

Trenco 818 Soundside Rd Edenton, NC 27932

Re: Alexander Michael Ryan Homes

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Builders FirstSource (Albermarle,NC).

Pages or sheets covered by this seal: E10264428 thruE10264441

My license renewal date for the state of North Carolina is December 31, 2017.

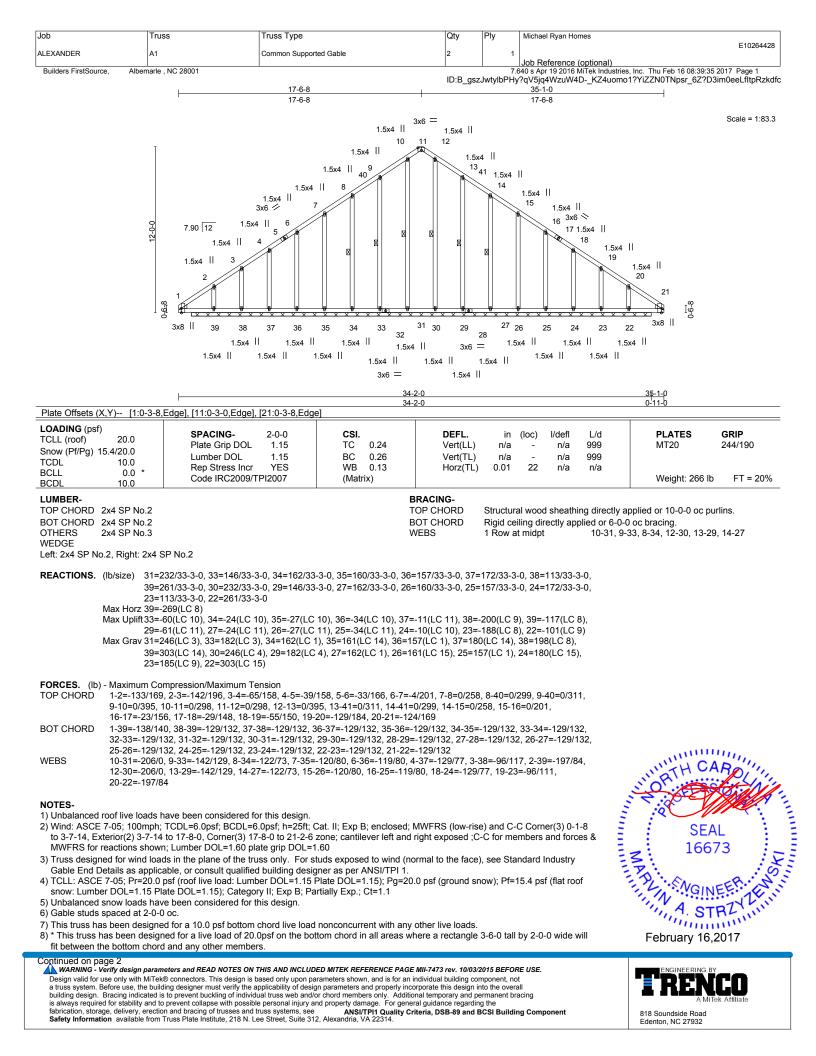
North Carolina COA: C-0844

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.



Strzyzewski, Marvin

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdictions(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to Trenco. Any project specific information included is for Trenco's customer's file reference purpose only, and was not taken into account in the preparation of these designs. Trenco has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of the design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Job	Truss	Truss Type	Qty	Ply	Michael Ryan Homes
					E10264428
ALEXANDER	A1	Common Supported Gable	2	1	
					Job Reference (optional)
Builders FirstSource, Albemarle, NC 28001 7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 16 08:39:36 2017 Page 2					
ID:B_gszJwtylbPHy?qV5jq4WzuW4D-SW7S68nQoJgZBjxD14K5OBekldPxVTuoaJ2RLtzkd					

