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ALL CONSTRUCTION SHALL CONFORM TO THE 2018 EDITION OF THE NC STATE BUILDING CODE.

CODES GOVERN OVER DRAWINGS.

DIMENSIONS GOVERN OVER SCALE.

VERIFY ALL MECHANICAL REQUIREMENTS BEFORE FRAMING.

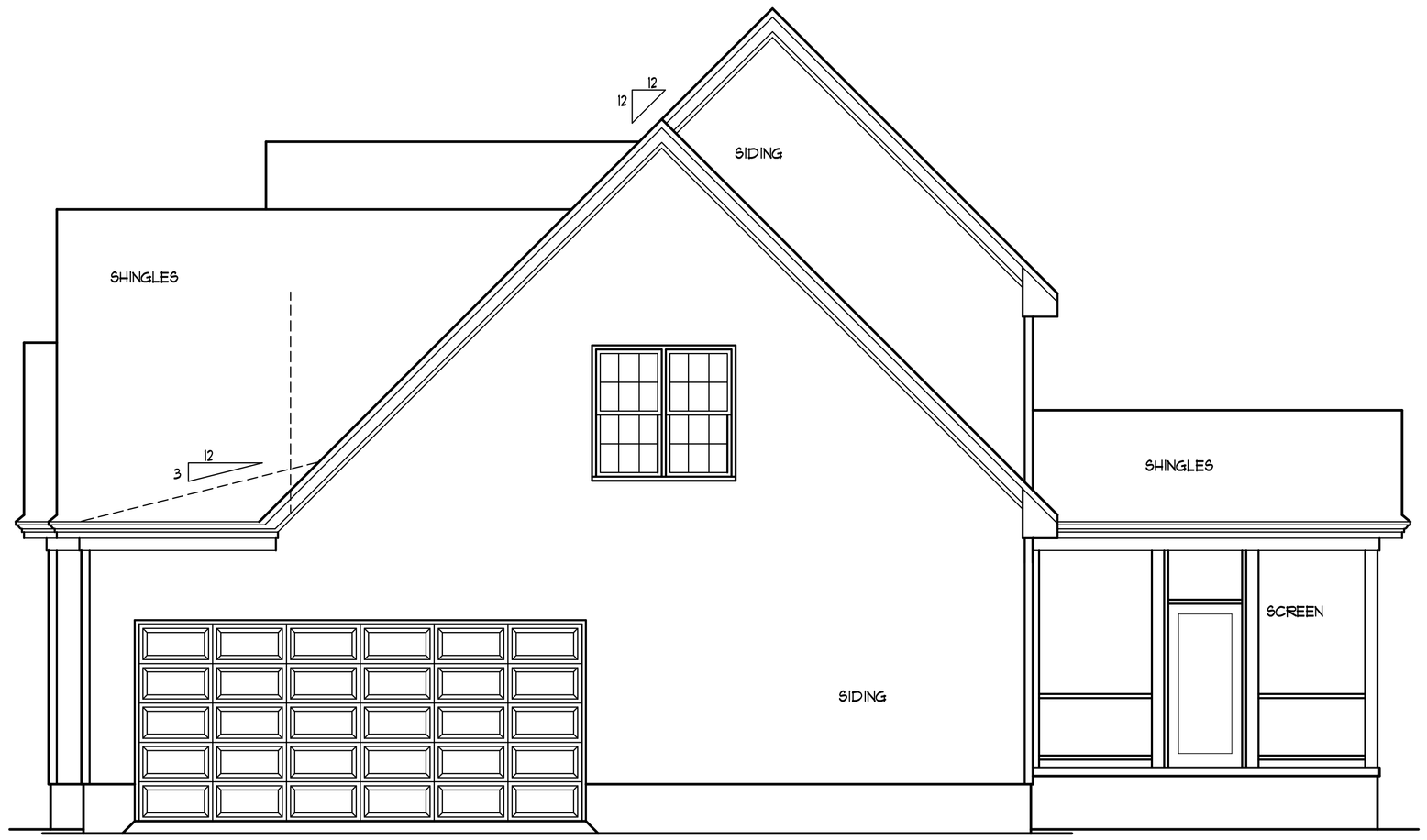
YUNCANNON DESIGNS DOES NOT ASSUME LIABILITY FOR ANY DEVIATION OF OR CONSTRUCTION METHODS OF THESE PLANS.

NOTICE TO CONTRACTOR
All construction must comply with current NC Building Codes and is subject to field inspection and verification.

APPROVED
Limited building only review
Permit holder responsible for full compliance with the code

06/30/2021

Harnett COUNTY NORTH CAROLINA



RIGHT SIDE ELEV
SCALE: 1/4" = 1'-0"

FRONT ELEVATION
SCALE: 1/4" = 1'-0"

Plan No. 2662-15

MILTON BUILT HOMES

YUNCANNON DESIGNS
CUSTOM HOME PLANS
FUQUAY-VARINA, NC - (919) 427-1374

DRAWN BY : WRV
CHK'D BY : WRV
DATE : 02/05/19
REVISIONS :

SHEET
A-1

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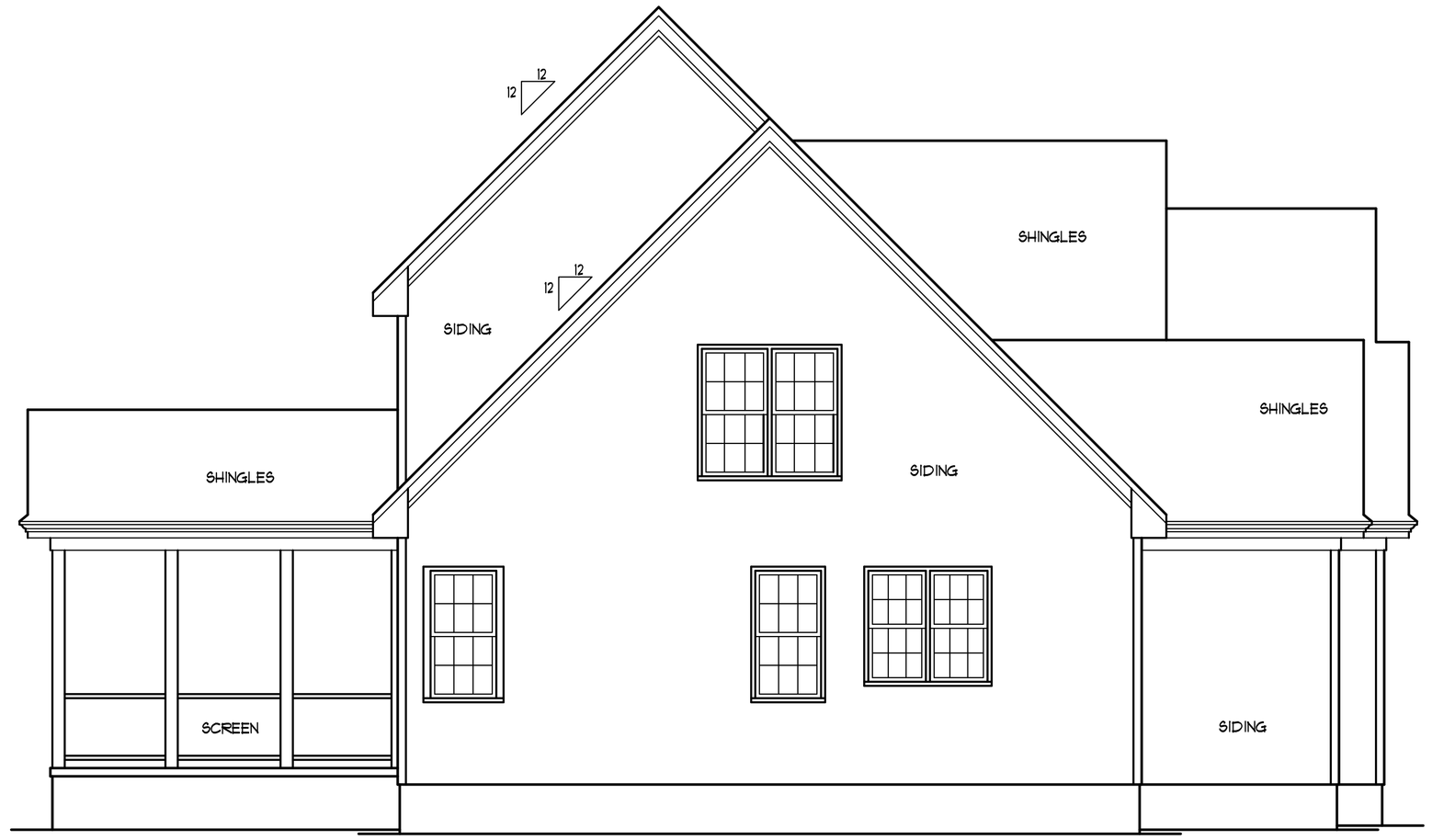
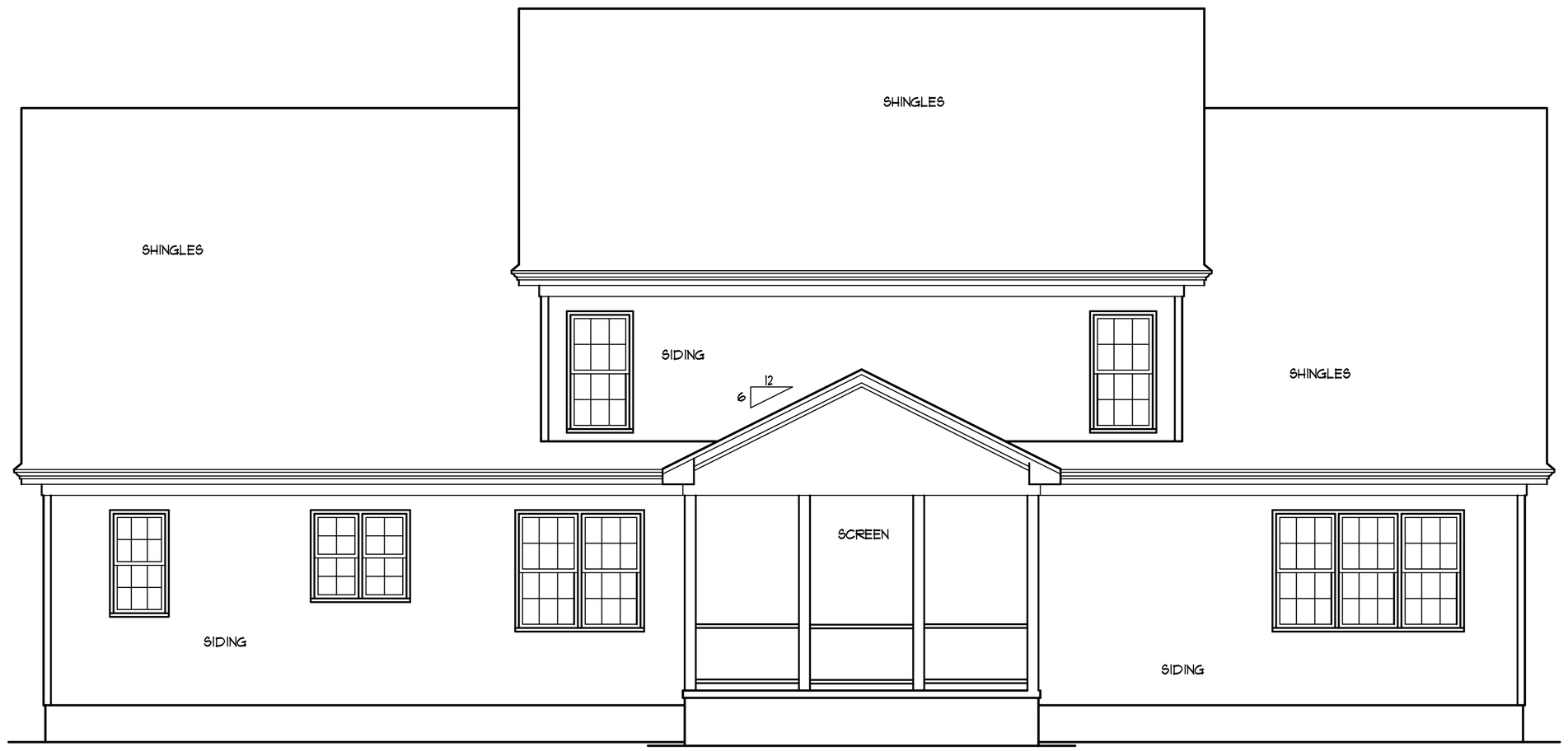


LEFT SIDE ELEV
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"

MILTON BUILT HOMES

VUNCANNON DESIGNS
CUSTOM HOME PLANS
FUQUAY-VARINA, NC - (919) 427-1374

DRAWN BY : WRV
CHK'D BY : WRV
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REVISIONS :

SHEET
A-2

Plan No. 2662-15

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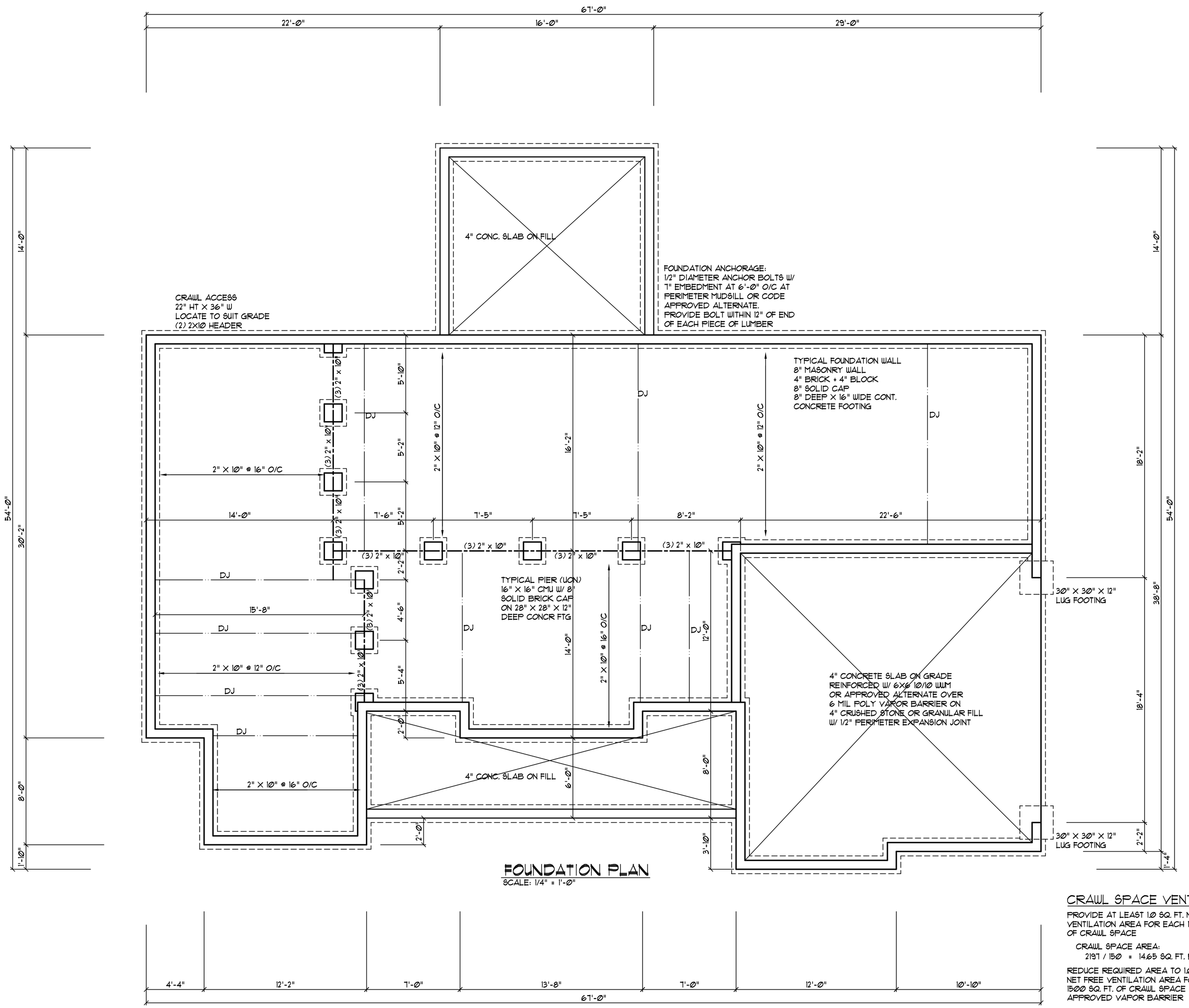

FOUNDATION PLAN
SCALE: 1/4" = 1'-Ø"

CRAWL SPACE VENTILATION

PROVIDE AT LEAST 1Ø SQ. FT. NET FREE VENTILATION AREA FOR EACH 15Ø SQ. FT. OF CRAWL SPACE

CRAWL SPACE AREA:
2197 / 15Ø = 14.65 SQ. FT. REQ'D.

REDUCE REQUIRED AREA TO 1Ø SQ. FT. NET FREE VENTILATION AREA FOR EACH 15ØØ SQ. FT. OF CRAWL SPACE WITH APPROVED VAPOR BARRIER

PROVIDE (1) VENT WITHIN 3'-Ø" OF EACH CORNER

REFER TO MANUFACTURER SPECIFICATIONS FOR ACTUAL VENTS USED TO DETERMINE NUMBER OF VENTS REQUIRED

FOUNDATION NOTES

1) SPECIFIC NOTES ON THE PLANS TAKE PRECEDENCE OVER THE FOLLOWING GENERAL NOTES. NOTED DIMENSIONS GOVERN OVER SCALE. PLAN DESIGNER AND CONTRACTOR ARE SOLELY RESPONSIBLE FOR DIMENSIONAL ACCURACY. ALL CONSTRUCTION TO COMPLY WITH NC STATE BUILDING CODE, VOL. VII WITH CURRENT REVISIONS.
2) STRUCTURAL CONCRETE TO BE F'C=3000 PSI, PREPARED AND PLACED IN ACCORDANCE WITH ACI STANDARD 318.
3) FOOTINGS TO BEAR ON UNDISTURBED EARTH, A MIN. OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE LOCAL INSPECTOR.
4) FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLEY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.
5) FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 3" MIN. FOOTING PROJECTION FROM FACE OF MASONRY.
6) MAXIMUN DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN VOLUME VII, SECTION R-3043 OF THE NC STATE CODE.
7) PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.
8) PROVIDE FOUNDATION WATERPROOFING AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.
9) PROVIDE PERIMETER INSULATION WITH BASEMENT SLABS.
10) CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEER.
11) CRAWL SPACE TO BE GRADED LEVEL AND CLEAR OF ALL DEBRIS.

ALLOWABLE PIER HEIGHTS		
SIZE	HOLLOW	SOLID
8"X16"	2'-8"	5'-Ø"
12"X16"	4'-Ø"	8'-Ø"
16"X16"	5'-4"	12'-Ø"
24"X24"	8'-Ø"	
FOOTINGS 30"X30"X10"-MINIMUM, UNLESS NOTED OTHERWISE OTHERWISE		

NOTE

1) USE 2X10 'S AT 16" O.C. #2 SPF. MINIMUM SPECIFIED (U.O.N.)
2) JOISTS DIRECTION DESIGNATED BY
3) DOUBLE ALL JOIST PARALLEL TO WALLS ABOVE.
4) GIRDERS TO BE (3)2X10'S #2 SPF. MINIMUM (U.O.N.)
5) MINIMUM CRAWL SPACE ACCESS TO BE 22" HEIGHT X 36" WIDTH.
6) FOUNDATION WALL AND PIER CAPS TO BE MINIMUM 8" SOLID.
7) FOOTING UNDER MASONRY FIREPLACE TO BE 12" THICK AND EXTEND 12" BEYOND.

Plan No. 2662-15

MILTON BUILT HOMES

VUNCANNON DESIGNS
CUSTOM HOME PLANS
RALEIGH-YARDNA, NC - (919) 427-1314

DRAWN BY : WRV
CHK'D BY : WRV
DATE : Ø2/Ø5/19
REVISIONS :

SHEET
A-3

BUILDER SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT TIME OF CONSTRUCTION.

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DIMENSIONS GOVERN OVER SCALE.

