

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 1625535\_Jill\_RF Sturtz Homes

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E12549241 thru E12549290

My license renewal date for the state of North Carolina is December 31, 2018.

North Carolina COA: C-0844



December 26,2018

Gilbert, Eric

**IMPORTANT NOTE:** Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Sturtz Homes
1625535 JUL RE	Δ1	GABLE	1	1	E12549241
1020000_011_111		SADEL .		•	Job Reference (optional)

Builders FirstSource. Albemarle, NC 28001



Plate Offsets (X,Y)--

20.0

10.0

10.0

0.0

15.4/20.0

LOADING (psf)

TCLL (roof)

TCDL

BCLL

BCDL

Snow (Pf/Pg)

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2 *Except*		except end verticals.
	16-25: 2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
WEBS	2x4 SP No.3 *Except*		10-0-0 oc bracing: 16-25
	9-15,5-7,3-26: 2x4 SP No.2, 2-29,10-12: 2x6 SP No.2		
OTHERS	2x4 SP No.3		
REACTIONS. (lb) -	All bearings 20-9-0. Max Horz 29=231(LC 9) Max Uplift All uplift 100 lb or less at joint(s) 29 except 15=-156(LC 11), 26=-153(LC 10) Max Grav All reactions 250 lb or less at joint(s) 27, 28, 14, 13 except 15=41 17), 26=455(LC 16), 12=520(LC 1), 22=264(LC 1), 24=274(LC 16) 18=274(LC 17), 29=520(LC 1)	55(LC ), 20=264(LC 1),	

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 3-5=-410/104, 7-9=-410/104, 2-29=-311/249, 10-12=-311/250
- BOT CHORD 28-29=0/259, 27-28=0/259, 26-27=0/259, 14-15=0/259, 13-14=0/259, 12-13=0/259 WEBS 15-16=-375/242, 9-16=-343/294, 25-26=-375/243, 3-25=-343/292, 3-29=-348/32,
- 9-12=-347/28

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Corner(3) -1-2-9 to 1-9-7, Exterior(2) 1-9-7 to 10-4-8, Corner(3) 10-4-8 to 13-4-8, Exterior(2) 13-4-8 to 21-11-9 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 7) Gable requires continuous bottom chord bearing. 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Ceiling dead load (5.0 psf) on member(s). 3-5, 7-9, 5-30, 7-30; Wall dead load (5.0 psf) on member(s).9-16, 3-25

12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 15, 26, and 29. This connection is for uplift only and does not consider lateral forces





Job	Truss	Truss Type	Qty	Ply	Sturtz Homes				
					E12549241				
1625535_Jill_RF	A1	GABLE	1	1					
					Job Reference (optional)				
Builders FirstSource,	Albemarle, NC 28001		8	.220 s Nov	16 2018 MiTek Industries, Inc. Fri Dec 21 12:46:16 2018 Page 2				
		ID:4ImeeSjiwdRzCfsAnBfHyayIMmJ-KWvoOHrOGz0tNLfdp?gtkL43Wk3nL19AETsC3ky6ke5							

NOTES-

13) Attic room checked for L/360 deflection.



Job	Truss	Truss Type	Qty	Ply	Sturtz Homes	
		_			E1:	2549242
1625535_Jill_RF	A2	Common	4	1		
					Job Reference (optional)	

Builders FirstSource. Albemarle, NC 28001

8.220 s Nov 16 2018 MiTek Industries, Inc. Fri Dec 21 12:46:17 2018 Page 1 ID:4ImeeSjiwdRzCfsAnBfHyayIMmJ-oiSAbds01G8k?VEpNjB6GZc8K7Ce4PDKT7bmcAy6ke4

Scale = 1.78.2



		5-2-12	7-3-10	9-4-8	11-4-8	13-5-6	15-6-4	20-3-8	20 <sub>1</sub> 9 <sub>1</sub> 0
		5-2-12	2-0-14	2-0-14	2-0-0	2-0-14	2-0-14	4-9-4	0-5-8
Plate Offsets (X,Y)	[2:0-3-0,0-2-12], [10:0-3-0,0-2-12], [1	6:0-4-0,0-3-0]							

LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2009/TPI2007	CSI. TC 0.50 BC 0.99 WB 0.69 Matrix-MS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL	in -0.23 1 -0.48 1 ) 0.06 ) 0.14	(loc) 17-19 17-19 12 24	l/defl >999 >502 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 190 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP DSS 2x4 SP No.2 *E: 14-23: 2x4 SP N 2x4 SP No.3 *E: 9-13,5-7,3-24: 2	xcept* lo.3 xcept* x4 SP No.2, 2-25,10-12: 2x6 SP No.2		BRACING- TOP CHORD BOT CHORD	Structural except en Rigid ceili 3-7-0 oc b	l wood s nd vertica ing direc bracing:	heathir als. tly app 14-23	ng directly app lied or 10-0-0	blied or 6-0-0 oc purlins oc bracing. Except:	i,
REACTIONS.	(lb/size) 12=1 Max Horz 25=2	569/0-3-8, 25=1569/0-3-8 229(LC 9)								
FORCES. (Ib	) - Max. Comp./M	lax. Ten All forces 250 (lb) or less exce	ept when shown.							
TOP CHORD	2-3=-964/158,	3-5=-891/95, 5-6=-5/460, 6-7=-5/460, 7-	-9=-891/95, 9-10=-96	64/158,						
BOT CHORD	24-25=-1007/20 24-25=0/848, 2 12-13=0/848, 2 14-15=-1374/0	22-24=-29/894, 20-22=0/2723, 18-20=0/ 21-23=-1374/0, 19-21=-1374/0, 17-19=-` )	2723, 16-18=0/2723, 1843/0, 15-17=-1374	13-16=0/894, /0,						
WEBS	9-14=0/803, 5- 19-22=-750/4 9-12=-657/33	-26=-1514/120, 7-26=-1514/120, 3-23=0 3, 16-17=-750/34, 15-16=-255/0, 14-16=	/803, 22-23=0/1373, 0/1373, 3-25=-657/3	21-22=-255/0, 4,						

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-2-9 to 1-9-7, Interior(1) 1-9-7 to 10-4-8, Exterior(2) 10-4-8 to 13-4-8, Interior(1) 13-4-8 to 21-11-9 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (5.0 psf) on member(s). 3-5, 7-9, 5-26, 7-26; Wall dead load (5.0 psf) on member(s).9-14, 3-23

9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 21-23, 19-21, 17-19, 15-17, 14-15

10) Attic room checked for L/360 deflection.



818 Soundside Road

Edenton, NC 27932

🖊 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. ARXING - Verify design parameters and READ NOTES ON THIS AND INCLODED MITER REFERENCE PAGE MIT-14's rev. Invozoris beroke use. Design valif for use only with MiTeR's connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Qua** Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	Sturtz Homes	
	4.0					E12549243
1625535_JIII_RF	A3	COMMON	3	1	lob Reference (ontional)	

Builders FirstSource, Albemarle , NC 28001

8.220 s Nov 16 2018 MiTek Industries, Inc. Fri Dec 21 12:46:19 2018 Page 1 ID:4ImeeSjiwdRzCfsAnBfHyayIMmJ-I5aw0JtHZuOSEpOCU7DaM\_iUpxxBYG9dwR4tg2y6ke2

Scale = 1.78.2



5-2-12	7-3-10	9-4-8	11-4-8	13-5-6	<sub>1</sub> 15-6-4	20-3-8	20 <sub>1</sub> 9 <sub>1</sub> 0
5-2-12	2-0-14	2-0-14	2-0-0	2-0-14	2-0-14	4-9-4	0-5-8

LOADING (psf) TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	) 20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2009/TPI2007	CSI. TC 0.50 BC 0.79 WB 0.86 Matrix-MS	<b>DEFL.</b> Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (loc) -0.24 14-16 -0.49 16-18 0.06 11 ) 0.14 12	l/defl >999 >496 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 186 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x6 SP DSS 2x4 SP No.2 2x4 SP No.3 *E: 9-12,5-7,3-23: 2	xcept* x4 SP No.2, 2-24,10-11: 2x6 SP No.2		BRACING- TOP CHORD BOT CHORD	Structural woo except end ver Rigid ceiling di 4-2-0 oc bracir	d sheathi ticals. rectly app ng: 13-22	ng directly app blied or 10-0-0	blied or 6-0-0 oc purlins oc bracing. Except:	5,
REACTIONS.	(lb/size) 11=1 Max Horz 24=2	1479/0-3-8, 24=1572/0-3-8 254(LC 9)							
FORCES. (Ib) TOP CHORD BOT CHORD	- Max. Comp./M 2-3=-964/158, 2-24=-1007/20 23-24=0/851, 1 11-12=0/855, 1	lax. Ten All forces 250 (lb) or less exce 3-5=-893/95, 5-6=-10/464, 6-7=-6/467, 7 36, 10-11=-793/135 21-23=-53/893, 19-21=0/2736, 17-19=0/ 20-22=-1379/0, 18-20=-1379/0, 16-18=-1	ept when shown. 7-9=-894/97, 9-10=-8 2736, 15-17=0/2736, 1852/0, 14-16=-1405.	11/141, 12-15=0/916, ⁄0,					
WEBS	13-14=-1405/0 9-13=0/798, 5- 18-21=-759/4 9-11=-813/0	, -25=-1528/131, 7-25=-1528/131, 3-22=0 3, 15-16=-735/38, 14-15=-256/0, 13-15=	/808, 21-22=0/1380, 0/1387, 3-24=-662/3	20-21=-256/0, 3,					
NOTES-									

1) Unbalanced roof live loads have been considered for this design.

