             | 2-0-0 | CSI.     | DEFL.    | in   | (loc) | l/defl | L/d | PLATES        | GRIP     |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|---------------|----------|
| TCLL 20.0     | Plate Grip DOL       | 1.15  | TC 0.17  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20          | 244/190  |
| TCDL 10.0     | Lumber DOL           | 1.15  | BC 0.09  | Vert(TL) | n/a  | -     | n/a    | 999 |               |          |
| BCLL 0.0 *    | Rep Stress Incr      | YES   | WB 0.04  | Horz(TL) | 0.00 | 3     | n/a    | n/a |               |          |
| BCDL 10.0     | Code IRC2009/TPI2007 |       | Matrix-P |          |      |       |        |     | Weight: 26 lb | FT = 20% |

| LUMBER-               | BRACING-                                                                  |
|-----------------------|---------------------------------------------------------------------------|
| TOP CHORD 2x4 SP No.1 | TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. |
| BOT CHORD 2x4 SP No.1 | BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.            |
| OTHERS 2x4 SP No.3    |                                                                           |

**REACTIONS.** (lb/size) 1=142/8-2-4, 3=142/8-2-4, 4=272/8-2-4  
 Max Horz 1=-22(LC 5)  
 Max Uplift 1=-22(LC 7), 3=-26(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
  - 6) Non Standard bearing condition. Review required.

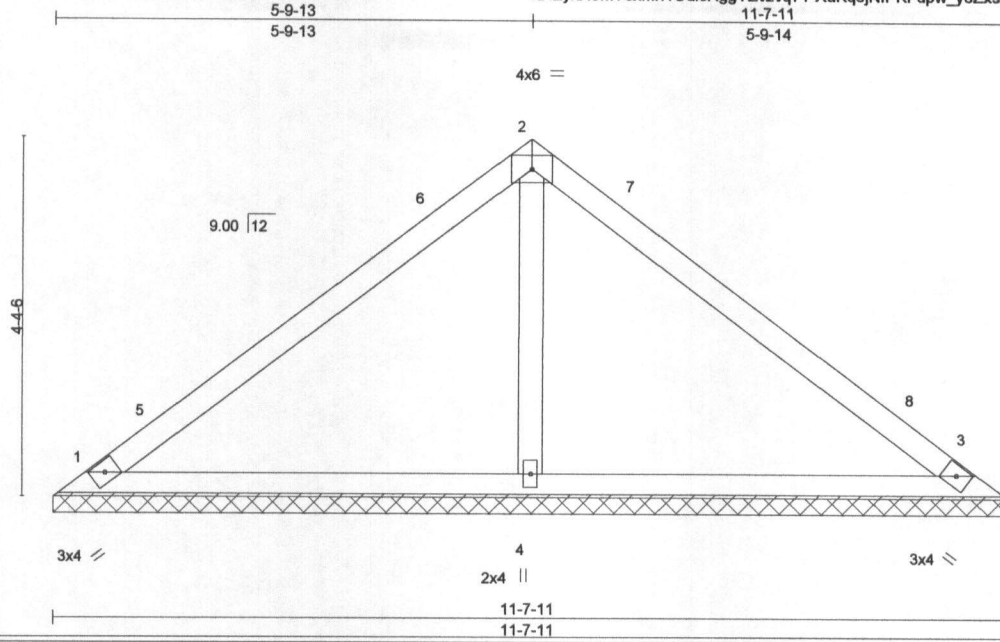


|                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                     |                                                                                                                                         |
|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-----------------------------------------------------------------------------------------------------------------------------------------|
| <p><b>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.</b><br/>         Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.</p> | <p>ENGINEERING BY<br/> <b>TRENCO</b><br/> <small>A MiTek Affiliate</small></p> <p>818 Soundside Road<br/>         Edenton, NC 27932</p> |
|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|-----------------------------------------------------------------------------------------------------------------------------------------|

|                   |               |                     |          |          |                                     |           |
|-------------------|---------------|---------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>VH-1 | Truss Type<br>GABLE | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391746 |
|-------------------|---------------|---------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:46 2018 Page 1  
 ID:ZyxR5MYexMn1OulsRggYZvzvq71-XuKq5jNfPKFdpw\_y6Zxs25qFst5WUQCFl\_Ls1kzs717  
 11-7-11  
 5-9-14



Scale = 1:27.3

|                      |                      |             |                         |               |             |
|----------------------|----------------------|-------------|-------------------------|---------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>            | <b>PLATES</b> | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.31     | in (loc) l/defl L/d     | MT20          | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.21     | Vert(LL) n/a - n/a 999  |               |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.09     | Vert(TL) n/a - n/a 999  |               |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-S    | Horz(TL) 0.00 3 n/a n/a |               |             |
|                      | Code IRC2009/TPI2007 |             |                         | Weight: 43 lb | FT = 20%    |

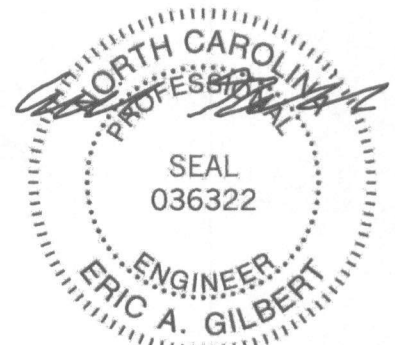
**LUMBER-**  
 TOP CHORD 2x4 SP No.1  
 BOT CHORD 2x4 SP No.1  
 OTHERS 2x4 SP No.3

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=224/11-7-11, 3=224/11-7-11, 4=412/11-7-11  
 Max Horz 1=-112(LC 5)  
 Max Uplift 1=-25(LC 7), 3=-34(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**WEBS** 2-4=-253/106