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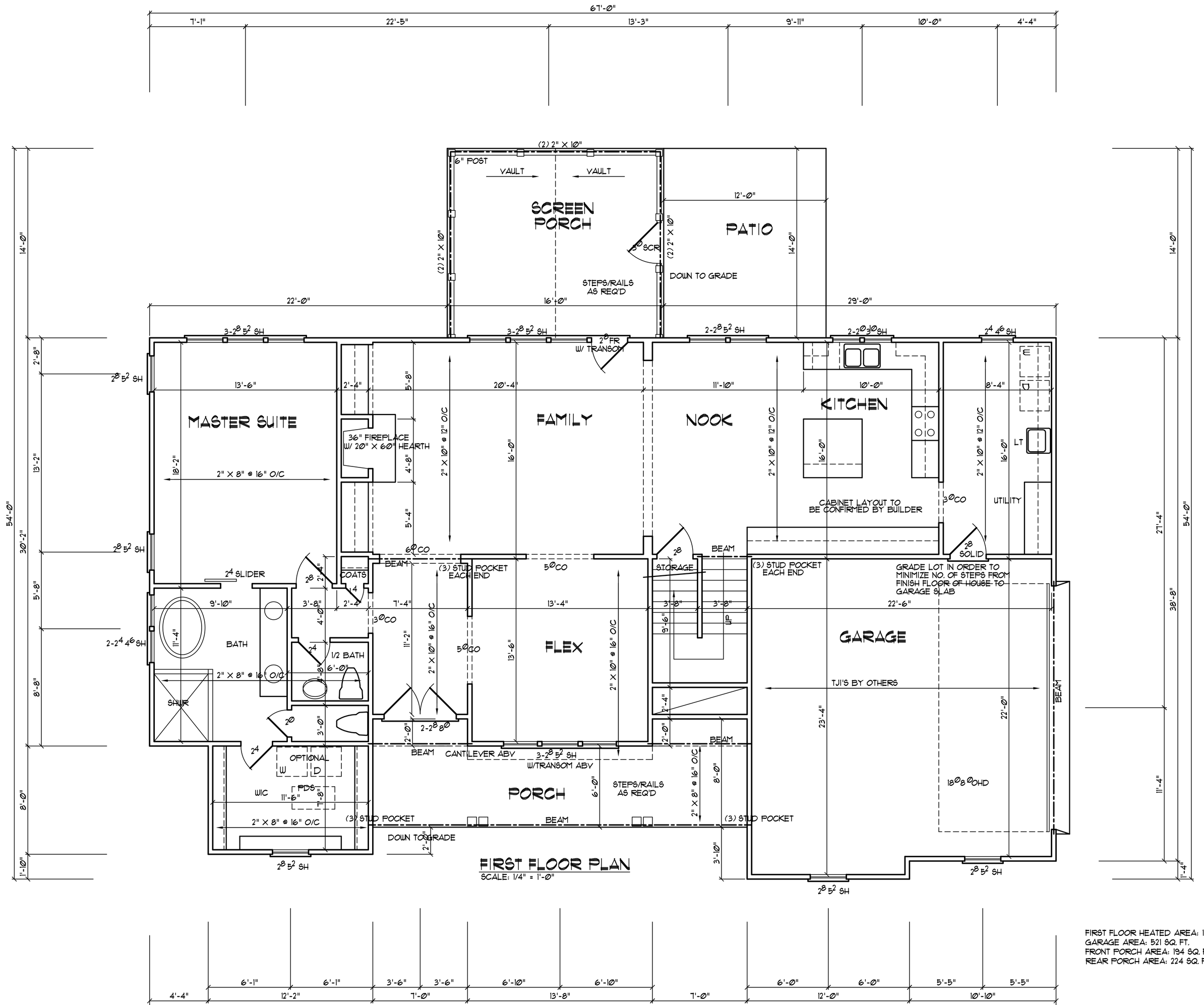

FIRST FLOOR PLAN
SCALE: 1/4" = 1'-0"

FRAMING NOTES

1) ALL FRAMING LUMBER SPF #2 GRADE (NLGA) OR BETTER, UNLESS NOTED.
2) JOIST SIZE AND SPACING BASED ON CODE MINIMUMS. BUILDER SHOULD CONSIDER STIFFENING FLOOR SYSTEM IN LONG SPAN AREAS, TO REDUCE FLOOR FLEXIBILITY.
3) PROVIDE DOUBLE JOIST UNDER WALLS PARALLEL TO JOIST DIRECTION.
4) PROVIDE SOLID BLOCKING BETWEEN ALL POINT LOADS AND TOP OF GIRDER, PIER OR FOUNDATION WALL.
5) JOISTS DIRECTION DESIGNATED BY

FIRST FLOOR HEATED AREA: 1779 SQ. FT.
GARAGE AREA: 521 SQ. FT.
FRONT PORCH AREA: 134 SQ. FT.
REAR PORCH AREA: 224 SQ. FT.

NOTE

1) 9'-0" CEILING HEIGHT
2) FRAME WINDOWS AT 7'-8" ABOVE FINISHED FLOOR
3) DOUBLE ALL JOISTS PARALLEL TO WALLS ABOVE
4) USE TEMPERED GLASS AT TUB & SHWR AREAS & WITH U.O.I. LESS THAN 18" ABOVE FINISHED FLOOR WITH MORE THAN 9 SQ. FT. OF GLASS AND U.O.I. WITHIN 2' OF SWINGING DOORS.
5) USE 2X10'S AS HEADERS (MIN.) U.O.N. ON STRUCTURAL SHEETS
6) ATTIC ACCESS 22"X30" MINIMUM.
7) SILLS AND PLATES TO BE 8" OR MORE ABOVE GRADE OR BE TREATED MATERIAL.
8) SIDING TO BE MINIMUM 6" ABOVE GRADE.
9) DOOR FROM GARAGE TO HOUSE TO BE 20 MINUTE FIRE RATED. ADD 1/2" SHEETROCK ON ALL WALLS & CEILINGS COMMON TO GARAGE & HEATED SPACE.
10) MINIMUM 15" SIDE CLEARANCE FROM CENTER LINE OF WATER CLOSET.
11) PROVIDE TREATED BAND AT CONCRETE SLAB.

Plan No. 2662-15

MILTON BUILT HOMES

VUNCANNON DESIGNS
CUSTOM HOME PLANS
FUQUAY-VARINA, NC - (919) 427-1374

DRAWN BY : WRV
CHK'D BY : WRV
DATE : 02/05/19
REVISIONS :

SHEET
A-4

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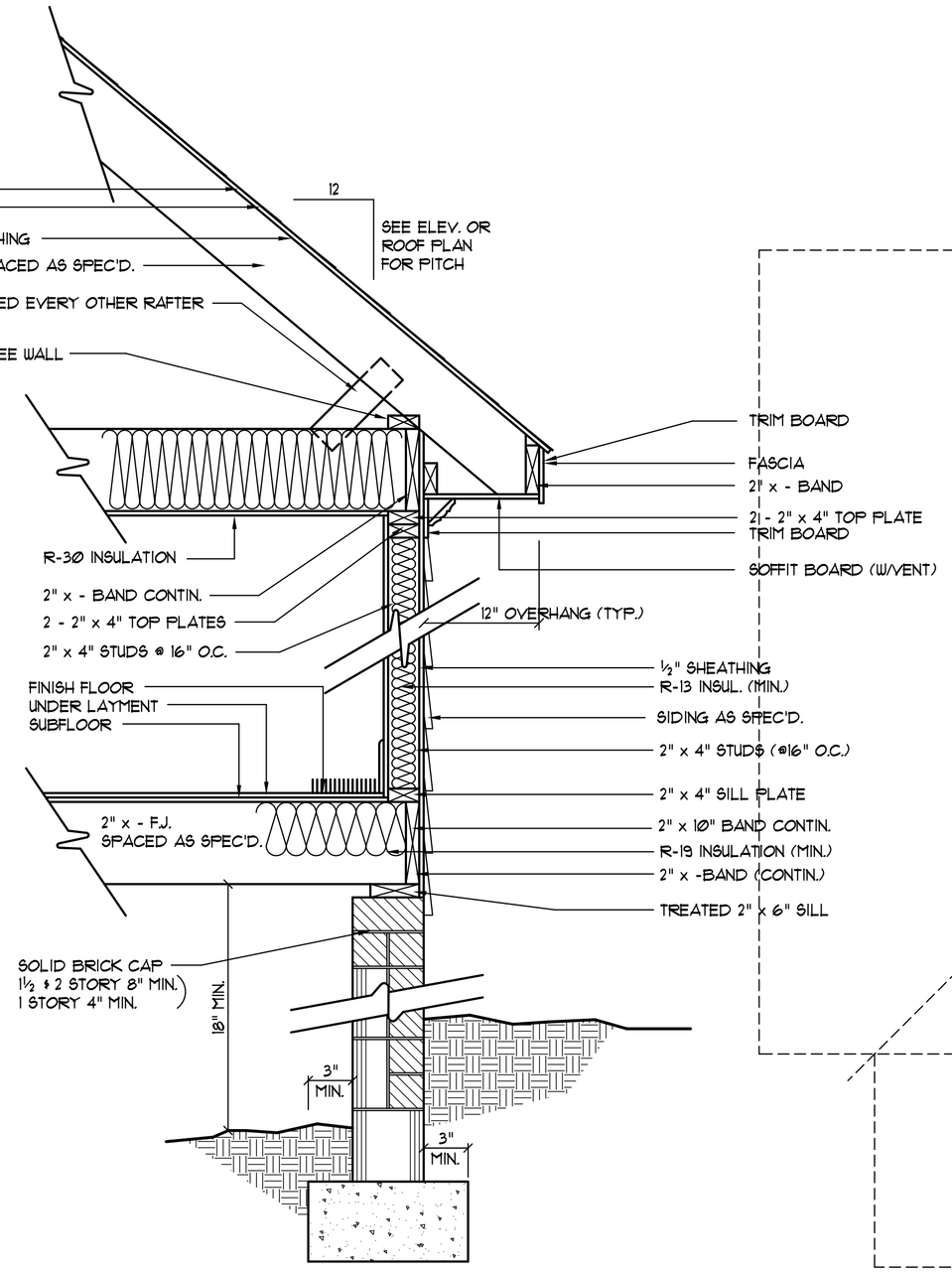
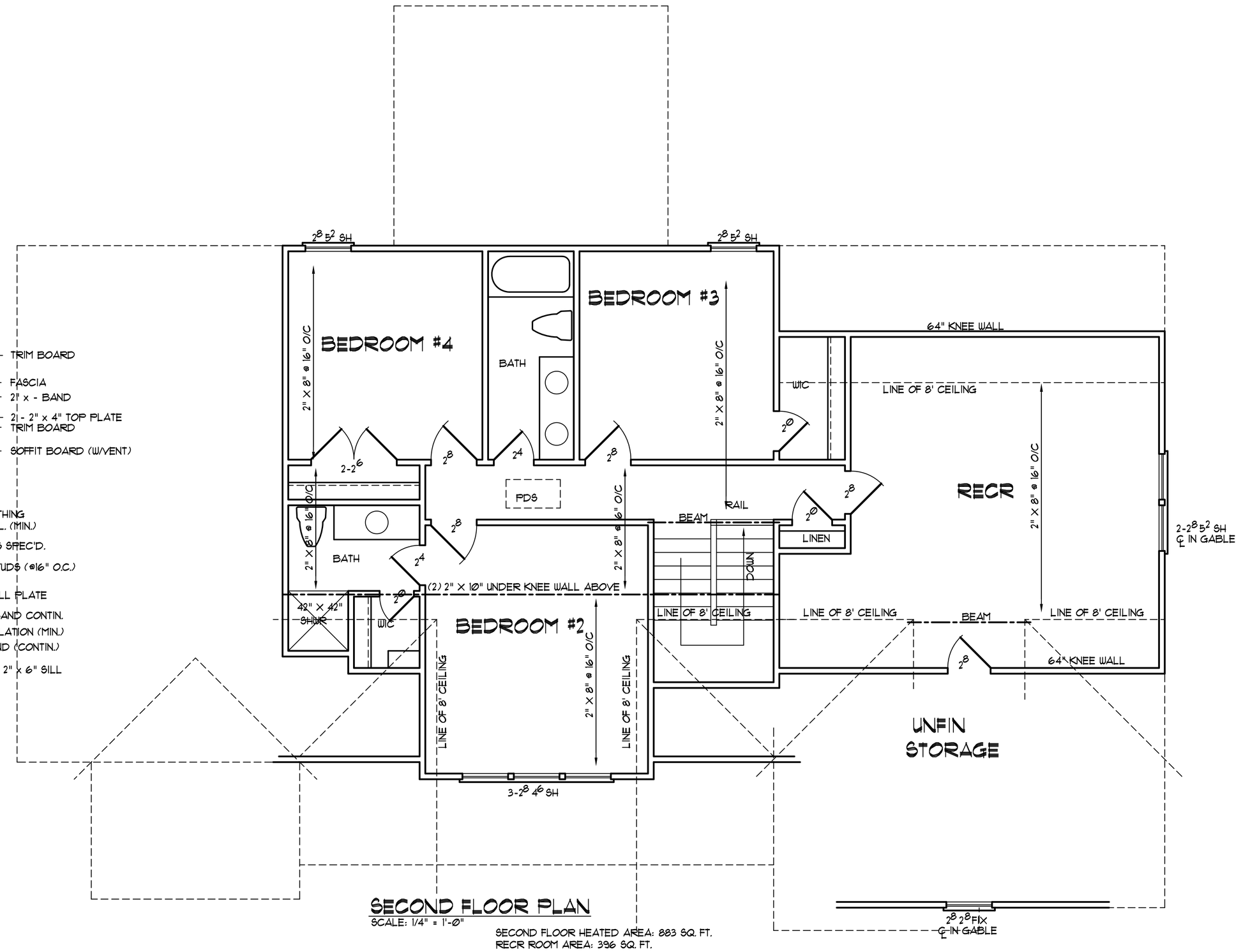


FOR FOOTING SIZES REFER TO FOUNDATION PLAN.

EXTERIOR WALL SECTION

SCALE: NTS



SECOND FLOOR PLAN

SCALE: 1/4" = 1'-0"

SECOND FLOOR HEATED AREA: 883 SQ. FT.
RECR ROOM AREA: 396 SQ. FT.

NOTE

1) 8'-0" CEILING HEIGHT
2) FRAME WINDOWS AT 6'-8" ABOVE FINISHED FLOOR
3) DOUBLE ALL JOISTS PARALLEL TO WALLS ABOVE
4) USE TEMPERED GLASS AT TUB ŧ SHWR. AREAS ŧ WITH WDWS. LESS THAN 18" ABOVE FINISHED FLOOR WITH MORE THAN 9 SQ. FT. OF GLASS AND WDWS. WITHIN 2" OF SWINGING DOORS.
5) USE 2X10'S AS HEADERS (TYP.) U.O.N. ON STRUCTURAL SHEETS
6) ATTIC ACCESS 22"X30" MINIMUM.
7) SILLS AND PLATES TO BE 8" OR MORE ABOVE GRADE OR BE TREATED MATERIAL.
8) SIDING TO BE MINIMUM 6" ABOVE GRADE.
9) DOOR FROM GARAGE TO HOUSE TO BE 20 MINUTE FIRE RATED. ADD 1/2" SHEETROCK ON ALL WALLS ŧ CEILINGS COMMON TO GARAGE ŧ HEATED SPACE.
10) MINIMUM 15" SIDE CLEARANCE FROM CENTER LINE OF WATER CLOSET.
11) PROVIDE TREATED BAND AT CONCRETE SLAB.

Plan No. 2662-15

MILTON BUILT HOMES

YUNCANNON DESIGNS
CUSTOM HOME PLANS
FUQUAY-VARINA, NC - (919) 427-7374

DRAWN BY : WRV
CHK'D BY : WRV
DATE : 02/05/19
REVISIONS :

SHEET A-5

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DIMENSIONS GOVERN OVER SCALE.

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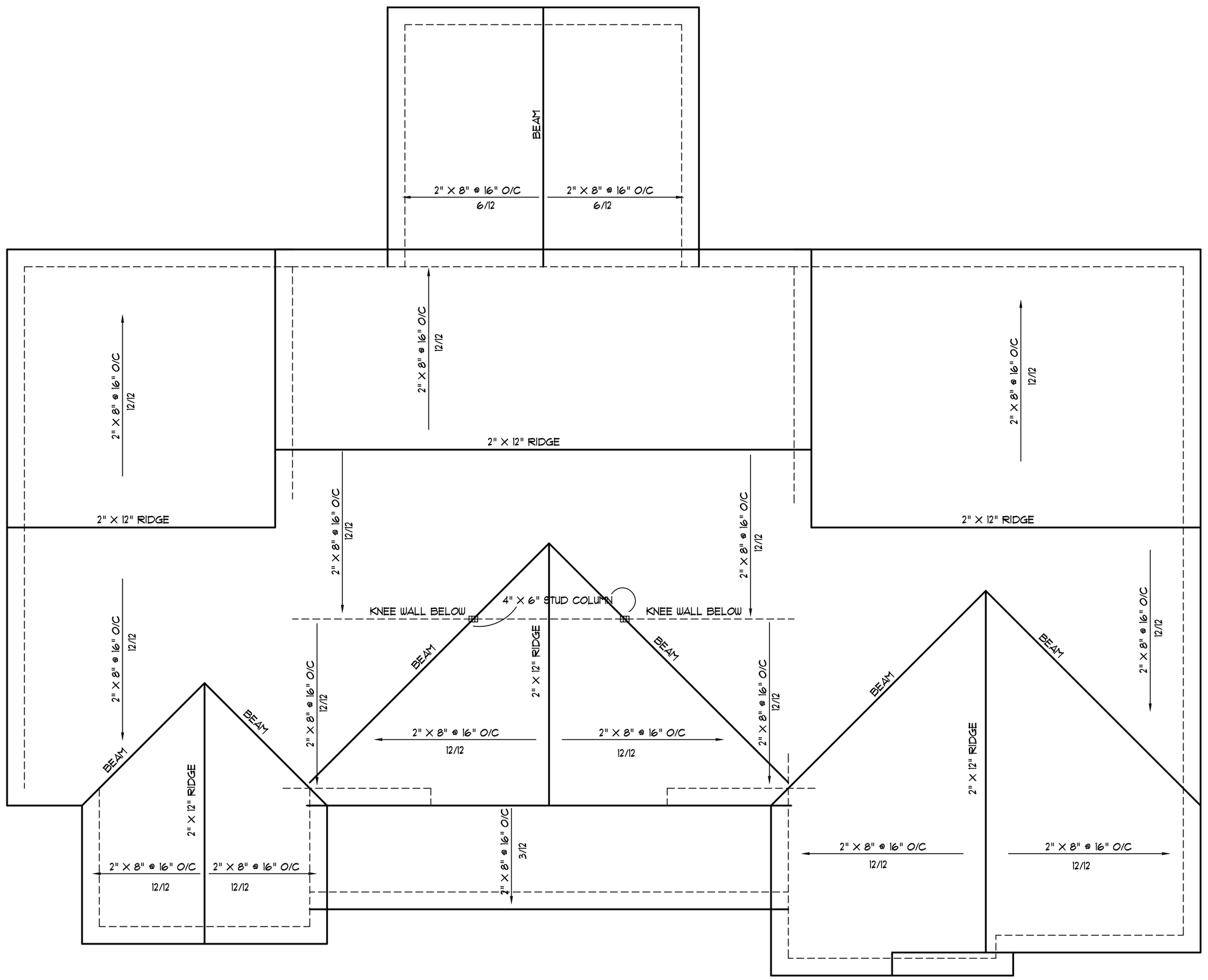


ROOF PLAN
SCALE: 1/4" = 1'-0"

ATTIC SPACE VENTILATION

MAIN ROOF
ATTIC AREA = 2718 SQ. FT.
REQUIRED AREA = A/300 = 9.06 SQ. FT.

