

RE: 757421 - H&H-NC/Roos Site Information: Project Customer: H and H Lot/Block: A Model: Address: City: General Truss Engineering O Drawings Show Special Load Design Code: IRC2009/TPI2 Wind Code: ASCE 7-05 Wind Roof Load: 40.0 psf	sevelt/ Project Name: Criteria & Desi ding Condition 007 Speed: 100 mp	757421 Subdivision State: NC gn Loads (ns): Dh E F	: All Individual Tru Design Program Design Method: Noor Load: N/2	u ss Design : MiTek 20/2 MWFRS(lov A psf	Trenco 818 Soundside Rd Edenton, NC 27932 20 7.6 w-rise)/C-C hybrid Wind ASCE 7-05
Mean Roof Height (feet): 25		I	Exposure Categ	ory: C	
No.Seal#Truss Nam1 131523379 A012 131523380 A023 131523381 A034 131523382 A045 131523383 A056 131523384 A107 131523385 B018 131523386 B029 131523387 B0310 131523387 B0310 131523387 B0310 131523397 C0113 131523393 D0116 131523393 D0116 131523397 E0120 131523397 E0120 131523400 G0223 131523401 H0124 131523402 H0225 131523403 J0326 131523404 J0427 131523406 J0629 131523407 J0730 131523410 J0831 131523410 J0932 131523412 PB0134 131523412 PB02	e Date No. 11/1/17 35 11/1/17 36 11/1/17 37 11/1/17	Seal# I31523413 I31523414 I31523415	Truss Name PB03 V01 V02	Date 11/1/17 11/1/17 11/1/17	

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters

In tex OSA, file: under my direct supervision based on the parameters provided by Builders FirstSource-Sumter,SC. Truss Design Engineer's Name: Garcia, Juan My license renewal date for the state of North Carolina is December 31, 2017 IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the **IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Garcia, Juan





Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314. November 1,2017





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 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 ANSI/TP14 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

ENGINE ERING BY





8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type	Qty	Ply	H&H-NC/Roosevelt/
757421	B03	Common	12	1	131523387
	500				Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s /	Aug 16 2017 MiTek Industries, Inc. Wed Nov 01 08:05:33 2017 Page 1



Plate Offsets (X,Y)-- [A:Edge,0-2-0]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.64	Vert(LL) -0.09 F-H >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.60	Vert(TL) -0.21 F-H >930 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.34	Horz(TL) 0.02 E n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.13 F-K >999 240	Weight: 101 lb FT = 20%

BRACING-TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except*
	A-H: 2x6 SP No.2
SLIDER	Right 2x4 SP No.3 1-11-12

REACTIONS. (lb/size) H=640/Mechanical, E=640/Mechanical Max Horz H=-531(LC 9)

Max Uplift H=-400(LC 9), E=-39(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

C-D=-618/0, D-E=-553/0 TOP CHORD

BOT CHORD G-H=0/602, G-M=0/602, F-M=0/602, F-N=0/602, E-N=0/602

C-F=0/357, C-H=-701/435 WEBS

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C

Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) E except (jt=lb) H=400.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

A-H. C-H

Rigid ceiling directly applied.

1 Row at midpt

Scale = 1:78.9

November 1,2017



Job	Truss	Truss Type	Qty	Ply	H&H-NC/Roosevelt/
757 101	504		~		I31523388
/5/421	804	Common	9	1	Job Reference (optional)
Builders FirstSource, S	Sumter, SC 29153	1		7.640 s /	Aug 16 2017 MiTek Industries, Inc. Wed Nov 01 08:05:34 2017 Page 1



Plate Offsets (X,Y)-- [A:Edge,0-2-0], [D:0-6-0,0-0-12]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.63	Vert(LL) -0.09 E-G >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.58	Vert(TL) -0.22 E-G >904 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.34	Horz(TL) 0.01 D n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0.06 E-J >999 240	Weight: 99 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 *Except* WEBS

A-G: 2x6 SP No.2

BRACING-TOP CHORD BOT CHORD WEBS

Rigid ceiling directly applied. A-G. C-G 1 Row at midpt

Structural wood sheathing directly applied, except end verticals.

