

RE: 1010683 - H&H-NC/Harmony/979/Carriage <b>Site Information:</b> Project Customer: H and H Project Name: 1010		<b>Trenco</b> 818 Soundside Rd Edenton, NC 27932
	vision: Carriage Glen at And	lerson Cre
Model:		
Address:		
City: SPRING LAKE State:	NC	
General Truss Engineering Criteria & Design Lo Drawings Show Special Loading Conditions):	ads (Individual Truss Desig	gn
Design Code: IRC2009/TPI2007	Design Program: MiTek 2	20/20 7.6
Wind Code: ASCE 7-05 Wind Speed: 100 mph	Design Method: MWFRS	(low-rise)/C-C hybrid Wind ASCE 7-05
Roof Load: 40.0 psf	Floor Load: N/A psf	

Exposure Category: C

Mean Roof Height (feet): 25

133

30242

No. Seal# **Truss Name Date** I33330211 I33330212 I33330213 I33330213 I33330214 I33330216 I33330217 I33330217 I33330218 I33330219 I33330221 5/15/18 123456789111111111122222222222333 A05 5/15/18 5/15/18 A07 A07A A15 A31 A32 A33 5/15/18 5/15/18 5/15/18 5/15/18 5/15/18 5/15/18 A A 5/15/18 5/15/18 133330220 133330221 133330222 133330223 133330224 133330225 133330226 133330226 133330227 133330228 A38 A39 5/15/18 A40 5/15/18 5/15/18 B01 5/15/18 5/15/18 5/15/18 5/15/18 5/15/18 5/15/18 5/15/18 5/15/18 Č03 J10 133330229 133330230 J102 J103 133330231 133330232 J104 5/15/18 J105 133330233 133330234 133330235 5/15/18 5/15/18 J106 1107 J109 5/15/18 13 36 J113 5/15/18 133330237 J114 5/15/18 30238 J115 /18 /18 30239 J116 J117 5/15/18 30240 30241 J118 5/15/18 1333

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Sumter,SC.

5/15/18

Truss Design Engineer's Name: Vance, Jeff

**PB02** 

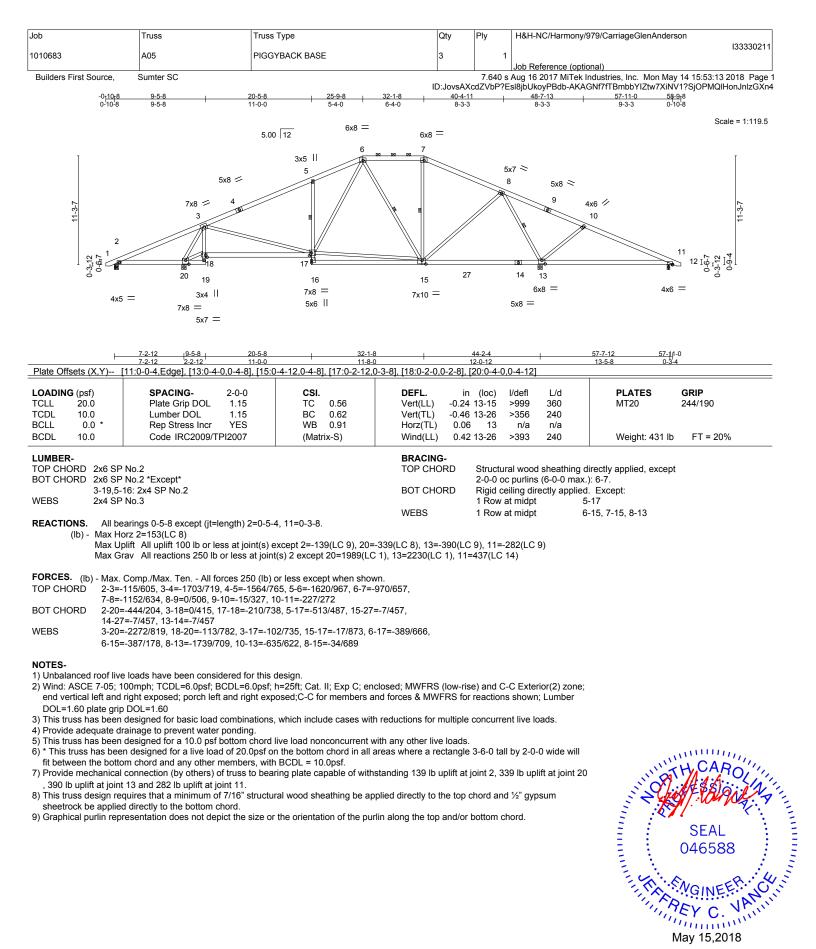
My license renewal date for the state of North Carolina is December 31, 2018.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Vance, Jeff

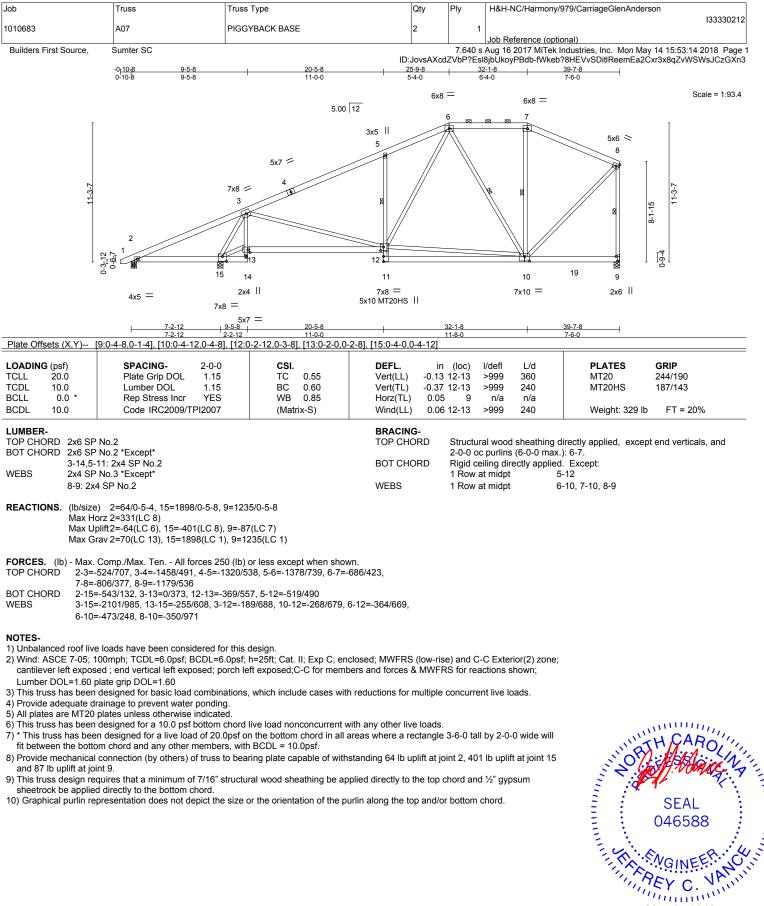
May 15,2018



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 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 **ANSI/TP14 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