NOTE:
ALL EAVES TO HAVE 2" CONTINUOUS SOFFIT VENT
ALLOW 1" AIR SPACE ABOVE INSULATION FOR AIR FLOW

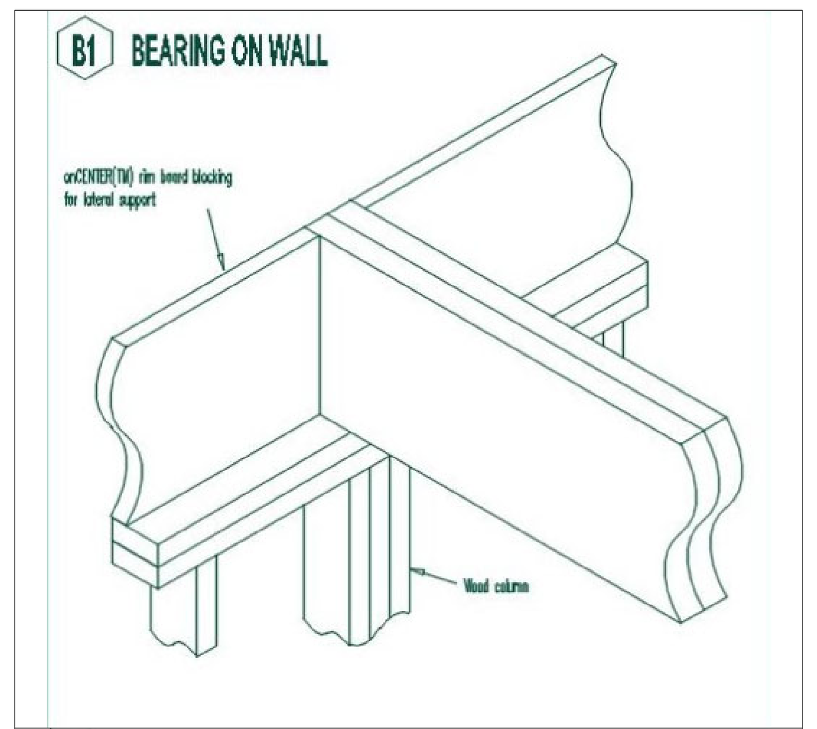
Plan No. 2662-15

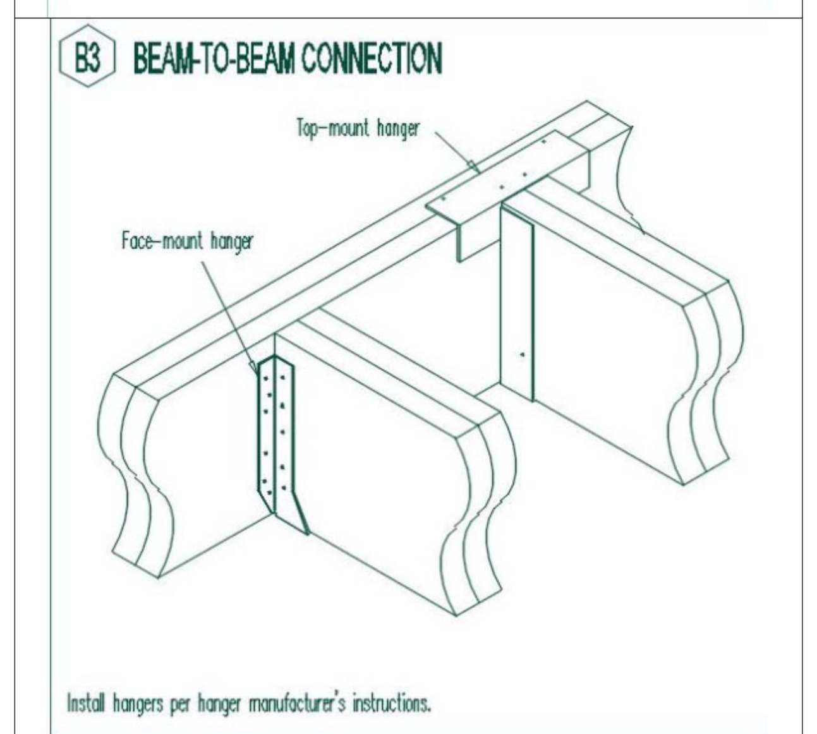
MILTON BUILT HOMES

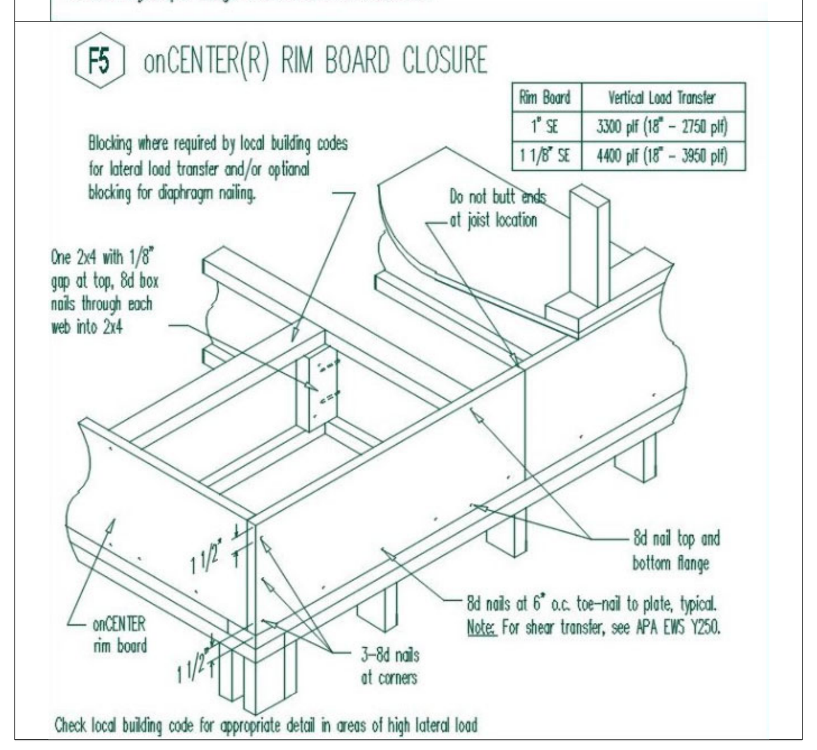
YUNCANNON DESIGNS
CUSTOM HOME PLANS
FUQUAY-VARINA, NC - (919) 427-7374

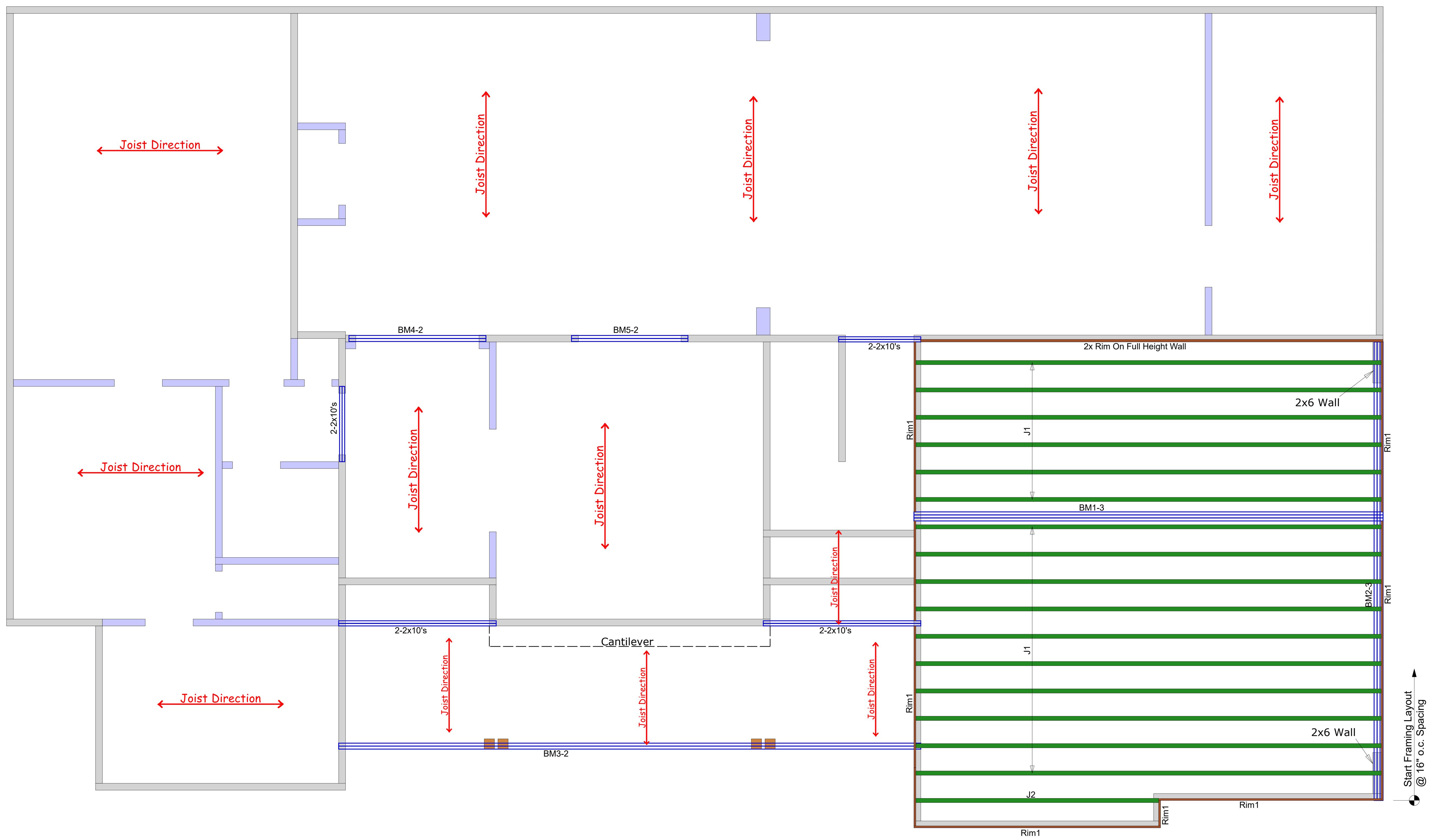
DRAWN BY : WRV
CHK'D BY : WRV
DATE : 01/18/19
REVISIONS :

SHEET
A-6



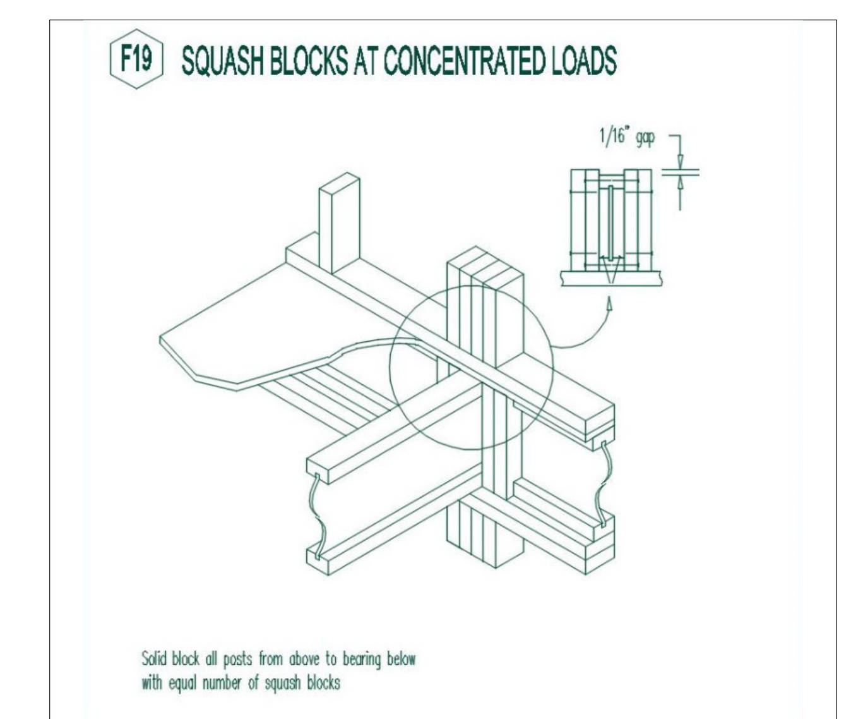


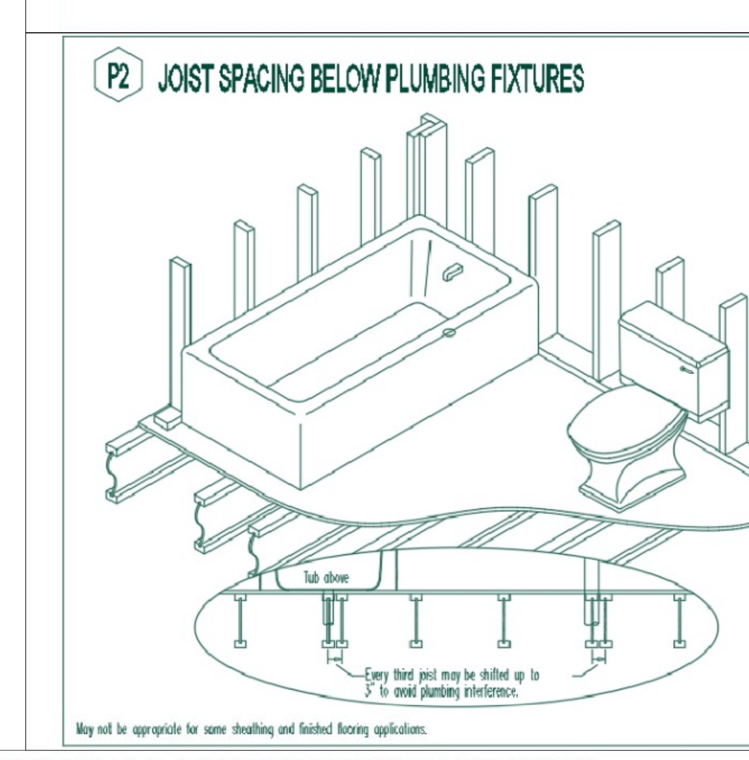




Products				
PlotID	Length	Product	Plies	Net Qty
J1	24' 0"	BLI 60 16"	1	16
J2	12' 0"	BLI 60 16"	1	1
BM1-3	24' 0"	onCENTER® LVL 2.0E 1 3/4" x 16"	3	3
BM2-3	24' 0"	onCENTER® LVL 2.0E 1 3/4" x 18"	3	3
Rim1	12' 0"	onCENTER® Rim SE 1 1/8" x 16"	1	6

Framing Material LVL's (By Others)				
PlotID	Length	Product	Plies	Net Qty
BM3-2	30' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	2
BM4-2	8' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	2
BM5-2	6' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	2



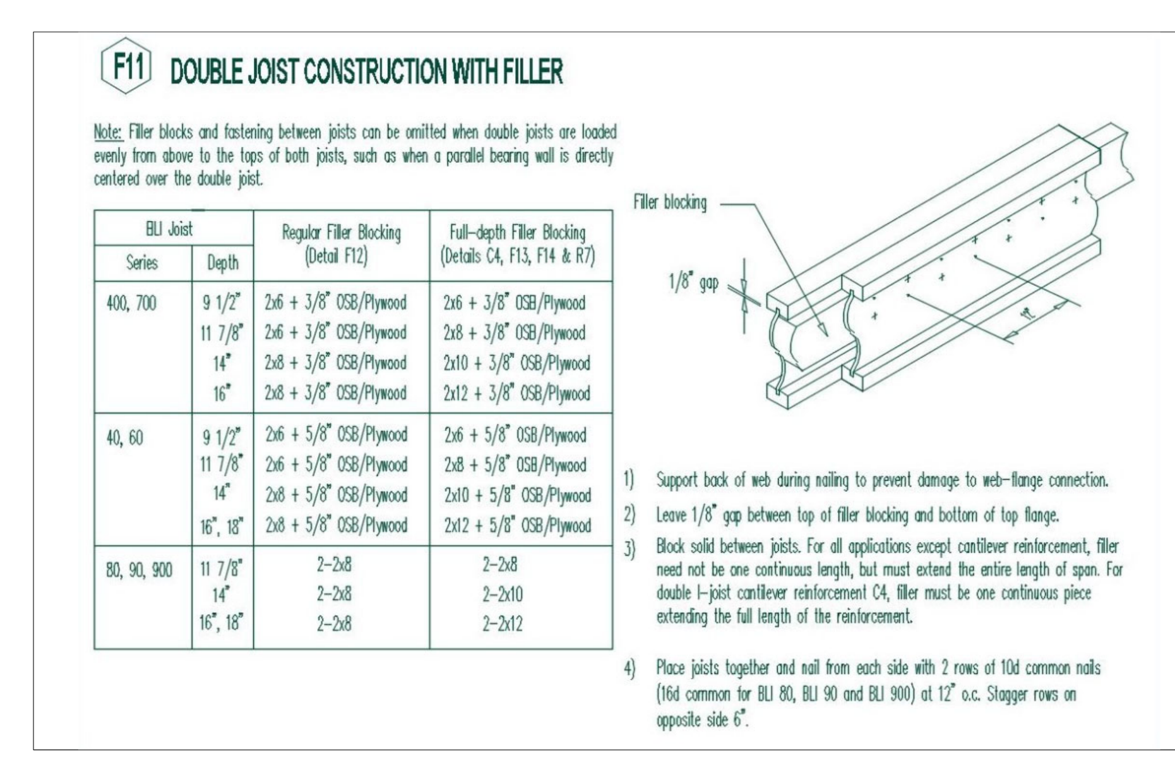


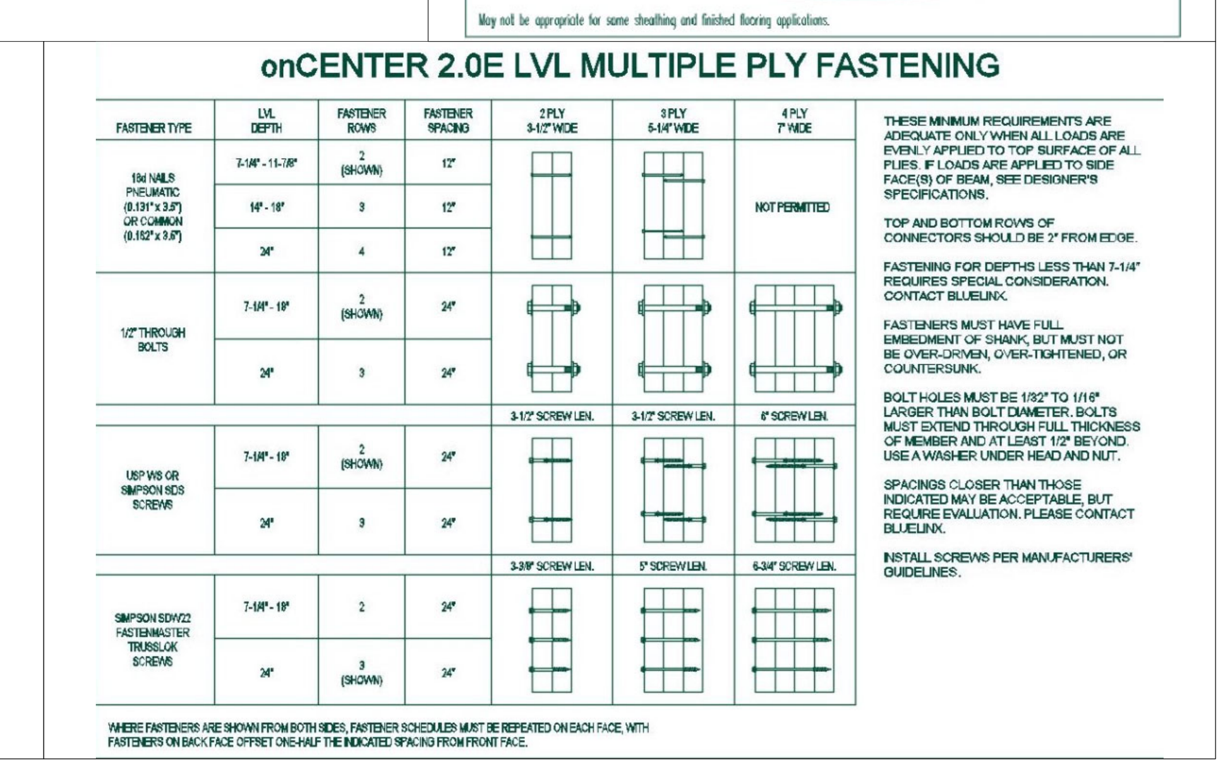
The attached materila list represents BlueLinx Engineered Lumber products needed to frame layout(s) shown based on the interpretation of user, but has not been reviewed by a BluLinx engineer. Purchaser is to verify material quantities, lengths, locations, and sizes and resolve clouded items.

Specified products are sized only for gravity loads shown. These loads should be verified by the purchaser. If additional loads or framing areas need to be accounted for, notify supplier of BlueLInx products so that material can be sized and price adjusted. Unless noted otherwise, hip, valley and ridge boards have not been designed and no products were designed to resist the building's lateral loads.

prior to installation, review layout with applicable product guide and/or installation sheet. If this information is not included, contact your supplier of BlueLinx products. Coordinate building plans and details with this layout. To prevent member damage from plumbing or mechanical cuts. Review this layout plan before placement. Follow I-joist web hole charts. Do not cut material beyond scope of product guide(s) withour prior approval.

Glue and nail minimum 23/32" APA rated OSB or plywood to floor I-joists. Use fastener schedule for side-loaded onCENTER(TM) LVL beams. For proper installation of hangers and connectors, follow manufacturer's guidelines. Connections not shown are by others.
Roofs shall have adequate drainage to prevent ponding.
These products were designed for "dry use" conditions only, and must be protected from long term exposure to high moistrue. Moisture protection (by others) may be required.
Detail F19 Squash Block/Columnn shall match size of column above





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...it's about the service!!!

3941 USHwy 421 North
Wilmington, NC 28401
(910) 386-4300

DRAWN BY:
JJC

DATE:
03-12-2019

SCALE:
1/4" = 1'-0"

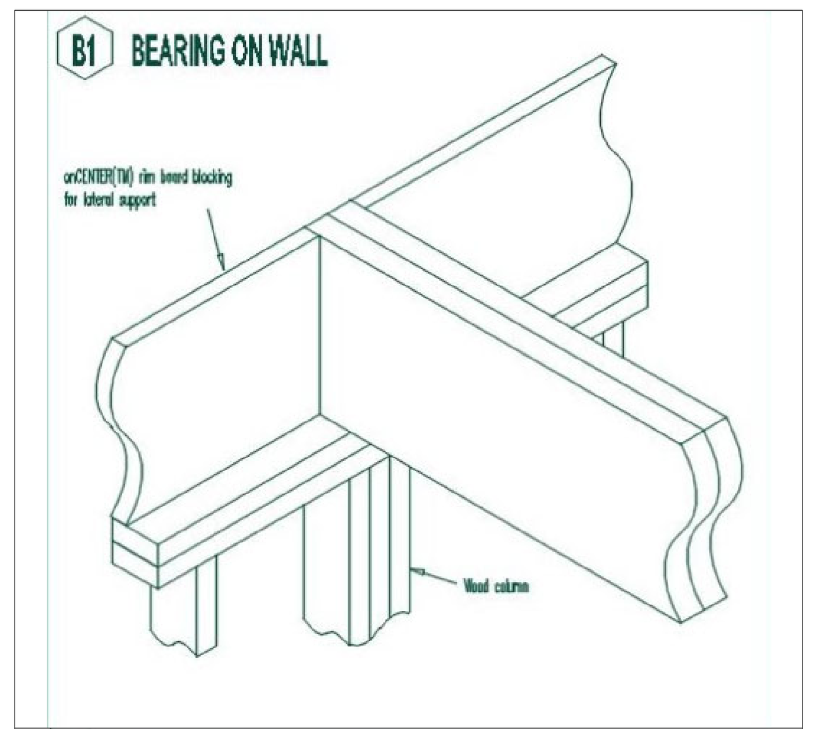
SALESPERSON:
Matthew Kemper

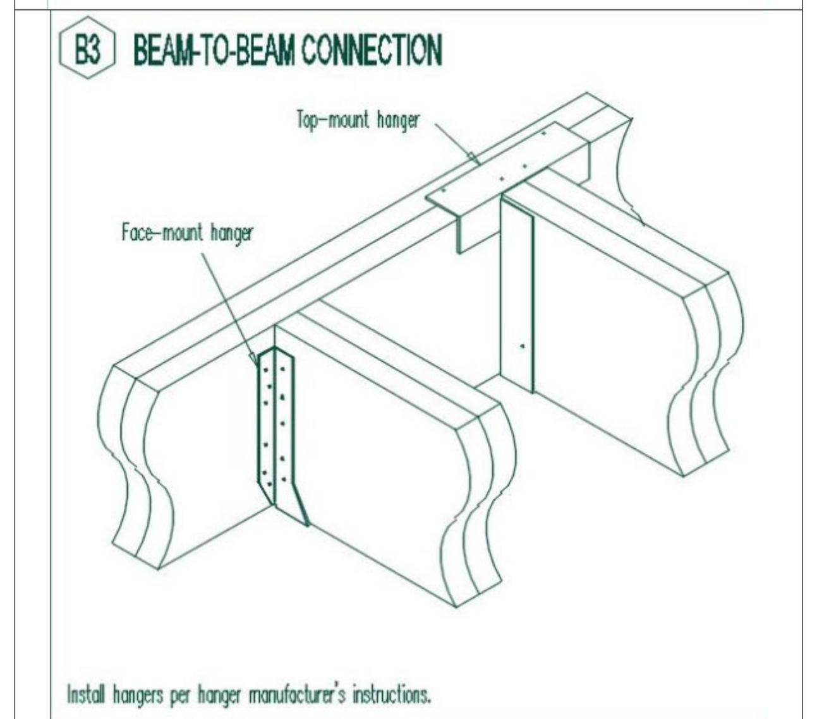
BUILDER NAME:
Milton Built Homes, LLC

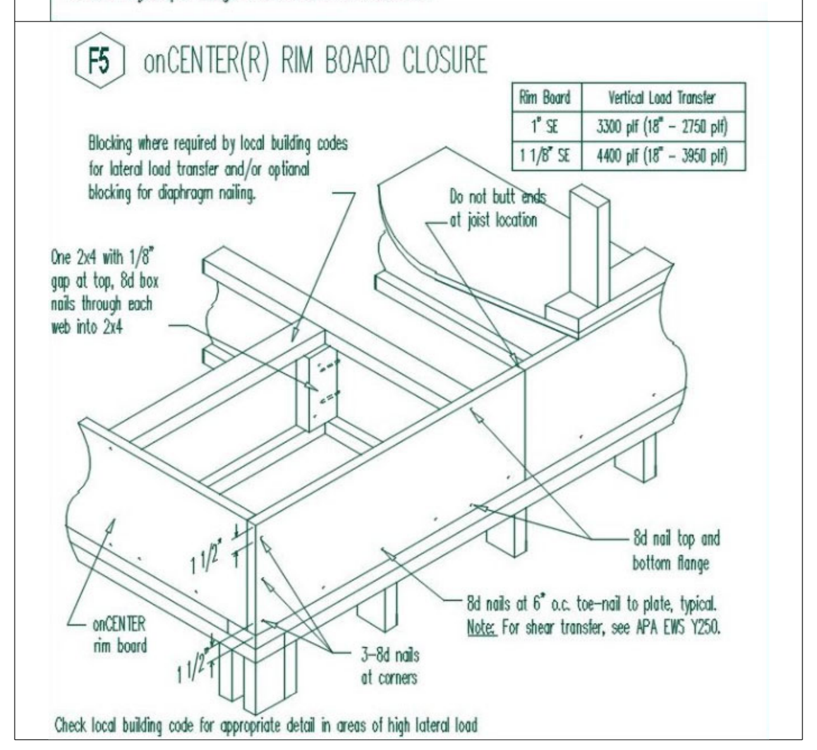
PROJECT NAME:
Lot 7 Raven Ridge
(Plan # 2662-15)