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
  - Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-5-4 to 4-10-1, Interior(1) 4-10-1 to 5-9-13, Exterior(2) 5-9-13 to 10-2-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - Gable requires continuous bottom chord bearing.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



January 25, 2018

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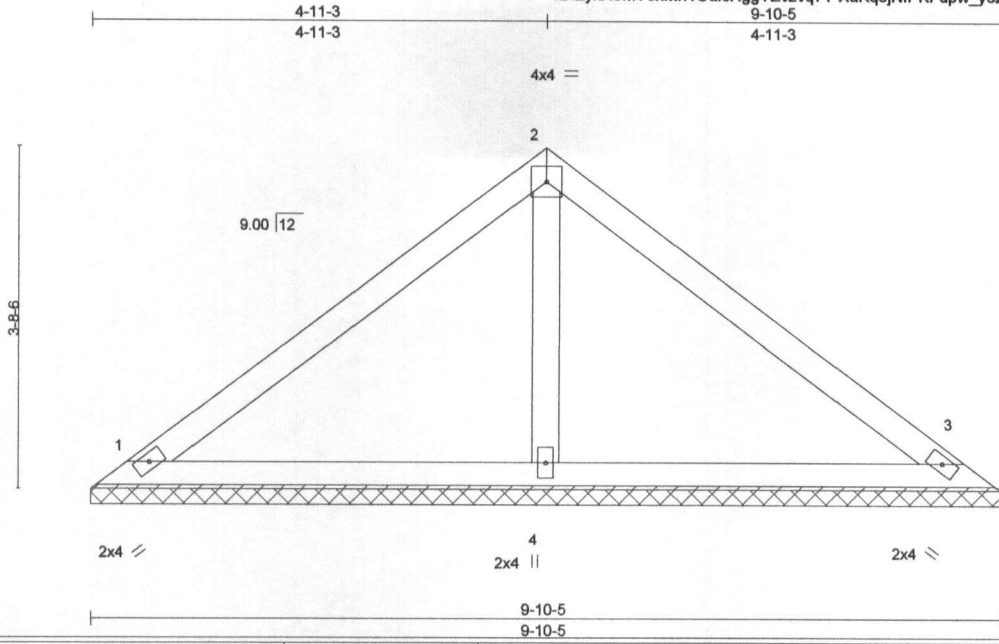


818 Soundside Road  
 Edenton, NC 27932

|                   |               |                      |          |          |                                     |           |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>VH-2 | Truss Type<br>Valley | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391747 |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:46 2018 Page 1  
 ID:ZyxR5MYexMn1OulsRggYZvzvq71-XuKq5jNPKFdpw\_y6Zxs25qG1t6WURFI\_Ls1kzs717



Scale = 1:24.3

|                      |                      |             |                         |               |             |
|----------------------|----------------------|-------------|-------------------------|---------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>            | <b>PLATES</b> | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.21     | in (loc) l/defl L/d     | MT20          | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.15     | Vert(LL) n/a - n/a 999  |               |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.06     | Vert(TL) n/a - n/a 999  |               |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-S    | Horz(TL) 0.00 3 n/a n/a |               |             |
|                      | Code IRC2009/TPI2007 |             |                         | Weight: 36 lb | FT = 20%    |

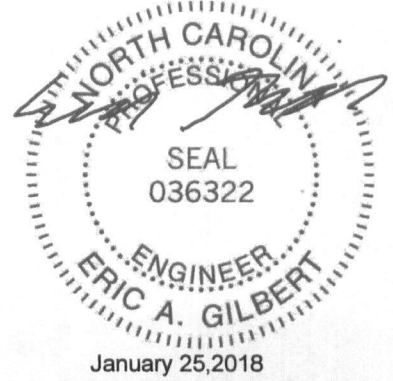
**LUMBER-**  
 TOP CHORD 2x4 SP No.1  
 BOT CHORD 2x4 SP No.1  
 OTHERS 2x4 SP No.3

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=187/9-10-5, 3=187/9-10-5, 4=344/9-10-5  
 Max Horz 1=-94(LC 5)  
 Max Uplift 1=-21(LC 7), 3=-29(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) Gable requires continuous bottom chord bearing.
  - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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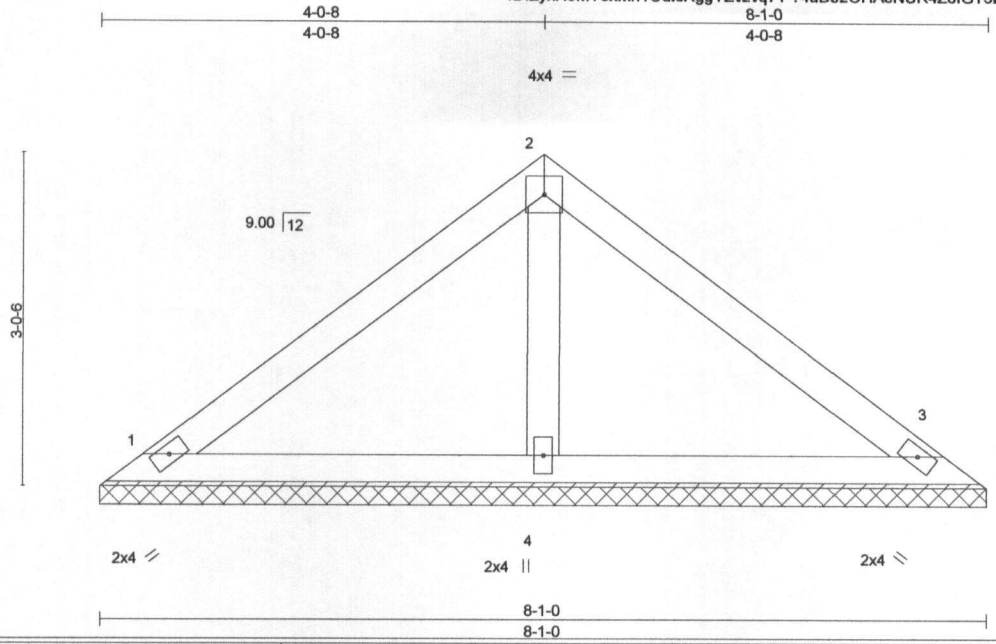
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2016 BEFORE USE.**  
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**ENGINEERING BY:**  
**TRENCO**  
 A MITEK AFFILIATE  
 818 Soundside Road  
 Edenton, NC 27832