Plate Offsets (X,Y)-- [2:0-3-0,0-2-12], [15:0-4-0,0-3-0]

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-2-9 to 1-9-7, Interior(1) 1-9-7 to 10-4-8, Exterior(2) 10-4-8 to 13-4-8, Interior(1) 13-4-8 to 20-6-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (5.0 psf) on member(s). 3-5, 7-9, 5-25, 7-25; Wall dead load (5.0 psf) on member(s).9-13, 3-22

9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 20-22, 18-20, 16-18, 14-16, 13-14

10) Attic room checked for L/360 deflection.







LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	<b>CSI.</b> TC 0.41 BC 0.31 WB 0.00 Matrix-AS	DEFL. Vert(LL) -0 Vert(TL) -0 Horz(TL) 0 Wind(LL) 0	in (loc) 0.04 4-7 0.11 4-7 0.01 2 0.03 4-7	l/defl >999 >609 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 22 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

REACTIONS. (lb/size) 2=309/0-3-0, 4=209/0-1-8 Max Horz 2=67(LC 8) Max Uplift 2=-41(LC 8), 4=-12(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 5-5-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.





Plate Offsets (X,Y) [5:0-2-7,	0-1-7]								
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2009/TPI2007	CSI. TC 0.79 BC 0.67 WB 0.04 Matrix-MP	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in -0.11 -0.26 0.03 0.08	(loc) 6 6-9 2 6	l/defl >618 >255 n/a >858	L/d 360 240 n/a 240	PLATES MT20 Weight: 23 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-		BR							

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.1 2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=474/0-3-0, 5=621/0-1-8 Max Horz 2=67(LC 6)

Max Uplift 2=-58(LC 6), 5=-52(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 187 lb down and 19 lb up at 2-0-12, and 189 lb down and 20 lb up at 4-0-12, and 201 lb down and 17 lb up at 5-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-4=-60, 5-7=-20

Concentrated Loads (lb) Vert: 5=-201 11=-186 12=-189



Structural wood sheathing directly applied or 5-7-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



🖊 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only design parameters and READ NOTES ON TIPS ON MICLODED MITER REPERIENCE PAGE mit-14/3 at 900, 1002/015 BEPORE 052. Design valid for use only with MITeR works connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



## LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.3

BRACING-TOP CHORD

TOP CHORD Structural wood sheathing directly applied or 3-7-8 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=234/0-3-0, 4=124/0-1-8 Max Horz 2=49(LC 8) Max Uplift 2=-44(LC 8), 4=-4(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 3-5-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.







		2-9-1 2-9-1	4-0-0 1-2-15	5-7-8 1-7-8		
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	<b>CSI.</b> TC 0.33 BC 0.57 WB 0.04 Matrix-AS	DEFL.         in         (lo           Vert(LL)         -0.06         6           Vert(TL)         -0.16         6           Horz(TL)         0.02         Wind(LL)	c) l/defl L/d -9 >999 360 -9 >405 240 2 n/a n/a -9 >999 240	PLATES MT20 Weight: 23 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

2-0-0 oc purlins: 3-4.

Rigid ceiling directly applied.

## LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=309/0-3-0, 5=209/0-1-8 Max Horz 2=54(LC 8) Max Uplift 2=-45(LC 8), 5=-8(LC 8) Max Grav 2=386(LC 16), 5=209(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 4-0-0, Exterior(2) 4-0-0 to 5-5-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied, except end verticals, and









	1	2-0-0		3-0-1	1			5-7-8		1	
		2-0-0		1-0-1				2-7-7			
Plate Offsets (X,Y) [2	0-0-0,0-0-14]										
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 20.4/20.0 TCDL 10.0	<b>SPACING</b> Plate Grip Lumber D	- 2-0-0 DOL 1.15 OL 1.15	CSI. TC 0.2 BC 0.6	26 65	<b>DEFL.</b> Vert(LL) Vert(TL)	in -0.06 -0.18	(loc) 5-6 5-6	l/defl >999 >370	L/d 360 240	PLATES MT20	<b>GRIP</b> 244/190
BCLL 0.0	* Rep Stres Code IRC	s Incr NO 2009/TPI2007	WB 0.0 Matrix-MI	03 P	Horz(TL) Wind(LL)	0.02 0.04	2 5-6	n/a >999	n/a 240	Weight: 21 lb	FT = 20%

BU		10.0				1
LU	MBER-		BF	RACING-		
TC	P CHORD	2x4 SP No.2	TC	OP CHORD	Structural wood sheathing directly appl	lied or 5-7-8 oc purlins,
BC	DT CHORD	2x4 SP No.2			except end verticals, and 2-0-0 oc purli	ins: 3-4.
WE	EBS	2x4 SP No.3	BC	OT CHORD	Rigid ceiling directly applied or 10-0-0 of	oc bracing.

REACTIONS. (lb/size) 2=306/0-3-0, 5=206/0-1-8

Max Horz 2=36(LC 4) Max Uplift 2=-49(LC 4), 5=-6(LC 5)

Max Grav 2=306(LC 1), 5=207(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This
  connection is for uplift only and does not consider lateral forces.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb)

Vert: 6=3(B) 11=3(B)

SEAL 036322 December 26,2018





TOP CHORD

BOT CHORD

LU	MB	ER-
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TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 3=40/Mechanical, 2=186/0-3-0, 4=13/Mechanical Max Horz 2=36(LC 6) Max Uplift 3=-8(LC 6), 2=-50(LC 6)

Max Grav 3=40(LC 1), 2=186(LC 1), 4=33(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 1-11-14 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 2-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





Scale = 1:35.1



L	9-7-8			19-3-0				
	9-7-8	1			9-7-8			1
Plate Offsets (X,Y) [8:0-4-0,	0-3-0]							
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2009/TPI2007	<b>CSI.</b> TC 0.28 BC 0.82 WB 0.20 Matrix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (I -0.13 8 -0.33 8 0.04 0.24 8	loc) l/defl 3-23 >999 3-23 >691 6 n/a 3-23 >967	L/d 360 240 n/a 240	PLATES MT20 Weight: 103 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3, Right: 2x4 SP REACTIONS. (lb/size) 2=88 Max Horz 2=56 Max Uplift 2=-2	P No.3 50/0-3-0, 6=850/0-3-0 5(LC 10) 37(LC 10), 6=-237(LC 11)	BI TC BC	RACING- DP CHORD DT CHORD	Structural v Rigid ceilin	wood sheath g directly ap	ing directly ap plied.	plied.	
FORCES.         (lb) - Max. Comp.//\/           TOP CHORD         2-3=-1382/100           BOT CHORD         2-8=-913/1220           WEBS         4-8=-493/520,	1ax. Ten All forces 250 (lb) or less exc 70, 3-4≕-1046/918, 4-5≕-1046/918, 5-6≡ 3, 6-8≕-921/1223 , 5-8≕-367/285, 3-8≕-367/285	ept when shown. -1382/1070						
NOTES- 1) Unbalanced roof live loads ha 2) Wind: ASCE 7-05; 100mph; -1-4-0 to 1-8-0, Exterior(2) 1- exposed ; porch left and right DOL=1.60	ave been considered for this design. TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat 8-0 to 9-7-8, Corner(3) 9-7-8 to 12-7-8, E exposed;C-C for members and forces &	. II; Exp B; Enclosed; MW Exterior(2) 12-7-8 to 20-7 & MWFRS for reactions sl	/FRS (low-rise) ai -0 zone; cantileve hown; Lumber DC	nd C-C Cor er left and ri DL=1.60 pla	rner(3) ight ate grip			

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







I	9-7-8	1			19-3-0			
I	9-7-8	I			9-7-8			1
Plate Offsets (X,Y) [8:0-4-0,0	0-3-0]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.28 BC 0.82 WB 0.20 Matrix-AS	DEFL.           Vert(LL)         -0.1           Vert(TL)         -0.3           Horz(TL)         0.0           Wind(LL)         0.2	in (loc)  3 8-11  3 8-11  4 6  22 8-14	l/defl >999 >691 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 87 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3, Right: 2x4 SP	2 No.3	BRACI TOP C BOT C	NG- HORD Struct HORD Rigid	tural wood ceiling dire	sheathing ectly appli	g directly applie ied.	id.	

REACTIONS. (lb/size) 2=850/0-3-0, 6=850/0-3-0 Max Horz 2=56(LC 10) Max Uplift 2=-237(LC 10), 6=-237(LC 11)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1382/889, 3-4=-1046/790, 4-5=-1046/790, 5-6=-1382/889
- BOT CHORD 2-8=-759/1223, 6-8=-776/1223
- WEBS 4-8=-488/520, 5-8=-367/193, 3-8=-367/193

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 9-7-8, Exterior(2) 9-7-8 to 12-7-8, Interior(1) 12-7-8 to 20-7-0 zone; cantilever left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







		9-7-8					18-7-0		
Plate Offsets (2	X,Y) [7:0-3-8,I	9-7-8 Edge], [8:0-4-0,0-3-0]					8-11-8		
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 15.4/20.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	<b>CSI.</b> TC 0.26 BC 0.75 WB 0.21 Matrix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (l -0.14 8 -0.36 8 0.04 0.24 8	loc) l/def -15 >999 -15 >61 7 n/a -15 >918	L/d 360 240 a n/a 3 240	PLATES MT20 Weight: 88 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SP No SLIDER	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 o.3 Right 2x6 SP No	o.2 2-6-0	1	BRACING- TOP CHORD BOT CHORD	Structural v Rigid ceiling	vood sheat g directly a	ning directly applied.	pplied.	
REACTIONS.	(Ib/size) 7=74 Max Horz 2=67 Max Uplift 7=-1	40/Mechanical, 2=826/0-3-0 7(LC 10) 87(LC 11), 2=-232(LC 10)							
FORCES. (Ib	) - Max. Comp./M	lax. Ten All forces 250 (lb) or less exc	ept when shown.						