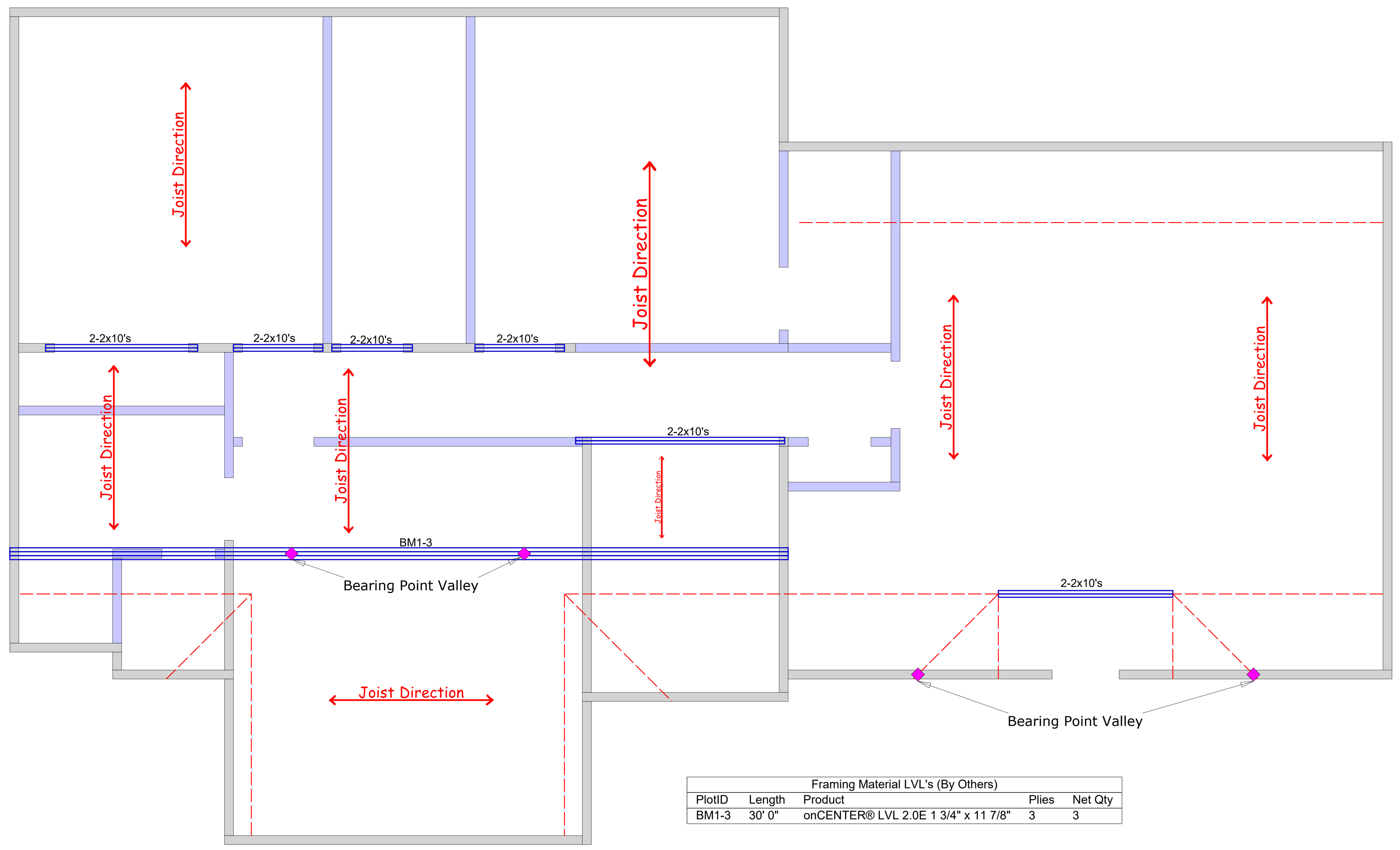
LEVEL NAME:
1st Floor Framing Layout (Ceiling)

PAGE: 1









Framing Material LVL's (By Others)

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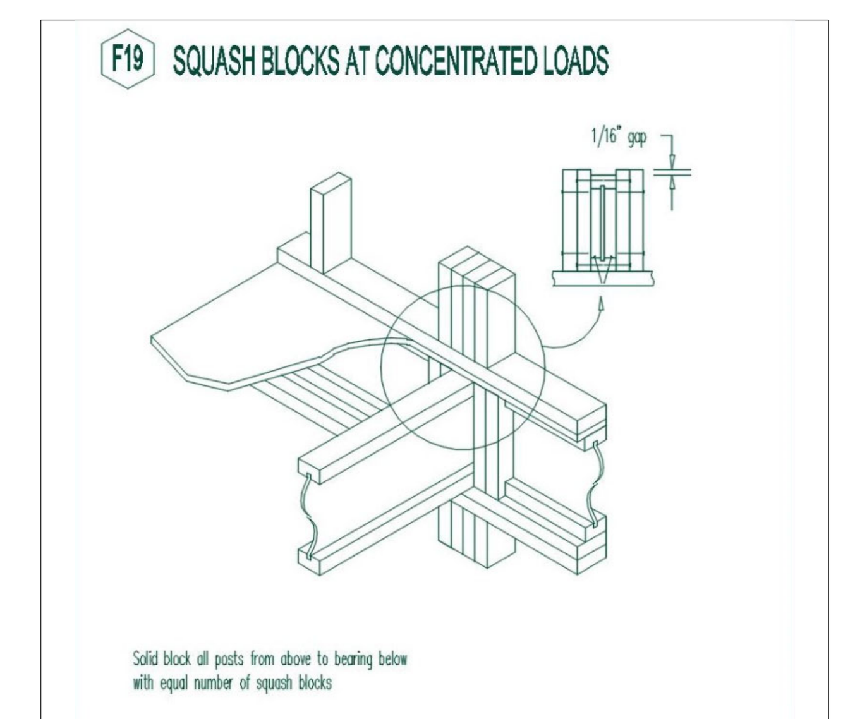
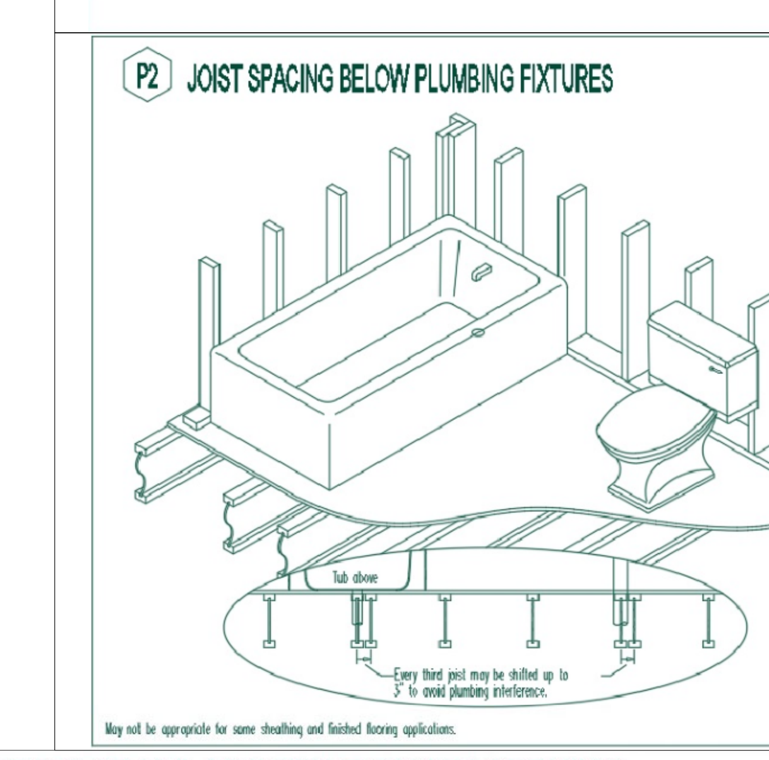
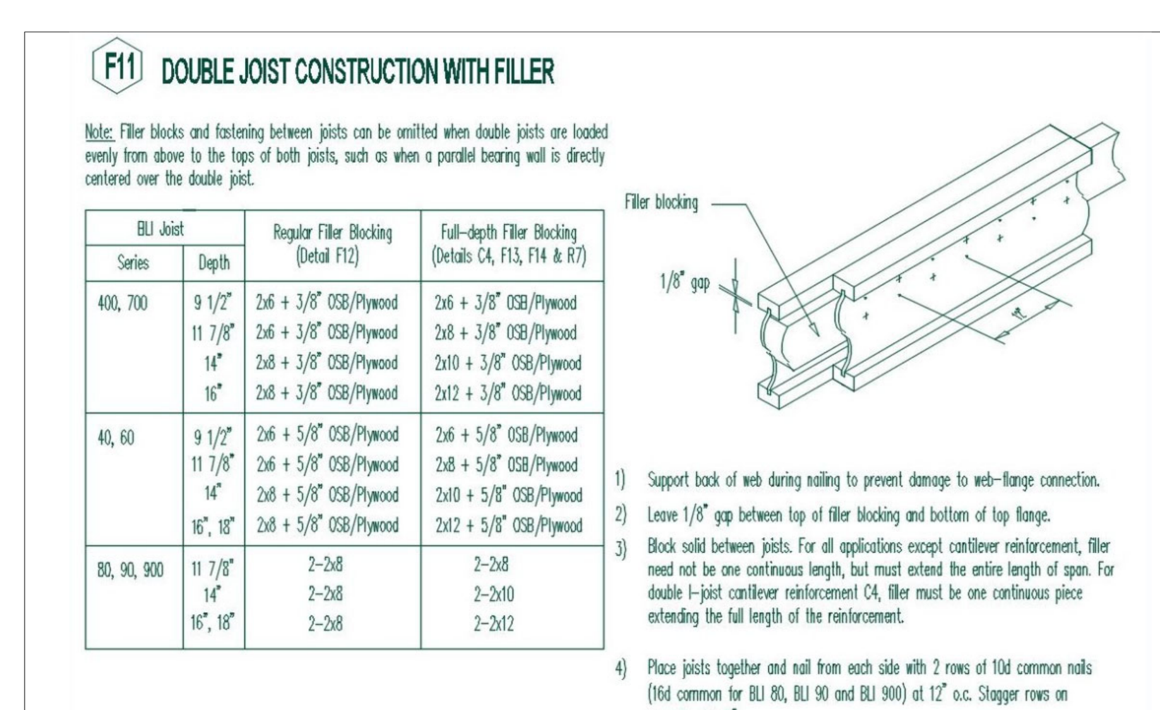
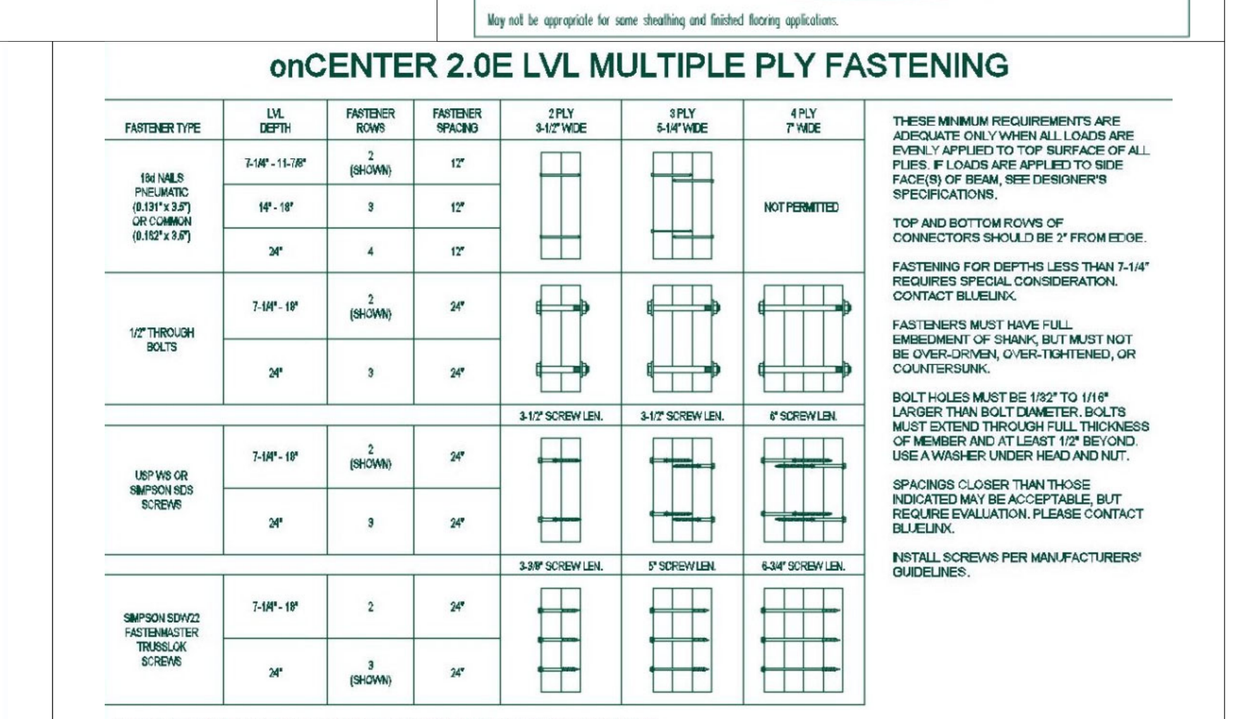
BM1-3	30' 0"	onCENTER® LVL 2.0E 1 3/4" x 11 7/8"	3	3









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Detail F19 Squash Block/Columnn shall match size of column above

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3941 USHwy 421 North
Wilmington, NC 28401
(910) 386-4300

DRAWN BY: JJC

DATE: 03-12-2019

SCALE: 1/4" = 1'-0"

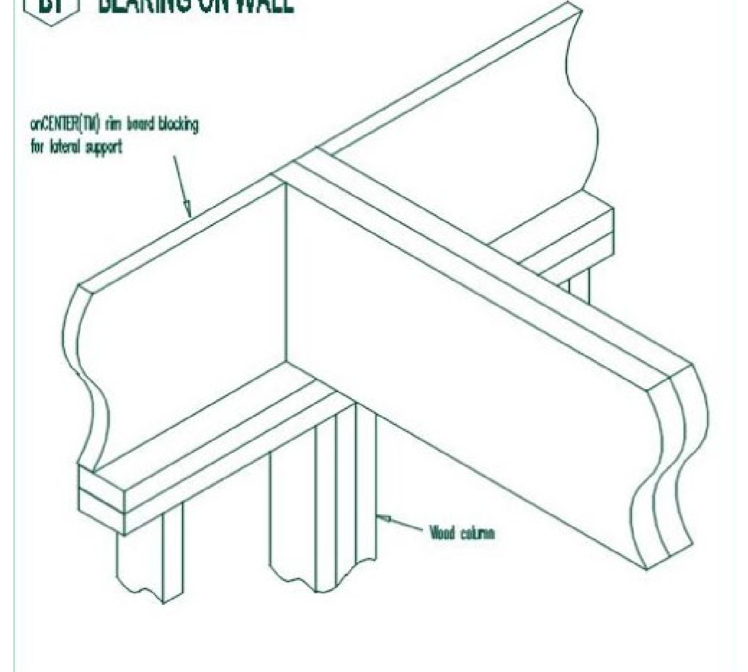
SALESPERSON: Matthew Kemper

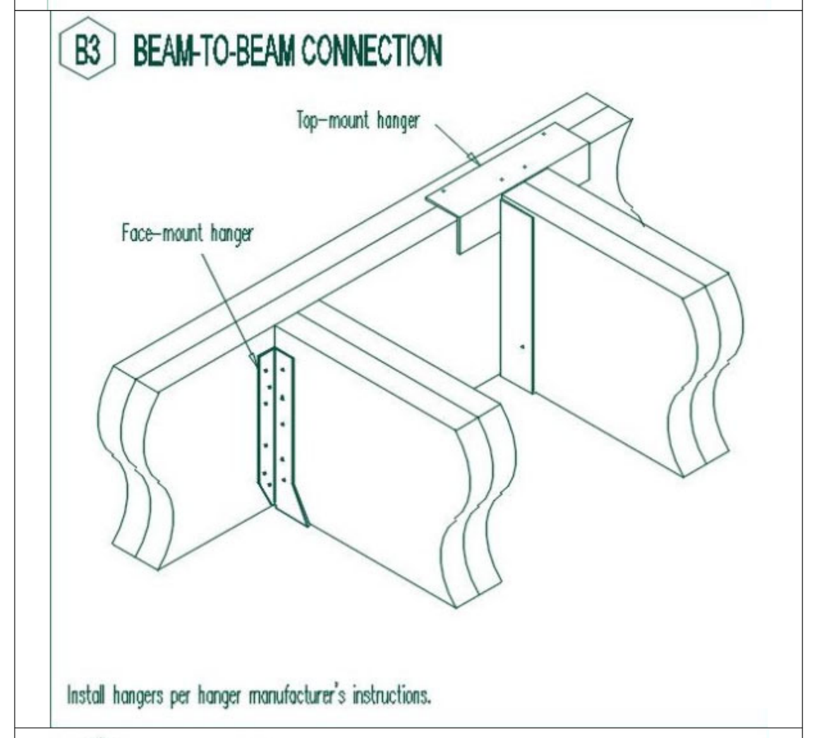
BUILDER NAME: Milton Built Homes, LLC

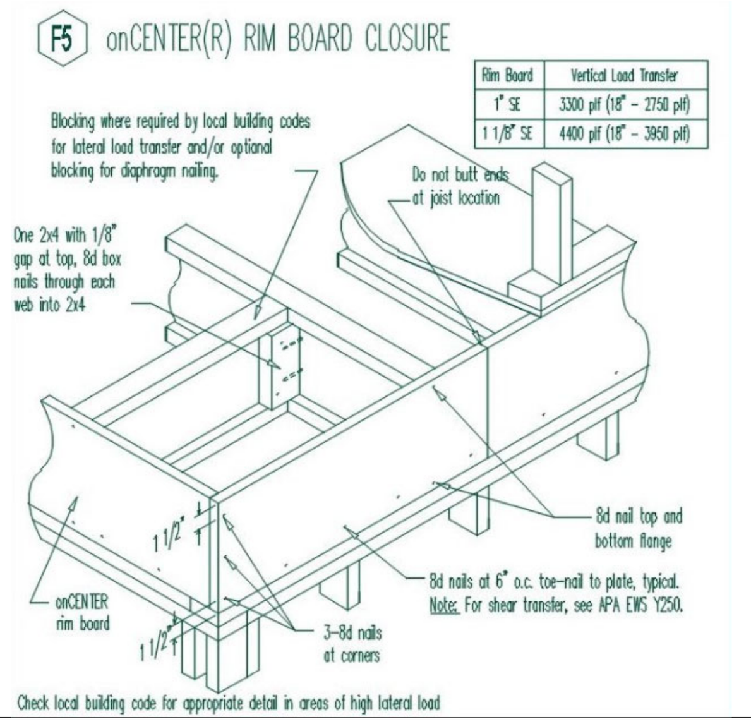
PROJECT NAME: Lot 7 Raven Ridge (Plan # 2662-15)

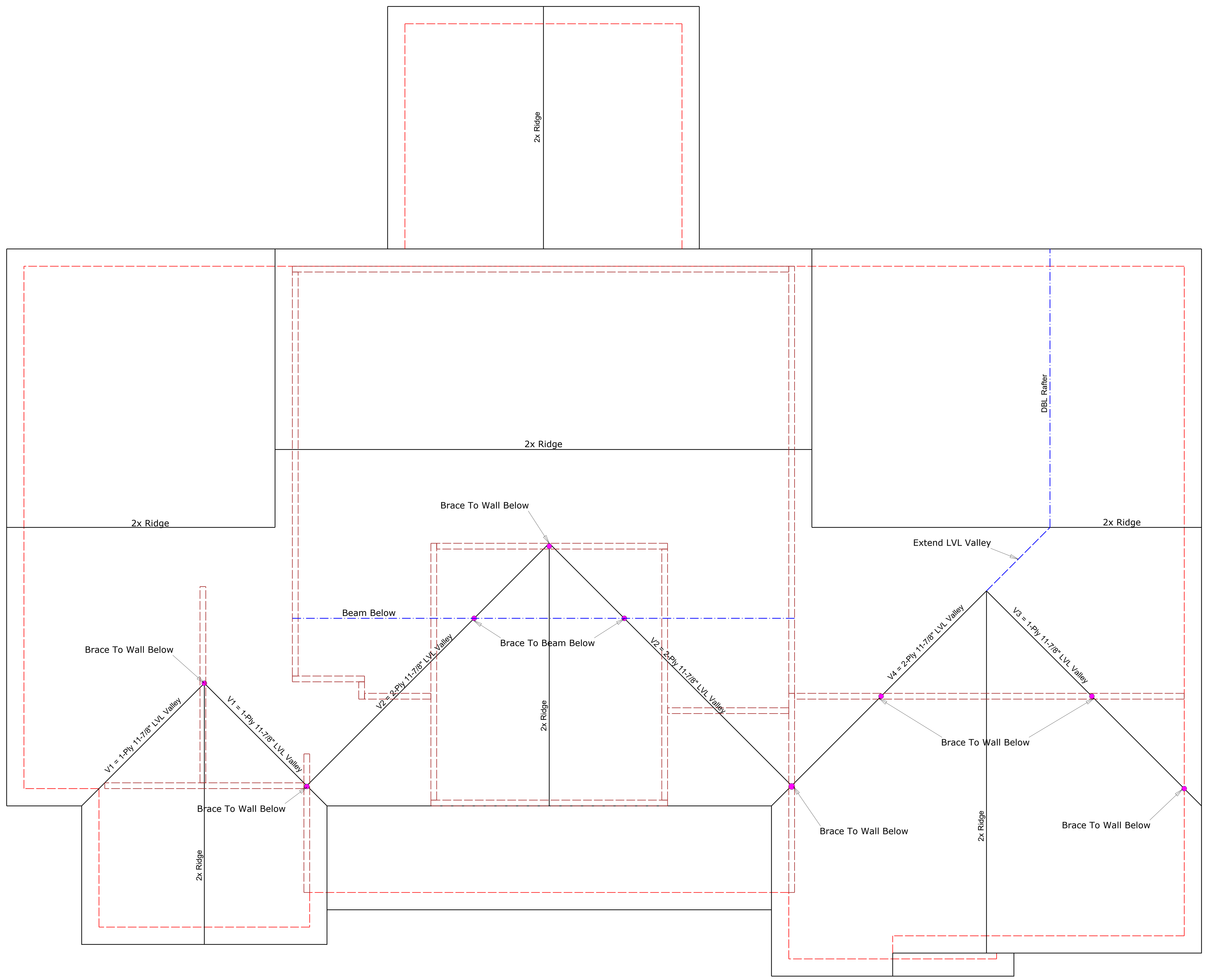
LEVEL NAME: 2nd Floor Framing Layout (Ceiling)