|                   |               |                      |          |          |                                     |           |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>VH-3 | Truss Type<br>Valley | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391748 |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:47 2018 Page 1  
 ID:ZyxR5MYexMn1OulsRggYZvzvq71-74uDJ2OHAeNUR4Z8fGT5bJNSCHTaDuEO\_e4QZAzs716



Scale = 1:20.4

|                      |                      |             |                         |               |             |
|----------------------|----------------------|-------------|-------------------------|---------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>            | <b>PLATES</b> | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.18     | in (loc) l/defl L/d     | MT20          | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.10     | Vert(LL) n/a - n/a 999  |               |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.04     | Vert(TL) n/a - n/a 999  |               |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-P    | Horz(TL) 0.00 3 n/a n/a |               |             |
|                      | Code IRC2009/TPI2007 |             |                         | Weight: 29 lb | FT = 20%    |

|                       |                                                                           |
|-----------------------|---------------------------------------------------------------------------|
| <b>LUMBER-</b>        | <b>BRACING-</b>                                                           |
| TOP CHORD 2x4 SP No.1 | TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. |
| BOT CHORD 2x4 SP No.1 | BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.            |
| OTHERS 2x4 SP No.3    |                                                                           |

**REACTIONS.** (lb/size) 1=163/8-1-0, 3=163/8-1-0, 4=250/8-1-0  
 Max Horz 1=-75(LC 5)  
 Max Uplift 1=-24(LC 7), 3=-30(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) Gable requires continuous bottom chord bearing.
  - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



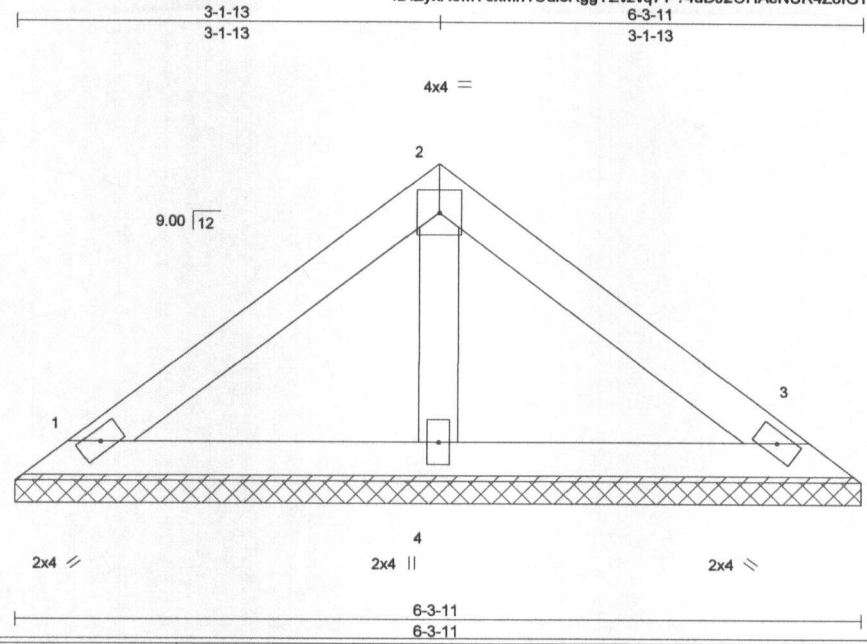
**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**  
 Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINEERING BY  
**TRENCO**  
 A MITEK AFFILIATE  
 818 Soundside Road  
 Edenton, NC 27932

|                   |               |                      |          |          |                                     |           |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>VH-4 | Truss Type<br>Valley | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391749 |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:47 2018 Page 1  
 ID:ZyxR5MYexMn1OulsRggYZvzq71-74uDj2OHAEeNUR4Z8FGT5bJNTWHUDDuQO\_e4QZAzs716



Scale = 1:16.7

|                      |                      |             |                         |               |             |
|----------------------|----------------------|-------------|-------------------------|---------------|-------------|
| <b>LOADING</b> (psf) | <b>SPACING-</b>      | <b>CSI.</b> | <b>DEFL.</b>            | <b>PLATES</b> | <b>GRIP</b> |
| TCLL 20.0            | 2-0-0                | TC 0.10     | in (loc) l/defl L/d     | MT20          | 244/190     |
| TCDL 10.0            | Plate Grip DOL 1.15  | BC 0.06     | Vert(LL) n/a - n/a 999  |               |             |
| BCLL 0.0 *           | Lumber DOL 1.15      | WB 0.03     | Vert(TL) n/a - n/a 999  |               |             |
| BCDL 10.0            | Rep Stress Incr YES  | Matrix-P    | Horz(TL) 0.00 3 n/a n/a |               |             |
|                      | Code IRC2009/TPI2007 |             |                         | Weight: 22 lb | FT = 20%    |

|                       |                                                                           |
|-----------------------|---------------------------------------------------------------------------|
| <b>LUMBER-</b>        | <b>BRACING-</b>                                                           |
| TOP CHORD 2x4 SP No.1 | TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. |
| BOT CHORD 2x4 SP No.1 | BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.            |
| OTHERS 2x4 SP No.3    |                                                                           |