REACTIONS. (lb/size) G=635/Mechanical, D=670/0-3-0 Max Horz G=-542(LC 9) Max Uplift G=-389(LC 9), D=-58(LC 9)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD C-D=-801/0

BOT CHORD F-G=0/593, F-K=0/593, E-K=0/593, E-L=0/593, D-L=0/593

WEBS C-E=0/354, C-G=-689/404

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) G=389.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.





Job	Truss	Truss Type	Qty	Ply	H&H-NC/Roosevelt/
757421	B05	GABLE	3	1	131523389
					Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s	Aug 16 2017 MiTek Industries, Inc. Wed Nov 01 08:05:34 2017 Page 1
		ID	5gbe_Q0	JNoiH4zfe	QirvLHzQqXF-P1J3?di4nnKIMkK6ncuM6cg9RqCMo1LmUFX9iCyNdPV



16-6-8 16-6-8

Plate Offsets (X,Y)-- [A:Edge,0-2-0] LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) l/defl L/d PLATES GRIP 244/190 TCLL 20.0 Plate Grip DOL 1.15 тс 0.48 Vert(LL) n/a n/a 999 MT20 TCDL 10.0 Lumber DOL 1.15 BC 0.36 Vert(TL) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.11 Horz(TL) 0.01 .1 n/a n/a Code IRC2009/TPI2007 BCDL 10.0 (Matrix) Weight: 134 lb FT = 20%

I.	U	M	R	F	R	-	

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x6 SP No.2
OTHERS	2x4 SP No.3

(lb) -

BRACING-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS

1 Row at midpt A-S. B-R. C-Q

REACTIONS. All bearings 16-6-8

Max Horz S=-532(LC 6) Max Uplift All uplift 100 lb or less at joint(s) S, Q, N, L except J=-113(LC 7), R=-106(LC 9), O=-101(LC 9), M=-101(LC 9), K=-143(LC 9) Max Grav All reactions 250 lb or less at joint(s) S, R, Q, O, N, M, L, K except J=263(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD B-C=-281/222, C-D=-350/221, D-E=-419/228, E-F=-426/222, F-G=-501/235, G-H=-577/242, H-I=-647/247, I-J=-755/263

R-S=-213/657, Q-R=-213/657, P-Q=-213/657, O-P=-213/657, N-O=-213/657, M-N=-213/657, BOT CHORD L-M=-213/657, K-L=-213/657, J-K=-213/657

NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) S, Q, N, L except (jt=lb) J=113, R=106, O=101, M=101, K=143.

10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Scale = 1:79.3

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Scale = 1:88.7

15-9-4 3-6-12 5-7-12 5-7-12 9-2-8 12-2-8 3-0-0 21-5-0 5-7-12

Plate Offsets (X,Y)-- [B:0-3-4,0-1-4], [E:0-5-8,0-3-0], [F:0-5-8,0-3-0], [I:0-3-4,0-1-4], [N:0-1-12,0-1-0], [S:0-1-12,0-1-0], [W:0-1-8,0-1-12], [X:0-2-0, Edge], [Y:0-2-0, Edge], [Z:0-1-8,0-1-12], [X:0-2-0, Edge], [Y:0-2-0, Edge <u>,0-1-12</u>]

	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.29	Vert(LL)	-0.11 Q-S	>999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.88	Vert(TL)	-0.24 P-R	>999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.54	Horz(TL)	0.04 K	n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL)	0.22 R-T	>999 240	Weight: 229 lb FT = 20%