TOP CHORD 2-3=-1326/865, 3-4=-985/751, 4-5=-973/758, 5-7=-1167/814

BOT CHORD 2-8=-777/1172, 7-8=-698/1047

WEBS 3-8=-371/200, 4-8=-466/475, 5-8=-262/125

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 9-7-8, Exterior(2) 9-7-8 to 12-7-8, Interior(1) 12-7-8 to 18-7-0 zone; cantilever left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=187, 2=232.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







LOADING         (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeUBC2000(TD)2007	CSI. TC 0.12 BC 0.04 WB 0.00	DEFL. Vert(LL) 0 Vert(TL) -0 Horz(TL) -0	in (loc) 0.00 7 0.00 4-7 0.00 3	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2009/1P12007	Matrix-IMP		0.00 7	>999	240	weight: 9 lb	FT = 20%

## LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=40/Mechanical, 2=186/0-3-8, 4=12/Mechanical Max Horz 2=54(LC 10) Max Uplift 3=-11(LC 10), 2=-47(LC 10) Max Grav 3=40(LC 1), 2=186(LC 1), 4=33(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 1-11-14 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.







		2-0-0			2-0-0			
Plate Offsets (X,Y) [2:0-0-12	2,0-1-8]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 20.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2009/TPI2007	CSI. TC 0.20 BC 0.32 WB 0.02 Matrix-MP	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (loc) -0.02 6 -0.04 6 0.01 2 0.01 6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 18 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-	·	BR	ACING-	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				

TOP CHORD

BOT CHORD

LU	MB	ER-
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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=247/0-3-8, 5=139/0-1-8 Max Horz 2=55(LC 8)

Max Uplift 2=-43(LC 8), 5=-4(LC 7)

Max Grav 2=318(LC 14), 5=170(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

## Continued on page

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and KEAD NOTES ON THIS AND INCLODED INTER REFERENCE FACE INTERVISED. INCLOSE DEL ONE OCL Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





ſ	Job	Truss	Truss Type	Qty	Ply	Sturtz Homes
						E12549254
ľ	1625535_Jill_RF	D2	Half Hip Girder	2	1	
L						Job Reference (optional)
	Builders FirstSource, A	lbemarle, NC 28001		8	.220 s Nov	/ 16 2018 MiTek Industries, Inc. Fri Dec 21 12:46:28 2018 Page 2
			ID:4lme	eSjiwdRzO	fsAnBfHy	aylMmJqdKvO_wRfXApBawWWuhDta4EZ8j9YRx?KmrU1y6kdv

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-7=-20

Concentrated Loads (lb)

Vert: 6=3(B)





	1		4-0-0		I		
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	<b>CSI.</b> TC 0.20 BC 0.15 WB 0.03 Matrix-MP	DEFL.         in           Vert(LL)         -0.01           Vert(TL)         -0.03           Horz(TL)         0.00           Wind(LL)         0.01	(loc) l/defl 4-7 >999 4-7 >999 2 n/a 4-7 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 19 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3

REACTIONS. (lb/size) 2=248/0-3-8, 4=140/0-1-8 Max Horz 2=79(LC 10) Max Uplift 2=-36(LC 10), 4=-13(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 3-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 4. This
  connection is for uplift only and does not consider lateral forces.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 4-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Sturtz Homes	
					E12549	9256
1625535_Jill_RF	E1	HIP GIRDER	1	2		
				<b>_</b>	Job Reference (optional)	
Builders FirstSource,	Albemarle, NC 28001		8	3.220 s No	v 16 2018 MiTek Industries, Inc. Fri Dec 21 12:46:34 2018 Page 2	2
		ID:4lm	eeSjiwdRz	CfsAnBfH	yaylMmJ-pb9R3h1VHKX614sn?5S8p_a_0UZ36qNGD9hhy6kdp	)

## NOTES-

13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 150 lb down and 16 lb up at 2-0-12, 120 lb down and 25 lb up at 4-0-12, 120 lb down and 25 lb up at 6-0-12, 120 lb down and 25 lb up at 8-0-12, 120 lb down and 25 lb up at 10-0-12, 120 lb down and 25 lb up at 12-0-12, 120 lb down

and 25 lb up at 14-0-12, 120 lb down and 25 lb up at 16-0-12, 120 lb down and 25 lb up at 18-0-12, 120 lb down and 25 lb up at 20-0-12, 120 lb down and 25 lb up at 22-0-12, 120 lb down and 25 lb up at 24-0-12, 120 lb down and 25 lb up at 26-0-12, and 120 lb down and 25 lb up at 27-11-12, and 150 lb down and 16 lb up at 29-11-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-9=-60, 9-11=-60, 20-23=-20

Concentrated Loads (lb)

Vert: 16=-120 19=-120 12=-120 30=-119 31=-120 32=-120 33=-120 34=-120 35=-120 35=-120 36=-120 37=-120 38=-120 39=-120 40=-120 41=-119





6-	0-0 12-7-9	19-4-15	26-0-8	32-0-8
6-	0-0 6-7-9	6-9-5	6-7-9	6-0-0
Plate Offsets (X,Y) [2:0-0-0,	0-1-9], [8:0-0-0,0-1-9]			
LOADING (psf) TCLL (roof) 20.0	<b>SPACING-</b> 2-0-0 Plate Grip DOL 1.15	CSI. DEFL. TC 0.59 Vert(LL)	in (loc) l/defl L/d -0.31 12-14 >999 360	PLATES GRIP MT20 244/190
Snow (Pf/Pg) 20.4/20.0	Lumber DOI 115	BC 0.98 Vert(TL)	-0.65 12-14 >591 240	MT18HS 244/190
TCDL 10.0	Bon Stroop Inor VES			WITTONS 244/190
BCLL 0.0 *				
BCDL 10.0	Code IRC2009/1P12007	Matrix-AS Wind(LL)	0.13 12-14 >999 240	weight: 153 lb $FT = 20\%$
LUMBER- TOP CHORD 2x4 SP No.2 *E 3-5,5-7: 2x4 SP BOT CHORD 2x4 SP No.1 *E 11-13: 2x4 SP I WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3, Right: 2x4 SP	xcept* 2400F 2.0E xcept* No.2 P No.3	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly a 2-0-0 oc purlins (3-4-12 max.): 3-7. Rigid ceiling directly applied.	ıpplied, except
REACTIONS. (Ib/size) 2=13 Max Horz 2=-4 Max Grav 2=1-	362/0-3-8, 8=1362/0-3-8 !9(LC 11) 411(LC 19), 8=1411(LC 19)			
FORCES.         (lb) - Max. Comp.//i           TOP CHORD         2-3=-2657/32           BOT CHORD         2-15=-191/23           WEBS         3-14=-180/18	/lax. Ten All forces 250 (lb) or less exc 3, 3-4=-4025/461, 4-6=-4025/461, 6-7=- 43, 14-15=-194/2340, 12-14=-333/4022, 64, 4-14=-656/159, 6-12=-658/160, 7-12	ept when shown. 4028/462, 7-8=-2657/322 10-12=-207/2339, 8-10=-203/2342 =-180/1867		
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads h</li> <li>2) Wind: ASCE 7-05; 100mph;</li> <li>-1-4-0 to 1-8-0, Interior(1) 1-6</li> <li>Interior(1) 30-3-7 to 33-4-8 zi</li> <li>Lumber DOL=1.60 plate grip</li> <li>3) TCLL: ASCE 7-05; Pr=20.0 p</li> <li>roof snow: Lumber DOL=1.1:</li> <li>governs. Rain surcharge api</li> <li>4) Unbalanced snow loads have</li> <li>5) This truss has been designed</li> <li>non-concurrent with other live</li> <li>6) Provide adequate drainage to</li> <li>7) All plates are MT20 plates ur</li> <li>8) This truss has been designed</li> <li>9) * This truss has been designed</li> <li>9) * This truss has been designed</li> <li>10) This truss design requires t</li> <li>sheetrock be applied directi</li> <li>11) Graphical purlin representa</li> </ul>	ave been considered for this design. TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat 3-0 to 6-0-0, Exterior(2) 6-0-0 to 10-2-15 DOL=1.60 usf (roof live load: Lumber DOL=1.15 Plate 5 Plate DOL=1.15); Category II; Exp B; F blied to all exposed surfaces with slopes a been considered for this design. d for greater of min roof live load of 12.0 a loads. b prevent water ponding. Ness otherwise indicated. d for a live load of 20.0psf on the bottor ord and any other members. hat a minimum of 7/16" structural wood s y to the bottom chord. tion does not depict the size or the orien	II; Exp B; Enclosed; MWFRS (low-rise) a , Interior(1) 10-2-15 to 26-0-8, Exterior(2) C for members and forces & MWFRS for the DOL=1.15); Pg=20.0 psf (ground snow Partially Exp.; Ct=1.10, Lu=50-0-0; Min. fla less than 0.500/12 in accordance with IB psf or 1.00 times flat roof load of 15.4 psf proconcurrent with any other live loads. In chord in all areas where a rectangle 3-6 sheathing be applied directly to the top ch tation of the purlin along the top and/or bo	and C-C Exterior(2) 26-0-8 to 30-3-7, reactions shown; reactions shown; r); Pf=20.4 psf (flat at roof snow load C 1608.3.4. on overhangs S-0 tall by 2-0-0 wide ord and 1/2" gypsum ottom chord.	SEAL 036322