**REACTIONS.** (lb/size) 1=123/6-3-11, 3=123/6-3-11, 4=188/6-3-11  
 Max Horz 1=57(LC 6)  
 Max Uplift 1=-18(LC 7), 3=-23(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  - 3) Gable requires continuous bottom chord bearing.
  - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
  - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.**  
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ENGINEERING BY  
**TRENCO**  
 A MITEK AFFILIATE  
 818 Soundside Road  
 Edenton, NC 27932



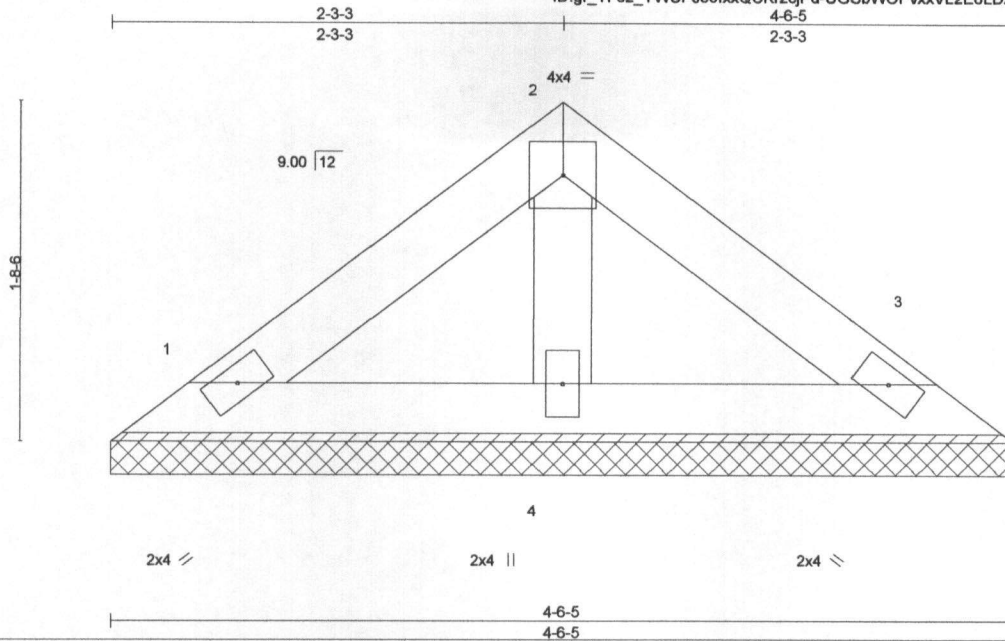
|                   |               |                      |          |          |                                     |           |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|
| Job<br>J0918-4409 | Truss<br>VH-5 | Truss Type<br>Valley | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County | E11391750 |
|-------------------|---------------|----------------------|----------|----------|-------------------------------------|-----------|

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8.130 s Sep 15 2017 MITek Industries, Inc. Wed Jan 24 14:14:48 2018 Page 1

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Job Reference (optional)



Scale = 1:11.2

| LOADING (psf) | SPACING-             | CSI.     | DEFL.    | in (loc) | l/defl | L/d | PLATES        | GRIP     |
|---------------|----------------------|----------|----------|----------|--------|-----|---------------|----------|
| TCLL 20.0     | 2-0-0                | TC 0.04  | Vert(LL) | n/a      | -      | n/a | MT20          | 244/190  |
| TCDL 10.0     | Plate Grip DOL 1.15  | BC 0.03  | Vert(TL) | n/a      | -      | n/a |               |          |
| BCLL 0.0 *    | Lumber DOL 1.15      | WB 0.02  | Horz(TL) | 0.00     | 3      | n/a |               |          |
| BCDL 10.0     | Rep Stress Incr YES  | Matrix-P |          |          |        |     | Weight: 15 lb | FT = 20% |
|               | Code IRC2009/TPI2007 |          |          |          |        |     |               |          |

**LUMBER-**  
 TOP CHORD 2x4 SP No.1  
 BOT CHORD 2x4 SP No.1  
 OTHERS 2x4 SP No.3

**BRACING-**  
 TOP CHORD Structural wood sheathing directly applied or 4-6-5 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** (lb/size) 1=83/4-6-5, 3=83/4-6-5, 4=127/4-6-5  
 Max Horz 1=38(LC 6)  
 Max Uplift 1=-12(LC 7), 3=-15(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