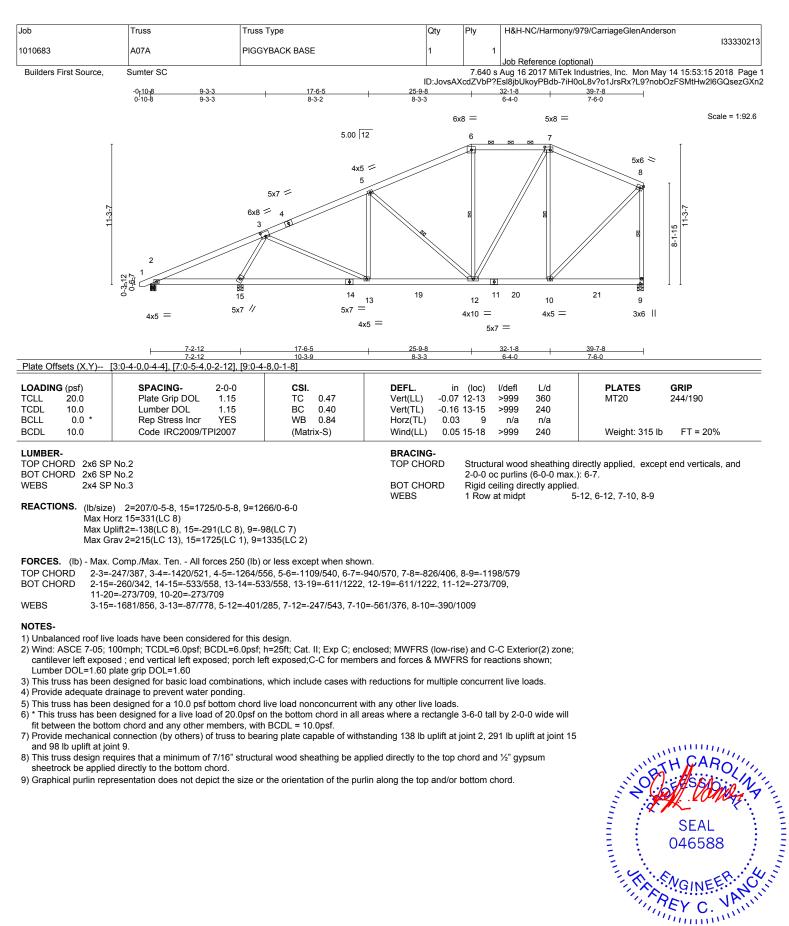
ENGINEERING BY A MITEK Atfiliate

818 Soundside Road Edenton, NC 27932



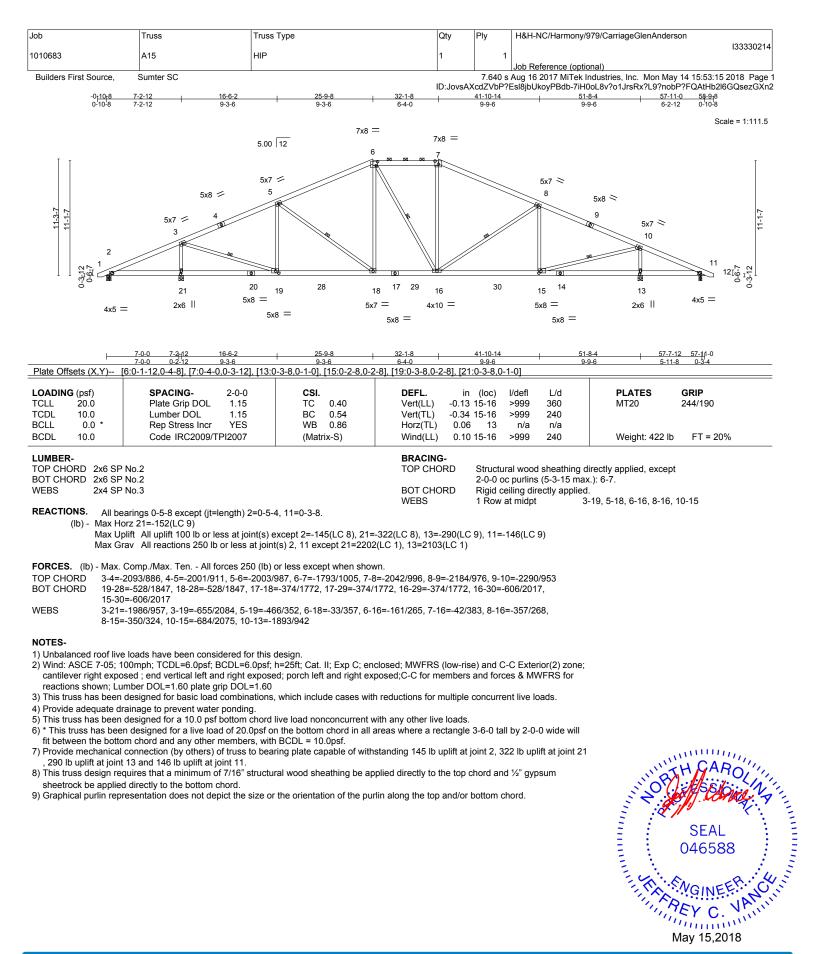


AMITEK Attiliate B18 Soundside Road Edenton, NC 27932



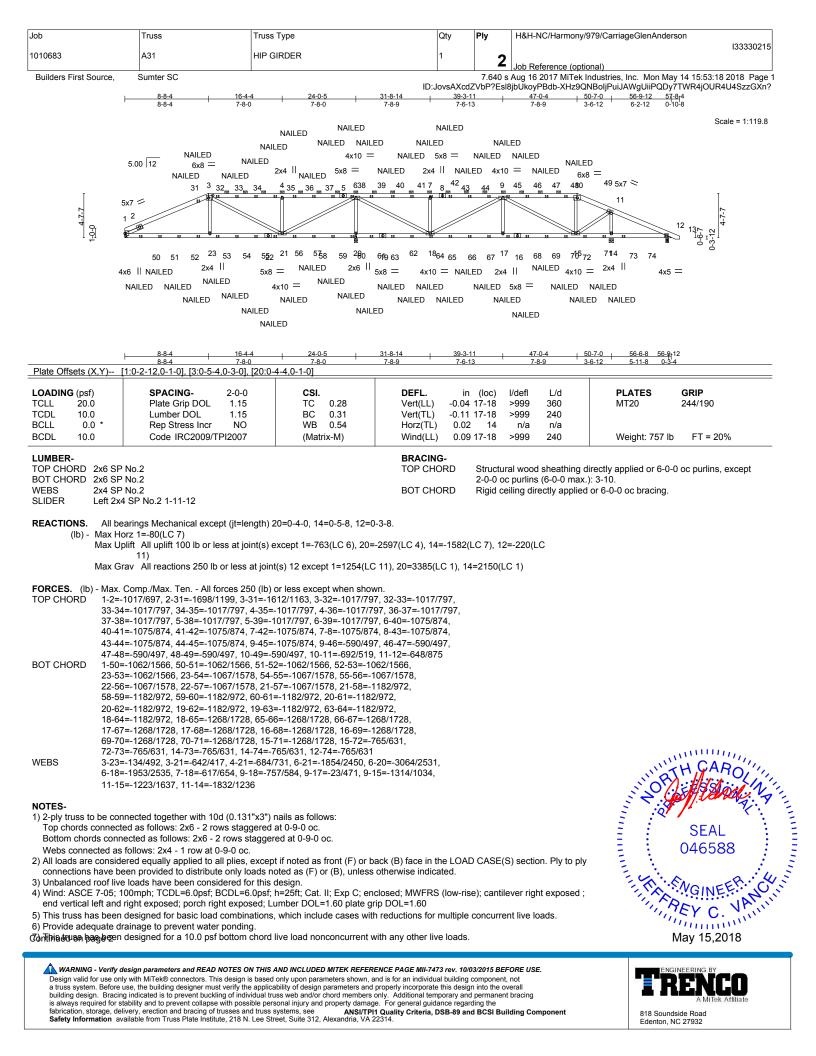
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Edenton, NC 27932



Job	Truss	Truss Type	Qty	Ply	H&H-NC/Harmony/979/CarriageGlenAnderson
1010683	A31		1		133330215
				2	Job Reference (optional)
Builders First Source,	Sumter SC				Aug 16 2017 MiTek Industries, Inc. Mon May 14 15:53:18 2018 Page 2
		I	D:JovsAXcd	ZVbP?Esl8	3jbUkoyPBdb-XHz9QNBoljPuiJAWgUiiPQDy7TWR4jOUR4U4SzzGXn?

## NOTES-

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Refer to girder(s) for truss to truss connections.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 763 lb uplift at joint 1, 2597 lb uplift at joint 20, 1582 lb uplift at joint 14 and 220 lb uplift at joint 12.

11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 13) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

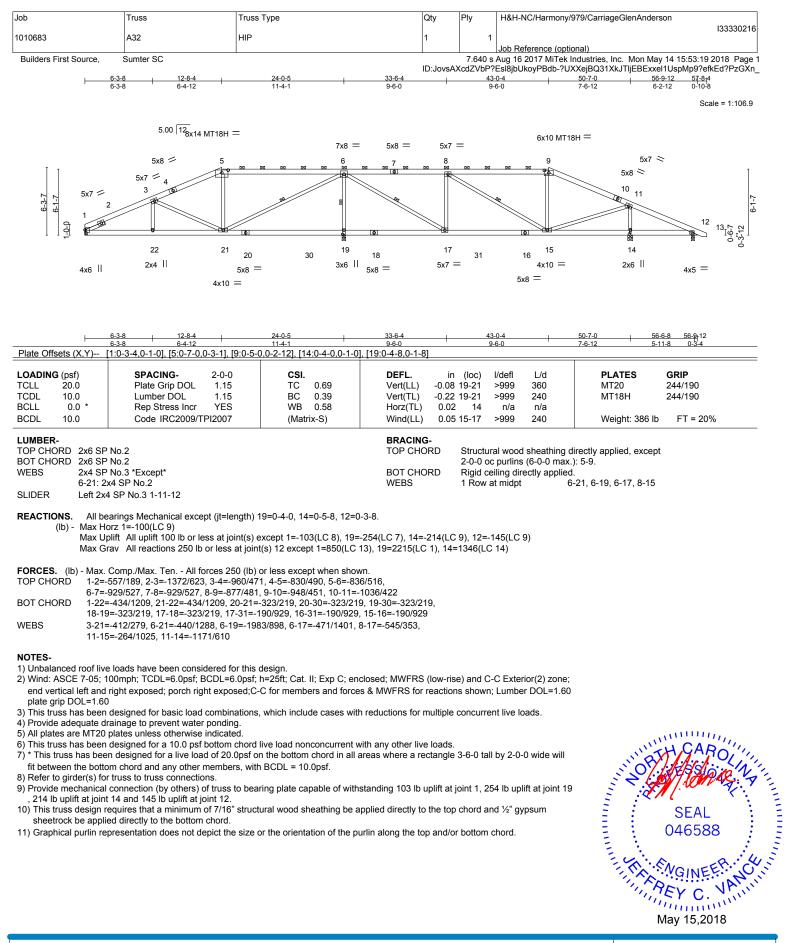
Uniform Loads (plf)