NOTES-

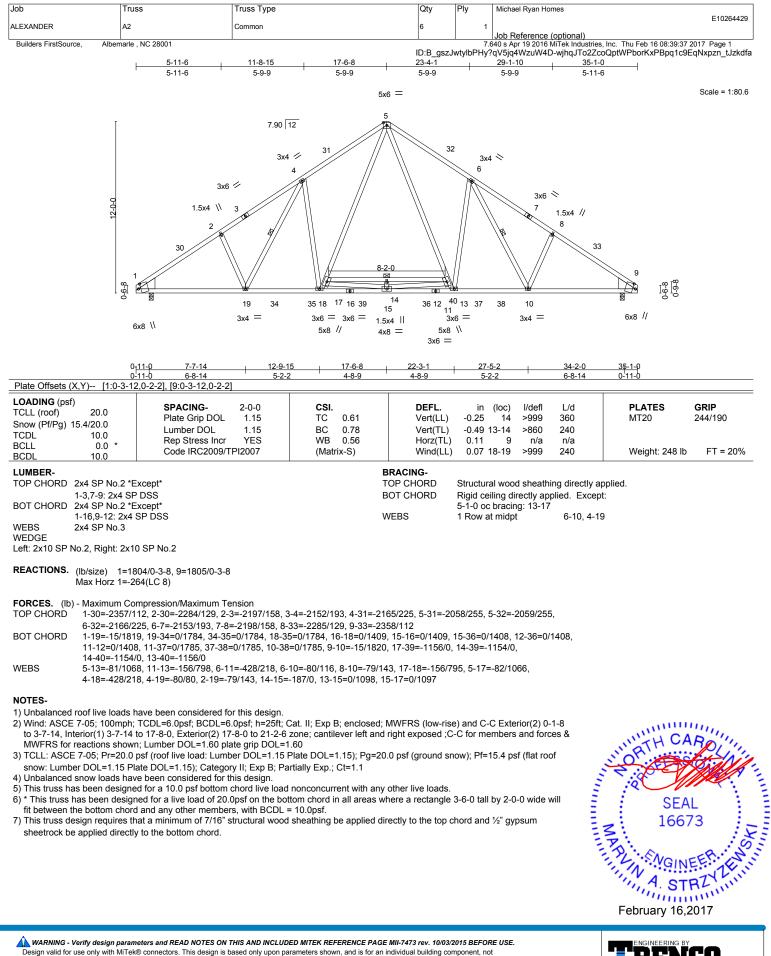
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 33, 34, 35, 36, 37, 38, 39, 29, 27, 26, 25, 24, 23, and 22. This connection is for uplift only and does not consider lateral forces.

10) Non Standard bearing condition. Review required.

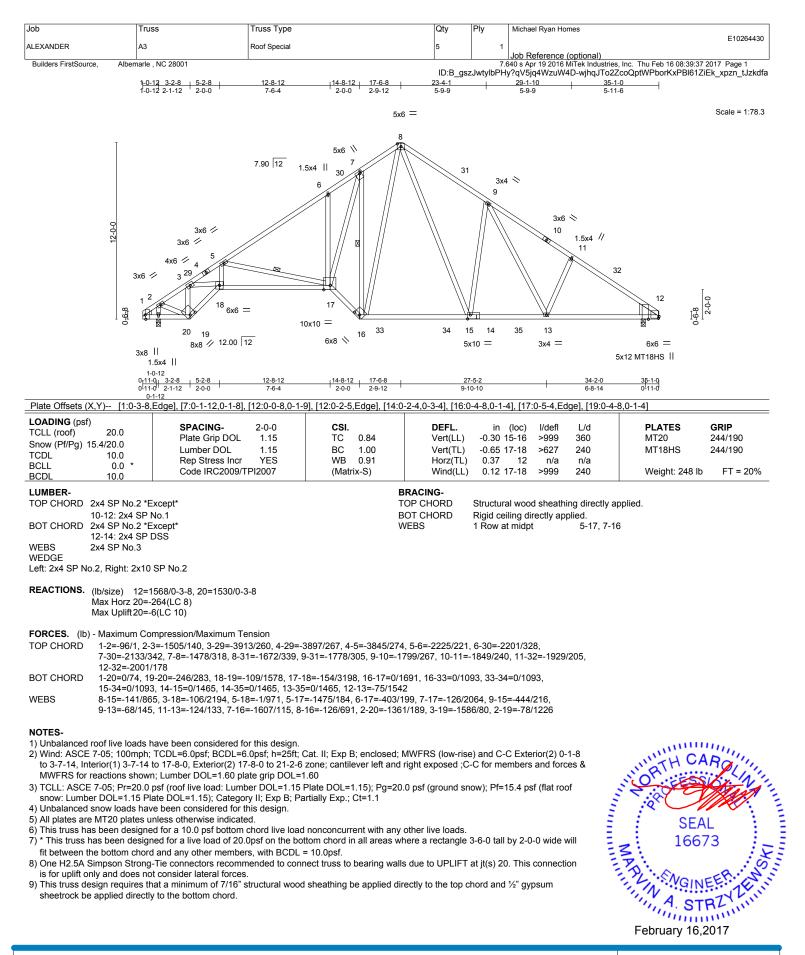
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