January 25, 2018

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.**

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

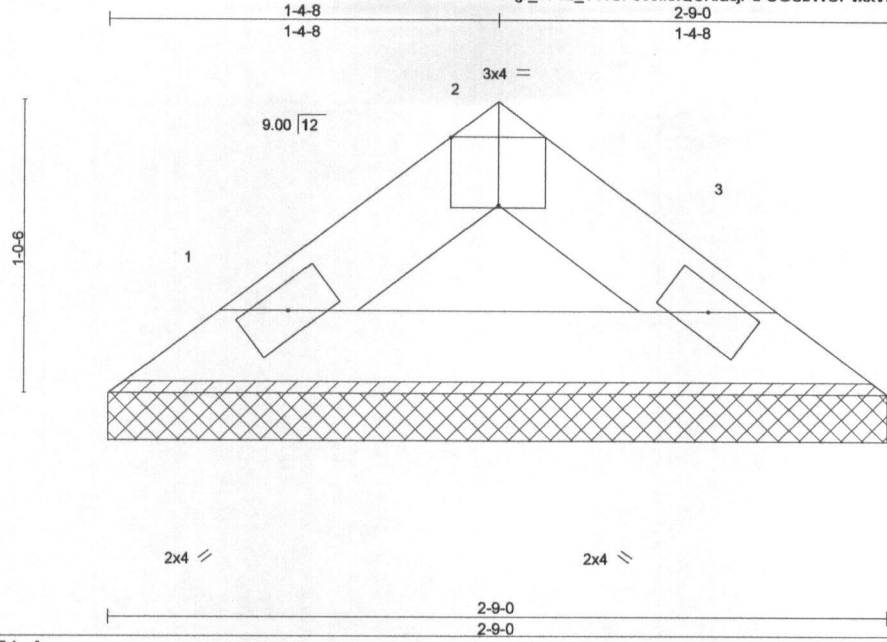


818 Soundside Road  
 Edenton, NC 27932

|                   |               |                      |          |          |                                                  |
|-------------------|---------------|----------------------|----------|----------|--------------------------------------------------|
| Job<br>J0918-4409 | Truss<br>VH-6 | Truss Type<br>Valley | Qty<br>1 | Ply<br>1 | Lot 38 South Creek / Harnett County<br>E11391751 |
|-------------------|---------------|----------------------|----------|----------|--------------------------------------------------|

Comtech, Inc., Fayetteville, NC 28309

8.130 s Sep 15 2017 MiTek Industries, Inc. Wed Jan 24 14:14:48 2018 Page 1  
ID:gr\_TFs2\_YWUFJe0lxxQCKrzcjPd-UGSbWOPvxxVL2E8LDz\_K7WwfdhqtlyL3XCiqz6dzs715



Scale = 1:7.9

Plate Offsets (X,Y)-- [2-0-2-0,Edge]

| LOADING (psf) | SPACING-             | 2-0-0 | CSI.     | DEFL.    | in   | (loc) | l/defl | L/d | PLATES       | GRIP     |
|---------------|----------------------|-------|----------|----------|------|-------|--------|-----|--------------|----------|
| TCLL 20.0     | Plate Grip DOL       | 1.15  | TC 0.01  | Vert(LL) | n/a  | -     | n/a    | 999 | MT20         | 244/190  |
| TCDL 10.0     | Lumber DOL           | 1.15  | BC 0.03  | Vert(TL) | n/a  | -     | n/a    | 999 |              |          |
| BCLL 0.0 *    | Rep Stress Incr      | YES   | WB 0.00  | Horz(TL) | 0.00 | 3     | n/a    | n/a |              |          |
| BCDL 10.0     | Code IRC2009/TPI2007 |       | Matrix-P |          |      |       |        |     | Weight: 8 lb | FT = 20% |

**LUMBER-**

TOP CHORD 2x4 SP No.1  
BOT CHORD 2x4 SP No.1

**BRACING-**

TOP CHORD Structural wood sheathing directly applied or 2-9-0 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

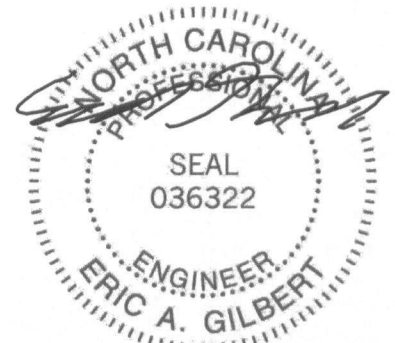
**REACTIONS.**

(lb/size) 1=75/2-9-0, 3=75/2-9-0  
Max Horz 1=-20(LC 5)  
Max Uplift 1=-3(LC 7), 3=-3(LC 8)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**NOTES-**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCCL=6.0psf; h=15ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas with a clearance greater than 6-0-0 between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



January 25, 2018

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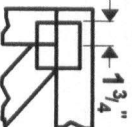
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



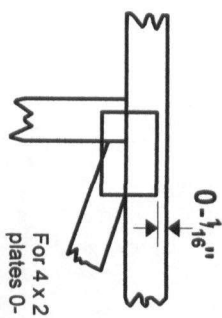
818 Soundside Road  
Edenton, NC 27832

# Symbols

## PLATE LOCATION AND ORIENTATION



Center plate on joint unless X, Y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in MITrak 20/20 software or upon request.

## PLATE SIZE

4 X 4

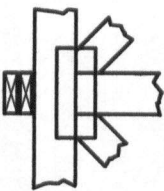
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

## BEARING

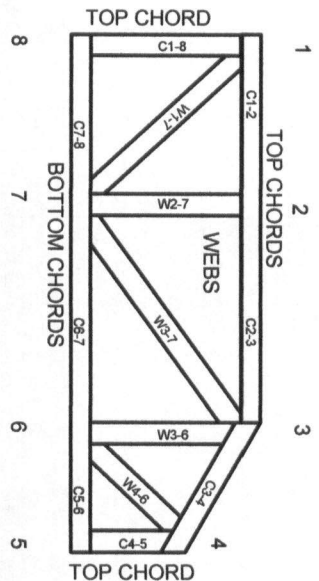


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

## Industry Standards:

- ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
- DSB-89: Design Standard for Bracing, Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVALS

- ICC-ES Reports:
- ESR-1311, ESR-1352, ESR1988
- ESR-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3 These truss designs rely on lumber values established by others.