Vert: 1-3=-60, 3-10=-60, 10-13=-60, 24-28=-20

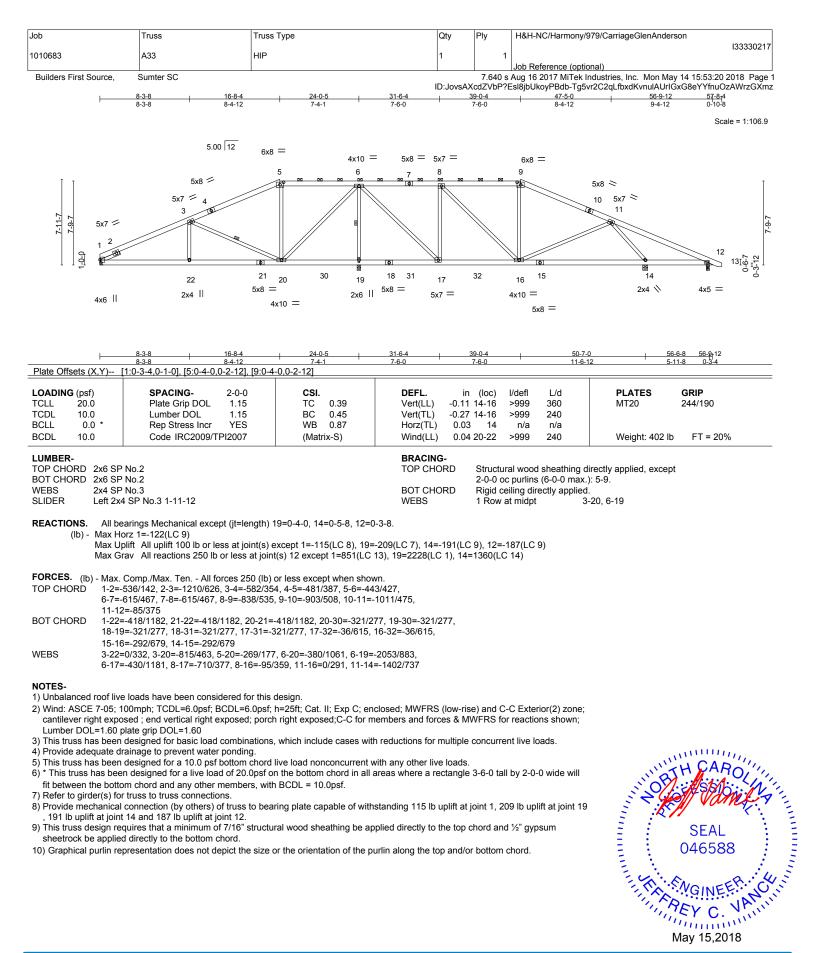
Concentrated Loads (lb) Vert: 7=-52(F) 18=-28(F) 31=-52(F) 32=-52(F) 33=-52(F) 34=-52(F) 35=-52(F) 36=-52(F) 37=-52(F) 38=-52(F) 39=-52(F) 40=-52(F) 41=-52(F) 42=-52(F) 43=-52(F) 45=-52(F) 4 44=-52(F) 45=-52(F) 46=-52(F) 46=-52(F) 46=-52(F) 48=-52(F) 49=-52(F) 50=-175(F) 51=-51(F) 52=-87(F) 53=-28(F) 54=-28(F) 55=-28(F) 55=-2 59=-28(F) 60=-28(F) 61=-28(F) 62=-28(F) 63=-28(F) 65=-28(F) 66=-28(F) 67=-28(F) 68=-28(F) 69=-28(F) 70=-28(F) 71=-28(F) 72=-87(F) 73=-51(F) 74=-76(F)

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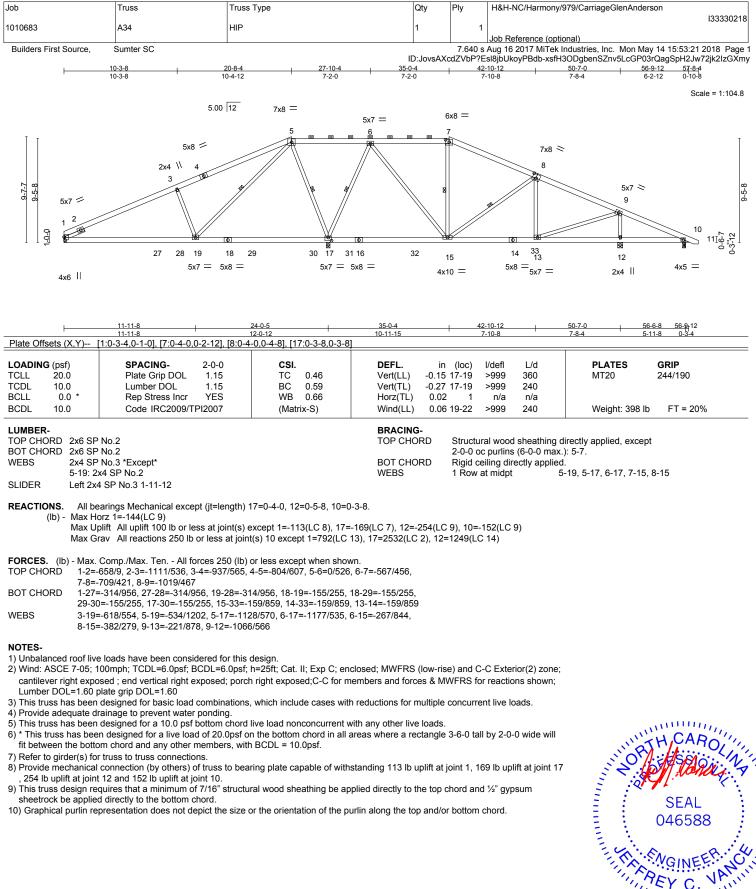








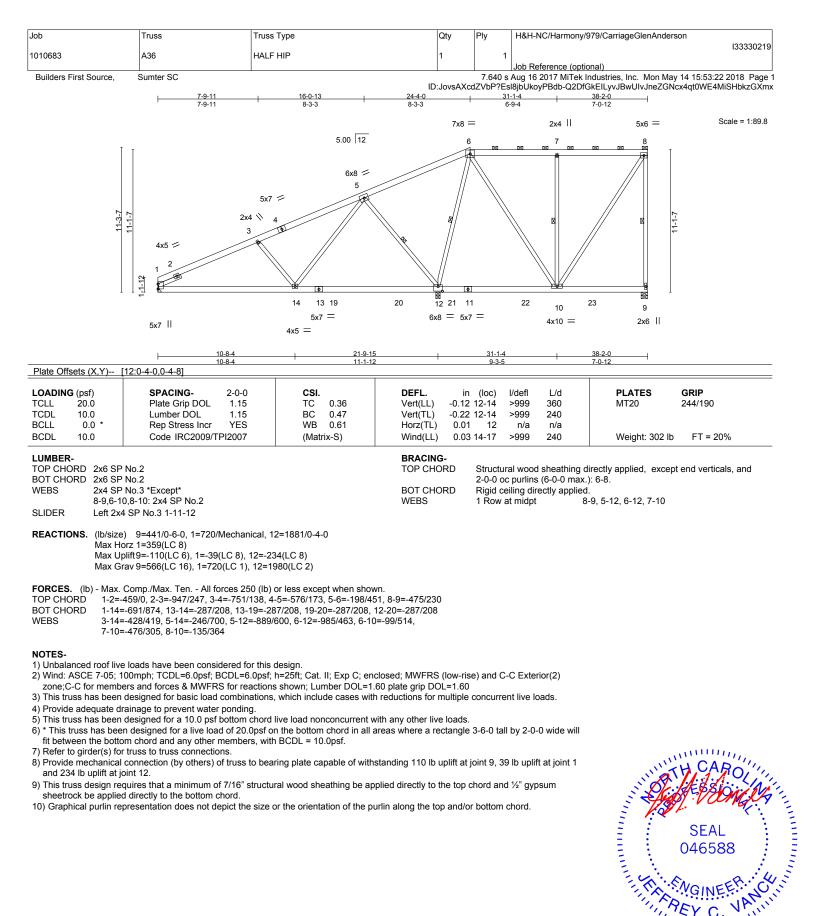






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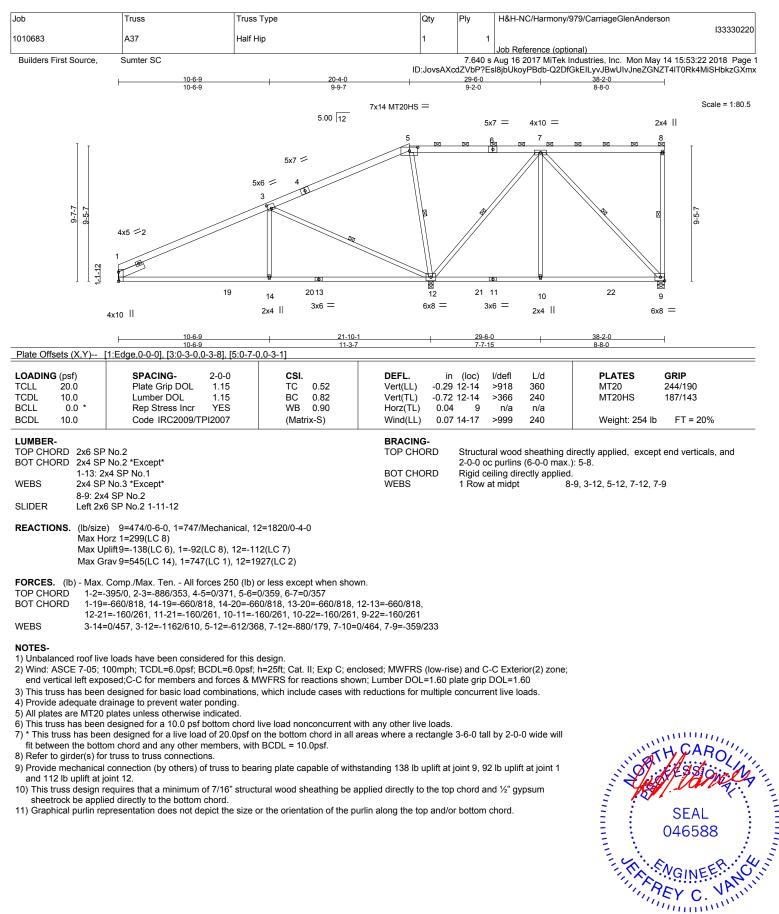