LUMBER-		BRACING-			
TOP CHORD	2x6 SP No.2 *Except*	TOP CHORD	Structural wood sheathing	g directly applied or 6-0-0 oc purlins, except	
	W-X,Y-Z: 2x4 SP No.2		end verticals, and 2-0-0 o	c purlins (6-0-0 max.): E-F.	
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:		
WEBS	2x4 SP No.3 *Except*		8-2-5 oc bracing: T-U.		
	C-T,H-L: 2x6 SP No.2, D-G,B-U,I-K: 2x4 SP No.2		4-7-0 oc bracing: N-S		
		WEBS	1 Row at midpt	C-S, H-N	
		JOINTS	1 Brace at Jt(s): V		
REACTIONS	(lb/size) =1188/0-5-8 K=1188/0-5-8				

Max Horz U=-505(LC 6) Max UpliftU=-22(LC 8), K=-22(LC 9) Max Grav U=1377(LC 2), K=1377(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD B-C=-1065/161, C-D=-710/287, D-E=-426/279, E-F=-321/146, F-G=-426/278, G-H=-710/288, H-I=-1065/161, B-U=-1339/167, I-K=-1339/168 BOT CHORD T-U=-471/485, R-T=0/2115, P-R=0/2115, M-P=0/2115, L-M=0/2115, Q-S=-419/509, O-Q=-1498/0, N-O=-446/537

C-S=-62/391, H-N=-62/391, D-V=-517/266, G-V=-517/267, B-T=-80/822, I-L=-82/822, WEBS L-O=-1724/101, Q-T=-1724/127

NOTES-(14)

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (5.0 psf) on member(s). C-D, G-H, D-V, G-V; Wall dead load (5.0 psf) on member(s).C-S, H-N
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. Q-S, O-Q, N-O

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- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) U, K.
- 11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Attic room checked for L/360 deflection.
- 14) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



November 1,2017



Edenton, NC 27932



ID:5gbe_Q0JNoiH4zfeQirvLHzQqXF-LQRqQJkKJOa0b1UVu0xqB1mY1dmqGq83yZ0Gm5yNdPT 7-4-11 15-9-4 15-4-9 1-4-4 5-7-12 5-7-12 6-0-7 0-4-11 21-5-0 5-7-12 1-4-4 6x8^{0-<u>4-1</u>1} 6x8 D E 3x6 = 3x6 = С 12.00 12 F в G 3x5 || 4x5 = 1-6-0 4x5 =н 8-2-4 7-2-0 7-2-0 Ø 4-1-5 4-1-5 9-8-0 -0 民 ĕ Κ R Р Ν s J $3x6 \overline{2x}6 =$ 2x6 || $4x10 \frac{1}{2x6} =$ 4x5 = 2x6 || 4x5 = 4x10 =

Scale = 1:88.7

15-9-4 3-6-12 5-7-12 5-7-12 9-2-8 3-6-12 12-2-8 21-5-0 5-7-12 -

Plate Offsets (X,Y)-- [A:0-3-4,0-0-8], [D:0-5-8,0-3-0], [E:0-5-8,0-3-0], [H:0-3-4,0-0-8], [L:0-1-12,0-1-0], [Q:0-1-12,0-1-0], [U:0-1-8,0-1-12], [V:0-2-0, Edge], [W:0-2-0, Edge], [X:0-1-8,0-1-12], [V:0-2-0, Edge], [W:0-2-0, Edge <u>,0-1-12</u>]

LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL. ir	(loc)	l/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.29	Vert(LL) -0.1	O-Q	>999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.88	Vert(TL) -0.24	N-P	>999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.54	Horz(TL) 0.0	F I	n/a n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL) 0.2	2 P-R	>999 240	Weight: 224 lb FT = 20%

LUMBER-		BRACING-				
TOP CHORD	2x6 SP No.2 *Except*	TOP CHORD	Structural wood sheathing	g directly applied or 6-0-0 oc purlins, except		
	U-V,W-X: 2x4 SP No.2		end verticals, and 2-0-0 o	c purlins (6-0-0 max.): D-E.		
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:			
WEBS	2x4 SP No.3 *Except*		8-3-4 oc bracing: R-S.			
	B-R,G-J: 2x6 SP No.2, C-F,A-S,H-I: 2x4 SP No.2		4-7-0 oc bracing: L-Q			
		WEBS	1 Row at midpt	B-Q, G-L		
		JOINTS	1 Brace at Jt(s): T			
REACTIONS.	(lb/size) S=1134/0-5-8, I=1134/0-5-8					