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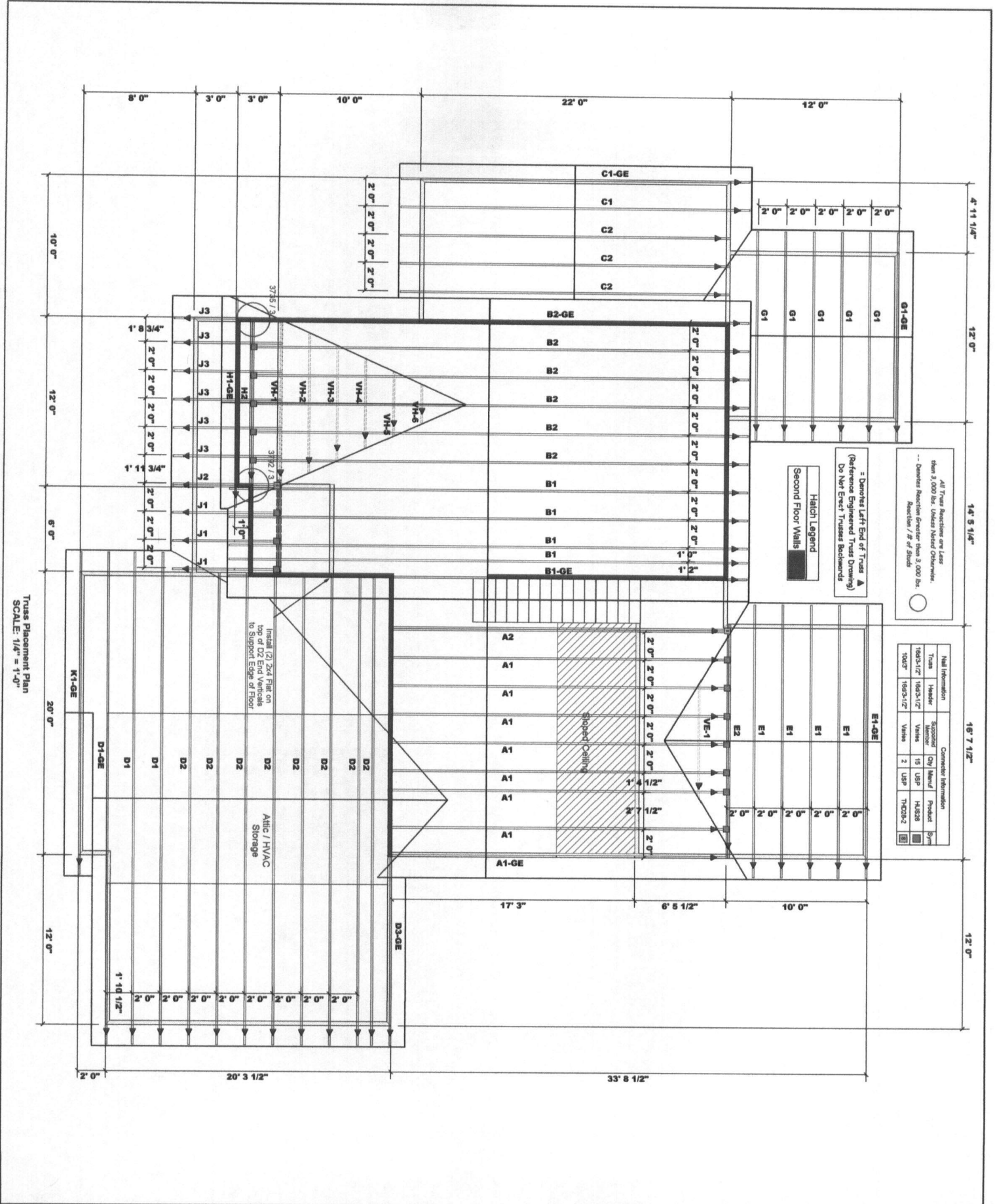


MITek Engineering Reference Sheet: Mill-7473 rev. 10/03/2015

# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/TP1 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.



|                  |                    |                  |                    |
|------------------|--------------------|------------------|--------------------|
| <b>BUILDER</b>   | Watermark Homes    | <b>COUNTY</b>    | Harnett            |
| <b>JOB NAME</b>  | Lot 38 South Creek | <b>ADDRESS</b>   | Lot 38 South Creek |
| <b>PLAN</b>      | The Ginko          | <b>MODEL</b>     | Roof               |
| <b>SEAL DATE</b> | 09/21/2018         | <b>DATE REV.</b> | 09/27/2018         |
| <b>QUOTE #</b>   |                    | <b>DRAWN BY</b>  | Anthony Williams   |
| <b>JOB #</b>     | J0918-4409         | <b>SALESMAN</b>  | Anthony Williams   |