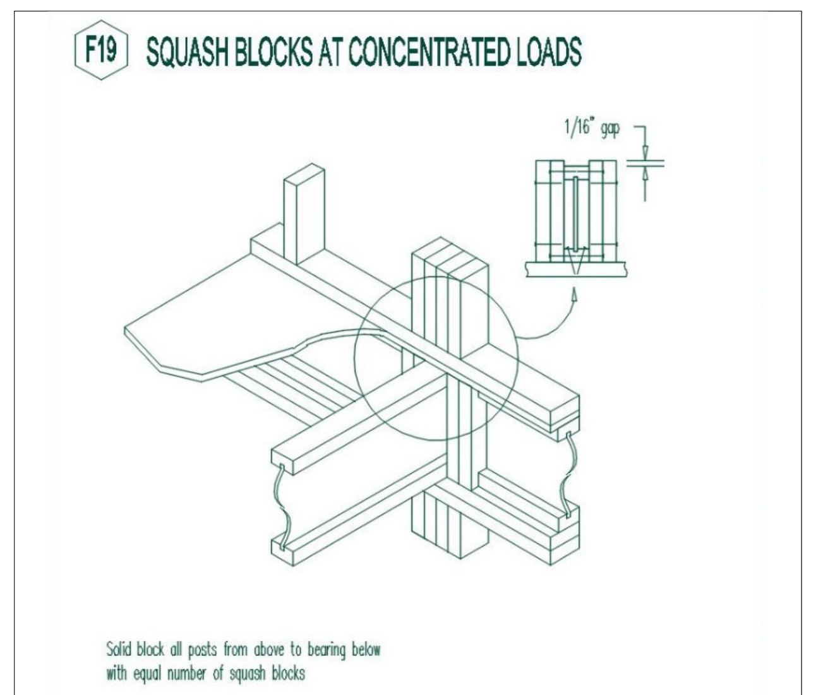
PAGE: 2

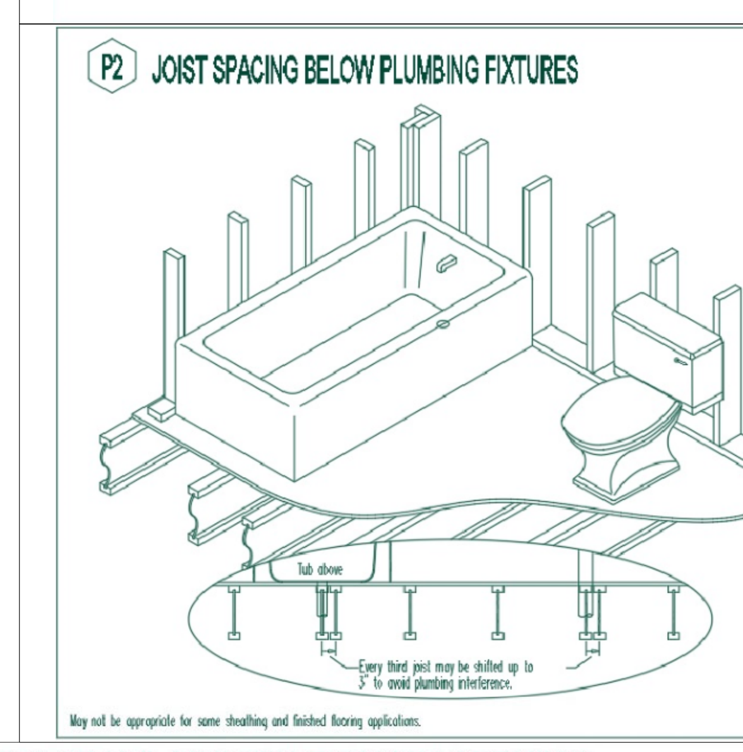












The attached materila list represents BlueLinx Engineered Lumber products needed to frame layout(s) shown based on the interpretation of user, but has not been reviewed by a BluLinx engineer. Purchaser is to verify material quantities, lengths, locations, and sizes and resolve clouded items.

Specified products are sized only for gravity loads shown. These loads should be verified by the purchaser. If additional loads or framing areas need to be accounted for, notify supplier of BlueLInx products so that material can be sized and price adjusted. Unless noted otherwise, hip, valley and ridge boards have not been designed and no products were designed to resist the building's lateral loads.

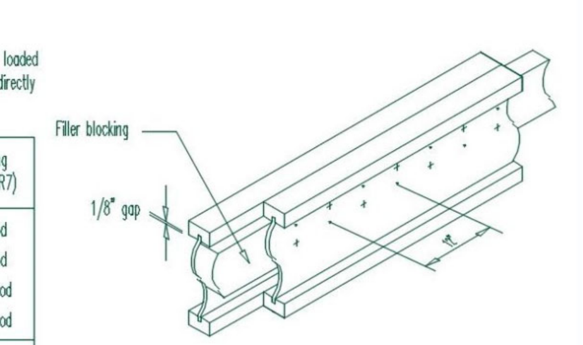
prior to installation, review layout with applicable product guide and/or installation sheet. If this information is not included, contact your supplier of BlueLinx products. Coordinate building plans and details with this layout. To prevent member damage from plumbing or mechanical cuts. Review this layout plan before placement. Follow I-joist web hole charts. Do not cut material beyond scope of product guide(s) withour prior approval.

Glue and nail minimum 23/32" APA rated OSB or plywood to floor I-joists. Use fastener schedule for side-loaded onCENTER(TM) LVL beams. For proper installation of hangers and connectors, follow manufacturer's guidelines. Connections not shown are by others.
Roofs shall have adequate drainage to prevent ponding.
These products were designed for "dry use" conditions only, and must be protected from long term exposure to high moistrue. Moisture protection (by others) may be required.
Detail F19 Squash Block/Columnn shall match size of column above

F11 DOUBLE JOIST CONSTRUCTION WITH FILLER

Note: Filler blocks and fastening between joists can be omitted when double joists are loaded evenly from above to the tops of both joists, such as when a parallel bearing wall is directly centered over the double joist.

BLI Joist		Regular Filler Blocking (Detail F12)	Full-depth Filler Blocking (Details C4, F13, F14 & R7)
Series	Depth		
400, 700	9 1/2"	2x6 + 3/8" OSB/Plywood	2x6 + 3/8" OSB/Plywood
	11 7/8"	2x6 + 3/8" OSB/Plywood	2x8 + 3/8" OSB/Plywood
	14"	2x8 + 3/8" OSB/Plywood	2x10 + 3/8" OSB/Plywood
	16"	2x8 + 3/8" OSB/Plywood	2x12 + 3/8" OSB/Plywood
40, 60	9 1/2"	2x6 + 5/8" OSB/Plywood	2x6 + 5/8" OSB/Plywood
	11 7/8"	2x6 + 5/8" OSB/Plywood	2x8 + 5/8" OSB/Plywood
	14"	2x8 + 5/8" OSB/Plywood	2x10 + 5/8" OSB/Plywood
	16", 18"	2x8 + 5/8" OSB/Plywood	2x12 + 5/8" OSB/Plywood
80, 90, 900	11 7/8"	2-2x8	2-2x8
	14"	2-2x8	2-2x10
	16", 18"	2-2x8	2-2x12



1) Support back of web during nailing to prevent damage to web–flange connection.
2) Leave 1/8" gap between top of filler blocking and bottom of top flange.
3) Block solid between joists. For all applications except cantilever reinforcement, filler need not be one continuous length, but must extend the entire length of span. For double I–joist cantilever reinforcement C4, filler must be one continuous piece extending the full length of the reinforcement.
4) Place joists together and nail from each side with 2 rows of 10d common nails (16d common for BLI 80, BLI 90 and BLI 900) at 12" o.c. Stagger rows on opposite side 6".

onCENTER 2.0E LVL MULTIPLE PLY FASTENING

FASTENER TYPE	LVL DEPTH	FASTENER ROWS	FASTENER SPACING	2 PLY 3-1/2" WIDE	3 PLY 5-1/4" WIDE	4 PLY 7" WIDE
16d NAILS PNEUMATIC (0.131" x 3.5") OR COMMON (0.162" x 3.5")	7-1/4" - 11-7/8"	2 (SHOWN)	12"			NOT PERMITTED
	14" - 18"	3	12"			
	24"	4	12"			
1/2" THROUGH BOLTS	7-1/4" - 18"	2 (SHOWN)	24"			
	24"	3	24"			
				3-1/2" SCREW LEN.	3-1/2" SCREW LEN.	6" SCREW LEN.
USP WS OR SIMPSON SDS SCREWS	7-1/4" - 18"	2 (SHOWN)	24"			
	24"	3	24"			
				3-3/8" SCREW LEN.	5" SCREW LEN.	6-3/4" SCREW LEN.
SIMPSON SDW22 FASTENMASTER TRUSSLOK SCREWS	7-1/4" - 18"	2	24"			
	24"	3 (SHOWN)	24"			

WHERE FASTENERS ARE SHOWN FROM BOTH SIDES, FASTENER SCHEDULES MUST BE REPEATED ON EACH FACE, WITH FASTENERS ON BACK FACE OFFSET ONE-HALF THE INDICATED SPACING FROM FRONT FACE.

THESE MINIMUM REQUIREMENTS ARE ADEQUATE ONLY WHEN ALL LOADS ARE EVENLY APPLIED TO TOP SURFACE OF ALL PLIES. IF LOADS ARE APPLIED TO SIDE FACE(S) OF BEAM, SEE DESIGNER'S SPECIFICATIONS.

TOP AND BOTTOM ROWS OF CONNECTORS SHOULD BE 2" FROM EDGE.

FASTENING FOR DEPTHS LESS THAN 7-1/4" REQUIRES SPECIAL CONSIDERATION. CONTACT BLUELINX.

FASTENERS MUST HAVE FULL EMBEDMENT OF SHANK, BUT MUST NOT BE OVER-DRIVEN, OVER-TIGHTENED, OR COUNTERSUNK.

BOLT HOLES MUST BE 1/32" TO 1/16" LARGER THAN BOLT DIAMETER. BOLTS MUST EXTEND THROUGH FULL THICKNESS OF MEMBER AND AT LEAST 1/2" BEYOND. USE A WASHER UNDER HEAD AND NUT.

SPACINGS CLOSER THAN THOSE INDICATED MAY BE ACCEPTABLE, BUT REQUIRE EVALUATION. PLEASE CONTACT BLUELINX.

INSTALL SCREWS PER MANUFACTURERS' GUIDELINES.

Professional BUILDERS SUPPLY
...it's about the service!!!

3941 USHwy 421 North
Wilmington, NC 28401
(910) 386-4300

DRAWN BY: JJC

DATE: 03-12-2019

SCALE: 1/4" = 1'-0"

SALESPERSON: Matthew Kemper

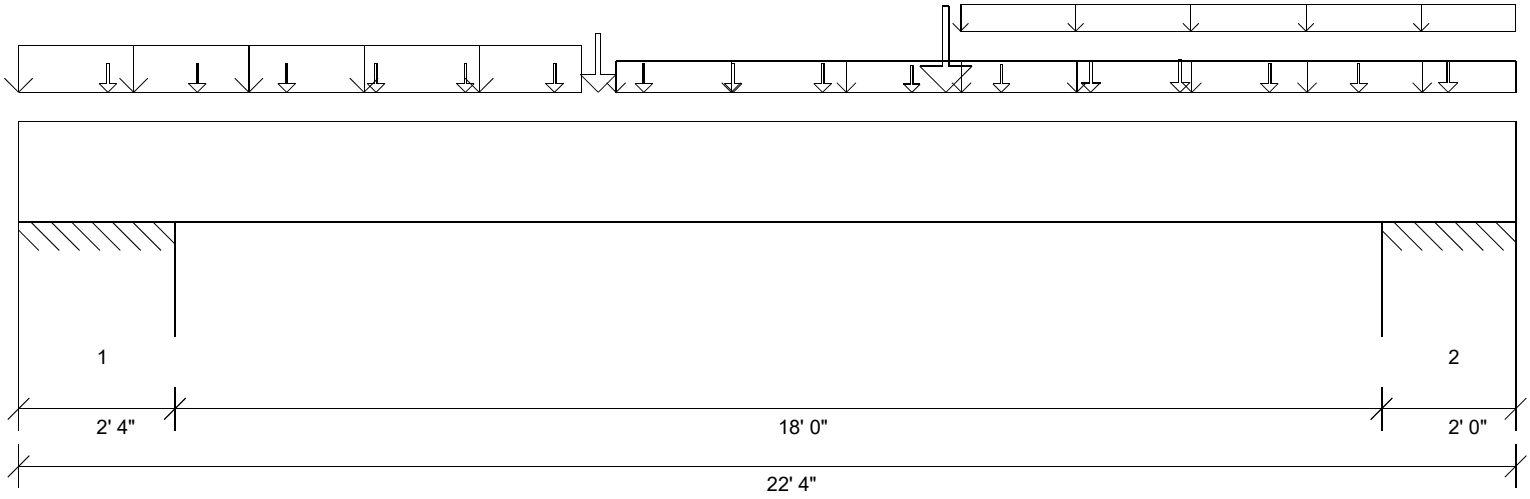
BUILDER NAME: Milton Built Homes, LLC

PROJECT NAME: Lot 7 Raven Ridge (Plan # 2662-15)

LEVEL NAME: Roof Framing Layout

Member: 3 - onCENTER LVL 2.0E 1 3/4" x 18"

Status: Design Passed



Graphical Illustration - Not To Scale
Member Cut Length - 22'- 4"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length Top:	0'	Bottom:	18'		

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	12'- 6 3/4"	48999.22 lb ft	84416.79 lb ft		Passed - 58%	1.25	D + 0.75(L + Lr)
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	18'- 10"	7244.56 lb	17955.00 lb		Passed - 40%	1.00	D + L
Live Load Deflection	11'- 4 5/16"	0'- 5/16"	N/A (L/360)		Passed - L/669	-	0.75(L + Lr)
Total Load Deflection	11'- 5 1/16"	0'- 9/16"	N/A (L/240)		Passed - L/385	-	D + 0.75(L + Lr)
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 1 1/2"	50.90 lb	38281.25 lb	32156.25 lb	Passed - 0%	1.25	D + 0.75(L + Lr)
	0'- 1 1/2"	-273.58 lb	27562.50 lb	-	Passed - 1%	0.90	D
	2'- 2 1/2"	8689.58 lb	27562.50 lb	32156.25 lb	Passed - 32%	1.00	D + L
	20'- 5 1/2"	9331.37 lb	27562.50 lb	32156.25 lb	Passed - 34%	1.00	D + L
	22'- 2 1/2"	-345.71 lb	27562.50 lb	-	Passed - 1%	0.90	D

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	22'- 4"	Self Weight	25 lb/ft	-	-	-
Uniform	-0'	8'- 4 3/4"	User Load	150 lb/ft	-	200 lb/ft	-
Uniform	8'- 11"	22'- 4"	User Load	60 lb/ft	-	80 lb/ft	-
Uniform	14'- 5/8"	22'- 4"	Rim1(i4060)	65 lb/ft	-	-	-
Point	1'- 4"	1'- 4"	J1(i4048)	153.00 lb	505.00 lb	-	-
Point	2'- 8"	2'- 8"	J1(i4073)	153.00 lb	505.00 lb	-	-
Point	4'	4'	J1(i4075)	153.00 lb	505.00 lb	-	-
Point	5'- 4"	5'- 4"	J1(i4074)	153.00 lb	505.00 lb	-	-
Point	6'- 8"	6'- 8"	J1(i4072)	153.00 lb	504.00 lb	-	-
Point	7'- 11 15/16"	7'- 11 15/16"	J1(i4062)	152.00 lb	504.00 lb	-	-
Point	8'- 7 3/4"	8'- 7 3/4"	User Load	1091.00 lb	-	2181.00 lb	-
Point	9'- 3 15/16"	9'- 3 15/16"	J1(i4079)	153.00 lb	504.00 lb	-	-
Point	10'- 7 15/16"	10'- 7 15/16"	J1(i4084)	153.00 lb	505.00 lb	-	-
Point	11'- 11 15/16"	11'- 11 15/16"	J1(i4083)	153.00 lb	505.00 lb	-	-
Point	13'- 3 15/16"	13'- 3 15/16"	J1(i4082)	105.00 lb	348.00 lb	-	-
Point	13'- 10"	13'- 10"	BM4-3(i4056)	2456.00 lb	252.00 lb	2925.00 lb	-
Point	14'- 7 15/16"	14'- 7 15/16"	J1(i4080)	124.00 lb	409.00 lb	-	-
Point	15'- 11 15/16"	15'- 11 15/16"	J1(i4087)	274.00 lb	589.00 lb	-	-
Point	17'- 3 15/16"	17'- 3 15/16"	J1(i4085)	361.00 lb	649.00 lb	-	-
Point	18'- 7 15/16"	18'- 7 15/16"	J1(i4081)	153.00 lb	505.00 lb	-	-
Point	19'- 11 15/16"	19'- 11 15/16"	J1(i4086)	163.00 lb	505.00 lb	-	-
Point	21'- 3 15/16"	21'- 3 15/16"	J1(i4088)	218.00 lb	633.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	2'- 4"	-	4523.00 lb	4309.00 lb	4484.00 lb	-
++>	0'- 1 1/2"	0'- 1 1/2"	E3(i3)	-	204.00 lb	228.00 lb	-
++>	2'- 2 1/2"	2'- 2 1/2"	E8(i7)	4523.00 lb	4105.00 lb	4256.00 lb	-

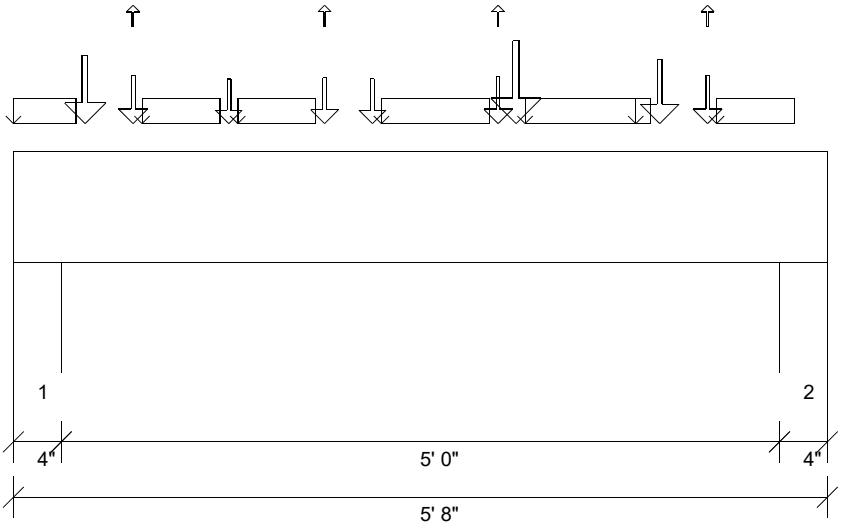
2	20'- 4"	22'- 4"	E9(i8)	4950.00 lb	4689.00 lb	3719.00 lb	-
==>	20'- 5 1/2"	20'- 5 1/2"	E9(i8)	4950.00 lb	4381.00 lb	3637.00 lb	-
==>	22'- 2 1/2"	22'- 2 1/2"	E9(i8)	-	308.00 lb	82.00 lb	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

Member: 2 - onCENTER LVL 2.0E 1 3/4" x 9 1/4"