Max Horz S=-500(LC 6) Max Grav S=1332(LC 2), I=1332(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD A-B=-1066/136, B-C=-713/278, C-D=-424/276, D-E=-318/144, E-F=-424/276,

11-6-0

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F-G=-713/279, G-H=-1066/135, A-S=-1294/116, H-I=-1294/117 R-S=-462/475, P-R=0/2116, N-P=0/2116, K-N=0/2116, J-K=0/2116, O-Q=-420/512, BOT CHORD M-O=-1498/0, L-M=-447/539 WEBS B-Q=-66/389, G-L=-66/389, C-T=-520/254, F-T=-520/255, A-R=-88/827, H-J=-90/827, J-M=-1724/102, O-R=-1724/128

NOTES-(13)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads. Provide adequate drainage to prevent water ponding.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Ceiling dead load (5.0 psf) on member(s). B-C, F-G, C-T, F-T; Wall dead load (5.0psf) on member(s).B-Q, G-L
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. O-Q, M-O, L-M
- 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) Attic room checked for L/360 deflection.

13) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Job	Truss	Truss Type	Qty	Ply	H&H-NC/Roosevelt/
757421	C03	Attic Girder	1		131523392
				Z	Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s /	Aug 16 2017 MiTek Industries, Inc. Wed Nov 01 08:05:38 2017 Page

ID:5gbe_Q0JNoiH4zfeQirvLHzQqXF-HpZar_lar?qkrLet0RzIHSrlERQrkfVMPtVMrzyNdPR 15-9-4 10-8-8 3-3-13 -11 16-2-1 -6 0-4-13 0-6-9 5-7-12 7-4-11 0-4-13 1-2-6 21-5-0 5-2-15 6x8 6x8 = 5x6 = F 4x5 =6x12 _ 3x5 II 6x12 = 5x8 || 12.00 12 Gн J AĤ ٩E V AG AF 7x8 || 6x8 || 6x8 II 8-2-4 7-2-0 7-2-0 4-6-13 4-6-13 9-8-0 -2-0 ٠ DX1 M U AC AD Р к $T_{7x10} = R$ L 6x10 MT18H = 3x8 MT20HS = 6x10 MT18H = 5x6 4x6 5-7-12 0-4-13 1-6-0 1-6-0 15-9+4 0-6-9 0-4-1 5-2-15 5-2-15 9-2-8 3-6-12 <u>15-2-11</u> 3-0-3 21-5-0 5-2-15

Plate Offsets (X,Y)-- [B:0-0-2,0-1-2], [D:0-2-0,Edge], [E:0-5-8,0-3-0], [F:0-5-8,0-3-0], [G:0-2-0,Edge], [H:0-3-8,0-3-0], [K:0-4-12,0-3-0], [N:0-5-8,0-3-0], [S:0-2-8,Edge], [V:0-3-0, [S:0-3-0, [S:0-<u>,0-2-12], [W:0-1-8,0-1-12], [X:0-1-8,0-1-12]</u>

LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES (GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.82	Vert(LL)	-0.27	N-O	>940	360	MT20 2	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.91	Vert(TL)	-0.37	N-O	>684	240	MT20HS 1	187/143
BCLL	0.0 *	Rep Stress Incr NO	WB 0.87	Horz(TL)	0.02	ĸ	n/a	n/a	MT18H 2	244/190
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL)	0.25	L-P	>999	240	Weight: 623 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x8 SP DSS *Except*	TOP CHORD	Structural wood sheathing directly applied or 4-9-8 oc purlins, except
	C-E,F-I: 2x6 SP No.1, E-F: 2x6 SP No.2, D-W,G-X: 2x4 SP No.2		end verticals, and 2-0-0 oc purlins (6-0-0 max.): A-B, B-C, E-F, H-I, H-J.
BOT CHORD	2x4 SP No.2 *Except*		Except:
	M-U: 2x4 SP No.1		1 Row at midpt B-C, H-I
WEBS	2x4 SP No.3 *Except*	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
	A-U,J-K,B-T,H-L: 2x6 SP No.2, D-G: 2x8 SP DSS		10-0-0 oc bracing: P-R.
	C-S,I-N,R-S,N-P: 2x4 SP No.2	WEBS	1 Row at midpt A-U, J-K
		JOINTS	1 Brace at Jt(s): A, J, V
DEACTIONO			