**LOAD CHART FOR JACK STUDS**

BASED ON 1" X 4" JACK STUDS @ 16" ON CENTER

| END REACTION TO TOP OF JACK STUD (K) PER MEMBER | END REACTION TO TOP OF JACK STUD (K) PER HEADING | END REACTION TO TOP OF JACK STUD (K) PER HEADING | END REACTION TO TOP OF JACK STUD (K) PER HEADING |
|-------------------------------------------------|--------------------------------------------------|--------------------------------------------------|--------------------------------------------------|
| 1700                                            | 2560                                             | 3400                                             | 4600                                             |
| 3400                                            | 5120                                             | 6800                                             | 9200                                             |
| 5100                                            | 7680                                             | 10200                                            | 13600                                            |
| 6800                                            | 10240                                            | 13600                                            | 18100                                            |
| 8500                                            | 12800                                            | 17000                                            | 22600                                            |
| 10200                                           | 15360                                            | 20400                                            | 27100                                            |
| 11900                                           | 17920                                            | 23800                                            | 31600                                            |
| 13600                                           | 19840                                            | 26200                                            | 34000                                            |
| 15300                                           | 21760                                            | 28600                                            | 38000                                            |
| 17000                                           | 23680                                            | 31000                                            | 41000                                            |
| 18700                                           | 25600                                            | 33400                                            | 44000                                            |
| 20400                                           | 27520                                            | 35800                                            | 47000                                            |
| 22100                                           | 29440                                            | 38200                                            | 50000                                            |
| 23800                                           | 31360                                            | 40600                                            | 53000                                            |
| 25500                                           | 33280                                            | 43000                                            | 56000                                            |
| 27200                                           | 35200                                            | 45400                                            | 59000                                            |
| 28900                                           | 37120                                            | 47800                                            | 62000                                            |
| 30600                                           | 39040                                            | 50200                                            | 65000                                            |
| 32300                                           | 40960                                            | 52600                                            | 68000                                            |
| 34000                                           | 42880                                            | 55000                                            | 71000                                            |
| 35700                                           | 44800                                            | 57400                                            | 74000                                            |
| 37400                                           | 46720                                            | 59800                                            | 77000                                            |
| 39100                                           | 48640                                            | 62200                                            | 80000                                            |
| 40800                                           | 50560                                            | 64600                                            | 83000                                            |
| 42500                                           | 52480                                            | 67000                                            | 86000                                            |
| 44200                                           | 54400                                            | 69400                                            | 89000                                            |
| 45900                                           | 56320                                            | 71800                                            | 92000                                            |
| 47600                                           | 58240                                            | 74200                                            | 95000                                            |
| 49300                                           | 60160                                            | 76600                                            | 98000                                            |
| 51000                                           | 62080                                            | 79000                                            | 101000                                           |
| 52700                                           | 64000                                            | 81400                                            | 104000                                           |
| 54400                                           | 65920                                            | 83800                                            | 107000                                           |
| 56100                                           | 67840                                            | 86200                                            | 110000                                           |
| 57800                                           | 69760                                            | 88600                                            | 113000                                           |
| 59500                                           | 71680                                            | 91000                                            | 116000                                           |
| 61200                                           | 73600                                            | 93400                                            | 119000                                           |
| 62900                                           | 75520                                            | 95800                                            | 122000                                           |
| 64600                                           | 77440                                            | 98200                                            | 125000                                           |
| 66300                                           | 79360                                            | 100600                                           | 128000                                           |
| 68000                                           | 81280                                            | 103000                                           | 131000                                           |
| 69700                                           | 83200                                            | 105400                                           | 134000                                           |
| 71400                                           | 85120                                            | 107800                                           | 137000                                           |
| 73100                                           | 87040                                            | 110200                                           | 140000                                           |
| 74800                                           | 88960                                            | 112600                                           | 143000                                           |
| 76500                                           | 90880                                            | 115000                                           | 146000                                           |
| 78200                                           | 92800                                            | 117400                                           | 149000                                           |
| 79900                                           | 94720                                            | 119800                                           | 152000                                           |
| 81600                                           | 96640                                            | 122200                                           | 155000                                           |
| 83300                                           | 98560                                            | 124600                                           | 158000                                           |
| 85000                                           | 100480                                           | 127000                                           | 161000                                           |
| 86700                                           | 102400                                           | 129400                                           | 164000                                           |
| 88400                                           | 104320                                           | 131800                                           | 167000                                           |
| 90100                                           | 106240                                           | 134200                                           | 170000                                           |
| 91800                                           | 108160                                           | 136600                                           | 173000                                           |
| 93500                                           | 110080                                           | 139000                                           | 176000                                           |
| 95200                                           | 112000                                           | 141400                                           | 179000                                           |
| 96900                                           | 113920                                           | 143800                                           | 182000                                           |
| 98600                                           | 115840                                           | 146200                                           | 185000                                           |
| 100300                                          | 117760                                           | 148600                                           | 188000                                           |
| 102000                                          | 119680                                           | 151000                                           | 191000                                           |
| 103700                                          | 121600                                           | 153400                                           | 194000                                           |
| 105400                                          | 123520                                           | 155800                                           | 197000                                           |
| 107100                                          | 125440                                           | 158200                                           | 200000                                           |
| 108800                                          | 127360                                           | 160600                                           | 203000                                           |
| 110500                                          | 129280                                           | 163000                                           | 206000                                           |
| 112200                                          | 131200                                           | 165400                                           | 209000                                           |
| 113900                                          | 133120                                           | 167800                                           | 212000                                           |
| 115600                                          | 135040                                           | 170200                                           | 215000                                           |
| 117300                                          | 136960                                           | 172600                                           | 218000                                           |
| 119000                                          | 138880                                           | 175000                                           | 221000                                           |
| 120700                                          | 140800                                           | 177400                                           | 224000                                           |
| 122400                                          | 142720                                           | 179800                                           | 227000                                           |
| 124100                                          | 144640                                           | 182200                                           | 230000                                           |
| 125800                                          | 146560                                           | 184600                                           | 233000                                           |
| 127500                                          | 148480                                           | 187000                                           | 236000                                           |
| 129200                                          | 150400                                           | 189400                                           | 239000                                           |
| 130900                                          | 152320                                           | 191800                                           | 242000                                           |
| 132600                                          | 154240                                           | 194200                                           | 245000                                           |
| 134300                                          | 156160                                           | 196600                                           | 248000                                           |