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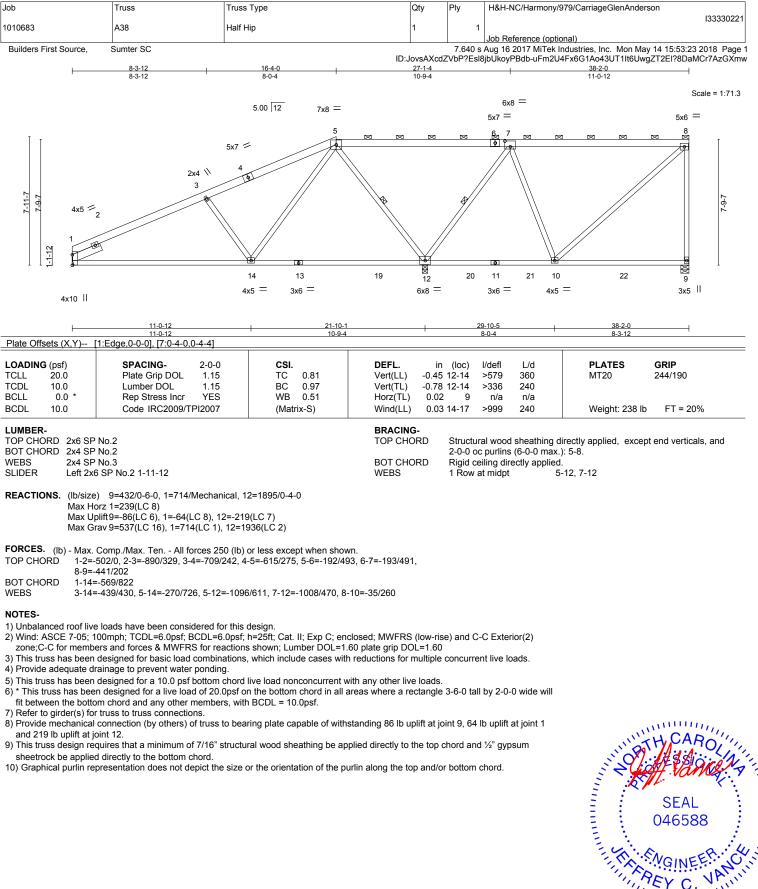


May 15,2018



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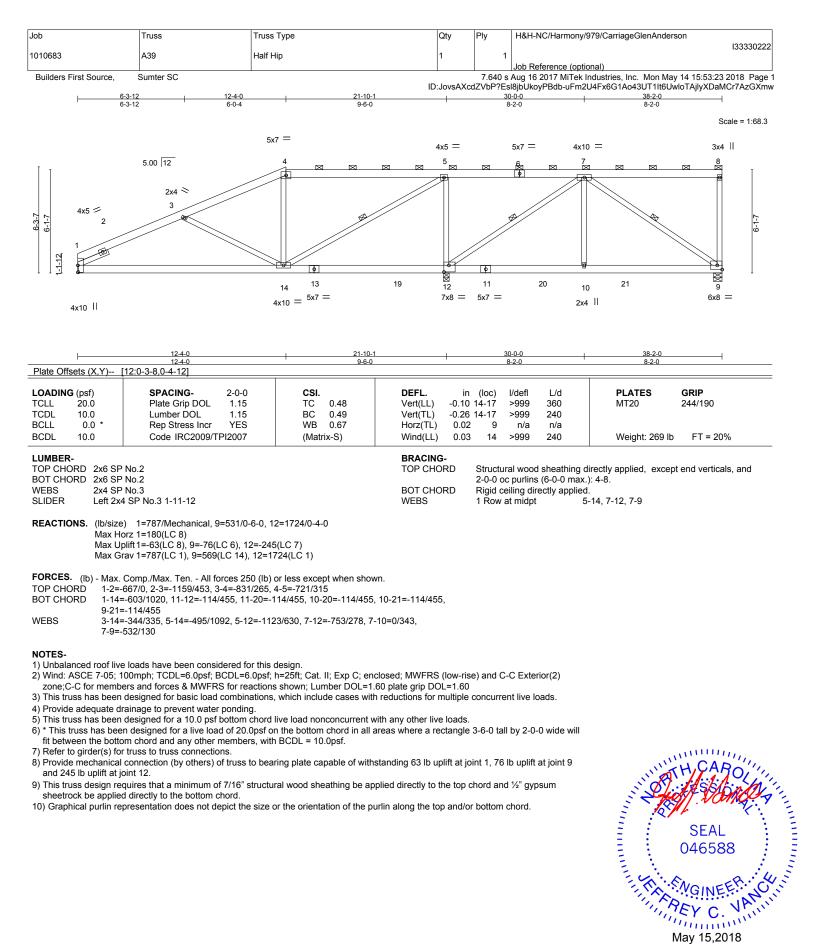






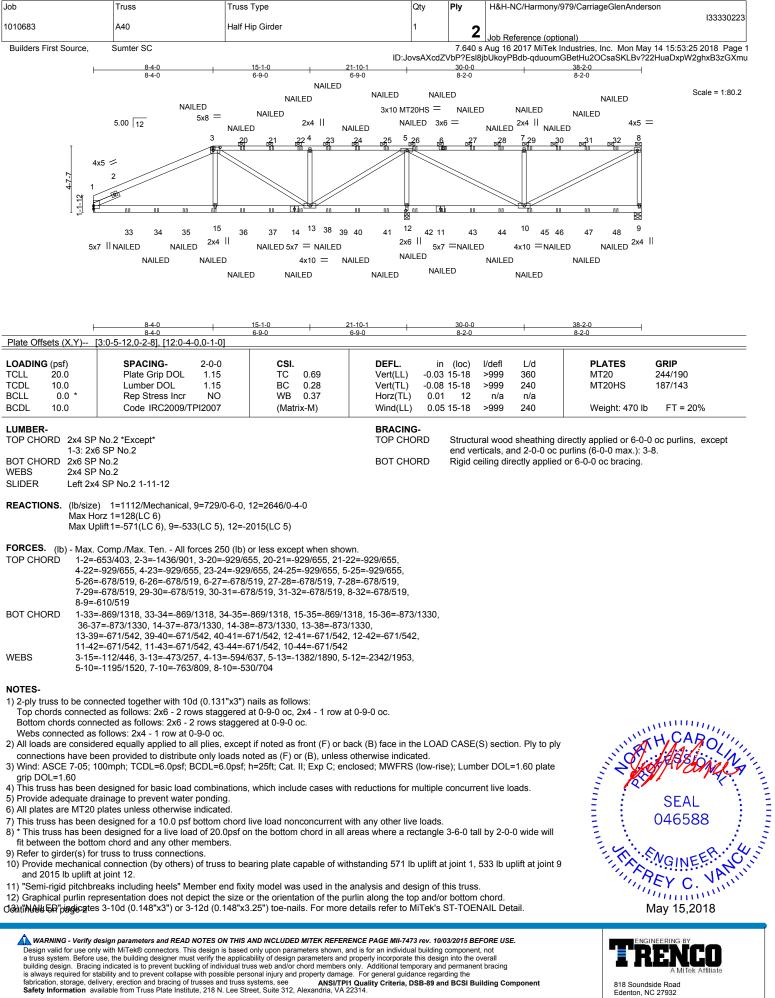
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Edenton, NC 27932



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Edenton, NC 27932



818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Harmony/979/CarriageGlenAnderson
					133330223
1010683	A40	Half Hip Girder	1	2	
				_	Job Reference (optional)
Builders First Source,	Sumter SC			7.640 s	Aug 16 2017 MiTek Industries, Inc. Mon May 14 15:53:25 2018 Page 2