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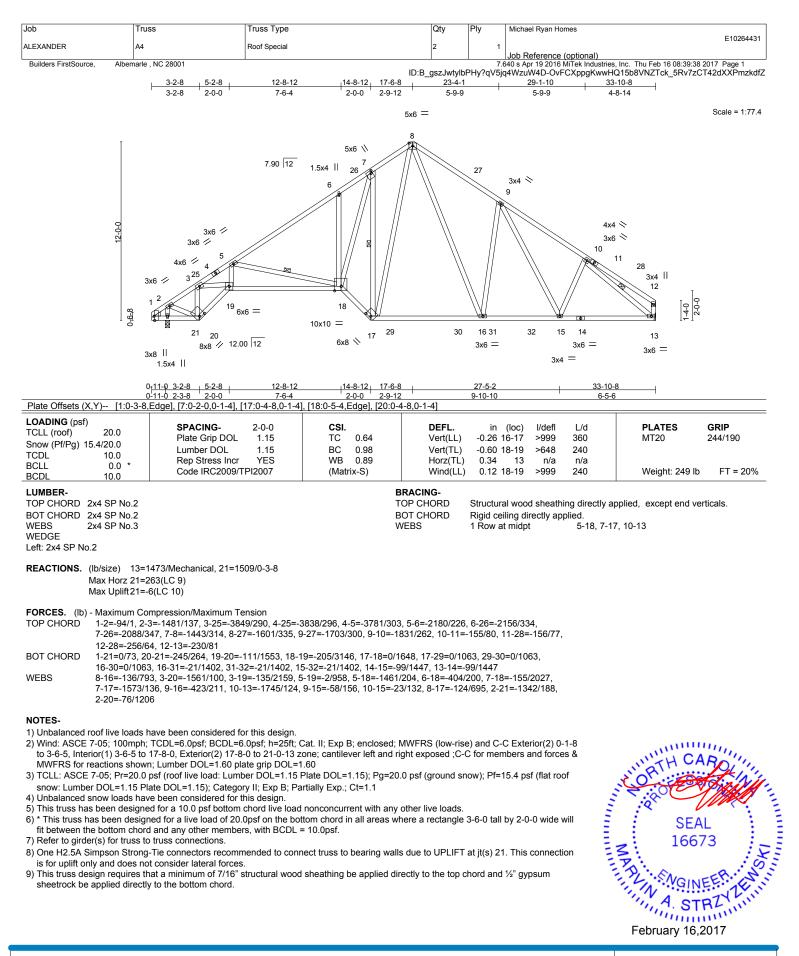