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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L	8-0-0	13-3-9		18-8-	-15	24-0-8	5		32-0-8	
	8-0-0	5-3-9	1	5-5-	-5	5-3-9		1	8-0-0	·
Plate Offsets (X,Y) [2:0-0-0	0,0-1-13], [10:0-0-0,0-0-13]								T	
LOADING         (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2009/TP	2-0-0 1.15 1.15 YES 12007	<b>CSI.</b> TC BC WB Matrix	0.56 0.82 0.42 -AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (lo -0.20 14- -0.46 14- 0.14 0.10	bc) I/de 16 >99 16 >84 10 n/ 16 >99	fl L/d 9 360 0 240 ′a n/a 9 240	PLATES MT20 Weight: 169 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-				BR	ACING-					
TOP CHORD 2x4 SP No.2				то	PCHORD	Structural w	ood shea	thina direct	ly applied, except	
BOT CHORD 2x4 SP No.2						2-0-0 oc pur	rlins (2-10	-13 max.):	4-8.	
WEBS 2x4 SP No.3				BO	T CHORD	Rigid ceiling	directly a	applied.		
WEDGE										
Left: 2x4 SP No.3, Right: 2x4 S	SP No.3									
REACTIONS. (Ib/size) 2= Max Horz 2= Max Grav 2=	1362/0-3-8, 10=1362/0-3-8 59(LC 10) 1371(LC 20), 10=1371(LC 2	0)								
FORCES. (lb) - Max. Comp./ TOP CHORD 2-3=-2313/3! 8-92205/3	Max. Ten All forces 250 () 58, 3-4=-2295/319, 4-5=-28 19, 9-10=-2313/358	b) or less excep 62/390, 5-7=-286	t when sho 60/389, 7-8	own. 3=-2864/390,						
BOT CHORD 2-17=-242/2	)10 16-17=-151/2064 14-1	6=-233/2861 12	2-14=-165/	2064 10-12=	-256/2010					
WEBS 3-17=-263/10	06, 4-17=0/267, 4-16=-120/	1012, 5-16=-516	/132, 7-14	=-516/134,	200,2010					
8-14=-121/10	014, 8-12=0/267, 9-12=-263	/106								
NOTES- 1) Unbalanced roof live loads I 2) Wind: ASCE 7-05; 100mph; -1-4-0 to 1-8-0, Interior(1) 14 Interior(1) 28-1-7 to 33-4-8 a; Lumber DOI =1 60 plate gri	nave been considered for th TCDL=6.0psf; BCDL=6.0p; 8-0 to 8-0-0, Exterior(2) 8-0 zone; cantilever left and righ DCl = 1 60	is design. sf; h=25ft; Cat. II I-0 to 12-2-15, In It exposed ;C-C	l; Exp B; En hterior(1) 12 for membe	nclosed; MW 2-2-15 to 24- ers and forces	FRS (low-rise) a 0-8, Exterior(2) a & MWFRS for	and C-C Exte 24-0-8 to 28- reactions sho	erior(2) 1-7, own;			
3) TCLL: ASCE 7-05; Pr=20.0	psf (roof live load: Lumber	DOL=1.15 Plate	DOL=1.15	); Pg=20.0 ps	sf (ground snow	); Pf=20.4 ps	f (flat		W'TH CAR	014

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







F		10-0-0	16-0-4	22-0-8			32-0-8			
Plate Offsets (X,	,Y) [2:0-0-0,0	D-1-13], [4:0-4-8,0-1-12], [6:0-4-8,0-1-12]	, [8:0-0-0,0-1-13]	0-0-4			10-0-0			
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 2 TCDL BCLL BCDL	20.0 0.4/20.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.73 BC 0.94 WB 0.33 Matrix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (loc) -0.18 14-17 -0.49 14-17 0.12 8 0.08 10-12	l/defl L/d >999 360 >790 240 n/a n/a >999 240	PLATES MT20 Weight: 166 lb	<b>GRIP</b> 244/190 FT = 20%		
LUMBER- TOP CHORD 3 BOT CHORD 3 WEBS 3 WEDGE Left: 2x4 SP No.	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 3, Right: 2x4 SF	P No.3	E	BRACING- TOP CHORD BOT CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling dire	sheathing direct (3-1-15 max.): 4 actly applied.	ly applied, except -6.			
REACTIONS.	(lb/size) 2=13 Max Horz 2=68 Max Uplift 2=-30 Max Grav 2=14	862/0-3-8, 8=1362/0-3-8 6(LC 10) (LC 10), 8=-3(LC 11) 88(LC 20), 8=1488(LC 20)								
FORCES. (lb) TOP CHORD	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         FOP CHORD       2-3=-2358/360, 3-4=-2002/306, 4-5=-2195/359, 5-6=-2195/359, 6-7=-2002/306, 7-8=-2358/359         FOP CHORD       2-14=-2358/359         GOT CHORD       2-14=-2368/359									
WEBS	3-14=-418/138 7-10=-418/138	8, 4-14=0/383, 4-12=-91/561, 5-12=-625/	146, 6-12=-91/561, 6-1	0=0/383,						
NOTES- 1) Unbalanced r 2) Wind: ASCE -1-4-0 to 1-8- Interior(1) 26- Lumber DOL= 3) TCLL: ASCE roof snow: Lu governs. Rai 4) Unbalanced s 5) This truss has 6) Provide adeq 7) This truss has 8) * This truss has 8) * This truss has 11) Graphical pu	oof live loads ha 7-05; 100mph; T 0, Interior(1) 1-8 3-7 to 33-4-8 zo =1.60 plate grip l 7-05; Pr=20.0 ps mber DOL=1.15 n surcharge app now loads have s been designed nt with other live uate drainage to s been designed as been designed as been designed n the bottom cho nanical connection esign requires the e applied directly urlin representation	Ave been considered for this design. CDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. -0 to 10-0-0, Exterior(2) 10-0-0 to 14-2-1 ne; cantilever left and right exposed; C-0 DOL=1.60 sf (roof live load: Lumber DOL=1.15 Plate i Plate DOL=1.15); Category II; Exp B; P lied to all exposed surfaces with slopes been considered for this design. for greater of min roof live load of 12.0 p loads. prevent water ponding. for a 10.0 psf bottom chord live load no ad for a live load of 20.0psf on the bottom ord and any other members. on (by others) of truss to bearing plate cat at a minimum of 7/16" structural wood s y to the bottom chord. ion does not depict the size or the orient	II; Exp B; Enclosed; MI 5, Interior(1) 14-2-15 to C for members and forc e DOL=1.15); Pg=20.0 artially Exp.; Ct=1.10, L ess than 0.500/12 in ac osf or 1.00 times flat roc n concurrent with any ot a chord in all areas whe upable of withstanding 1 heathing be applied dire ation of the purlin along	WFRS (low-rise) ; o 22-0-8, Exterior( es & MWFRS for psf (ground snow u=50-0-0; Min. fit ccordance with IB of load of 15.4 psf her live loads. re a rectangle 3-6 100 lb uplift at join ectly to the top ch the top and/or bo	and C-C Exterior( 2) 22-0-8 to 26-3 reactions shown; -); Pf=20.4 psf (fla at roof snow load C 1608.3.4. on overhangs -0 tall by 2-0-0 w t(s) 2, 8. ord and 1/2" gyps	2) -7, at ide sum	SEAL 03632	2 P. Human		

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6-0-1	5 <u>12-0-0</u> 5 5-11-1	16-0-4	20-0-8	25-11-9		32-0-8	
Plate Offsets (X,Y) [2:0-0-0,	0-1-5], [4:0-4-8,0-1-12], [6:0-4-8,0-1-12],	[8:Edge,0-1-5]				0 0 10	
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2009/TPI2007	CSI. TC 0.65 BC 0.79 WB 0.52 Matrix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (loc) l/defl -0.13 15-16 >999 -0.33 15-16 >999 0.13 8 n/a 0.07 13 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 181 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3, Right: 2x4 SI	P No.3	BI TC BC	RACING- OP CHORD OT CHORD	Structural wood sheathir 2-0-0 oc purlins (4-2-9 m Rigid ceiling directly app	g directly ap ax.): 4-6. ied.	pplied, except	
REACTIONS. (Ib/size) 2=13 Max Horz 2=-7 Max Uplift 2=-1 Max Grav 2=1!	362/0-3-8, 8=1362/0-3-8 ′8(LC 11) /4(LC 10), 8=-14(LC 11) 550(LC 20), 8=1550(LC 20)						
FORCES. (lb) - Max. Comp.//V TOP CHORD 2-3=-2600/308 7-8=-2600/308 BOT CHORD 2-16=-184/22° 8-10=-197/22° WEBS 3-15=-621/11° 7-11=-621/11°	Aax. Ten All forces 250 (lb) or less exc 5, 3-4=-1984/308, 4-5=-1713/322, 5-6=- 5 15, 15-16=-184/2215, 13-15=-88/1664, 1 15 1, 4-15=0/427, 4-13=-50/311, 5-13=-412 1	ept when shown. 1713/322, 6-7=-1984/308 1-13=-101/1664, 10-11= /89, 6-13=-50/311, 6-11=	3, =-197/2215, =0/427,				
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads ha</li> <li>2) Wind: ASCE 7-05; 100mph; 1 -1-4-0 to 1-8-0, Interior(1) 1-8 Interior(1) 24-3-7 to 33-4-8 zc Lumber DOL=1.60 plate grip</li> <li>3) TCLL: ASCE 7-05; Pr=20.0 p roof snow: Lumber DOL=1.16 governs. Rain surcharge app;</li> <li>4) Unbalanced snow loads have</li> <li>5) This truss has been designed:</li> <li>6) Provide adequate drainage tx</li> <li>7) This truss has been designed;</li> <li>8) This truss has been designed;</li> <li>8) This truss has been designed;</li> <li>9) Trovide adequate drainage tx</li> <li>10) This truss design requires tf sheetrock be applied direct1</li> <li>11) Graphical purlin representar</li> </ul>	ave been considered for this design. TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat 3-0 to 12-0-0, Exterior(2) 12-0-0 to 16-0- one; cantilever left and right exposed ;C- DOL=1.60 sof (roof live load: Lumber DOL=1.15 Plate 5 Plate DOL=1.15); Category II; Exp B; F blied to all exposed surfaces with slopes a been considered for this design. d for greater of min roof live load of 12.0 e loads. b prevent water ponding. d for a 10.0 psf bottom chord live load no ed for a live load of 20.0psf on the bottor lord and any other members. on (by others) of truss to bearing plate c hat a minimum of 7/16" structural wood s y to the bottom chord. tion does not depict the size or the orien	. II; Exp B; Enclosed; MV 4, Interior(1) 16-0-4 to 20 C for members and force te DOL=1.15); Pg=20.0 p Partially Exp.; Ct=1.10, Lu less than 0.500/12 in acc psf or 1.00 times flat roof enconcurrent with any oth n chord in all areas wher apable of withstanding 10 sheathing be applied dire tation of the purlin along	WFRS (low-rise) a )-0-8, Exterior(2) as & MWFRS for psf (ground snow u=50-0-0; Min. fla cordance with IB f load of 15.4 psf her live loads. re a rectangle 3-6 00 lb uplift at join her top and/or bo	and C-C Exterior(2) 20-0-8 to 24-3-7, reactions shown; ); Pf=20.4 psf (flat at roof snow load C 1608.3.4. on overhangs 6-0 tall by 2-0-0 wide t(s) 2, 8. ord and 1/2" gypsum ottom chord.	Wannah	SEAL 036322 A. GINER December 26,20	BHUILING 18