Status: Design Passed



Graphical Illustration - Not To Scale
Member Cut Length - 5'- 8"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 0'- 3 1/16"	Bottom: 5'			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	3'- 4 1/2"	3685.51 lb ft	13320.27 lb ft		Passed - 28%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	4'- 6 3/4"	2167.52 lb	6151.25 lb		Passed - 35%	1.00	D + L
Live Load Deflection	2'- 10 3/8"	0'	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	2'- 10 1/2"	0'- 1/16"	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	2960.38 lb	11484.44 lb	12250.07 lb	Passed - 26%	1.00	D + L
	5'- 5"	2694.06 lb	11484.41 lb	12250.04 lb	Passed - 23%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	5'- 8"	Self Weight	8 lb/ft	-	-	-
Uniform	0'	0'- 5 1/4"	Bk1(i4205)	65 lb/ft	-	-	-
Uniform	0'- 10 3/4"	1'- 5 1/4"	Bk1(i4267)	65 lb/ft	-	-	-
Uniform	1'- 6 3/4"	2'- 1 1/4"	Bk1(i4263)	65 lb/ft	-	-	-
Uniform	2'- 6 3/4"	3'- 3 3/4"	Bk1(i4203)	65 lb/ft	-	-	-
Uniform	3'- 6 3/4"	4'- 5 1/4"	Bk1(i4135)	65 lb/ft	-	-	-
Uniform	4'- 10 3/4"	5'- 5 1/4"	Bk1(i4231)	65 lb/ft	-	-	-
Point	0'- 6"	0'- 6"	J3(i4132)	315.00 lb	513.00 lb	-	-
Point	0'- 10"	0'- 10"	J4(i4094)	98.00 lb	384.00/-14.00 lb	-	-
Point	1'- 6"	1'- 6"	J3(i4183)	89.00 lb	322.00 lb	-	-
Point	2'- 2"	2'- 2"	J4(i4111)	90.00 lb	350.00/-13.00 lb	-	-
Point	2'- 6"	2'- 6"	J3(i4134)	96.00 lb	322.00 lb	-	-
Point	3'- 4 1/2"	3'- 4 1/2"	J4(i4096)	87.00 lb	367.00/-13.00 lb	-	-
Point	3'- 6"	3'- 6"	J3(i4235)	365.00 lb	726.00 lb	-	-
Point	4'- 6"	4'- 6"	J3(i4176)	311.00 lb	450.00 lb	-	-
Point	4'- 10"	4'- 10"	J4(i4099)	98.00 lb	384.00/-14.00 lb	-	-

Support Information:

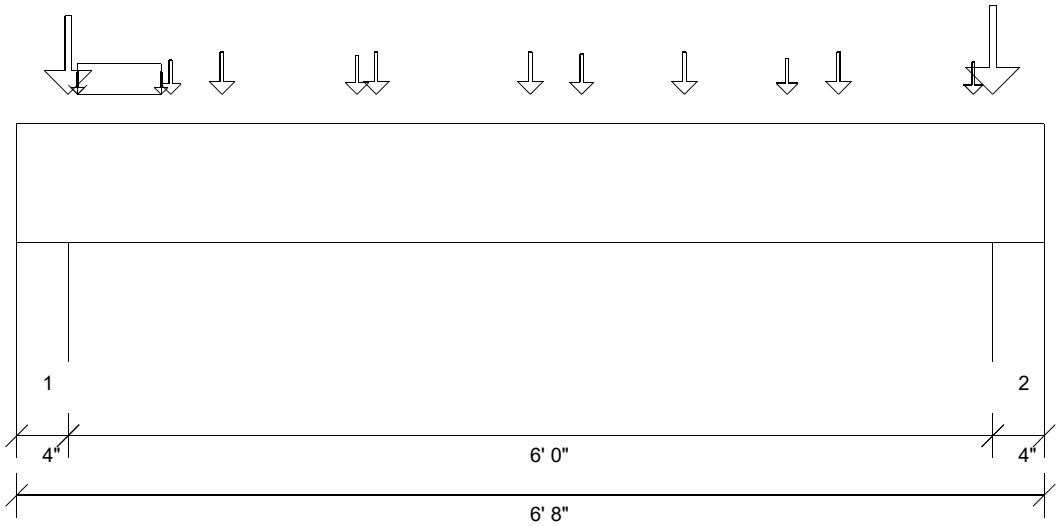
				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E46(i103)	950.00 lb	1998.00/-27.00 lb	-	-
2	5'- 4"	5'- 8"	E44(i101)	887.00 lb	1820.00/-27.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

Member: 2 - onCENTER LVL 2.0E 1 3/4" x 9 1/4"

Status: Design Passed



Graphical Illustration - Not To Scale
Member Cut Length - 6'- 8"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 0'- 3 1/16"	Bottom: 6'			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	3'- 4"	3640.19 lb ft	13320.27 lb ft		Passed - 27%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1 1/4"	1907.79 lb	6151.25 lb		Passed - 31%	1.00	D + L
Live Load Deflection	3'- 4"	0'- 1/16"	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	3'- 4"	0'- 1/16"	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	3301.93 lb	11484.41 lb	12250.04 lb	Passed - 29%	1.00	D + L
	6'- 5"	3554.12 lb	11484.44 lb	12250.08 lb	Passed - 31%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	6'- 8"	Self Weight	8 lb/ft	-	-	-
Uniform	0'- 4 3/4"	0'- 11 1/4"	Bk1(i4141)	22 lb/ft	88 lb/ft	-	-
Point	0'- 4"	0'- 4"	J3(i4149)	445.00 lb	835.00 lb	-	-
Point	0'- 4 3/4"	0'- 4 3/4"	Bk1(i4141)	1.00 lb	6.00 lb	-	-
Point	0'- 11 1/4"	0'- 11 1/4"	Bk1(i4141)	1.00 lb	6.00 lb	-	-
Point	1'	1'	J5(i4116)	67.00 lb	207.00 lb	-	-
Point	1'- 4"	1'- 4"	J3(i4115)	141.00 lb	322.00 lb	-	-
Point	2'- 2 1/2"	2'- 2 1/2"	J5(i4102)	135.00 lb	246.00 lb	-	-
Point	2'- 4"	2'- 4"	J3(i4171)	141.00 lb	322.00 lb	-	-
Point	3'- 4"	3'- 4"	J3(i4179)	141.00 lb	322.00 lb	-	-
Point	3'- 8"	3'- 8"	J5(i4175)	154.00 lb	258.00 lb	-	-
Point	4'- 4"	4'- 4"	J3(i4129)	141.00 lb	322.00 lb	-	-
Point	5'	5'	J5(i4103)	78.00 lb	235.00 lb	-	-
Point	5'- 4"	5'- 4"	J3(i4126)	141.00 lb	322.00 lb	-	-
Point	6'- 2 1/2"	6'- 2 1/2"	J5(i4212)	55.00 lb	175.00 lb	-	-
Point	6'- 4"	6'- 4"	J3(i4248)	591.00 lb	930.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E48(i107)	1074.00 lb	2210.00/-1.00 lb	-	-
2	6'- 4"	6'- 8"	E46(i103)	1227.00 lb	2346.00/-1.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
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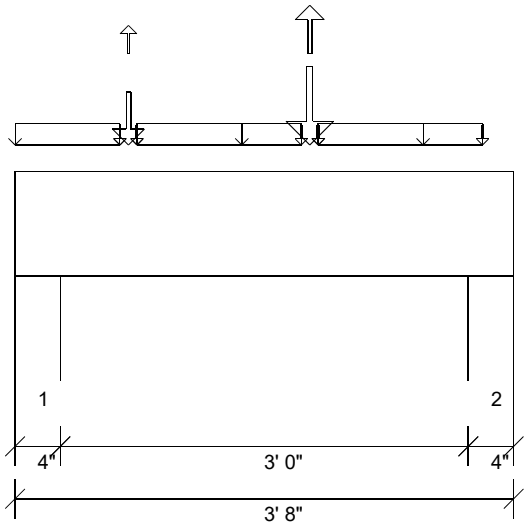


Graphical Illustration - Not To Scale
Member Cut Length - 3'- 8"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 0'- 4 5/8"	Bottom: 3'			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	2'- 2"	977.91 lb ft	3429.65 lb ft		Passed - 29%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	2'- 6 3/4"	773.90 lb	2497.50 lb		Passed - 31%	1.00	D + L
Live Load Deflection	1'- 10 3/8"	0'	N/A (L/360)		Passed - L/999	-	0.75(L + Lr)
Total Load Deflection	1'- 10 3/8"	0'	N/A (L/240)		Passed - L/999	-	D + 0.75(L + Lr)
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	1165.03 lb	5578.16 lb	10500.08 lb	Passed - 21%	1.25	D + 0.75(L + Lr)
	0'- 3"	-2.78 lb	5578.16 lb	-	Passed - 0%	1.25	D + Lr
	3'- 5"	969.75 lb	5578.16 lb	10500.08 lb	Passed - 17%	1.25	D + 0.75(L + Lr)
	3'- 5"	-93.76 lb	5578.16 lb	-	Passed - 2%	1.25	D + Lr

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

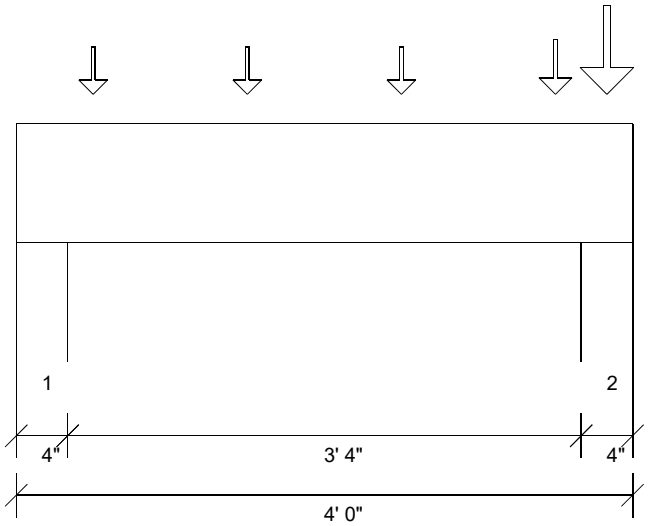
				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	3'- 8"	Self Weight	6 lb/ft	-	-	-
Uniform	0'	0'- 9 1/4"	Bk1(i4131)	7 lb/ft	27 lb/ft	-	-
Uniform	0'- 10 3/4"	2'- 1 1/4"	Bk1(i4119)	7 lb/ft	27 lb/ft	-	-
Uniform	2'- 2 3/4"	3'- 5 1/4"	Bk1(i4254)	7 lb/ft	27 lb/ft	-	-
Point	0'- 9 1/4"	0'- 9 1/4"	Bk1(i4131)	-	2.00 lb	-	-
Point	0'- 10"	0'- 10"	J3(i4169)	241.00 lb	486.00/-52.00 lb	168.00/-170.00 lb	-
Point	0'- 10 3/4"	0'- 10 3/4"	Bk1(i4119)	-	2.00 lb	-	-
Point	2'- 1 1/4"	2'- 1 1/4"	Bk1(i4119)	-	2.00 lb	-	-
Point	2'- 2"	2'- 2"	J3(i4136)	373.00 lb	636.00/-181.00 lb	582.00/-586.00 lb	-
Point	2'- 2 3/4"	2'- 2 3/4"	Bk1(i4254)	-	2.00 lb	-	-
Point	3'- 5 1/4"	3'- 5 1/4"	Bk1(i4254)	-	2.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E13(i57)	355.00 lb	674.00/-109.00 lb	352.00/-355.00 lb	-
2	3'- 4"	3'- 8"	E39(i96)	305.00 lb	543.00/-124.00 lb	398.00/-401.00 lb	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 4'
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length Top:	0'	Bottom:	0'- 10 1/2"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	1'- 6"	673.49 lb ft	3429.65 lb ft	Passed - 20%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	1'- 1 1/4"	545.11 lb	2497.50 lb	Passed - 22%	1.00	D + L
Live Load Deflection	1'- 11 3/4"	0'	N/A (L/360)	Passed - L/999	-	L
Total Load Deflection	1'- 11 13/16"	0'	N/A (L/240)	Passed - L/999	-	D + L
Max. Reaction			Supported Mtl / Supporting Mtl			
	0'- 3"	912.74 lb	5578.12 lb / 10500.00 lb	Passed - 16%	1.00	D + L
	3'- 9"	2290.36 lb	5578.12 lb / 10500.00 lb	Passed - 41%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

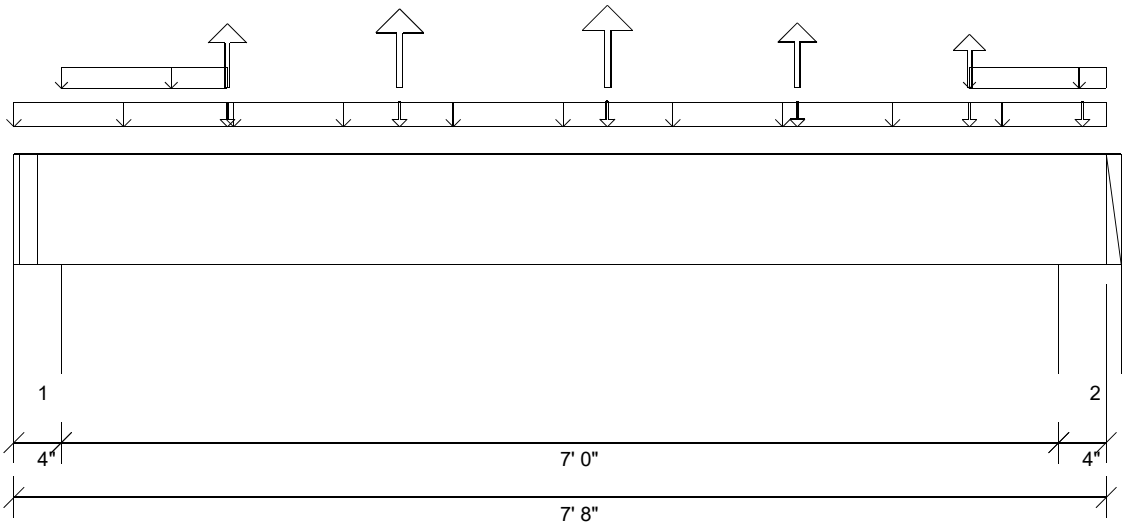
				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	4'	Self Weight	6 lb/ft	-	-	-
Point	0'- 6"	0'- 6"	J4(i4184)	133.00 lb	327.00 lb	-	-
Point	1'- 6"	1'- 6"	J4(i4090)	133.00 lb	327.00 lb	-	-
Point	2'- 6"	2'- 6"	J4(i4193)	133.00 lb	327.00 lb	-	-
Point	3'- 6"	3'- 6"	J4(i4236)	265.00 lb	327.00 lb	-	-
Point	3'- 10"	3'- 10"	E97(i547)	446.00 lb	761.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E44(i101)	278.00 lb	654.00 lb	-	-
2	3'- 8"	4'	E4(i1)	856.00 lb	1415.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 7'- 8"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length Top:	0'	Bottom:	1'- 4"		

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	4'- 2"	2889.51 lb ft	3429.65 lb ft	Passed - 84%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	1'- 6"	1346.39 lb	2497.50 lb	Passed - 54%	1.00	D + L
Live Load Deflection	3'- 10 1/16"	0'- 1/16"	N/A (L/360)	Passed - L/999	-	L
Total Load Deflection	3'- 9 15/16"	0'- 1/8"	N/A (L/240)	Passed - L/875	-	D + L
Max. Reaction			Supported Mtl / Supporting Mtl			
	0'- 3"	-1245.99 lb	5578.08 lb / -	Passed - 22%	1.00	D + L
	7'- 5"	-1261.06 lb	5578.13 lb / -	Passed - 23%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

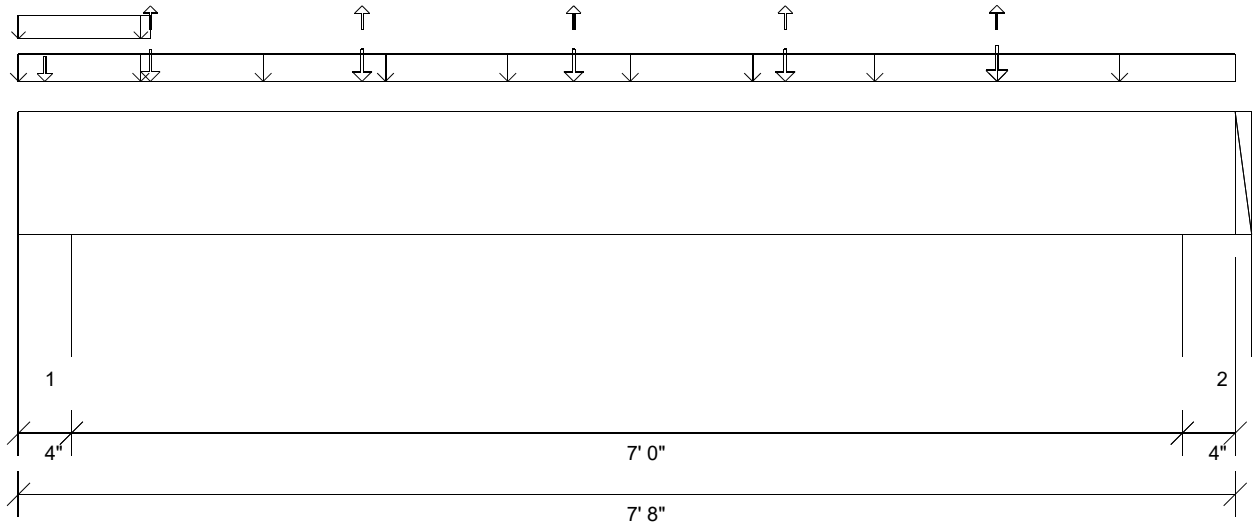
				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	7'- 8"	Self Weight	6 lb/ft	-	-	-
Uniform	0'	7'- 8"	User Load	60 lb/ft	-	-	-
Uniform	0'- 4"	1'- 6"	FC1 Floor Material	1 lb/ft	5 lb/ft	-	-
Uniform	6'- 8 1/2"	7'- 8"	FC1 Floor Material	1 lb/ft	5 lb/ft	-	-
Point	1'- 6"	1'- 6"	J5(i4116)	-165.00 lb	55.00/-390.00 lb	-	-
Point	2'- 8 1/2"	2'- 8 1/2"	J5(i4102)	-332.00 lb	61.00/-413.00 lb	-	-
Point	4'- 2"	4'- 2"	J5(i4175)	-358.00 lb	64.00/-433.00 lb	-	-
Point	5'- 6"	5'- 6"	J5(i4103)	-172.00 lb	58.00/-394.00 lb	-	-
Point	6'- 8 1/2"	6'- 8 1/2"	J5(i4212)	-125.00 lb	42.00/-294.00 lb	-	-
Point	7'- 6"	7'- 6"	E68(i136)	60.00 lb	-	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E12(i54)	-321.00 lb	140.00/-925.00 lb	-	-
2	7'- 4"	7'- 8"	E33(i87)	-262.00 lb	151.00/-999.00 lb	-	-

Errors, Warnings & Notes:

* CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 7'- 8"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length Top:	0'	Bottom:	1'- 2 1/2"		

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	3'- 7 1/2"	779.05 lb ft	3429.65 lb ft		Passed - 23%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	6'- 6 3/4"	363.04 lb	2497.50 lb		Passed - 15%	1.00	D + L
Live Load Deflection	3'- 10 1/4"	0'	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	3'- 10 1/16"	0'	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	470.80 lb	5577.56 lb	10498.83 lb	Passed - 8%	1.00	D + L
	7'- 5"	435.95 lb	5578.13 lb	10500.00 lb	Passed - 8%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