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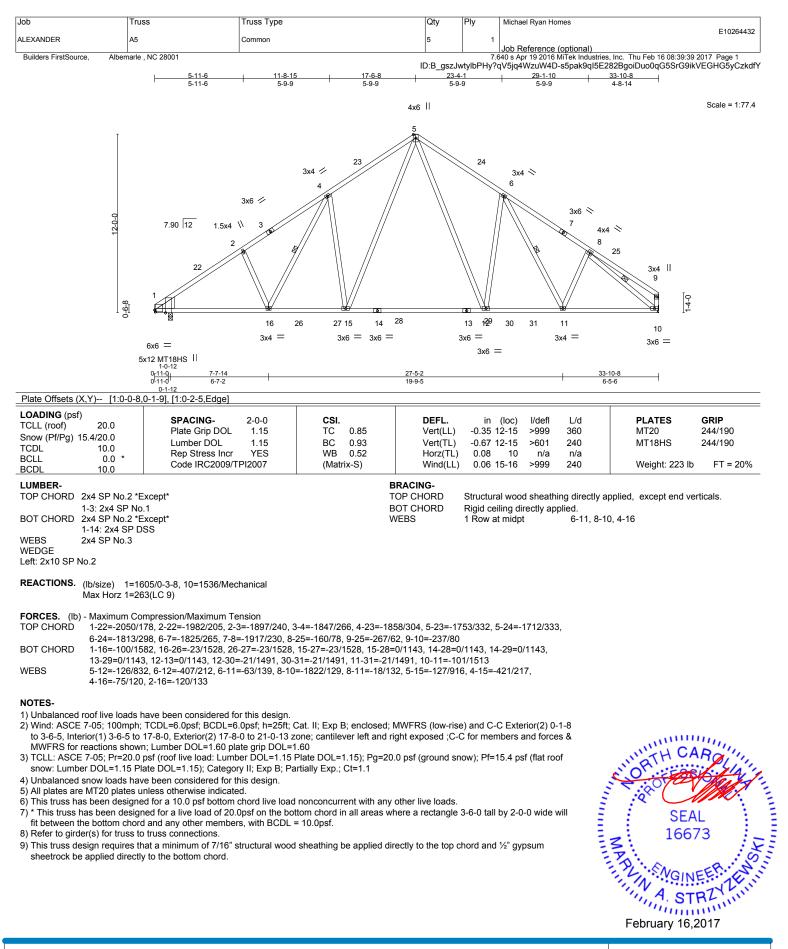


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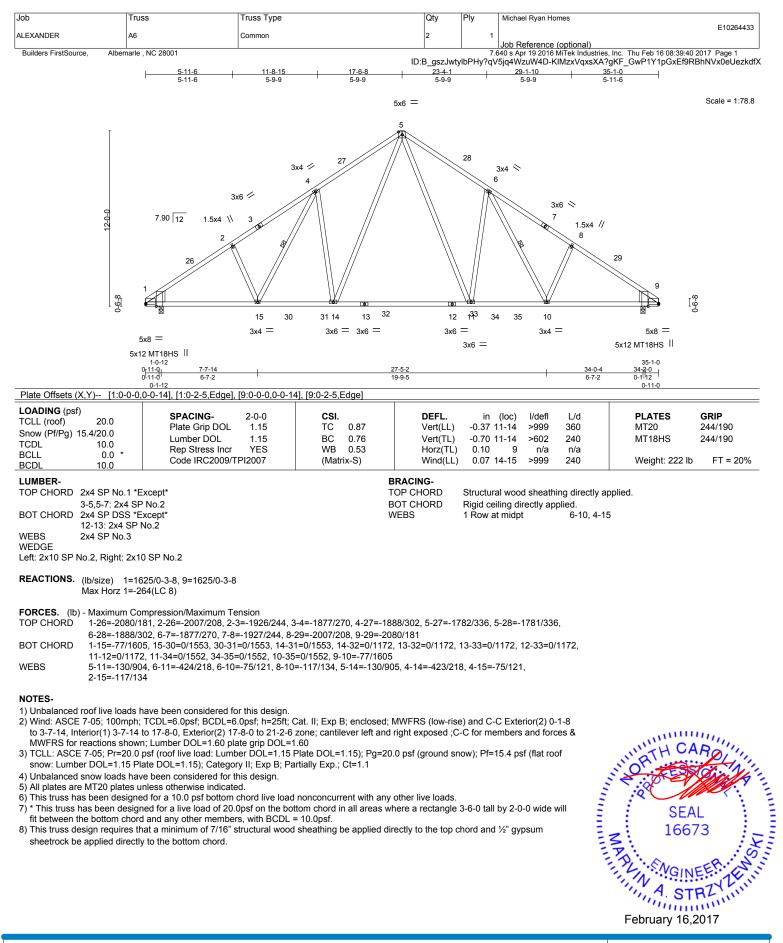
A MiTek 818 Soundside Road Edenton, NC 27932