		7-0-15	14-0-0	18-0-8	24-9-0		31-9-0	
Plate Offsets (2	X,Y) [2:0-0-0,0	7-0-15 D-1-5], [5:0-4-8,0-1-12], [9:0-3-13,	0-0-5]	4-0-8	6-8-8		7-0-0	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 20.0 20.4/20.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.89 BC 0.83 WB 0.89 Matrix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (loc) // -0.16 13-14 > -0.38 13-14 > 0.14 9 0.06 13-14 >	/defl L/d •999 360 •999 240 n/a n/a •999 240	<b>PLATES</b> MT20 Weight: 171 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SP No SLIDER	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 o.3 Right 2x4 SP No	o.3 2-6-0		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sh 2-0-0 oc purlins (4- Rigid ceiling directl 1 Row at midpt	neathing directly app -1-6 max.): 5-6. ly applied. 5-11	lied, except	
REACTIONS.	(lb/size) 9=12 Max Horz 2=98 Max Uplift 2=-2 Max Grav 9=15	268/Mechanical, 2=1352/0-3-8 3(LC 10) 4(LC 10) 112(LC 20), 2=1577(LC 20)						
FORCES. (lb	) - Max. Comp./M	ax. Ten All forces 250 (lb) or le	ss except when shown.					

- TOP CHORD 2-3=-2720/294, 3-5=-1995/297, 5-6=-1642/309, 6-7=-1976/299, 7-9=-2592/294
- BOT CHORD 2-14=-207/2336, 13-14=-207/2336, 11-13=-94/1648, 10-11=-186/2241, 9-10=-186/2241
- WEBS 3-14=0/271, 3-13=-775/129, 5-13=-1/492, 6-11=-26/475, 7-11=-685/123, 7-10=0/254
- WEBS 3-14=0/2/1, 3-13=-775/129, 5-13=-1/492, 6-11=-26/475, 7-11=-685/123, 7-10=0/2

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 14-0-0, Exterior(2) 14-0-0 to 22-3-7, Interior(1) 22-3-7 to 31-9-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
   Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.









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	L	6-6-0	12-10-3		16-0-4	19-2-5	2	25-6-8			32-0-8	
		6-6-0	6-4-3	1	3-2-1	3-2-1		6-4-3		T	6-6-0	
Plate Offsets (X,	Y) [2:0-0-0,0	-1-9], [10:Edge,0-1	-9], [12:0-3-0,0-3-0]									
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 1 TCDL BCLL BCDL	20.0 5.4/20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DO Lumber DOL Rep Stress Ir Code IRC20	2-0-0 DL 1.15 1.15 ner YES 09/TPI2007	<b>CSI.</b> TC BC WB Matri	0.62 0.90 0.44 x-AS	DEFL. Vert(LL) Vert(TL) Horz(TL Wind(LL	in -0.21 -0.46 0.13 ) 0.07	(loc) 15 15 10 18-19	l/defl >999 >833 n/a >999	L/d 360 240 n/a 240	<b>PLATES</b> MT20 Weight: 185 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2 WEDGE Left: 2x4 SP No.	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 3, Right: 2x4 SP	No.3			E T E	RACING- OP CHORD OT CHORD	Structura Rigid ceil	l wood ling dire	sheathin ectly appl	g directly appl ied.	lied.	
REACTIONS.	(lb/size) 2=15 Max Horz 2=10	26/0-3-8, 10=1443/ 6(LC 10)	0-3-8									
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	- Max. Comp./Ma 2-3=-2611/161 9-10=-2625/19 2-19=-88/2252 10-11=-105/22 6-14=-39/824, 6-17=-39/821, 5	ax. Ten All forces , 3-5=-2493/202, 5- 3 , 18-19=-24/1970, 1 67 12-14=-66/728, 7-1 5-18=-482/158, 5-1	250 (lb) or less exca 6=-2094/211, 6-7=-2 13-18=0/1567, 12-13 2=-487/160, 7-11=-6 9=-39/360	ept when sh 2095/219, 7- =0/1567, 11 56/376, 17-1	iown. -9=-2507/23 1-12=-20/19 <sup>-</sup> 8=-65/726,	5, 73,						
NOTES-												

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 16-0-4, Exterior(2) 16-0-4 to 19-0-4, Interior(1) 19-0-4 to 32-0-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10

- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



nt 818 Soundside Road Edenton, NC 27932



BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

## LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3, Right: 2x4 SP No.3

### REACTIONS. (lb/size) 2=1427/0-3-8, 10=1427/0-3-8 Max Horz 2=98(LC 10) Max Uplift 2=-31(LC 10), 10=-31(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-2410/232, 3-5=-2292/272, 5-6=-1876/287, 6-7=-1876/287, 7-9=-2292/272, 9-10=-2410/232
- BOT CHORD
   2-17=-116/2075, 16-17=-58/1778, 14-16=0/1356, 12-14=-70/1778, 10-12=-129/2075

   WEBS
   6-14=-75/718, 7-14=-490/156, 7-12=-32/380, 6-16=-75/718, 5-16=-490/156, 5-17=-32/380

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 16-0-4, Exterior(2) 16-0-4 to 19-0-4, Interior(1) 19-0-4 to 33-4-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type	Qty	Ply	Sturtz Homes	
					E12	549265
1625535_Jill_RF	E10	Hip Girder	1	2		
				2	Job Reference (optional)	
Builders FirstSource,	Albemarle, NC 28001		i	3.220 s No	v 16 2018 MiTek Industries, Inc. Fri Dec 21 12:46:37 2018 Pag	ge 2
		ID:4Ir	neeSjiwdRzO	fsAnBfHya	aylMmJ-DZgkoT5ZKQgvOamfYvYo4nRVtB5?mSRG3ERqI0y6k	dm

## NOTES-

13) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 15-11-12 from the left end to 29-11-12 to connect truss(es) to front face of bottom chord.

14) Fill all nail holes where hanger is in contact with lumber.

15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 538 lb down at 2-0-12, 538 lb down at 4-0-12, 538 lb down at 6-0-12, 591 lb down and 24 lb up at 8-0-12, 532 lb down and 21 lb up at 10-0-12, and 620 lb down and 47 lb up at 12-0-12, and 559 lb down and 78 lb up at 13-11-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-5=-60, 5-10=-60, 1-9=-20

Veril. 1-4=-60, 4-5=-60, 5-10=-60,

Concentrated Loads (lb)

Vert: 15=-538(F) 16=-538(F) 14=-532(F) 29=-538(F) 30=-538(F) 31=-538(F) 32=-532(F) 33=-532(F) 34=-532(F) 35=-611(F) 36=-532(F) 37=-532(F) 38=-532(F) 39=-532(F) 40=-523(F) 40=-5





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# TREERING BY A Mi Tek Atfiliate 818 Soundside Road

Edenton, NC 27932



#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 7-2-0, Exterior(2) 7-2-0 to 10-2-0, Interior(1) 10-2-0 to 15-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



TERENGINEERING BY A MITCH Affiliate 818 Soundside Road

Edenton, NC 27932



TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.2

SLIDER Right 2x4 SP No.3 2-6-0

#### REACTIONS. (lb/size) 7=558/Mechanical, 2=645/0-3-8 Max Horz 2=65(LC 10) Max Uplift 2=-38(LC 10)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-1299/197, 3-4=-1288/292, 4-5=-1250/286, 5-7=-1227/206
- BOT CHORD 2-9=-125/1149, 8-9=-38/801, 7-8=-118/1125

4-9=-183/654, 4-8=-179/600 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 7-2-0, Exterior(2) 7-2-0 to 10-2-0, Interior(1) 10-2-0 to 14-0-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



818 Soundside Road Edenton, NC 27932

🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not besign valid for use only with with every connectors. This design is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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818 Soundside Road Edenton, NC 27932