ID:JovsAXcdZVbP?Esl8jbUkoyPBdb-qduoumGBetHu2OCsaSKLBv?22HuaDxpW2ghxB3zGXmu

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

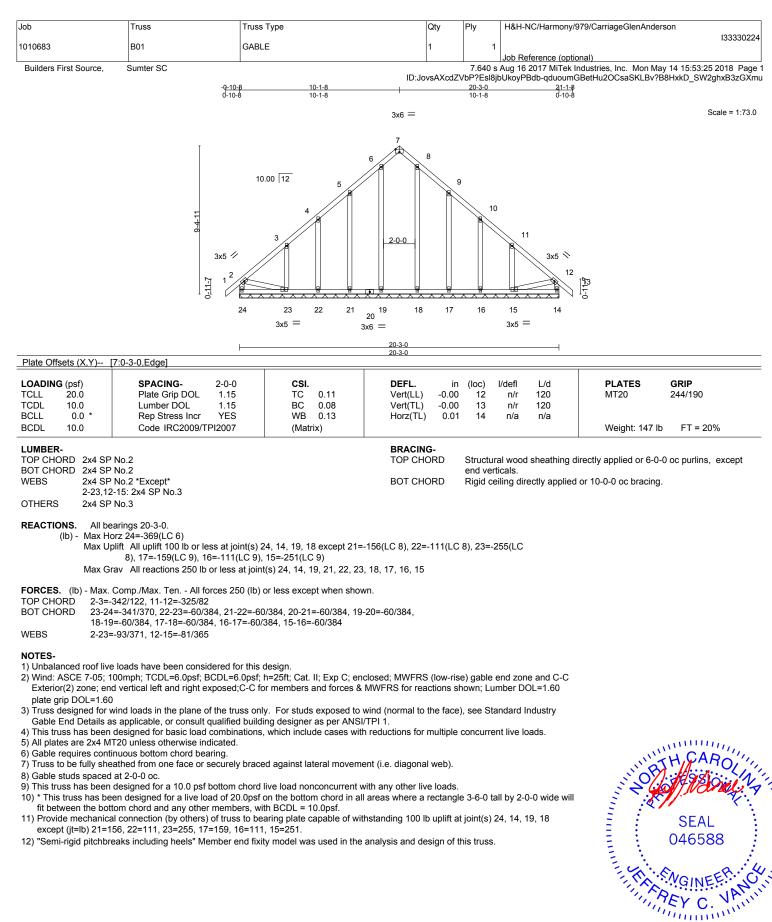
Uniform Loads (plf)

Vert: 1-3=-60, 3-8=-60, 9-16=-20

Concentrated Loads (lb)

Vert: 3=-29(B) 6=-52(B) 15=-31(B) 11=-28(B) 20=-52(B) 21=-52(B) 22=-52(B) 23=-52(B) 24=-52(B) 25=-52(B) 26=-52(B) 27=-52(B) 28=-52(B) 29=-52(B) 30=-52(B) 30 31=-52(B) 32=-52(B) 33=-170(B) 34=-34(B) 35=-60(B) 36=-28(B) 37=-28(B) 38=-28(B) 39=-28(B) 40=-28(B) 41=-28(B) 42=-28(B) 43=-28(B) 43=-28(B) 45=-28(B) 45=-2





May 15,2018



Job	Truss	Truss Type	Qty	Ply	H&H-NC/Harmony/9	79/CarriageGlenAnderson	
1010683	C01	GABLE	1	1			133330225
Builders First Source,	Sumter SC					dustries, Inc. Mon May 14 15	
		7-4-0	ID:JovsAX <u>13-7-12</u> 6-3-12	cdZVbP?E 20-3-0 6-7-4	sl8jbUkoyPBdb-IqSA6 <u>21-1-</u> β 0-10-8	6HpPBPlfYn389saj6YGGhCJ	lyRlgGKQVkVzGXmt
		5x6		0-7-4	0-10-0		Scale: 1/8"=1'
		10.00 12 2					
			3 3x6 ×	<u>,</u>			
		6x8 1/	4 3x5 3x6 / 5	~			
	11- 1- 10		340 12				
	~				5x6 📏		
		2-17			× <sup>6</sup>		
		15 State	g				
		12 11	1 <sup>10</sup> 299	30	a o 4x10		
			3x6 = 20HS =				
		7-4-0	13-7-12 6-3-12	<u>20-3-0</u> 6-7-4			
Plate Offsets (X,Y) [1	:0-3-0,0-1-12], [7:0-6-14,Edg		0012	014			
LOADING (psf) TCLL 20.0	SPACING- 2-0- Plate Grip DOL 1.1			1 (loc) 11-12	l/defl L/d >999 360	PLATES GRI MT20 244/	
TCDL 10.0 BCLL 0.0 *	Lumber DOL 1.1 Rep Stress Incr YE	5 BC 0.44		' 11-12	>999 240 n/a n/a	MT20HS 187/	
BCDL 10.0	Code IRC2009/TPI200		( )		>999 240	Weight: 182 lb F	T = 20%
LUMBER- TOP CHORD 2x4 SP 1	lo 2 *Excent*		BRACING- TOP CHORD	Structur	al wood sheathing di	rectly applied, except end	verticals
1-2: 2x6 BOT CHORD 2x4 SP1	SP No.2		BOT CHORD WEBS		iling directly applied.		verticals.
WEBS 2x4 SP M	No.3		JOINTS		at midpt 5- at Jt(s): 13, 14	11	
OTHERS 2x4 SP 1 SLIDER Right 2x	NO.3 8 SP DSS 1-11-12						
	12=803/0-5-8, 7=858/0-5-8						
	z 12=-415(LC 6) ift12=-258(LC 9), 7=-234(LC	9)					
		50 (lb) or less except when show					
12-15=	-730/313, 1-15=-670/271	.88/296, 4-5=-578/273, 5-6=-914/2	, ,				
7-30=-	38/635	-29=-38/635, 9-29=-38/635, 9-30=	,				
	-97/314, 2-13=-96/304, 5-11	389/328, 5-9=0/291, 1-14=-57/3	352, 11-14=-58/353				
	loads have been considered						
		6.0psf; h=25ft; Cat. II; Exp C; enc r members and forces & MWFRS					
DOL=1.60 3) Truss designed for wi	nd loads in the plane of the t	uss only. For studs exposed to w	vind (normal to the face	e), see Sta	andard Industry		
		ed building designer as per ANSI/ nations, which include cases with		concurre	nt live loads.		
	lates unless otherwise indica 20 unless otherwise indicate					Number of the second se	CAP
<ol> <li>Gable studs spaced a</li> <li>This truss has been c</li> </ol>		n chord live load nonconcurrent wi	ith any other live loads.			N' A.	adria dal
	designed for a live load of 20 n chord and any other memb	0.0psf on the bottom chord in all a ers, with BCDL = 10.0psf.	reas where a rectangle	e 3-6-0 tall	by 2-0-0 wide will		Counces
		s to bearing plate capable of with	nstanding 100 lb uplift a	at joint(s) e	except (jt=lb)		EAL
	quires that a minimum of 7/10 d directly to the bottom chord	" structural wood sheathing be ap	oplied directly to the top	o chord ar	id ½" gypsum	04	6588
						S O4	
						The NG	INEER
						TIREY	C. VALLIN
						11111	15,2018

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ENGINEERING BY A MITEK Affiliate B18 Soundside Road Edenton, NC 27932

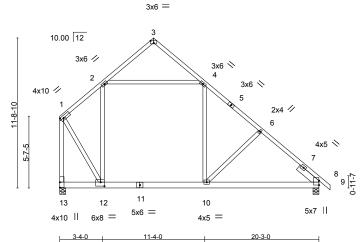
May 15,2018

Job	Truss	Truss Type	Qty	Ply	H&H-NC/Harmonv/	979/CarriageGlenAnders	on
1010683	C03	COMMON	7				133330226
Builders First Source,	Sumter SC		•		Job Reference (opti		14 15:53:27 2018 Page 1
Builders First Source,	Sumer SC	<u>3-4-0</u> <u>7-4-0</u> 3-4-0 4-0-0	ID:JovsAXcd 11-4-0 15-7-12 4-0-0 4-3-12		8jbUkoyPBdb-m00YJ - <u>02∥-1-</u> 8		u5UKhpcpV_A2GxzGXms
		3×	<sub>6</sub> =				Scale = 1:94.1
		10.00 12	3				
		3x6 1/					
		2	3x6 \\ 4				
	0	4x5 1	3x6 5		,		
	11-8-10	I R I		2x4 %			
				$\times$	4x5 📎		
	ע א ע				7		
					-1 e 8		
		<sup>™</sup> 11 13 12 11 5x10 MT20	10		ax10		
		$4 \times 10^{-11} = 6 \times 10^{-5} \times 10^{-$	4x5 =				
		<u>  3-4-0   11-</u> 3-4-0 8-0		20-3-0 3-11-0			
Plate Offsets (X,Y) [1	:0-2-0,0-1-12], [3:0-3-0,Edge],					1	
LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.67		(loc) 5 10-16	l/defl L/d >967 360	PLATES MT20	<b>GRIP</b> 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.64	Vert(TL) -0.61	10-16	>396 240	MT20HS	187/143
BCLL 0.0 * BCDL 10.0	Rep Stress Incr NO Code IRC2009/TPI2007	WB 0.41 (Matrix-M)	Horz(TL) -0.05 Wind(LL) 0.36	5 8 5 10-16	n/a n/a >662 240	Weight: 152 lb	FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SP I BOT CHORD 2x6 SP I	DSS *Except*		TOP CHORD	end ver	ticals.	directly applied or 6-0-0	oc purlins, except
8-11: 2x WEBS 2x4 SP I	6 SP No.1 No.3		BOT CHORD	Rigid ce	eiling directly applied	l or 6-0-0 oc bracing.	
SLIDER Right 2x	6 SP No.2 1-11-12						
	13=803/0-5-8, 8=858/0-5-8 rz 13=-439(LC 6)						
Max Upl	lift 13=-80(LC 9), 8=-102(LC 9) av 13=857(LC 2), 8=858(LC 1)						
TOP CHORD 1-2=-6	Comp./Max. Ten All forces 25 10/322, 2-3=-276/127, 4-5=-61						
BOT CHORD 12-13=	-1185/439 =-251/394, 11-12=-40/468, 10-1						
	350/924, 2-4=-363/327, 6-10=-	2///2/2					
NOTES- 1) Unbalanced roof live	loads have been considered fo	r this design.					
	0mph; TCDL=6.0psf; BCDL=6. ight exposed;C-C for members						
,	lesigned for basic load combina lates unless otherwise indicate	,	h reductions for multiple	concurre	nt live loads.		
5) This truss has been of	lesigned for a 10.0 psf bottom of designed for a live load of 20.0	chord live load nonconcurrent			l by 2-0-0 wide will		
fit between the bottor	n chord and any other member connection (by others) of truss t	s, with BCDL = 10.0psf.	· ·				
8=102.							H CARO
o, semi-ngiù pitchorea	ks including heels" Member end	a many model was used in the a	analysis and design of tr	ແລ ແປຣຣ.		STOP.	ABBAN IN'
							A CANCE A
							SEAL
						E :	046588