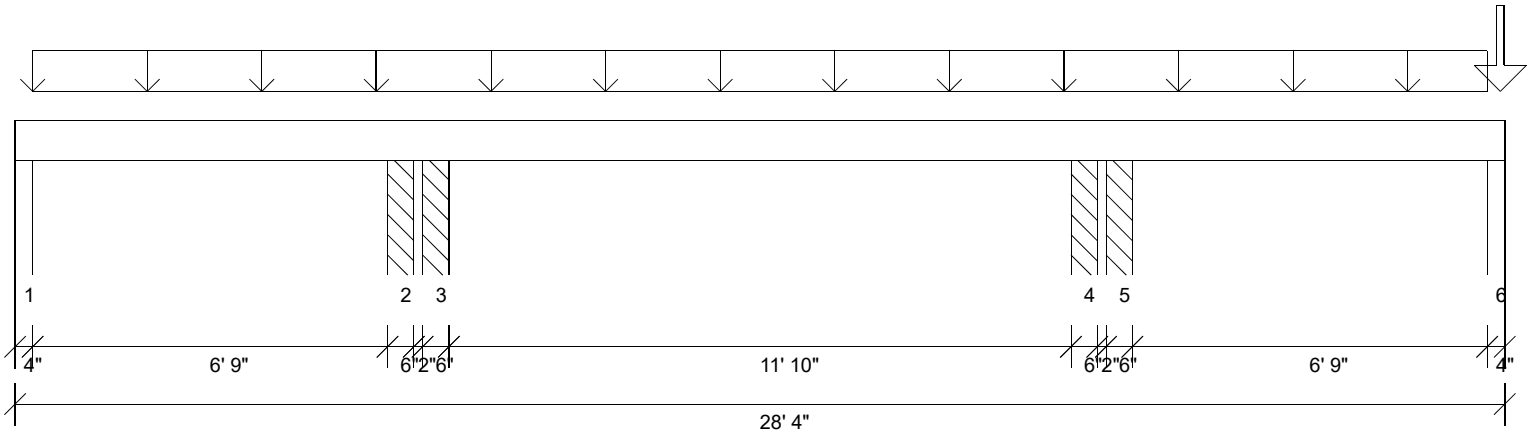
				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	7'- 8"	Self Weight	6 lb/ft	-	-	-
Uniform	0'	7'- 8"	User Load	60 lb/ft	-	-	-
Uniform	0'	0'- 10"	FC1 Floor Material	1 lb/ft	5 lb/ft	-	-
Point	0'- 2"	0'- 2"	E93(i157)	16.00 lb	-	-	-
Point	0'- 10"	0'- 10"	J7(i4147)	21.00 lb	47.00/-8.00 lb	-	-
Point	2'- 2"	2'- 2"	J7(i4166)	12.00 lb	59.00/-9.00 lb	-	-
Point	3'- 6"	3'- 6"	J7(i4170)	12.00 lb	59.00/-9.00 lb	-	-
Point	4'- 10"	4'- 10"	J7(i4221)	12.00 lb	59.00/-9.00 lb	-	-
Point	6'- 2"	6'- 2"	J7(i4241)	17.00 lb	81.00/-13.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E33(i87)	313.00 lb	158.00/-24.00 lb	-	-
2	7'- 4"	7'- 8"	E6(i753)	285.00 lb	151.00/-24.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 28'- 4"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 27'- 8"	Bottom: 27'- 8"			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	20'- 4"	3286.53 lb ft	10813.27 lb ft		Passed - 30%	1.25	D + Lr
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	19'- 11 3/4"	1653.99 lb	7689.06 lb		Passed - 22%	1.25	D + Lr
Live Load Deflection	20'- 9"	0'	N/A (L/360)		Passed - L/414	-	Lr
Total Load Deflection	20'- 9"	0'	N/A (L/240)		Passed - L/394	-	D + Lr
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	665.70 lb	11484.28 lb	12249.89 lb	Passed - 6%	1.25	D + Lr
	7'- 4"	-917.17 lb	16734.31 lb	-	Passed - 6%	0.90	D
	8'	2677.31 lb	15750.00 lb	15225.00 lb	Passed - 18%	0.90	D
	20'- 4"	2695.61 lb	15750.00 lb	15225.00 lb	Passed - 18%	0.90	D
	21'	-940.59 lb	15750.00 lb	-	Passed - 6%	0.90	D
	28'- 1"	1108.57 lb	11484.40 lb	12250.02 lb	Passed - 10%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	28'- 4"	Self Weight	8 lb/ft	-	-	-
Uniform	0'- 4"	28'	User Load	150 lb/ft	-	120 lb/ft	-
Point	28'- 3"	28'- 3"	J1(i4073)	152.00 lb	500.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E12(i54)	382.00 lb	-	390.00/-122.00 lb	-
2	7'- 1"	7'- 7"	PBO1(i532)	-	-	530.00 lb	-
3	7'- 9"	8'- 3"	PBO3(i534)	1813.00 lb	-	1373.00 lb	-
4	20'- 1"	20'- 7"	PBO4(i535)	1813.00 lb	-	1373.00 lb	-
5	20'- 9"	21'- 3"	PBO2(i533)	-	-	530.00 lb	-
6	28'	28'- 4"	E7(i752)	534.00 lb	500.00 lb	390.00/-122.00 lb	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
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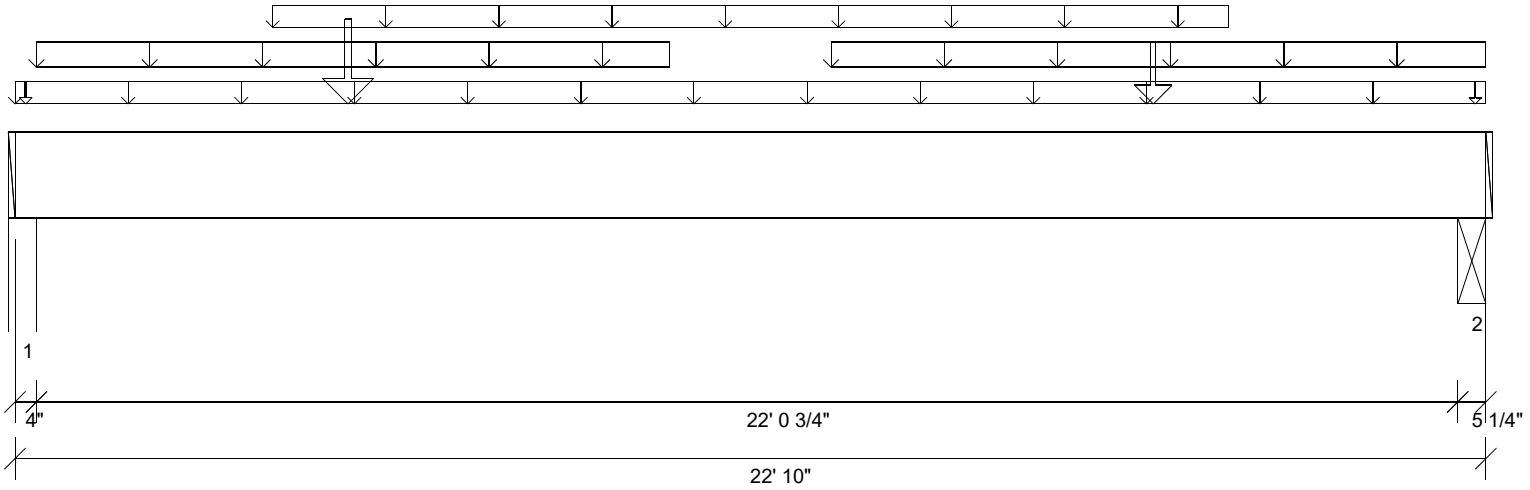
Member: 3 - onCENTER LVL 2.0E 1 3/4" x 16"

Status: Design Passed



Graphical Illustration - Not To Scale
Member Cut Length - 22'- 10"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length Top:	0'	Bottom:	22'- 3/4"		

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	5'- 2 1/16"	31174.67 lb ft	68289.58 lb ft		Passed - 46%	1.25	D + Lr
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 8"	6435.13 lb	19950.00 lb		Passed - 32%	1.25	D + Lr
Live Load Deflection	11'- 1/4"	0'- 7/16"	N/A (L/360)		Passed - L/595	-	Lr
Total Load Deflection	11'- 15/16"	0'- 13/16"	N/A (L/240)		Passed - L/335	-	D + Lr
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	6597.35 lb	17226.50 lb	18374.92 lb	Passed - 38%	1.25	D + Lr
	22'- 5 3/4"	5381.33 lb	22148.53 lb	20671.96 lb	Passed - 26%	1.25	D + Lr

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	22'- 10"	Self Weight	22 lb/ft	-	-	-
Uniform	-0'	22'- 10"	FC2 Floor Material	7 lb/ft	13 lb/ft	-	-
Uniform	0'- 4"	10'- 2"	E58(i122)	65 lb/ft	-	-	-
Uniform	4'	18'- 10"	FC2 Floor Material	-	13 lb/ft	-	-
Uniform	12'- 8"	22'- 10"	E59(i124)	65 lb/ft	-	-	-
Point	0'- 2"	0'- 2"	E91(i162)	28.00 lb	-	-	-
Point	5'- 2 1/16"	5'- 2 1/16"	E58(i122)	2043.00 lb	-	4086.00 lb	-
Point	17'- 8 1/16"	17'- 8 1/16"	E59(i124)	1289.00 lb	-	2578.00 lb	-
Point	22'- 8"	22'- 8"	E60(i125)	3.00 lb	-	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E5(i2)	2859.00 lb	250.00 lb	3739.00 lb	-
2	22'- 4 3/4"	22'- 10"	BM2-3(i4049)	2456.00 lb	252.00 lb	2925.00 lb	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
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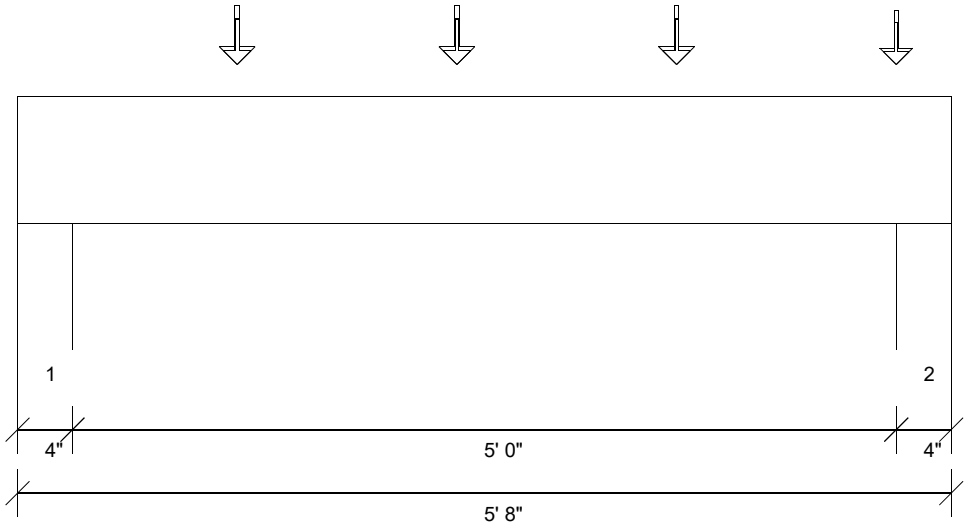


Graphical Illustration - Not To Scale
Member Cut Length - 5'- 8"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 0'- 3/8"	Bottom: 5'			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	2'- 8"	1040.97 lb ft	3429.65 lb ft		Passed - 30%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1 1/4"	653.11 lb	2497.50 lb		Passed - 26%	1.00	D + L
Live Load Deflection	2'- 9 7/8"	0'	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	2'- 9 7/8"	0'	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	659.76 lb	5578.28 lb	10500.32 lb	Passed - 12%	1.00	D + L
	5'- 5"	914.39 lb	5578.28 lb	10500.32 lb	Passed - 16%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	5'- 8"	Self Weight	6 lb/ft	-	-	-
Point	1'- 4"	1'- 4"	J2(i3771)	83.00 lb	166.00 lb	-	-
Point	1'- 4"	1'- 4"	J4(i3758)	50.00 lb	100.00 lb	-	-
Point	2'- 8"	2'- 8"	J2(i3742)	83.00 lb	166.00 lb	-	-
Point	2'- 8"	2'- 8"	J4(i3735)	50.00 lb	100.00 lb	-	-
Point	4'	4'	J2(i3732)	83.00 lb	166.00 lb	-	-
Point	4'	4'	J4(i3740)	50.00 lb	100.00 lb	-	-
Point	5'- 4"	5'- 4"	J2(i3779)	71.00 lb	143.00 lb	-	-
Point	5'- 4"	5'- 4"	J4(i3759)	43.00 lb	86.00 lb	-	-

Support Information:

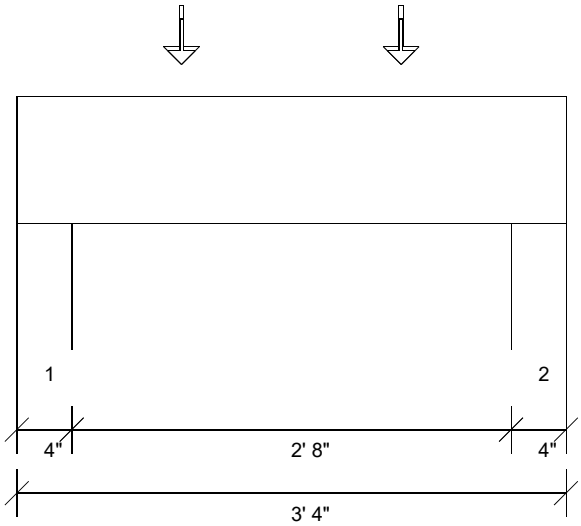
				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E77(i147)	229.00 lb	425.00 lb	-	-
2	5'- 4"	5'- 8"	E76(i146)	318.00 lb	602.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

Member: 2 - 2x10 SPF No.2

Status: Design Passed



Graphical Illustration - Not To Scale
Member Cut Length - 3'- 4"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 0'- 6"	Bottom: 2'- 8"			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	1'- 8 3/16"	305.13 lb ft	3429.65 lb ft		Passed - 9%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1 1/4"	348.42 lb	2497.50 lb		Passed - 14%	1.00	D + L
Live Load Deflection	1'- 8"	0'	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	1'- 8"	0'	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	409.13 lb	5578.13 lb	10500.02 lb	Passed - 7%	1.00	D + L
	3'- 1"	408.96 lb	5578.13 lb	10500.02 lb	Passed - 7%	1.00	D + L

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

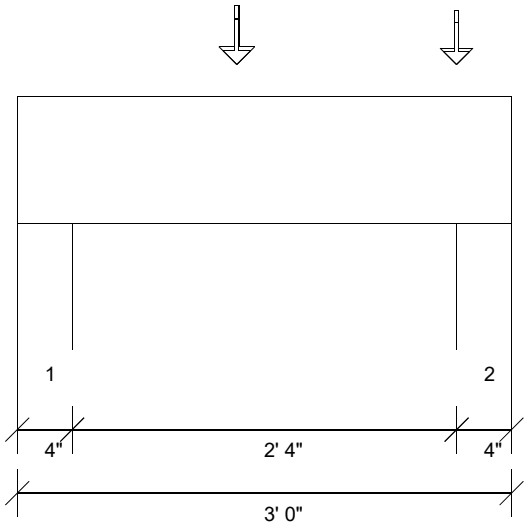
				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	3'- 4"	Self Weight	6 lb/ft	-	-	-
Point	1'	1'	J2(i3712)	83.00 lb	166.00 lb	-	-
Point	1'	1'	J4(i3717)	50.00 lb	100.00 lb	-	-
Point	2'- 4"	2'- 4"	J2(i3750)	83.00 lb	166.00 lb	-	-
Point	2'- 4"	2'- 4"	J4(i3769)	50.00 lb	100.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E76(i146)	143.00 lb	266.00 lb	-	-
2	3'	3'- 4"	E78(i148)	143.00 lb	266.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 3'
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 0'- 3/8"	Bottom: 2'- 4"			

Design Results:

	Location	Design	Control	Result	LDF	Load Combination
Critical Moment (Pos)	1'- 4"	261.81 lb ft	3429.65 lb ft	Passed - 8%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft			
Critical Shear	1'- 1 1/4"	240.04 lb	2497.50 lb	Passed - 10%	1.00	D + L
Live Load Deflection	1'- 5 9/16"	0'	N/A (L/360)	Passed - L/999	-	L
Total Load Deflection	1'- 5 9/16"	0'	N/A (L/240)	Passed - L/999	-	D + L
Max. Reaction			Supported Mtl / Supporting Mtl			
	0'- 3"	246.70 lb	5578.13 lb / 10500.02 lb	Passed - 4%	1.00	D + L
	2'- 9"	513.38 lb	5578.12 lb / 10499.99 lb	Passed - 9%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

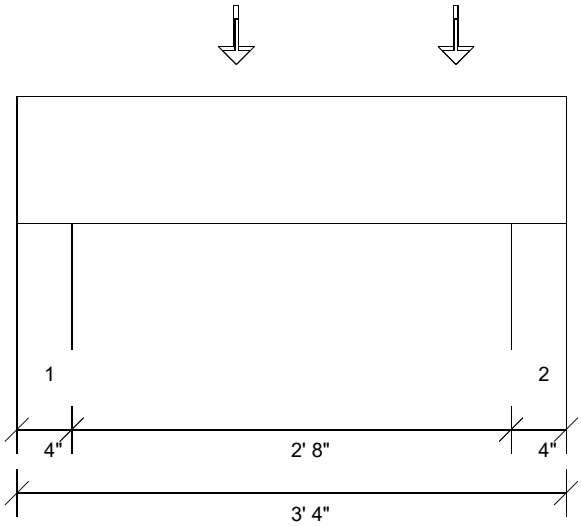
				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	3'	Self Weight	6 lb/ft	-	-	-
Point	1'- 4"	1'- 4"	J2(i3780)	83.00 lb	166.00 lb	-	-
Point	1'- 4"	1'- 4"	J4(i3763)	50.00 lb	100.00 lb	-	-
Point	2'- 8"	2'- 8"	J2(i3786)	71.00 lb	143.00 lb	-	-
Point	2'- 8"	2'- 8"	J4(i3760)	43.00 lb	86.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E78(i148)	84.00 lb	151.00 lb	-	-
2	2'- 8"	3'	E80(i150)	181.00 lb	344.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 3'- 4"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 0'- 3 5/8"	Bottom: 2'- 8"			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	1'- 4"	336.09 lb ft	3429.65 lb ft		Passed - 10%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 1 1/4"	308.67 lb	2497.50 lb		Passed - 12%	1.00	D + L
Live Load Deflection	1'- 7 7/8"	0'	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	1'- 7 7/8"	0'	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	315.33 lb	5578.12 lb	10499.99 lb	Passed - 6%	1.00	D + L
	3'- 1"	502.76 lb	5578.12 lb	10499.99 lb	Passed - 9%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

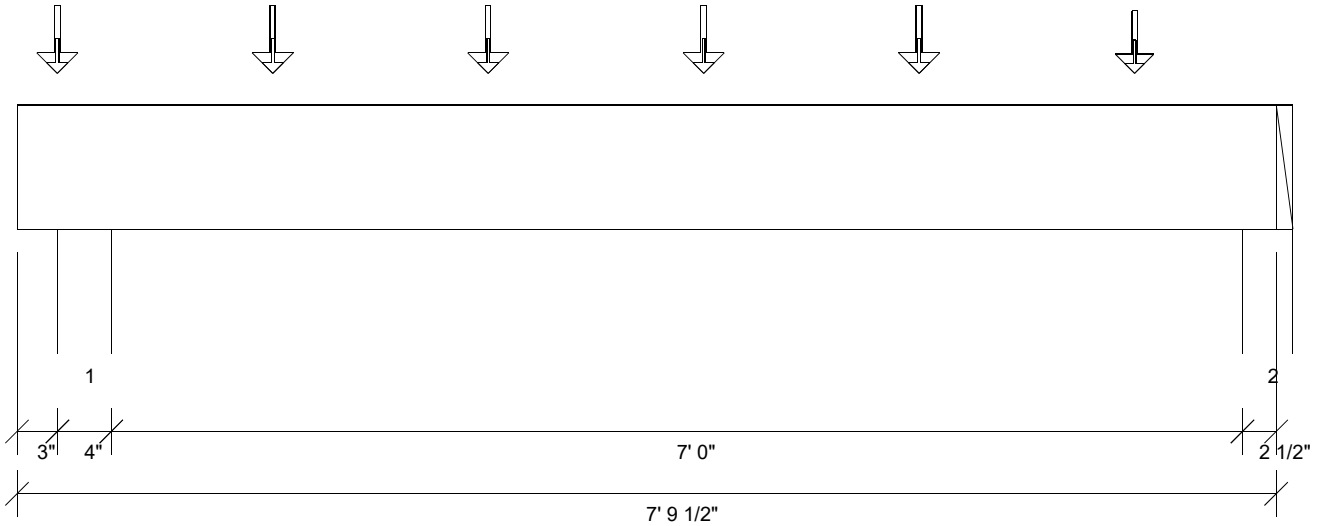
				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	3'- 4"	Self Weight	6 lb/ft	-	-	-
Point	1'- 4"	1'- 4"	J2(i3719)	83.00 lb	166.00 lb	-	-
Point	1'- 4"	1'- 4"	J4(i3778)	50.00 lb	100.00 lb	-	-
Point	2'- 8"	2'- 8"	J2(i3772)	83.00 lb	166.00 lb	-	-
Point	2'- 8"	2'- 8"	J4(i3724)	50.00 lb	100.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E80(i150)	112.00 lb	203.00 lb	-	-
2	3'	3'- 4"	E82(i152)	174.00 lb	329.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 7'- 9 1/2"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length Top:	0'	Bottom:	1'- 2 1/2"		

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	4'- 3"	2010.95 lb ft	3429.65 lb ft		Passed - 59%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 4 1/4"	967.82 lb	2497.50 lb		Passed - 39%	1.00	D + L
Live Load Deflection	4'- 11/16"	0'- 1/16"	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	4'- 11/16"	0'- 1/16"	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 5"	1377.98 lb	5578.13 lb	10500.00 lb	Passed - 25%	1.00	D + L
	7'- 8"	1030.98 lb	3187.44 lb	6562.38 lb	Passed - 32%	1.00	D + L

Design Notes:

* The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	7'- 9 1/2"	Self Weight	6 lb/ft	-	-	-
Point	0'- 3"	0'- 3"	J1(i3716)	107.00 lb	213.00 lb	-	-
Point	0'- 3"	0'- 3"	J4(i3784)	27.00 lb	55.00 lb	-	-
Point	1'- 7"	1'- 7"	J1(i3747)	107.00 lb	213.00 lb	-	-
Point	1'- 7"	1'- 7"	J4(i3777)	27.00 lb	55.00 lb	-	-
Point	2'- 11"	2'- 11"	J1(i3726)	107.00 lb	213.00 lb	-	-
Point	2'- 11"	2'- 11"	J4(i3783)	27.00 lb	55.00 lb	-	-
Point	4'- 3"	4'- 3"	J1(i3713)	107.00 lb	213.00 lb	-	-
Point	4'- 3"	4'- 3"	J4(i3715)	27.00 lb	55.00 lb	-	-
Point	5'- 7"	5'- 7"	J1(i3739)	107.00 lb	213.00 lb	-	-
Point	5'- 7"	5'- 7"	J4(i3749)	27.00 lb	55.00 lb	-	-
Point	6'- 11"	6'- 11"	J1(i3736)	93.00 lb	187.00 lb	-	-
Point	6'- 11"	6'- 11"	J4(i3738)	24.00 lb	48.00 lb	-	-

Support Information:

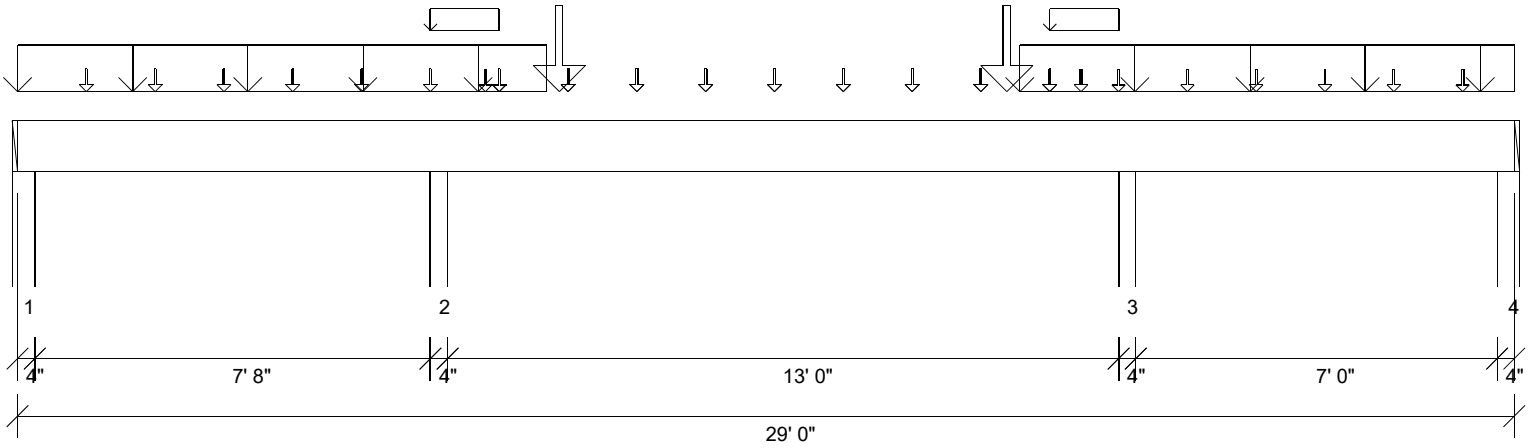
				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'- 3"	0'- 7"	E95(i168)	480.00 lb	912.00 lb	-	-
2	7'- 7"	7'- 9 1/2"	E97(i547)	354.00 lb	663.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.