A MITEK Affiliate

Edenton, NC 27932

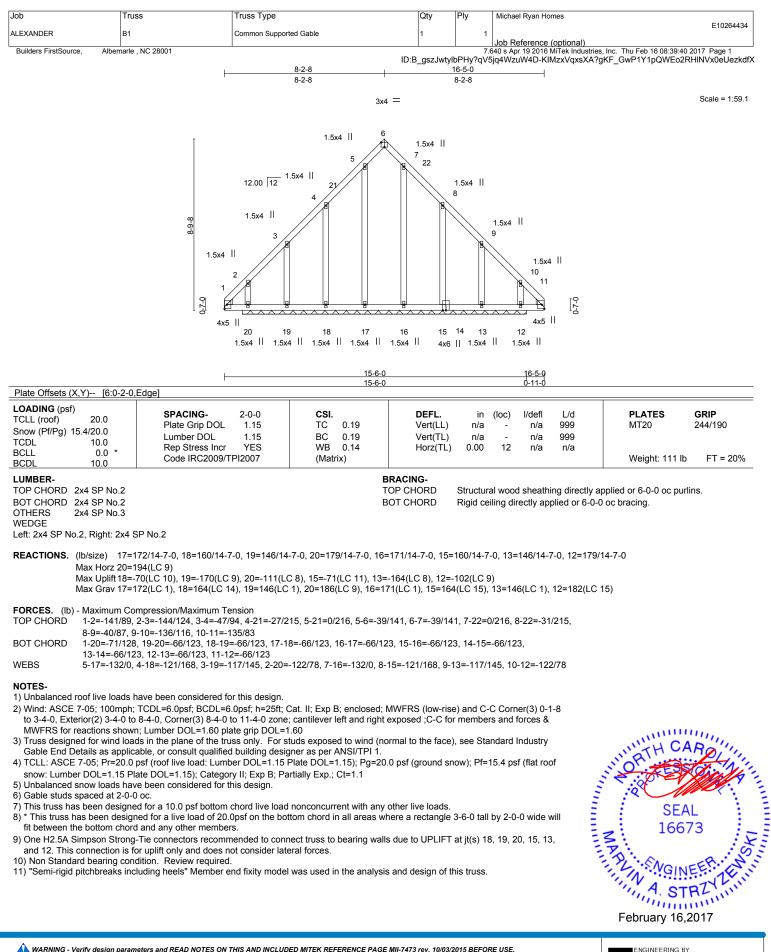
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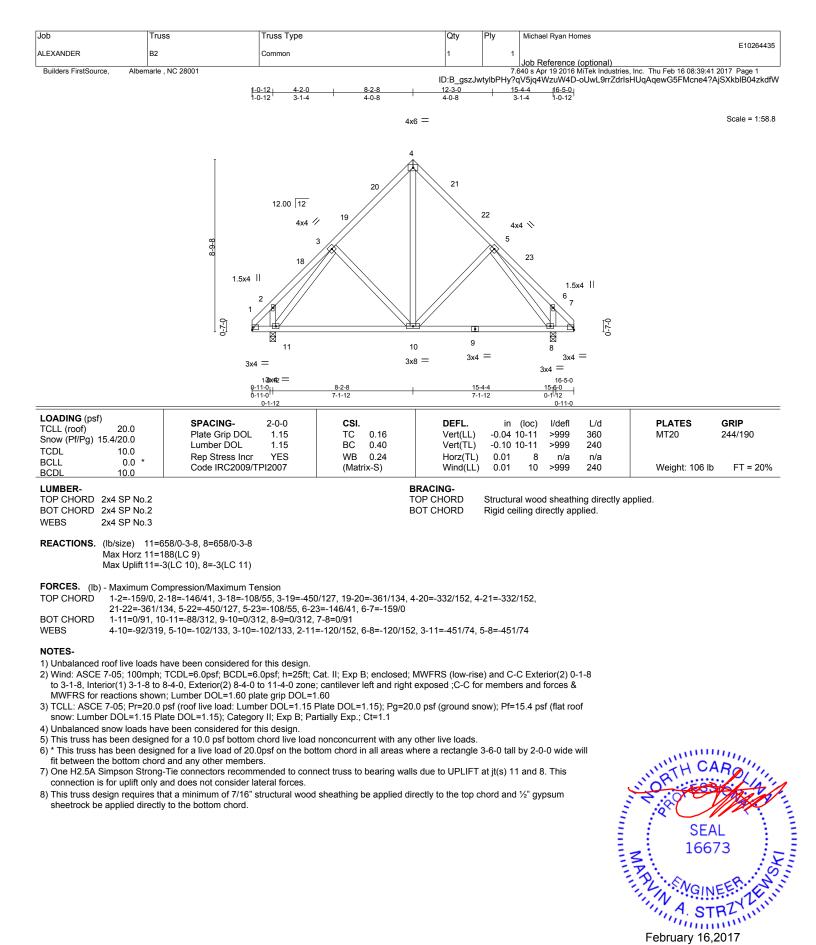