TRENCIO AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932



Snow (Pf/Pg)         20.4/20.           TCDL         10           BCLL         0           BCDL         10	0 Plate Grip DOL Lumber DOL 0 Rep Stress Incr 0 Code IRC2009/	1.15         TC           1.15         BC           YES         WB           TPI2007         Matrix	0.46 0.52 0.55 -AS	Vert(LL) -0.07 Vert(TL) -0.14 Horz(TL) 0.08 Wind(LL) 0.03	7 9-12 4 9-12 3 6 3 9-12	>999 >999 n/a >999	360 240 n/a 240	MT20 Weight: 82 lb	244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP WEDGE Left: 2x4 SP No.2	No.2 No.2 No.3		BRACING TOP CHO BOT CHO	3- )RD Structu 2-0-0 c )RD Rigid c	ural wood oc purlins eiling dir	l sheathin (6-0-0 m ectly appl	g directly app ax.): 4-5. ied.	lied, except end verti	cals, and
REACTIONS. (Ib/size Max H Max U Max G	<ul> <li>e) 6=552/Mechanical, 2=640/0</li> <li>orz 2=184(LC 10)</li> <li>plift 6=-35(LC 10), 2=-21(LC 10)</li> <li>rav 6=640(LC 16), 2=810(LC 16)</li> </ul>	-3-8							
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-9=-           WEBS         3-8=-	Comp./Max. Ten All forces 25( 1709/180, 3-4=-921/102 323/1506, 8-9=-307/1457, 7-8=- 40/457, 3-7=-953/221, 4-7=-122/	) (lb) or less except when sho 307/1457 /862, 4-6=-710/228	own.						
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-05; 1 -1-4-0 to 1-8-0, Inter and forces & MWFR 3) TCLL: ASCE 7-05; F roof snow: Lumber D governs. Rain surch 4) Unbalanced snow lo 5) This truss has been non-concurrent with 6) Provide adequate dr 7) This truss has been 8) * This truss has been 8) * This truss has been 9) Refer to girder(s) for 10) Bearing at joint(s) 2 capacity of bearing 11) Provide mechanica 12) One H2.5A Simpso connection is for up 13) This truss design ru sheetrock be applied 14) Graphical purlin rep	e loads have been considered for 00mph; TCDL=6.0psf; BCDL=6.0 ior(1) 1-8-0 to 12-0-0, Exterior(2) S for reactions shown; Lumber D Pr=20.0 psf (roof live load: Lumbe DOL=1.15 Plate DOL=1.15); Cate large applied to all exposed surfa ads have been considered for th designed for greater of min roof other live loads. ainage to prevent water ponding designed for a 10.0 psf bottom c n designed for a 10.0 psf bottom c n designed for a live load of 20.0 ottom chord and any other memi truss to truss connections. 2 considers parallel to grain value surface. I connection (by others) of truss on Strong-Tie connectors recomm Jift only and does not consider la equires that a minimum of 7/16" sed directly to the bottom chord. presentation does not depict the	this design. )psf; h=25ft; Cat. II; Exp B; Er 12-0-0 to 13-10-12 zone; car )OL=1.60 plate grip DOL=1.60 ar DOL=1.15 Plate DOL=1.15; agory II; Exp B; Partially Exp.; aces with slopes less than 0.5 is design. live load of 12.0 psf or 1.00 tir hord live load nonconcurrent to psf on the bottom chord in all bers. e using ANSI/TPI 1 angle to g to bearing plate capable of wi nended to connect truss to be ateral forces. structural wood sheathing be size or the orientation of the p	nclosed; MWFRS (I ntilever left and righ 0 ); Pg=20.0 psf (groi Ct=1.10, Lu=50-0- 500/12 in accordance mes flat roof load of with any other live I areas where a rect arain formula. Build ithstanding 100 lb u earing walls due to I applied directly to the purlin along the top	ow-rise) and C-C t exposed ;C-C fr and snow); Pf=20 ); Min. flat roof si e with IBC 1608. 15.4 psf on over oads. angle 3-6-0 tall b ng designer shor plift at joint(s) 6. JPLIFT at jt(s) 2. ne top chord and and/or bottom ch	Exterior or memb ).4 psf (fl. now load 3.4. thangs y 2-0-0 v uld verify Uld verify This 1/2" gyp ord.	(2) ers at vide	and a company and a second second	SEAL 03632	2 E.B.F.A.I.I.I.I.I.I.I.I.I.I.I.I.I.I.I.I.I.I
WARNING - Verify de Design valid for use only a truss system. Before u	esign parameters and READ NOTES ON with MiTek® connectors. This design is b se, the building designer must verify the a	THIS AND INCLUDED MITEK REFER based only upon parameters shown, a upplicability of design parameters and	RENCE PAGE MII-7473 re and is for an individual bu properly incorporate this prob. Additional temperate	v. 10/03/2015 BEFOR ilding component, no design into the over	<b>RE USE.</b> t all				:0

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





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818 Soundside Road Edenton, NC 27932



BRACING-

TOP CHORD

BOT CHORD

	ED	
LU	ER-	

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3

REACTIONS. (Ib/size) 2=638/0-3-8, 8=558/Mechanical Max Horz 2=214(LC 10) Max Uplift 2=-9(LC 10), 8=-65(LC 10) Max Grav 2=638(LC 1), 8=578(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-766/0

BOT CHORD 2-9=-145/612, 8-9=-145/612

WEBS 3-9=0/306, 3-8=-673/157

## NOTES-

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 14-0-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.





		7-0-15 7-0-15	<u>14-0-8</u> 6-11-9			
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.50 BC 0.47 WB 0.78 Matrix-AS	DEFL.         in         (loc)           Vert(LL)         -0.05         6-7           Vert(TL)         -0.13         6-7           Horz(TL)         0.02         6           Wind(LL)         0.02         7-10	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES MT20 Weight: 74 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

## LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 WEDGE
 2x4 SP No.3

Left: 2x4 SP No.3

REACTIONS. (lb/size) 2=640/0-3-8, 6=552/Mechanical Max Horz 2=212(LC 10) Max Uplift 2=-10(LC 10), 6=-65(LC 10) Max Grav 2=640(LC 1), 6=564(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-763/0

BOT CHORD 2-7=-132/608, 6-7=-132/608

WEBS 3-7=0/310, 3-6=-678/146

## NOTES-

- Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 13-10-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
  4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.



	6-	11-7		7-1-1			
Plate Offsets (X,Y) [2:0-0-0,0	)-1-5], [4:0-4-8,0-1-12]						
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2009/TPI2007	CSI. TC 0.51 BC 0.47 WB 0.52 Matrix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (loc) -0.09 6-7 -0.18 6-7 0.01 6 0.02 7-10	l/defl L/d >999 360 >911 240 n/a n/a >999 240	PLATES MT20 Weight: 81 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 POT CHORD 2x4 SP No.2		BR/ TO	ACING- P CHORD	Structural wood	sheathing directly ap	plied, except end vert	icals, and
WEBS 2x4 SP No.3		BO	T CHORD	Rigid ceiling dire	ectly applied.		

WEDGE Left: 2x4 SP No.3

REACTIONS. (lb/size) 2=658/0-3-8, 6=631/Mechanical Max Horz 2=184(LC 10) Max Uplift 2=-22(LC 10), 6=-34(LC 10) Max Grav 2=823(LC 16), 6=719(LC 16)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-1118/30, 3-4=-980/94

BOT CHORD 2-7=-1178/904

WEBS 3-7=-502/181, 4-7=-109/882, 4-6=-659/218

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 12-0-0, Exterior(2) 12-0-0 to 13-10-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



December 26,2018

818 Soundside Road

Edenton, NC 27932

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TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.94 BC 0.72 WB 0.68 Matrix-AS	Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in ( -0.14 -0.29 0.03 0.07	(loc) l/ 6-9 > 6-9 > 2 6-9 >	//defl L/d >999 360 >573 240 n/a n/a >999 240	Weight: 68 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.2		BF TC BC	RACING- DP CHORD DT CHORD	Structural v 2-0-0 oc pu Rigid ceilin	wood sh urlins (6- ng direct	neathing directly -0-0 max.): 3-4. Ily applied.	y applied, except end verti	cals, and
REACTIONS. (Ib/size) 2=64 Max Horz 2=13 Max Uplift 2=-3 Max Grav 2=79	0/0-3-8, 5=552/Mechanical 11(LC 10) 1(LC 10), 5=-12(LC 9) 16(LC 16), 5=611(LC 15)							
FORCES.         (lb) - Max. Comp.///           TOP CHORD         2-3=-763/93, 4           BOT CHORD         2-6=-141/564,           WEBS         3-6=0/316, 3-5	ax. Ten All forces 250 (lb) or less exce ŀ-5=-250/69 5-6=-143/557 i≕-678/172	pt when shown.						
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads ha</li> <li>2) Wind: ASCE 7-05; 100mph; 1</li> <li>-1-4-0 to 1-8-0, Interior(1) 1-8</li> <li>exposed ;C-C for members a</li> <li>3) TCLL: ASCE 7-05; Pr=20.0 p</li> <li>roof snow: Lumber DOL=1.15</li> <li>governs. Rain surcharge app</li> <li>4) Unbalanced snow loads have</li> <li>5) This truss has been designed</li> <li>non-concurrent with other live</li> <li>6) Provide adequate drainage to</li> <li>7) This truss has been designed</li> <li>8) * This truss has been designed</li> <li>will fit between the bottom ch</li> <li>9) Refer to girder(s) for truss to</li> <li>10) Provide mechanical connect</li> <li>11) One H2.5A Simpson Strong</li> <li>connection is for uplift only a</li> <li>12) This truss design requires the sheetrock be applied directive</li> <li>13) Graphical purlin representation</li> </ul>	Ave been considered for this design. CDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. -0 to 8-0-0, Exterior(2) 8-0-0 to 12-2-15, nd forces & MWFRS for reactions shown sf (roof live load: Lumber DOL=1.15 Plat Plate DOL=1.15); Category II; Exp B; P lied to all exposed surfaces with slopes been considered for this design. for greater of min roof live load of 12.0 p e loads. prevent water ponding. for a 10.0 psf bottom chord live load noi ad for a live load of 20.0psf on the bottom ord and any other members. truss connections. tion (by others) of truss to bearing plate of -Tie connectors recommended to conne- and does not consider lateral forces. to the bottom chord. to the bottom chord. to ndoes not depict the size or the orient	II; Exp B; Enclosed; MW, Interior(1) 12-2-15 to 13- ; Lumber DOL=1.60 plat e DOL=1.15); Pg=20.0 p artially Exp.; Ct=1.10, Lu ess than 0.500/12 in acc osf or 1.00 times flat roof neconcurrent with any othe chord in all areas where expable of withstanding 1 ct truss to bearing walls of neathing be applied direct ation of the purlin along t	/FRS (low-rise) an -10-12 zone; cant e grip DOL=1.60 sf (ground snow) I=50-0-0; Min. flat cordance with IBC load of 15.4 psf of er live loads. e a rectangle 3-6- 100 lb uplift at join due to UPLIFT at ctly to the top cho the top and/or bot	nd C-C Ext illever left a ; Pf=20.4 p t roof snow ; 1608.3.4. on overhan 0 tall by 2- nt(s) 5. jt(s) 2. This ord and 1/2 tom chord.	terior(2) and right osf (flat / load	t e	SEAL 03632	2 E.F. H.