Member: 3 - onCENTER LVL 2.0E 1 3/4" x 11 7/8"

Status: Design Passed



Graphical Illustration - Not To Scale
Member Cut Length - 29'
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length Top:	0'	Bottom:	1'- 2 1/2"		

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	21'- 6"	12716.31 lb ft	39896.69 lb ft		Passed - 32%	1.25	D + Lr
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	20'- 4 1/8"	8186.51 lb	14806.64 lb		Passed - 55%	1.25	D + Lr
Live Load Deflection	14'- 9 9/16"	0'- 1/8"	N/A (L/360)		Passed - L/999	-	Lr
Total Load Deflection	14'- 9 3/4"	0'- 3/16"	N/A (L/240)		Passed - L/896	-	D + 0.75(L + Lr)
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 3"	978.78 lb	17226.63 lb	18375.08 lb	Passed - 6%	1.25	D + Lr
	0'- 3"	-660.28 lb	17226.63 lb	-	Passed - 4%	1.25	D + Lr
	8'- 2"	11754.03 lb	17226.68 lb	18375.14 lb	Passed - 68%	1.25	D + Lr
	21'- 6"	11812.03 lb	17226.68 lb	18375.14 lb	Passed - 69%	1.25	D + Lr
	28'- 9"	756.11 lb	17226.59 lb	18375.04 lb	Passed - 4%	1.25	D + Lr
	28'- 9"	-959.77 lb	17226.59 lb	-	Passed - 6%	1.25	D + Lr

Design Notes:

* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	29'	Self Weight	16 lb/ft	-	-	-
Uniform	0'	10'- 3"	User Load	150 lb/ft	-	200 lb/ft	-
Uniform	8'	9'- 4"	FC3 Floor Material	1 lb/ft	3 lb/ft	-	-
Uniform	19'- 5"	29'	User Load	150 lb/ft	-	200 lb/ft	-
Uniform	20'	21'- 4"	FC3 Floor Material	1 lb/ft	3 lb/ft	-	-
Point	1'- 4"	1'- 4"	J4(i3728)	54.00 lb	108.00 lb	-	-
Point	2'- 8"	2'- 8"	J4(i3758)	54.00 lb	108.00 lb	-	-
Point	4'	4'	J4(i3735)	54.00 lb	108.00 lb	-	-
Point	5'- 4"	5'- 4"	J4(i3740)	54.00 lb	108.00 lb	-	-
Point	6'- 8"	6'- 8"	J4(i3759)	54.00 lb	108.00 lb	-	-
Point	8'	8'	J4(i3721)	53.00 lb	106.00 lb	-	-
Point	9'- 3/4"	9'- 3/4"	J3(i3775)	9.00 lb	17.00 lb	-	-
Point	9'- 4"	9'- 4"	J4(i3717)	52.00 lb	105.00 lb	-	-
Point	9'- 4"	9'- 4"	J3(i3720)	45.00 lb	89.00 lb	-	-
Point	10'- 6"	10'- 6"	User Load	2392.00 lb	-	4783.00 lb	-
Point	10'- 8"	10'- 8"	J3(i3753)	72.00 lb	144.00 lb	-	-
Point	10'- 8"	10'- 8"	J4(i3769)	52.00 lb	105.00 lb	-	-
Point	12'	12'	J3(i3745)	72.00 lb	144.00 lb	-	-
Point	12'	12'	J4(i3722)	52.00 lb	104.00 lb	-	-
Point	13'- 4"	13'- 4"	J3(i3743)	72.00 lb	144.00 lb	-	-
Point	13'- 4"	13'- 4"	J4(i3763)	52.00 lb	105.00 lb	-	-
Point	14'- 8"	14'- 8"	J3(i3751)	72.00 lb	144.00 lb	-	-
Point	14'- 8"	14'- 8"	J4(i3760)	52.00 lb	104.00 lb	-	-
Point	16'	16'	J3(i3746)	72.00 lb	144.00 lb	-	-
Point	16'	16'	J4(i3752)	52.00 lb	104.00 lb	-	-
Point	17'- 4"	17'- 4"	J3(i3785)	72.00 lb	144.00 lb	-	-
Point	17'- 4"	17'- 4"	J4(i3755)	52.00 lb	104.00 lb	-	-
Point	18'- 8"	18'- 8"	J3(i3774)	72.00 lb	144.00 lb	-	-
Point	18'- 8"	18'- 8"	J4(i3778)	52.00 lb	105.00 lb	-	-

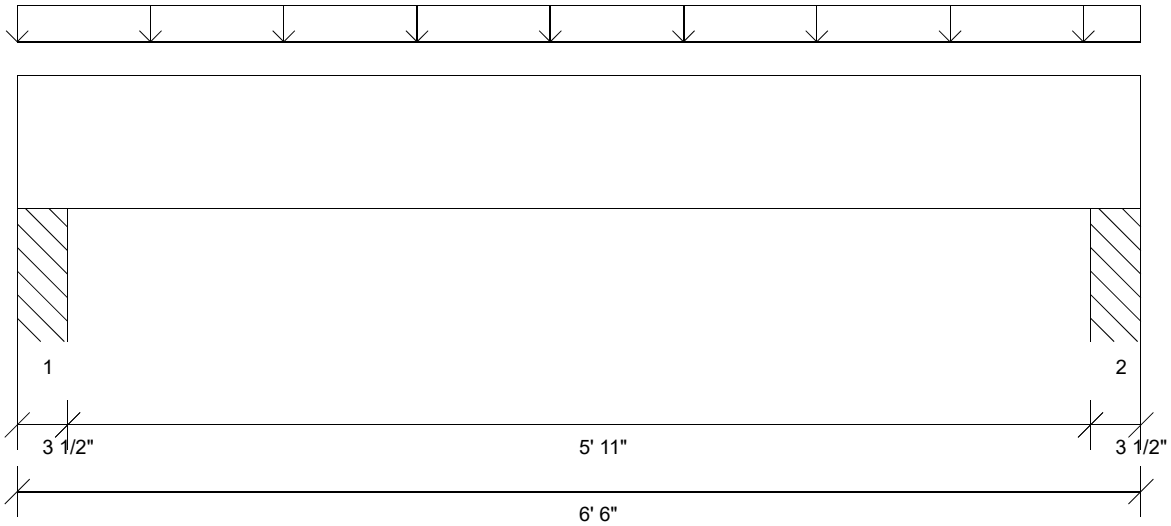
Point	19'- 2"	19'- 2"	User Load	2392.00 lb	-	4783.00 lb	-
Point	20'	20'	J3(i3725)	54.00 lb	107.00 lb	-	-
Point	20'	20'	J4(i3724)	52.00 lb	105.00 lb	-	-
Point	20'- 7 1/4"	20'- 7 1/4"	J3(i3767)	18.00 lb	35.00 lb	-	-
Point	21'- 4"	21'- 4"	J4(i3784)	30.00 lb	59.00 lb	-	-
Point	22'- 8"	22'- 8"	J4(i3777)	31.00 lb	61.00 lb	-	-
Point	24'	24'	J4(i3783)	31.00 lb	61.00 lb	-	-
Point	25'- 4"	25'- 4"	J4(i3715)	31.00 lb	61.00 lb	-	-
Point	26'- 8"	26'- 8"	J4(i3749)	31.00 lb	61.00 lb	-	-
Point	28'	28'	J4(i3738)	27.00 lb	54.00 lb	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 4"	E64(i134)	233.00 lb	257.00/-242.00 lb	781.00/-787.00 lb	-
2	8'	8'- 4"	E69(i138)	4820.00 lb	1769.00 lb	6901.00 lb	-
3	21'- 4"	21'- 8"	E95(i168)	4719.00 lb	1602.00 lb	6968.00 lb	-
4	28'- 8"	29'	E91(i162)	63.00 lb	150.00/-276.00 lb	734.00/-900.00 lb	-

Errors, Warnings & Notes:

* CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
* Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.



Graphical Illustration - Not To Scale
Member Cut Length - 6'- 6"
MemberPitch - 0/12

Design Information:

Building Code:	IBC 2012	Floor Dead Load:	10.0 lb/ft²	Roof Dead Load:	10.0 lb/ft²	Ground Snow Load:	0.0 lb/ft²
Design Methodology:	ASD	Floor Live Load:	40.0 lb/ft²	Roof Live Load:	20.0 lb/ft²		
		Unbraced Length	Top: 6'- 6"	Bottom: 6'- 6"			

Design Results:

	Location	Design	Control		Result	LDF	Load Combination
Critical Moment (Pos)	3'- 3"	672.33 lb ft	3429.65 lb ft		Passed - 20%	1.00	D + L
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Moment (Neg)		0.00 lb ft	0.00 lb ft				
Critical Shear	1'- 3/4"	319.43 lb	2497.50 lb		Passed - 13%	1.00	D + L
Live Load Deflection	3'- 3"	0'	N/A (L/360)		Passed - L/999	-	L
Total Load Deflection	3'- 3"	0'	N/A (L/240)		Passed - L/999	-	D + L
Max. Reaction			Supported Mtl	Supporting Mtl			
	0'- 2 1/2"	474.59 lb	4940.63 lb	7612.51 lb	Passed - 10%	1.00	D + L
	6'- 3 1/2"	474.59 lb	4940.63 lb	7612.51 lb	Passed - 10%	1.00	D + L

Design Notes:
* Member design assumed proper ply to ply connection. Verify connection between plies according to code specification

Loading:

				Maximum Load Magnitudes			
Type	Start	End	Source	Dead	Floor Live	Roof Live	Snow
Self Weight	0'	6'- 6"	Self Weight	6 lb/ft	-	-	-
Uniform	0'	6'- 6"	User Load	70 lb/ft	70 lb/ft	-	-

Support Information:

				Maximum Analysis Reactions			
Support	Start	End	Source	Dead	Floor Live	Roof Live	Snow
1	0'	0'- 3 1/2"	PBO5(i3789)	247.00 lb	227.00 lb	-	-
2	6'- 2 1/2"	6'- 6"	PBO6(i3816)	247.00 lb	228.00 lb	-	-

Errors, Warnings & Notes:

* The dead loads used in the design of this member were applied to the structure as sloped dead loads.
* Calculation of lateral stability factor (KL) is based on the width of one ply.
* The member graphic, dimensions, and locations shown on this report are based on the centerline of the member.
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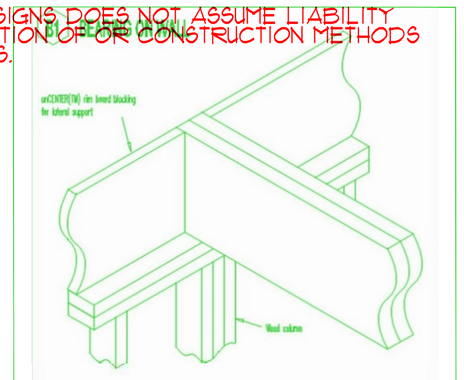

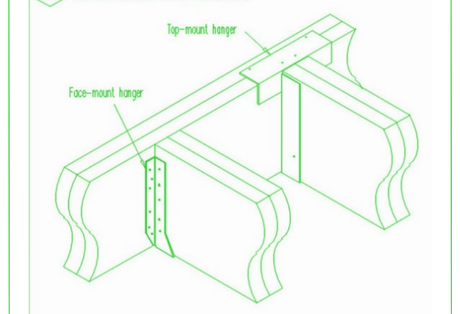

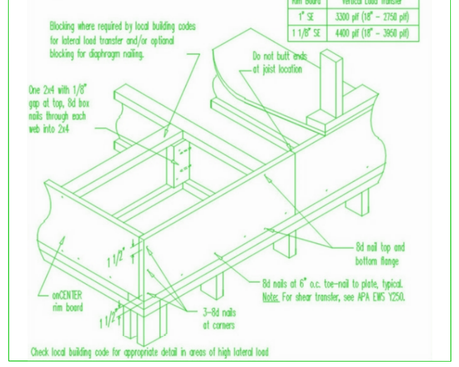

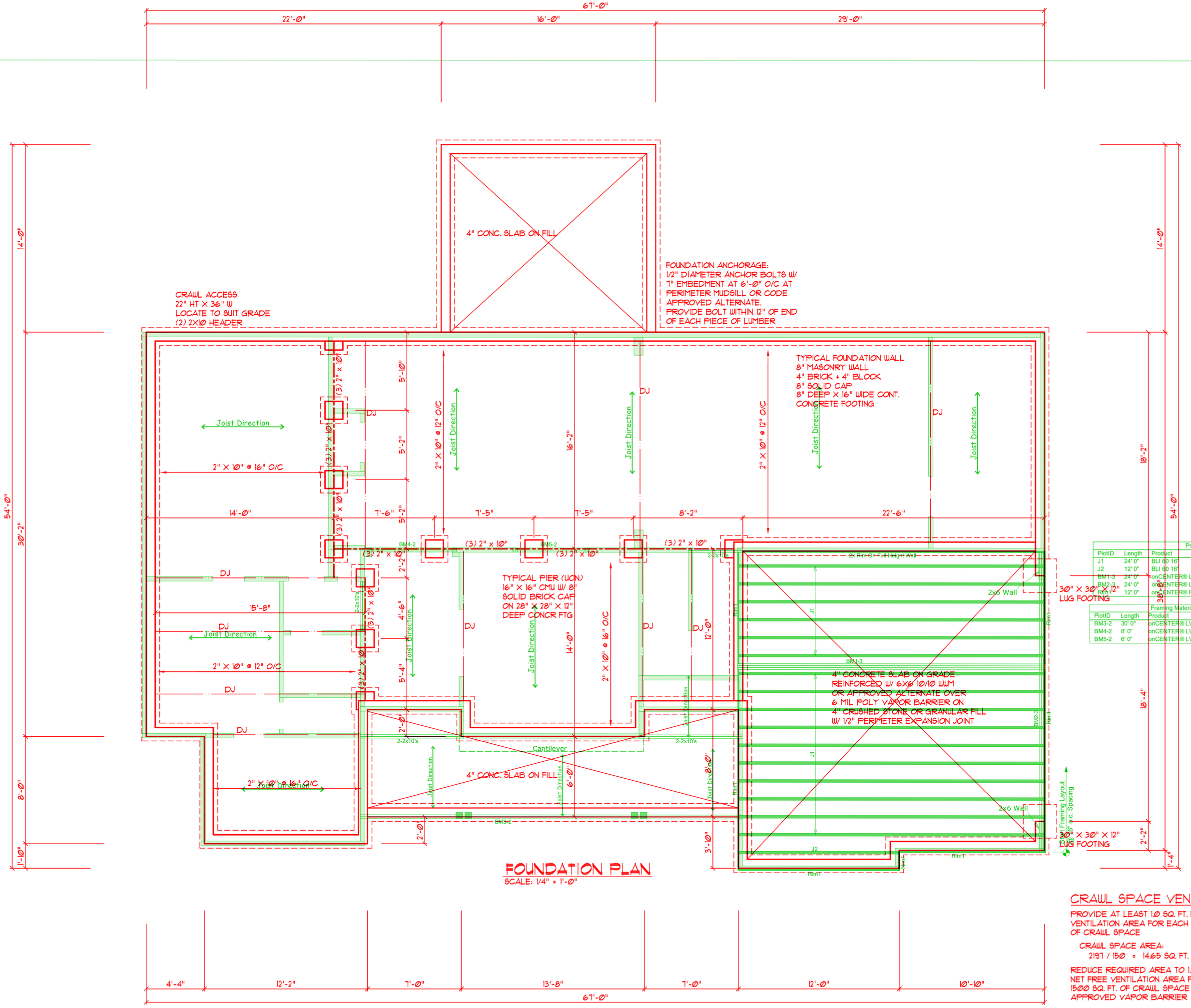

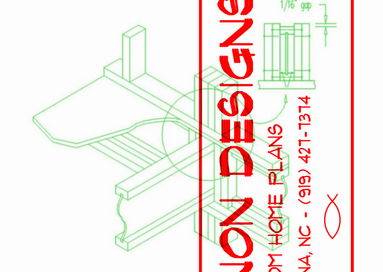

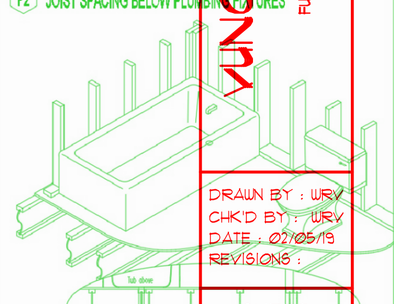

BUILDER SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT TIME OF CONSTRUCTION.

ALL CONSTRUCTION SHALL CONFORM TO THE 2018 EDITION OF THE NC STATE BUILDING CODE.

CODES GOVERN OVER DRAWINGS.

DIMENSIONS GOVERN OVER SCALE.

VERIFY ALL MECHANICAL REQUIREMENTS BEFORE FRAMING.

VUNCANNON DESIGNS DOES NOT ASSUME LIABILITY FOR ANY DEVIATION OF OR CONSTRUCTION METHODS OF THESE PLANS.

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

1) SPECIFIC NOTES ON THE PLANS TAKE PRECEDENCE OVER THE FOLLOWING GENERAL NOTES. NOTED DIMENSIONS GOVERN OVER SCALE. PLAN DESIGNER AND CONTRACTOR ARE SOLELY RESPONSIBLE FOR DIMENSIONAL ACCURACY. ALL CONSTRUCTION TO COMPLY WITH NC STATE BUILDING CODE, VOL. VII WITH CURRENT REVISIONS.

2) STRUCTURAL CONCRETE TO BE F'C=3000 PSI, PREPARED AND PLACED IN ACCORDANCE WITH ACI STANDARD 318.

3) FOOTINGS TO BEAR ON UNDISTURBED EARTH, A MIN. OF 12" BELOW ADJACENT FINISHED GRADE, OR AS OTHERWISE DIRECTED BY THE LOCAL INSPECTOR.

4) FOOTING SIZES BASED ON A PRESUMPTIVE SOIL BEARING CAPACITY OF 2000 PSF. CONTRACTOR IS SOLEY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION.

5) FOOTINGS AND PIERS SHALL BE CENTERED UNDER THEIR RESPECTIVE ELEMENTS. PROVIDE 3" MIN. FOOTING PROJECTION FROM FACE OF MASONRY.

6) MAXIMUN DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN VOLUME VII, SECTION R-304.3 OF THE NC STATE CODE.

7) PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.

8) PROVIDE FOUNDATION WATERPROOFING AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS.

9) PROVIDE PERIMETER INSULATION WITH BASEMENT SLABS.

10) CORBEL FOUNDATION WALL AS REQUIRED TO ACCOMMODATE BRICK VENEER.

11) CRAWL SPACE TO BE GRADED LEVEL AND CLEAR OF ALL DEBRIS.

ALLOWABLE PIER HEIGHTS		
SIZE	HOLLOW	SOLID
8"X16"	2'-8"	5'-0"
12"X16"	4'-0"	8'-0"
16"X16"	5'-4"	12'-0"
24"X24"	8'-0"	

FOOTINGS 30"X30"X10"-MINIMUM, UNLESS NOTED OTHERWISE OTHERWISE

NOTE

1) USE 2X10 'S AT 16" O.C. #2 S.P.F. MINIMUM SPECIFIED (U.O.N.)

2) JOISTS DIRECTION DESIGNATED BY ⟷

3) DOUBLE ALL JOIST PARALLEL TO WALLS ABOVE.

4) GIRDERS TO BE (3)2X10'S #2 S.P.F. MINIMUM (U.O.N.)

5) MINIMUM CRAWL SPACE ACCESS TO BE 22" HEIGHT X 36" WIDTH.

6) FOUNDATION WALL AND PIER CAPS TO BE MINIMUM 8" SOLID.

7) FOOTING UNDER MASONRY FIREPLACE TO BE 12" THICK AND EXTEND 12" BEYOND.

Products				
PlotID	Length	Product	Plies	Net Qty
J1	24' 0"	BLI 60 16"	1	16
J2	12' 0"	BLI 60 16"	1	1
BM1-3	24' 0"	onCENTER® LVL 2.0E 1 3/4" x 16"	3	3
BM2-3	24' 0"	onCENTER® LVL 2.0E 1 3/4" x 18"	3	3
Rim1	12' 0"	onCENTER® Rim SE 1 1/8" x 16"	1	6

Framing Material LVL's (By Others)				
PlotID	Length	Product	Plies	Net Qty
BM3-2	30' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	2
BM4-2	8' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	2
BM5-2	6' 0"	onCENTER® LVL 2.0E 1 3/4" x 9 1/4"	2	2

CRAWL SPACE VENTILATION

PROVIDE AT LEAST 1.0 SQ. FT. NET FREE VENTILATION AREA FOR EACH 150 SQ. FT. OF CRAWL SPACE

CRAWL SPACE AREA:
2197 / 150 = 14.65 SQ. FT. REQ'D.

REDUCE REQUIRED AREA TO 1.0 SQ. FT. NET FREE VENTILATION AREA FOR EACH 1500 SQ. FT. OF CRAWL SPACE WITH APPROVED VAPOR BARRIER

PROVIDE (1) VENT WITHIN 3'-0" OF EACH CORNER

REFER TO MANUFACTURER SPECIFICATIONS FOR ACTUAL VENTS USED TO DETERMINE NUMBER OF VENTS REQUIRED

MILTON BUILT HOMES

VUNCANNON DESIGNS
CUSTOM HOME PLANS
FUQUAY-VARINA, NC - (919) 427-1374

DRAWN BY : WRV
CHK'D BY : WRV
DATE : 02/05/19
REVISIONS :

SHEET
A-3

Plan No. 2662-15

onCENTER 2.0E LVL MULTIPLE PLY FASTENING

F11 DOUBLE JOIST CONSTRUCTION WITH FILLER

The attached materila list represents BlueLinx Engineered Lumber products needed to frame layout(s) shown based on the interpretation of user, but has not been reviewed by a BluLinx engineer. Purchaser is to verify material quantities, lengths, locations, and sizes and resolve clouded items.

Specified products are sized only for gravity loads shown. These loads should be verified by the purchaser. If additional loads or framing areas need to be accounted for, notify supplier of BlueLInx products so that material can be sized and price adjusted. Unless noted otherwise, hip, valley and ridge boards have not been designed and no products were designed to resist the building's lateral loads.