REACTIONS. (lb/size) U=5158/0-5-8, K=5800/0-5-8 (req. 0-5-15) Max Horz U=-897(LC 13) Max Uplift U=-1754(LC 13), K=-1900(LC 16) Max Grav U=9005(LC 12), K=10048(LC 11)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD C-U=-9114/2049, A-C=-1366/374, A-Y=-282/253, Y-Z=-392/445, B-Z=-608/635, B-C=-7372/1825, B-D=-5820/1264, D-E=-2204/578, E-F=-4732/940, F-G=-2460/614
- G-H=-6507/1378, H-I=-7526/1880, H-AA=-613/588, AA-AB=-501/561, J-AB=-447/378, I-K=-9671/2166, I-J=-1705/421 BOT CHORD U-AC=-3181/3140, T-AC=-3181/3140, R-T=-3505/3374, P-R=0/3490, M-P=-3375/4604,
- L-M=-3375/4604, L-AD=-2991/4289, K-AD=-2991/4106, Q-S=-709/1298, O-Q=-709/1298, N-O=-709/1298 WFBS S-T=0/418, B-S=-958/751, L-N=0/408, H-N=-2130/912, D-AE=-4191/941, AE-AF=-4191/941,
- V-AF=-4191/941, V-AG=-3492/868, AG-AH=-3492/868, AH-AI=-3492/868, G-AI=-3492/868, E-V=-1482/4629, F-V=-1372/3846, S-U=-1427/1827, C-S=-1659/6000, I-N=-1694/6043, K-N=-3634/2208, Q-R=-979/332, O-P=-551/310, R-S=-2806/6580, N-P=-2583/4172
- NOTES-(20)
- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x8 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x8 - 2 rows staggered at 0-2-0 oc, 2x4 - 1 row at 0-9-0 oc. 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

- 5) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated

All plates are 2x4 MT20 unless otherwise indicated.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will Contfit let weepable bottom chord and any other members.

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Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Scale = 1:88.7





Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Roosevelt/
757421	C03	Attic Girder	1	2	131523392
				_	Job Reference (optional)
Builders FirstSource, S	Sumter, SC 29153			7.640 s	Aug 16 2017 MiTek Industries, Inc. Wed Nov 01 08:05:38 2017 Page 2
			ID:5gbe	Q0JNoiH	4zfeQirvLHzQqXF-HpZar lar?qkrLet0RzIHSrlERQrkfVMPtVMrzyNdPR

NOTES- (20)

- 11) Ceiling dead load (5.0 psf) on member(s). B-D, G-H, D-V, G-V; Wall dead load (5.0psf) on member(s).B-S, H-N
- 12) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. Q-S, O-Q, N-O
- 13) WARNING: Required bearing size at joint(s) K greater than input bearing size.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) U=1754, K=1900.
- 15) This truss has been designed for a total drag load of 100 plf. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 1-0-0, 20-5-0 to 21-5-0 for 1071.0 plf.
- 16) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 17) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 18) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s). The design/selection of such connection device(s) is the responsibility of others.
- 19) Attic room checked for L/360 deflection.
- 20) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: A-B=-60, B-D=-70, D-E=-60, E-F=-60, F-G=-60, G-H=-70, H-J=-60, K-U=-35(F=-15), N-S=-45(F=-15), D-G=-10

Drag: B-S=-10, H-N=-10

Concentrated Loads (lb)

Vert: B=-670 Y=-670 Z=-670 AA=-1230 AB=-1230 AE=-640(B) AF=-640(B) AG=-640(B) AH=-640(B) AI=-1233(B)