Edenton, NC 27932

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1-h T		T	Oti	DL		
Job T ALEXANDER B:	russ	Truss Type Common Girder	Qty 1	Ply	Michael Ryan Homes	E10264436
	rle , NC 28001		1	2	Job Reference (optional)	ies, Inc. Thu Feb 16 08:39:42 2017 Page 1
Duliders Firstoodice, Albemai	ie , NC 20001	3-9-8 7-3-8	ID:B_gszJwty 10-9-8	lbPHy?qV		9QjvePMNLRVeSud42Syv3bgyFVIYXzkdfV
		3-9-8 3-6-0	3-6-0		-9-8	
		4x6	II			Scale = 1:56.2
	۹ ۳ ۳ ۳ ۳ ۹ ۳ ۹ ۳ ۹ ۳ ۹ ۳ ۹ ۳ ۹ ۳ ۹		141000	3x6 *	4x6 5 18 6	
		MUS26 MUS26 MUS26 MUS26 <u>3-9-8</u> 7-3-8 3-9-8 3-6-0		14	5x8 MT18HS US26 7-0 -9-8	
	<u>ie,0-3-8], [7:0-4-8,0-2-4], [</u>	8:0-4-0,0-4-8], [9:0-4-8,0-2-4]				
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2009/7	2-0-0 CSI. 1.15 TC 0.83 1.15 BC 0.29 NO WB 0.69 Pl2007 (Matrix-M)	DEFL. Vert(LL) Vert(TL) Horz(TL) Wind(LL)	in -0.05 -0.11 0.01 0.01	(loc) l/defi L/d 7-8 >999 360 7-8 >999 240 6 n/a n/a 8 >999 240	PLATES GRIP MT20 244/190 MT18HS 244/190 Weight: 241 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP DSS WEBS 2x4 SP No.3 3-8: 2x4 SP N REACTIONS. (lb/size) 10: Max Horz 10:	vo.2 . =6188/0-3-8, 6=5443/0-3-	тс ВС	RACING- DP CHORD DT CHORD	end verti	• •	applied or 5-6-0 oc purlins, except -0-0 oc bracing.
FORCES. (lb) - Maximum C TOP CHORD 1-2=-5025/ BOT CHORD 10-13=-122 7-18=0/272 WEBS 3-8=0/5054	Compression/Maximum Te 0, 2-11=-3795/0, 3-11=-36 2/312, 13-14=-122/312, 9- 2, 6-18=0/272	ension 376/0, 3-12=-3676/0, 4-12=-3795/0, 4-5= 14=-122/312, 9-15=0/3490, 15-16=0/349 0, 2-8=-1452/0, 2-9=0/1863, 1-9=0/3349	0, 8-16=0/3490,			
connections have been pr 3) Unbalanced roof live loads 4) Wind: ASCE 7-05; 100mp exposed ; Lumber DOL=1 5) TCLL: ASCE 7-05; Pr=20. snow: Lumber DOL=1.15 6) Unbalanced snow loads h	follows: 2x4 - 1 row at 0-5 as follows: 2x6 - 2 rows s ys: 2x4 - 1 row at 0-9-0 oc qually applied to all plies, ovided to distribute only lu s have been considered for h; TCDL=6.0psf; BCDL=6 .60 plate grip DOL=1.60 0 psf (roof live load: Lumi Plate DOL=1.15); Catego ave been considered for t	 0 oc. taggered at 0-6-0 oc. except if noted as front (F) or back (B) fa bads noted as (F) or (B), unless otherwis or this design. 0psf; h=25ft; Cat. II; Exp B; enclosed; Noter DOL=1.15 Plate DOL=1.15); Pg=20.0 or JI; Exp B; Partially Exp.; Ct=1.1 his design. 	e indicated. IWFRS (low-rise	e); cantilev	ver left and right	H CARDINI
 9) * This truss has been desi fit between the bottom chc 10) "Semi-rigid pitchbreaks in 11) Use Simpson Strong-Tie to 8-10-4 to connect truss 12) Use Simpson Strong-Tie 	ned for a 10.0 psf bottom igned for a live load of 20. ord and any other membe ncluding heels" Member e MUS26 (6-10d Girder, 6- s(es) to back face of botto MUS26 (6-10d Girder, 6- l to 12-10-4 to connect tru	chord live load nonconcurrent with any c Opsf on the bottom chord in all areas wh rs. and fixity model was used in the analysis 10d Truss) or equivalent spaced at 2-0-0 m chord. 10d Truss, Single Ply Girder) or equivale ss(es) to back face of bottom chord.	ere a rectangle (and design of th) oc max. starting	iis truss. g at 0-10-	by 2-0-0 wide will 4 from the left end x. starting at	SEAL 16673
						February 16,2017
Continued on page 2						