		6-0-0		1				8-0-8			
Plate Offsets (X,Y)	) [2:0-0-0,0-	-1-5]									
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 20. TCDL BCLL BCDL	20.0 .4/20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2009/TPI2	2-0-0 <b>CSI</b> . 1.15 TC 1.15 BC YES WB 2007 Mat	0.53 0.52 0.37 rix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in -0.10 -0.26 0.02 0.02	(loc) 6-7 6-7 6 7-10	l/defl >999 >635 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 69 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x WEDGE Left: 2x4 SP No.3	(4 SP No.2 (4 SP No.2 (4 SP No.3			E T E	BRACING- FOP CHORD BOT CHORD	Structura 2-0-0 oc Rigid cei	al wood purlins ling dire	sheathin (6-0-0 m ectly appl	g directly ap ax.): 3-5. ied.	oplied, except end vert	icals, and
REACTIONS. (III M M M	b/size) 2=640 lax Horz 2=105 lax Uplift 2=-28 lax Grav 2=739	0/0-3-8, 6=552/Mechanica 5(LC 10) (LC 10), 6=-14(LC 9) 9(LC 16), 6=669(LC 15)	I								
FORCES. (lb) - M TOP CHORD 2 BOT CHORD 2 WEBS 2	Max. Comp./Ma 2-3=-812/114, 3 2-7=-148/660, 6 4-7=-26/299, 4-	ıx. Ten All forces 250 (lb }-4=-658/154 }-7=-132/603 6=-709/176	o) or less except when s	hown.							

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 6-0-0, Exterior(2) 6-0-0 to 10-2-15, Interior(1) 10-2-15 to 13-10-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



ENGINEERING BY A MiTek Affiliate 818 Soundside Road

Edenton, NC 27932



	4-0-0 4-0-0			14-0-8 10-0-8			
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.91 BC 0.80 WB 0.63 Matrix-AS	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in (loc) -0.24 6-7 -0.62 6-7 0.03 6 0.02 6-7	l/defl L/d >685 360 >267 240 n/a n/a >999 240	PLATES MT20 Weight: 67 lb	<b>GRIP</b> 244/190 FT = 20%
		DI					

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS WEDGE

# Left: 2x4 SP No.3

REACTIONS. (lb/size) 2=640/0-3-8, 6=552/Mechanical Max Horz 2=78(LC 10) Max Uplift 2=-21(LC 10), 6=-16(LC 9)

Max Grav 2=664(LC 16), 6=711(LC 15)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1035/100, 3-4=-890/129

```
BOT CHORD
               2-7=-123/902, 6-7=-208/1088
```

```
WEBS
               3-7=0/297, 4-6=-1059/250
```

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) -1-4-0 to 1-8-0, Interior(1) 1-8-0 to 4-0-0, Exterior(2) 4-0-0 to 8-2-15, Interior(1) 8-2-15 to 13-10-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied, except end verticals, and

2-0-0 oc purlins (5-7-14 max.): 3-5.

Rigid ceiling directly applied.

818 Soundside Road

Edenton, NC 27932

🔥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE WARNING - Verify design parameters and KEAD NOTES ON THIS AND INCLODED MITER REFERENCE FAGE MIT F14's rev. twosevis bervice osc. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSTP11 Quality Criteria, DSB-89 and BCSI Building Component** fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Qua Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



## Scale = 1.25.7



	2-0-0 2-0-0	6-0-12 4-0-12		9-11-12 3-11-0					14-0-8 4-0-12	
Plate Offsets (X,Y) [3:0-5	5-0,0-1-7]									
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2009/TI	2-0-0 1.15 1.15 NO PI2007	CSI. TC 0.4 BC 0.5 WB 0.6 Matrix-M3	9         Vert(LL)           9         Vert(TL)           9         Horz(TL)           9         Wind(LL)	in -0.10 -0.19 0.04 0.04	(loc) 8-9 8-9 7 8-9	l/defl >999 >861 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 68 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2	2			BRACING- TOP CHORD	Structura	l wood	sheathin	g directly a	pplied or 5-3-13 oc purl	ins,

BOT CHORD

LUMBER-	
TOP CHORD	2x

2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

#### REACTIONS. (lb/size) 7=542/Mechanical, 2=631/0-3-8 Max Horz 2=55(LC 8) Max Uplift 7=-19(LC 7), 2=-15(LC 8) Max Grav 7=734(LC 13), 2=678(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1094/21, 3-4=-2037/55, 4-5=-2037/55

- BOT CHORD 2-10=-24/949, 9-10=-20/949, 8-9=-45/1700, 7-8=-45/1700
- 3-9=-49/1139, 4-9=-366/63, 5-9=-11/354, 5-7=-1694/45 WEBS

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=20.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

## Continued on page

🔺 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



except end verticals, and 2-0-0 oc purlins (3-7-13 max.): 3-6.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Job	Truss	Truss Type	Qty	Ply	Sturtz Homes
					E12549280
1625535_Jill_RF	G15	Half Hip Girder	1	1	
					Job Reference (optional)
Builders FirstSource, A	lbemarle , NC 28001		8	.220 s Nov	/ 16 2018 MiTek Industries, Inc. Fri Dec 21 12:46:53 2018 Page 2

ID:4ImeeSjiwdRzCfsAnBfHyayIMmJ-leen9xHcYKhdJ1\_kUGqYk96ISecrWiedljJgs4y6kdW

LOAD CASE(S) Standard Uniform Loads (plf)

Vert: 1-3=-60, 3-6=-60, 7-11=-20

Concentrated Loads (lb)

Vert: 10=3(F) 9=3(F) 8=3(F) 17=3(F) 18=3(F) 19=3(F)





7-2-8 Plate Offsets (X,Y)--[4:0-2-0,Edge], [6:0-4-0,0-1-8], [11:0-4-0,0-1-8] LOADING (psf) SPACING-CSI. DEFL. PLATES GRIP 2-0-0 in (loc) I/defl I/d TCLL (roof) 20.0 Plate Grip DOL TC 0.09 Vert(LL) -0.00 120 244/190 1.15 n/r MT20 Snow (Pf/Pg) 15.4/20.0 Lumber DOL 1.15 BC 0.06 Vert(TL) -0.00 n/r 120 7 TCDL 10.0 Rep Stress Incr YES WB 0.03 Horz(TL) 0.00 8 n/a n/a BCLL 0.0 Code IRC2009/TPI2007 Matrix-R Weight: 37 lb FT = 20% BCDL 10.0 LUMBER-BRACING-

TOP CHORD2x4 SP No.2TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins,<br/>except end verticals.BOT CHORD2x4 SP No.2BOT CHORDBOT CHORDRigid ceiling directly applied or 6-0-0 oc bracing.OTHERS2x4 SP No.3BOT CHORDRigid ceiling directly applied or 6-0-0 oc bracing.

## REACTIONS. All bearings 7-2-8.

(lb) - Max Horz 11=65(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 11, 8, 10, 9 Max Grav All reactions 250 lb or less at joint(s) 11, 8, 10, 9

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Corner(3) -1-0-0 to 2-0-0, Exterior(2) 2-0-0 to 3-7-4, Corner(3) 3-7-4 to 6-7-4, Exterior(2) 6-7-4 to 8-2-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

9) Gable studs spaced at 2-0-0 oc.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 11, 8, 10, and 9. This connection is for uplift only and does not consider lateral forces.