| 136000                                          | 158080                                           | 199000                                           | 251000                                           |
| 137700                                          | 160000                                           | 201400                                           | 254000                                           |
| 139400                                          | 161920                                           | 203800                                           | 257000                                           |
| 141100                                          | 163840                                           | 206200                                           | 260000                                           |
| 142800                                          | 165760                                           | 208600                                           | 263000                                           |
| 144500                                          | 167680                                           | 211000                                           | 266000                                           |
| 146200                                          | 169600                                           | 213400                                           | 269000                                           |
| 147900                                          | 171520                                           | 215800                                           | 272000                                           |
| 149600                                          | 173440                                           | 218200                                           | 275000                                           |
| 151300                                          | 175360                                           | 220600                                           | 278000                                           |
| 153000                                          | 177280                                           | 223000                                           | 281000                                           |
| 154700                                          | 179200                                           | 225400                                           | 284000                                           |
| 156400                                          | 181120                                           | 227800                                           | 287000                                           |
| 158100                                          | 183040                                           | 230200                                           | 290000                                           |
| 159800                                          | 184960                                           | 232600                                           | 293000                                           |
| 161500                                          | 186880                                           | 235000                                           | 296000                                           |
| 163200                                          | 188800                                           | 237400                                           | 299000                                           |
| 164900                                          | 190720                                           | 239800                                           | 302000                                           |
| 166600                                          | 192640                                           | 242200                                           | 305000                                           |
| 168300                                          | 194560                                           | 244600                                           | 308000                                           |
| 170000                                          | 196480                                           | 247000                                           | 311000                                           |
| 171700                                          | 198400                                           | 249400                                           | 314000                                           |
| 173400                                          | 200320                                           | 251800                                           | 317000                                           |
| 175100                                          | 202240                                           | 254200                                           | 320000                                           |
| 176800                                          | 204160                                           | 256600                                           | 323000                                           |
| 178500                                          | 206080                                           | 259000                                           | 326000                                           |
| 180200                                          | 208000                                           | 261400                                           | 329000                                           |
| 181900                                          | 209920                                           | 263800                                           | 332000                                           |
| 183600                                          | 211840                                           | 266200                                           | 335000                                           |
| 185300                                          | 213760                                           | 268600                                           | 338000                                           |
| 187000                                          | 215680                                           | 271000                                           | 341000                                           |
| 188700                                          | 217600                                           | 273400                                           | 344000                                           |
| 190400                                          | 219520                                           | 275800                                           | 347000                                           |
| 192100                                          | 221440                                           | 278200                                           | 350000                                           |
| 193800                                          | 223360                                           | 280600                                           | 353000                                           |
| 195500                                          | 225280                                           | 283000                                           | 356000                                           |
| 197200                                          | 227200                                           | 285400                                           | 359000                                           |
| 198900                                          | 229120                                           | 287800                                           | 362000                                           |
| 200600                                          | 231040                                           | 290200                                           | 365000                                           |
| 202300                                          | 232960                                           | 292600                                           | 368000                                           |
| 204000                                          | 234880                                           | 295000                                           | 371000                                           |
| 205700                                          | 236800                                           | 297400                                           | 374000                                           |
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| 210800                                          | 242560                                           | 304600                                           | 383000                                           |
| 212500                                          | 244480                                           | 307000                                           | 386000                                           |
| 214200                                          | 246400                                           | 309400                                           | 389000                                           |
| 215900                                          | 248320                                           | 311800                                           | 392000                                           |
| 217600                                          | 250240                                           | 314200                                           | 395000                                           |
| 219300                                          | 252160                                           | 316600                                           | 398000                                           |
| 221000                                          | 254080                                           | 319000                                           | 401000                                           |
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| 224400                                          | 257920                                           | 323800                                           | 407000                                           |
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| 227800                                          | 261760                                           | 328600                                           | 413000                                           |
| 229500                                          | 263680                                           | 331000                                           | 416000                                           |
| 231200                                          | 265600                                           | 333400                                           | 419000                                           |
| 232900                                          | 267520                                           | 335800                                           | 422000                                           |
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| 236300                                          | 271360                                           | 340600                                           | 428000                                           |
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| 241400                                          | 277120                                           | 347800                                           | 437000                                           |
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| 282200                                          | 323200                                           | 405400                                           | 509000                                           |
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| 285600                                          | 327040                                           | 410200                                           | 515000                                           |
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| 289000                                          | 330880                                           | 415000                                           | 521000                                           |
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| 292400                                          | 334720                                           | 419800                                           | 527000                                           |
| 294100                                          | 336640                                           | 422200                                           | 530000                                           |
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| 300900                                          | 344320                                           | 431800                                           | 542000                                           |
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| 304300                                          | 348160                                           | 436600                                           | 548000                                           |
| 306000                                          | 350080                                           | 439000                                           | 551000                                           |
| 307700                                          | 352000                                           | 441400                                           | 554000                                           |
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| 311100                                          | 355840                                           | 446200                                           | 560000                                           |
| 312800                                          | 357760                                           | 448600                                           | 563000                                           |
| 314500                                          | 359680                                           | 451000                                           | 566000                                           |
| 316200                                          | 361600                                           | 453400                                           | 569000                                           |
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| 319600                                          | 365440                                           | 458200                                           | 575000                                           |
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| 340000                                          | 388480                                           | 487000                                           | 611000                                           |
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| 346800                                          | 396160                                           | 496600                                           | 623000                                           |
| 348500                                          | 398080                                           | 499000                                           | 626000                                           |
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| 351900                                          | 401920                                           | 503800                                           | 632000                                           |
| 353600                                          | 403840                                           | 506200                                           | 635000                                           |
| 355300                                          | 405760                                           | 508600                                           | 638000                                           |
| 357000                                          | 407680                                           | 511000                                           | 641000                                           |