SEAL 046588 MGINEER May 15,2018



Job	Truss	Truss Type	Qty	Ply	H&H-NC/Harmony/979/CarriageGlenAnderson
1010683	C03A	COMMON	2	1	133330227
					Job Reference (optional)
Builders First Source,	Sumter SC			7.640 s	Aug 16 2017 MiTek Industries, Inc. Mon May 14 15:53:27 2018 Page 1
			D:JovsAXcdZ	VbP?Esl8j	bUkoyPBdb-m00YJSIRAUXcHiMFitNpGK5NH5U?hpPpV_A2GxzGXms
		<u>3-4-0</u> <u>7-4-0</u> <u>11-4-0</u> 3-4-0 4-0-0 4-0-0	<u>15-7-12</u> 4-3-12	<u>+ 20-3</u> 4-7-	



#### Plate Offsets (X,Y)-- [3:0-3-0.Edge], [12:0-3-8.0-3-12]

LOADING	G (psf)	SPACING- 2-0-12	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.70	Vert(LL) -0.26 10-16 >937 360 MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.66	Vert(TL) -0.63 10-16 >384 240
BCLL	0.0 *	Rep Stress Incr NO	WB 0.43	Horz(TL) -0.05 8 n/a n/a
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-M)	Wind(LL) 0.38 10-16 >642 240 Weight: 152 lb FT = 20%

8-0-0

#### LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP DSS *Except*
	8-11: 2x6 SP No.1
WEBS	2x4 SP No.3
SLIDER	Right 2x6 SP No.2 1-11-12

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-11-5 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

8-11-0

REACTIONS. (lb/size) 13=828/0-5-8, 8=885/0-5-8 Max Horz 13=-453(LC 6) Max Uplift 13=-82(LC 9), 8=-105(LC 9) Max Grav 13=884(LC 2), 8=885(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-629/332, 2-3=-284/131, 4-5=-629/331, 5-6=-731/315, 6-7=-769/323, 7-8=-844/177, 1-13=-1222/452

BOT CHORD 12-13=-259/406, 11-12=-42/483, 10-11=-42/483, 8-10=-84/649

1-12=-360/953, 2-4=-374/337, 6-10=-286/281 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3-4-0

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

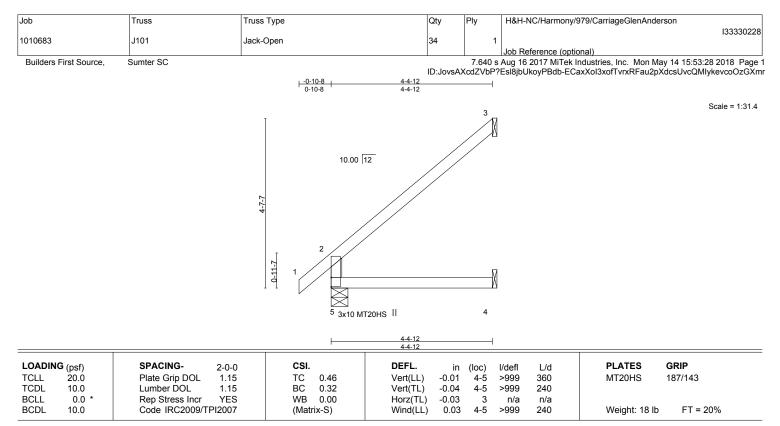
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 8=105.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Scale = 1:90.0





### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals. BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 5=236/0-5-8, 3=112/Mechanical, 4=48/Mechanical Max Horz 5=192(LC 8) Max Uplift 3=-99(LC 8) Max Grav 5=236(LC 1), 3=112(LC 1), 4=79(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;

- end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

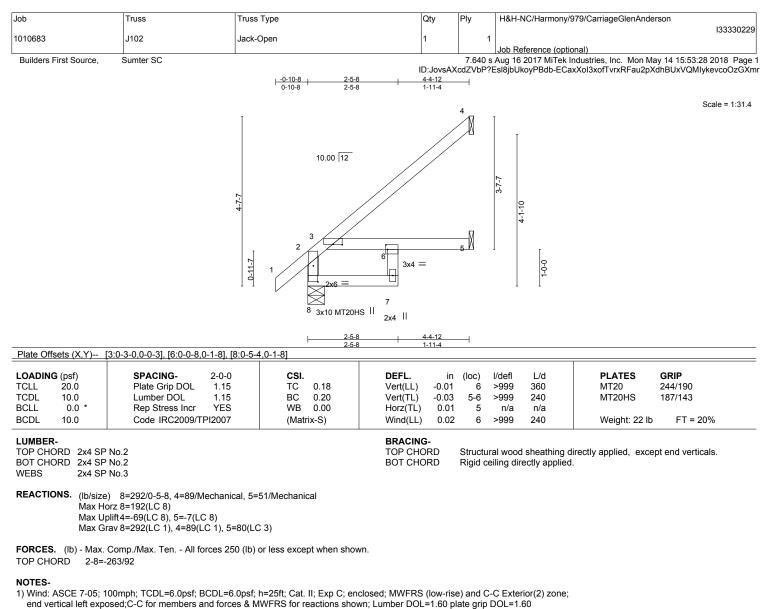
4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



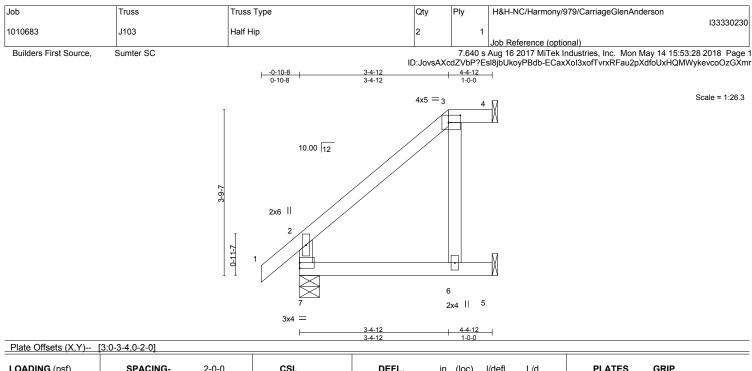




- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.







LOADIN	G (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.27	Vert(LL) -0	.01 6-7	>999	360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.21	Vert(TL) -0	.04 6-7	>999	240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.05	Horz(TL) -0	.05 4	n/a	n/a	
BCDL	10.0	Code IRC2009/TPI2007	(Matrix-S)	Wind(LL) 0	.04 6-7	>999	240	Weight: 22 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied.

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 4=53/Mechanical, 7=236/0-5-8, 5=107/Mechanical Max Horz 7=166(LC 8) Max Uplift4=-7(LC 6), 7=-19(LC 8), 5=-65(LC 8) Max Grav 4=55(LC 14), 7=236(LC 1), 5=107(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

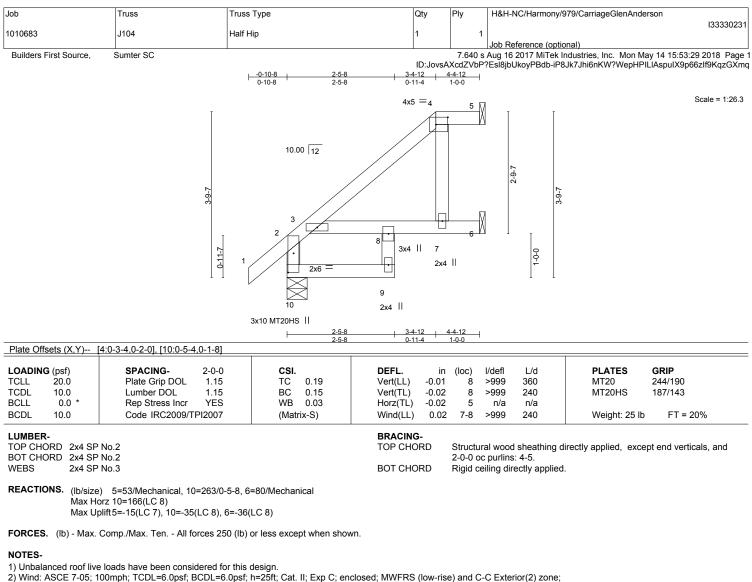
### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;
- end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 7, 5.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied, except end verticals, and





end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

Provide adequate drainage to prevent water ponding.