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 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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Edenton, NC 27932



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Job	Truss	Truss Type	Qty	Ply	H&H-NC/Roosevelt/
757421	D04	Common Girder	3	2	131523396
				_	Job Reference (optional)
Builders FirstSource,	Sumter, SC 29153			7.640 s /	Aug 16 2017 MiTek Industries, Inc. Wed Nov 01 08:05:40 2017 Page 2
		I	D:5gbe_Q	0JNoiH4zf	eQirvLHzQqXF-EBhKGgnrNd4R4foG7s?mMtwEjE66CiofsB_TvsyNdPP

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: F=-620(F) O=-615(F) P=-615(F) Q=-615(F) R=-620(F) T=-620(F) U=-620(F)





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ENGINEERING BY AMITOR ATTILIATE

Edenton, NC 27932



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a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
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MSISTPH Quality Criteria, DSB-89 and BCSI Building Component
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Plate Offsets (X,Y)-- [B:1-0-0,0-1-8], [B:0-0-4,Edge]

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.68 BC 0.27 WB 0.13 (Matrix)	DEFL. Vert(LL) 0.0 Vert(TL) 0.0 Horz(TL) 0.0	n (loc) 0 E 10 E 10 G	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES GRIP MT20 244/190 Weight: 57 lb FT = 20%
LUMBER-	•		BRACING-				

TOP CHORD

BOT CHORD

end verticals.

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP No.2
WEBS	2x4 SP No.2
OTHERS	2x4 SP No.3

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz B=165(LC 7) Max Uplift All uplift 100 lb or less at joint(s) G except B=-127(LC 6), H=-232(LC 1), I=-287(LC 8) Max Grav All reactions 250 lb or less at joint(s) G, H except B=312(LC 1), I=773(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS C-I=-487/437

NOTES-(10)

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) G except (jt=lb) B=127, H=232, I=287.
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

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Plate Offsets (X,Y)	[B:0-2-4,Edge]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.58 BC 0.52 WB 0.00	DEFL. in Vert(LL) 0.00 Vert(TL) 0.05 Horz(TL) 0.00	(loc) l/defl L/d) A n/r 120 5 A n/r 120) n/a n/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2009/TPI2007	(Matrix)			Weight: 23 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	2 No.1 2 No.2 2 No.3		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing d end verticals. Rigid ceiling directly applied	irectly applied or 6-0-0 oc purlins, except or 10-0-0 oc bracing.

REACTIONS. (lb/size) D=251/6-6-0, B=310/6-6-0 Max Horz B=96(LC 6) Max Uplift D=-94(LC 6), B=-133(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-(11)

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) B=133
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) B.
- 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 11) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(10)

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) B=131
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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NOTES- (9

- Wind: ASCÉ 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=231, D=146.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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Max Uplift A=-176(LC 6), C=-148(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (10)

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) C considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) C.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) A=176, C=148.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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LOADING (p TCLL 20 TCDL 10 BCLL 0 BCLL 0 BCDL 10	osf) 0.0 0.0 0.0 * 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2009/TI	2-0-0 1.15 1.15 YES Pl2007	CSI. TC BC WB (Matr	0.56 0.51 0.00 ix)	DEFL. Vert(LL) Vert(TL) Horz(TL)	in 0.00 0.05 0.00	(loc) A A D	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 23 lb	GRIP 244/190 FT = 20%	
LUMBER-					-	BRACING					_		

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(10)

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 4) Gable requires continuous bottom chord bearing.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D except (jt=lb) B=136

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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REACTIONS. (lb/size) D=247/6-5-0, B=307/6-5-0 Max Horz B=109(LC 7) Max Uplift D=-89(LC 8), B=-136(LC 6)



NOTES-(9)

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left exposed ; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) D considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) D.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) B=244, D=156
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 9) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint A, 114 lb uplift at joint E, 307 lb uplift at joint B and 280 lb uplift at joint D.

10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss

11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

12) This manufactured truss is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.



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