TERGINEERING BY AMITEK Affiliate 818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Sturtz Homes	
1625535 Jill RF	H2	Roof Special Girder	1			E12549282
Builders FirstSource				8 220 e No	Job Reference (optional)	Fri Dec 21 12:47:01 2018 Dage 1
Dunuers Firstouurce,	ADDITIONE, NO 20001		ID:4ImeeSjiwdF	RzCfsAnBfH	yaylMmJ-WA7oqgOdgohVHGc	Gxy_Q2rRl7tKyOLpobzF58dy6kdO
		<u>  3-8-1</u> 3-8-1	7-2-8 3-6-7			
						Scale = 1:42.0
		1 1 5v4 - 11				
		9				
			10.00 12			
			3x4 🚿			
			2			
		6-8-9	$\mathbb{R}$			
				3		
				Fm-	8-8	
			11	12	14	
		$5_{4x4} = MUS26$	4 ··· 3x6 II	·		
		MUS26	MUS26 I	6x6 — MUS26		
		<u>→ 3-8-1</u> 3-8-1	7-2-8 3-6-7			
Plate Offsets (X,Y) [	2:0-0-12,0-1-8], [3:0-0-0,0-2-1	12], [4:0-4-8,0-1-8]	1			1
LOADING (psf) TCLL (roof) 20.0	SPACING-	2-0-0 <b>CSI</b> .	DEFL.	in	(loc) I/defl L/d	PLATES GRIP
Snow (Pf/Pg) 15.4/20.0	Lumber DOL	1.15 IC 0.12	Vent(LL)	) -0.01	4-8 >999 360 4-8 >999 240	MT20 244/190
BCLL 0.0	0 * Rep Stress Inc	cr NO WB 0.52 9/TPI2007 Matrix-MS	Horz(TL Wind(L	.) 0.00	3 n/a n/a 4-8 ⊳999 240	Weight: 103 lb ET = 20%
BCDL 10.0	0					
TOP CHORD 2x4 SP	No.2		TOP CHORD	Structur	al wood sheathing directly app	lied or 6-0-0 oc purlins,
BOT CHORD 2x6 SP WEBS 2x4 SP	No.2 No.3		BOT CHORD	except e Rigid ce	nd verticals. iling directly applied or 10-0-0	oc bracing.
WEDGE Right: 2x4 SP No 3				0		·
Max Ho	orz 5=-156(LC 9)	5-0				
Max Gr	av 3=2660(LC 4), 5=3154(LC	C 1)				
FORCES. (lb) - Max. (	Comp./Max. Ten All forces 2	250 (lb) or less except when shown.				
BOT CHORD 4-5=0/	/1589, 3-4=0/1589					
WEBS 2-5=-2	2144/0, 2-4=0/2517					
NOTES-	posted together with 10d (0.12	21"v2") poile of follows:				
Top chords connecte	d as follows: 2x4 - 1 row at 0-	9-0 oc.				
Bottom chords conne Webs connected as f	cted as follows: 2x6 - 2 rows ollows: 2x4 - 1 row at 0-9-0 o	staggered at 0-5-0 oc. c.				
<ol> <li>All loads are considered ply connections have</li> </ol>	red equally applied to all plies	, except if noted as front (F) or back (B) and the second se	face in the LOAD (	CASE(S) s	ection. Ply to	
3) Wind: ASCE 7-05; 10	00mph; TCDL=6.0psf; BCDL=	6.0psf; h=25ft; Cat. II; Exp B; Enclosed;	MWFRS (low-rise)	; cantileve	r left and	
4) TCLL: ASCE 7-05; Pi	r=20.0 psf (roof live load: Lum	nber DOL=1.15 Plate DOL=1.15); Pg=20	.0 psf (ground sno	w); Pf=15.4	4 psf (flat	IN ATH CARO
roof snow: Lumber D 5) Unbalanced snow loa	OL=1.15 Plate DOL=1.15); C ads have been considered for	ategory II; Exp B; Partially Exp.; Ct=1.10 this design.	)			STEER STATISTICS
6) This truss has been d	lesigned for a 10.0 psf bottom	h chord live load nonconcurrent with any	other live loads.	6.0 toll by	2.0.0 wide	the should
will fit between the bo	ottom chord and any other me	mbers.	nere a rectarigie 3	-0-0 tall by	2-0-0 wide	SEAL
<ol> <li>Use Simpson Strong- end to 4-5-0 to conner</li> </ol>	Tie MUS26 (6-10d Girder, 4- ect truss(es) to back face of be	10d Truss) or equivalent spaced at 2-3-4 ottom chord.	l oc max. starting a	at 0-1-12 fr	om the left	036322
9) Use Simpson Strong-	Tie MUS26 (6-10d Girder, 6-	10d Truss) or equivalent at 6-5-0 from th	e left end to conne	ect truss(es	i) to back	$\chi = 1$
10) Fill all nail holes whe	ere hanger is in contact with I	umber.				A NOINEER A
LOAD CASE(S) Standa	ard					CAREER
1) Dead + Roof Live (ba	lanced): Lumber Increase=1.	15, Plate Increase=1.15				A. GILLIN
						December 26,2018
Continued on page 2					I	
WARNING - Verify des	ign parameters and READ NOTES C	ON THIS AND INCLUDED MITEK REFERENCE PAG	GE MII-7473 rev. 10/03/2	2015 BEFORE	USE.	ENGINEERING BY
a truss system. Before use building design. Bracing i	e, the building designer must verify th ndicated is to prevent buckling of indi	e applicability of design parameters and properly in vidual truss web and/or chord members only. Addit	corporate this design in tional temporary and pe	to the overall	ina	i kenlu

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss we band/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

#### 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Sturtz Homes
					E12549282
1625535_Jill_RF	H2	Roof Special Girder	1	2	
				2	Job Reference (optional)
Builders FirstSource, A	lbemarle, NC 28001		8	.220 s Nov	/ 16 2018 MiTek Industries, Inc. Fri Dec 21 12:47:02 2018 Page 2

ID:4ImeeSjiwdRzCfsAnBfHyayIMmJ-\_NgB100FR5pMuQATVfVfb2\_wtGgB7o3ypd?eg3y6kdN

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-60, 5-6=-20 Concentrated Loads (lb)

Vert: 5=-1321(B) 10=-1314(B) 11=-1314(B) 12=-1251(B)











			11-11-4			I				
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL	if) 20.0 15.4/20.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	<b>CSI.</b> TC 0.13 BC 0.08 WB 0.14	DEFL. Vert(LL) Vert(TL) Horz(TL)	in n/a n/a 0.00	(loc) - - 7	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code IRC2009/TPI2007	Matrix-S						Weight: 78 lb	FT = 20%
LUMBER- TOP CHORD	2x4 SP No.2		B Tr	RACING- OP CHORD	Structural	wood	sheathin	a directlv ap	polied or 6-0-0 oc purlin	IS.
BOT CHORD WEBS OTHERS	2x4 SP No.2 2x4 SP No.3 2x4 SP No.3		В	OT CHORD	except ene Rigid ceilir	d verti ng dire	cals. ectly appl	ied or 10-0-0	0 oc bracing.	- /

11-11-4

REACTIONS. All bearings 11-11-4.

(lb) - Max Horz 1=174(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 9, 10, 11

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 8, 9, 10 except 11=264(LC 14)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-4-13 to 3-7-10, Interior(1) 3-7-10 to 9-7-10, Exterior(2) 9-7-10 to 11-9-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 7, 9, 10, and 11. This connection is for uplift only and does not consider lateral forces.







Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=397(LC 1), 9=436(LC 14), 6=434(LC 15)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-9=-284/193, 4-6=-284/193

## NOTES-

- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 8-5-4, Exterior(2) 8-5-4 to 11-5-4, Interior(1) 11-5-4 to 16-5-10 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 9, and 6. This connection is for uplift only and does not consider lateral forces.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocllapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

818 Soundside Road Edenton, NC 27932

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-4-13 to 3-2-13, Interior(1) 3-2-13 to 7-2-13, Exterior(2) 7-2-13 to 10-2-13, Interior(1) 10-2-13 to 14-0-13 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 8, and 6. This connection is for uplift only and does not consider lateral forces.







TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. All bearings 12-0-14.

Max Horz 1=-108(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=295(LC 14), 6=295(LC 15)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 6-0-7, Exterior(2) 6-0-7 to 9-0-7, Interior(1) 9-0-7 to 11-8-0 zone; cantilever left and right exposed :C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1, 5, 8, and 6. This connection is for uplift only and does not consider lateral forces.



818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



		9-8-1							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 PCLL 0.0 t	SPACING-     2-0-0       Plate Grip DOL     1.15       Lumber DOL     1.15       Rep Stress Incr     YES	CSI. TC 0.28 BC 0.20 WB 0.06	<b>DEFL.</b> Vert(LL) Vert(TL) Horz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code IRC2009/TPI2007	Matrix-S						Weight: 37 lb	FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD

BOT CHORD

## -OMBER

TOP CHORD 2x4 SP No.2 BOT CHORD OTHERS

2x4 SP No.2 2x4 SP No.3

REACTIONS. (lb/size) 1=188/9-8-1, 3=188/9-8-1, 4=334/9-8-1 Max Horz 1=-85(LC 8) Max Uplift 1=-7(LC 11), 3=-14(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 4-10-0, Exterior(2) 4-10-0 to 7-10-0, Interior(1) 7-10-0 to 9-3-3 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





REACTIONS. (lb/size) 1=137/7-2-10, 3=137/7-2-10, 4=243/7-2-10 Max Horz 1=-62(LC 8) Max Uplift 1=-5(LC 11), 3=-10(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.







BRACING-

TOP CHORD

BOT CHORD

LL	JM	BE	R-
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TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTUEDS 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 1=93/4-10-7, 3=93/4-10-7, 4=139/4-10-7 Max Horz 1=-39(LC 8) Max Uplift 1=-7(LC 11), 3=-10(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.



Structural wood sheathing directly applied or 4-10-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





2x4 🥢

2x4 🚿

		2-5-10						
	I	2-5-10						
Plate Offsets (X,Y) [2:0-2-0,E	dge]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2009/TPI2007	CSI. TC 0.01 BC 0.03 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(TL) Horz(TL)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a s n/a	L/d 999 999 n/a	PLATES MT20 Weight: 7 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2		BR/ TOI	ACING- P CHORD S	structural woo	d sheathin	g directly app	plied or 2-5-10 oc pu	rlins.
BOT CHORD 2x4 SP No.2		BO	CHORD R	liaid ceilina d	irectly appl	ied or 10-0-0	oc bracing.	

REACTIONS. (Ib/size) 1=67/2-5-10, 3=67/2-5-10 Max Horz 1=-16(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.10; Min. flat roof snow load governs.
 Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.