5) All plates are MT20 plates unless otherwise indicated

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

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8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 10, 6.

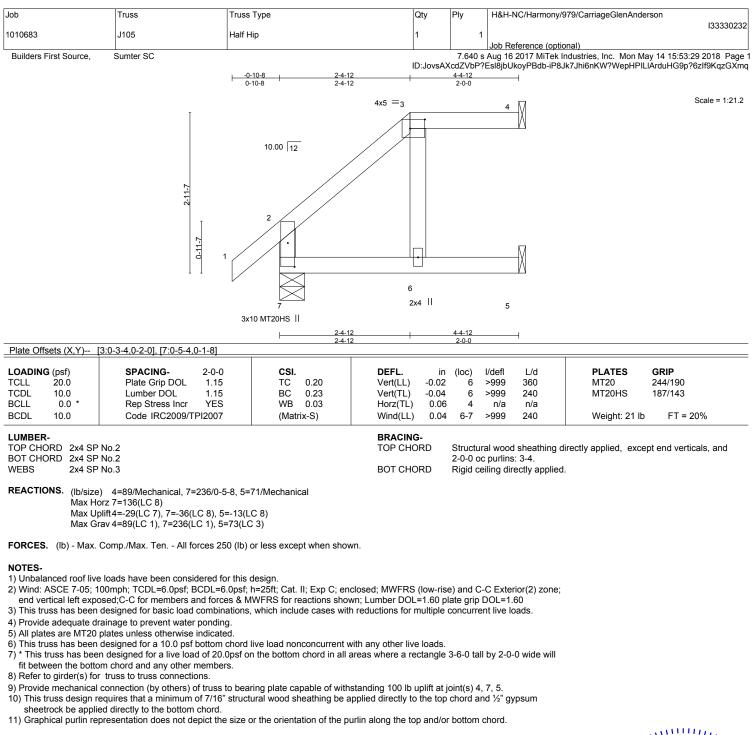
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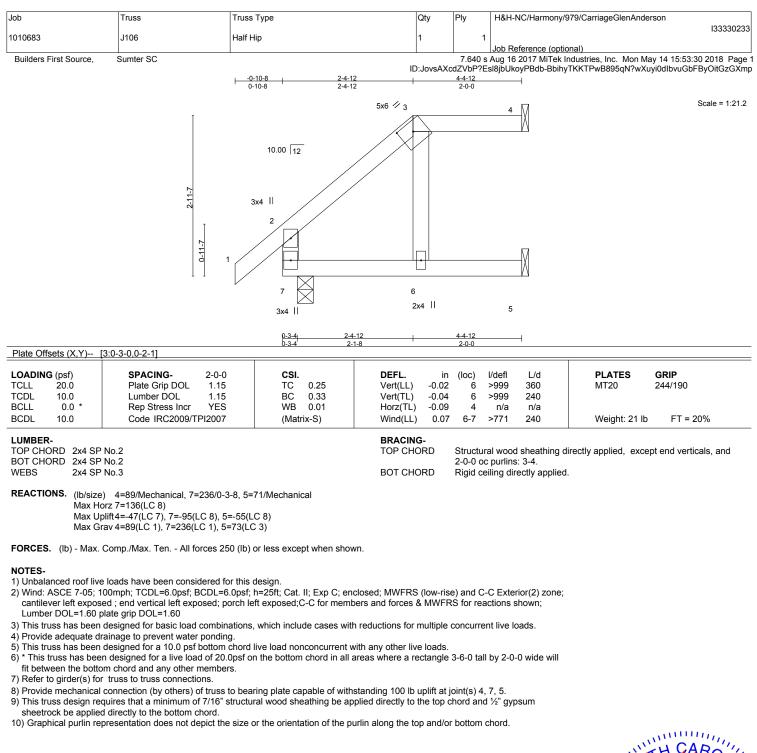
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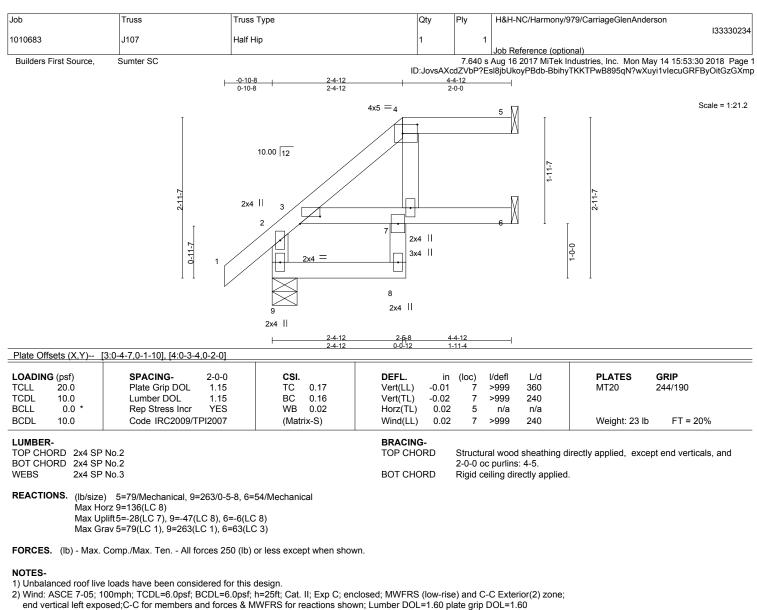






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3) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 9, 6.

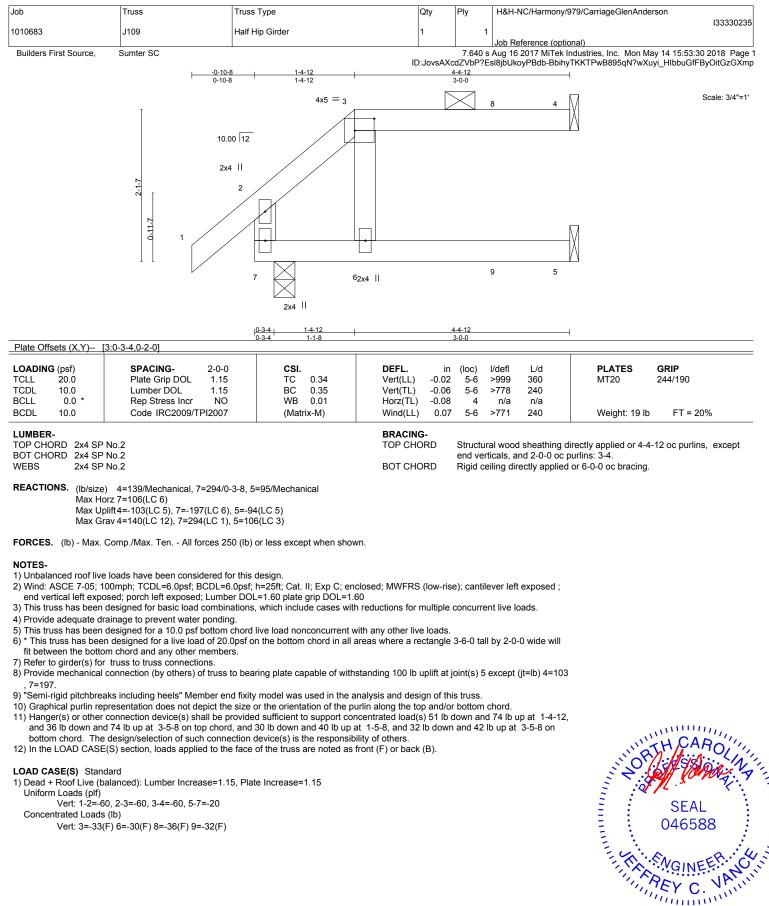
9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and ½" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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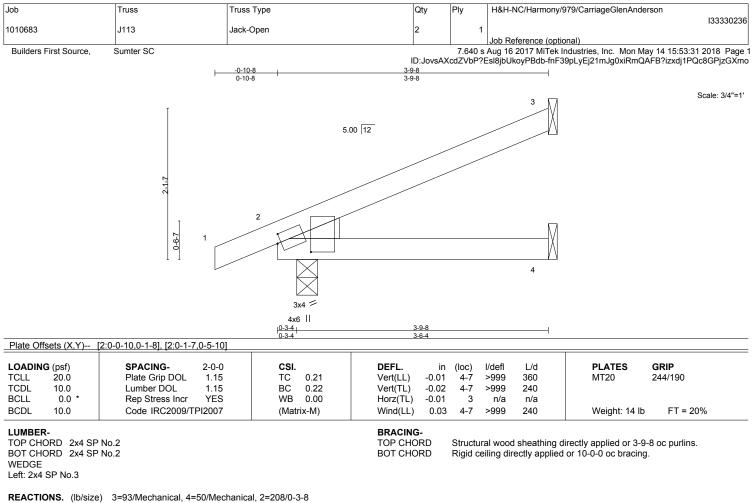
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Max Uplift3=50(LC 8) Max Grav 3=93(LC 1), 4=67(LC 3), 2=-102(LC 8) Max Grav 3=93(LC 1), 4=67(LC 3), 2=-208(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

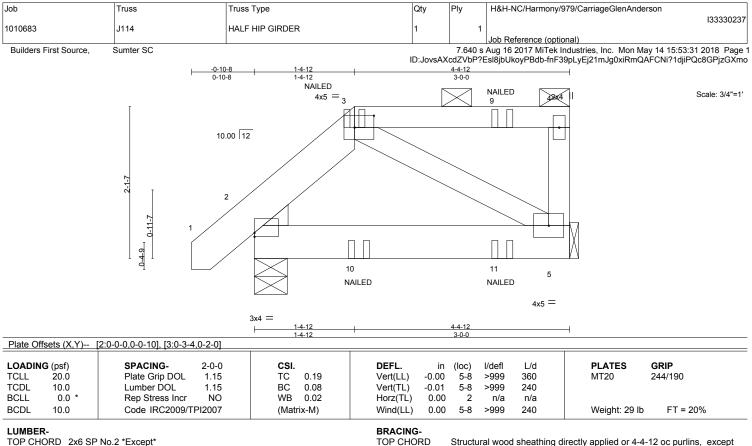
#### NOTES-

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will
- fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=102.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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BOT CHORD

end verticals, and 2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x6 SP No.2 \*Except\* 3-4: 2x4 SP No.2 BOT CHORD 2x6 SP No.2 2x4 SP No.2 WEBS WEDGE

Left: 2x4 SP No.3

REACTIONS. (lb/size) 2=238/0-5-8, 5=195/Mechanical Max Horz 2=82(LC 6) Max Uplift2=-82(LC 6), 5=-84(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail. 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

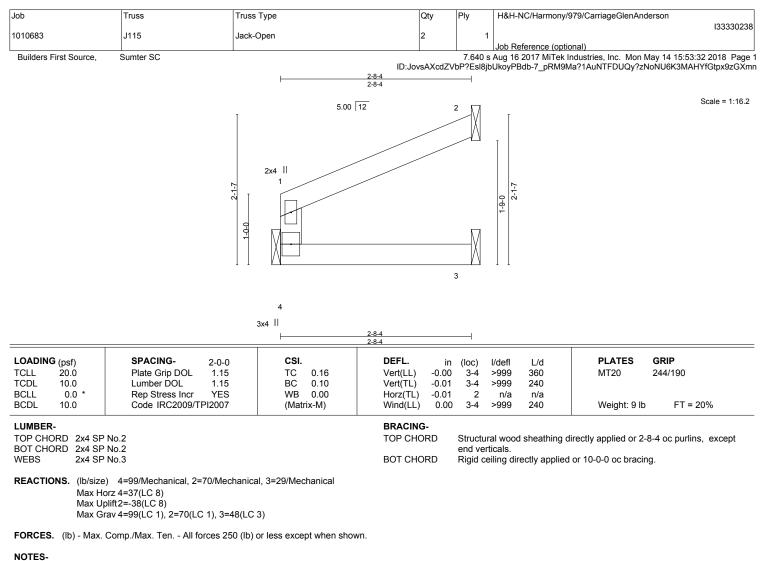
Vert: 1-3=-60, 3-4=-60, 5-6=-20

Concentrated Loads (Ib) Vert: 3=-10(B) 9=-16(B) 10=-9(B) 11=-11(B)



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Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

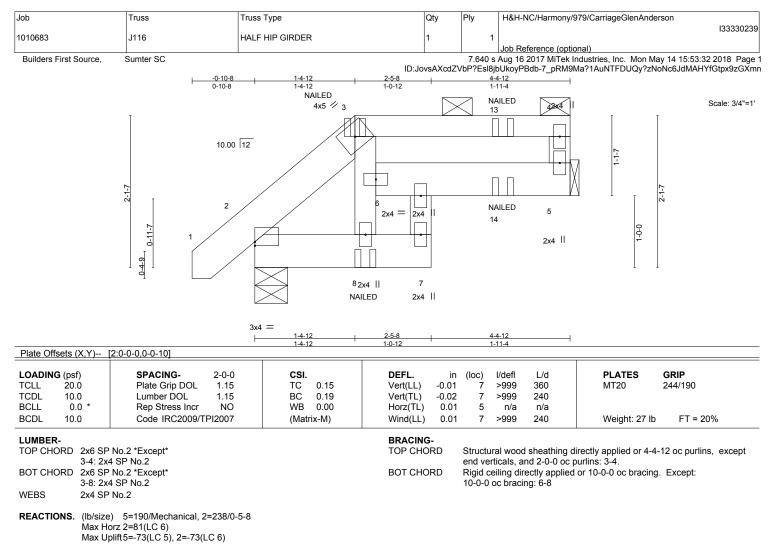
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

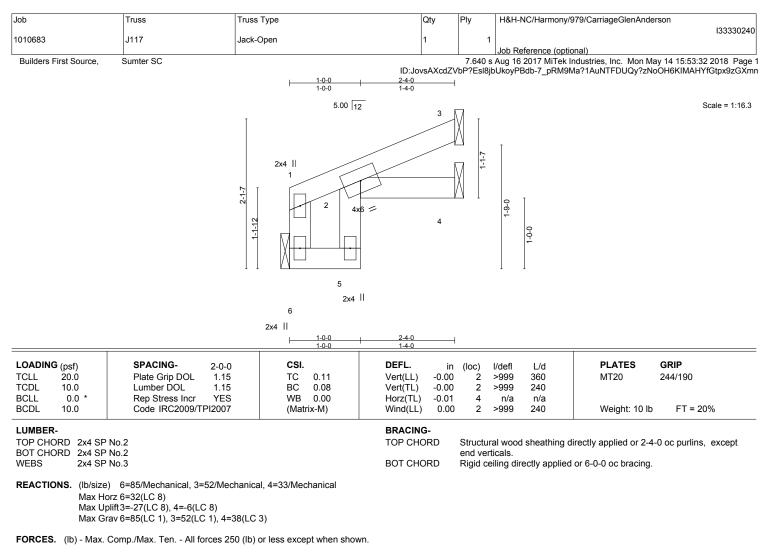
Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 8-10=-20, 7-8=-20, 5-6=-20

Concentrated Loads (lb)

Vert: 3=-1(F) 8=-5(F) 13=-3(F) 14=-15(F)



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## NOTES-

1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2)

zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4)\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will

fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

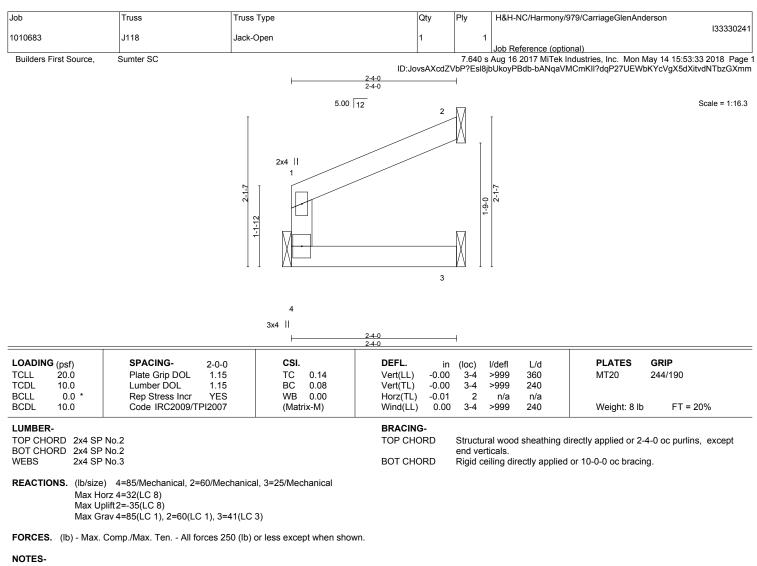
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



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 building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
 **NoISITPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



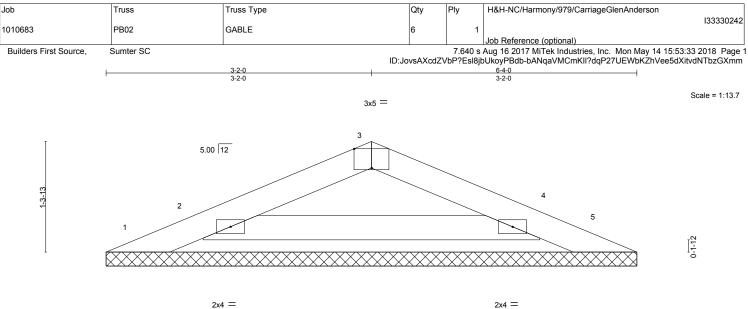


- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2)
- zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4)\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will
- fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
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2x4 =

#### Plate Offsets (X,Y)-- [3:0-2-8,Edge] LOADING (psf) SPACING-2-0-0 CSI DEFL in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.15 тс 0.07 Vert(LL) n/a n/a 999 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 BC 0.20 Vert(TL) n/a n/a 999 0.0 \* BCLL Rep Stress Incr YES WB 0.00 Horz(TL) 0.00 5 n/a n/a Code IRC2009/TPI2007 BCDL 10.0 (Matrix) Weight: 16 lb FT = 20%

6-4-0

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. All bearings 6-4-0.

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 2, 4 Max Grav All reactions 250 lb or less at joint(s) 1, 5, 2, 4

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 2, 4.

- 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
- 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign value to be only with with these contractions. This besign is based only upon parameters shown, and is to rain individual outdarg component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932



BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

<sup>(</sup>lb) - Max Horz 1=-15(LC 9)