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818 Soundside Road Edenton, NC 27932

CO

Job	Truss	Truss Type	Qty	Ply	Michael Ryan Homes
ALEXANDER	B3	Common Girder	1		E10264436
				Z	Job Reference (optional)
Builders FirstSource, Albemarle, NC 28001 7.640 s Apr 19 2016 MiTek Industries, Inc. Thu Feb 16 08:39:42 2017 Page 2					
lD:B_gszJwtylbPHy?qV5jq4WzuW4D-GgUjMBsBO9QjvePMNLRVeSud42Syv3bgyFVIYXzkdfV					

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

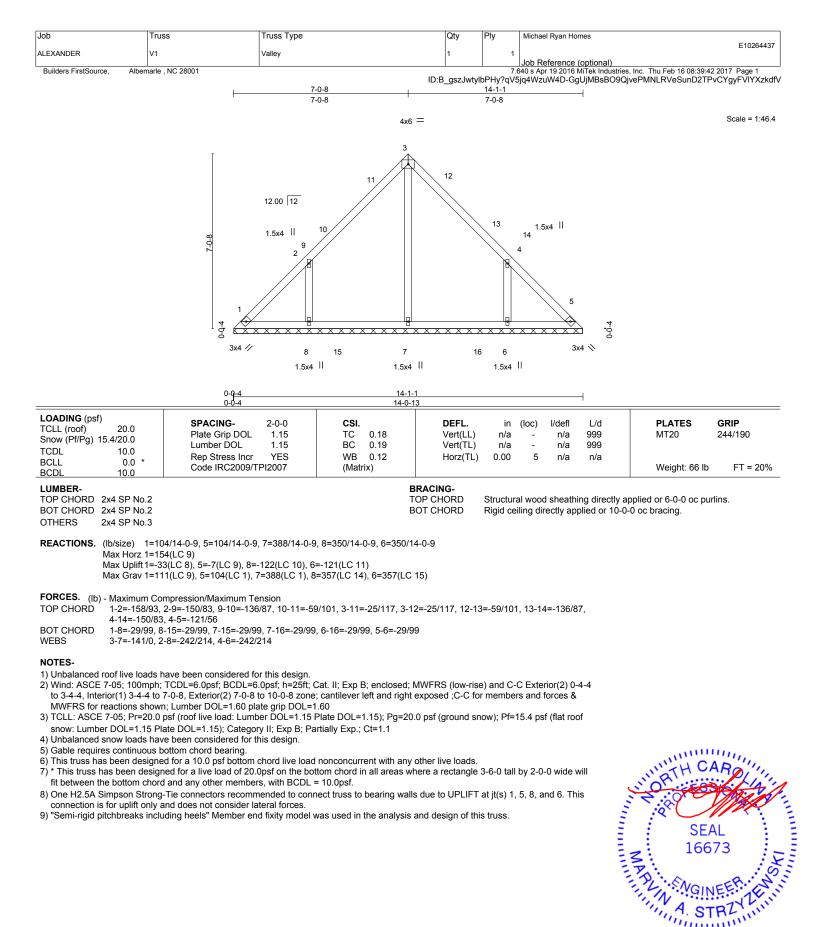
Vert: 1-3=-60, 3-5=-60, 6-10=-20

Concentrated Loads (lb)

Vert: 7=-1453(B) 13=-1519(B) 14=-1516(B) 15=-1516(B) 16=-1516(B) 17=-1516(B) 18=-1453(B)

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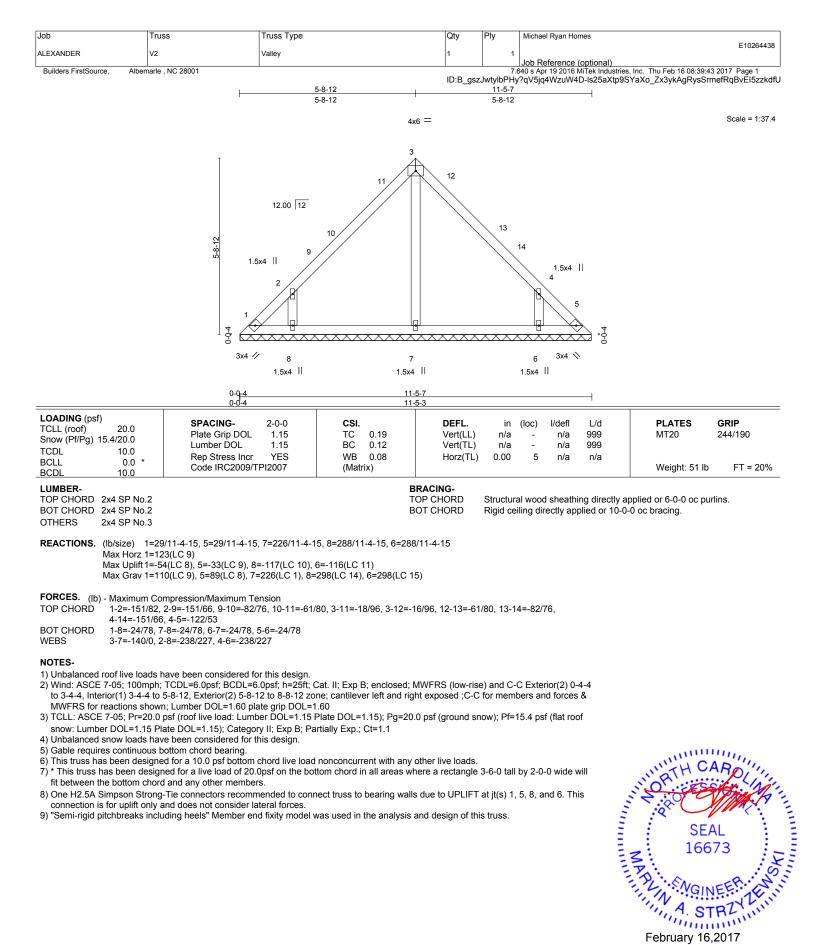




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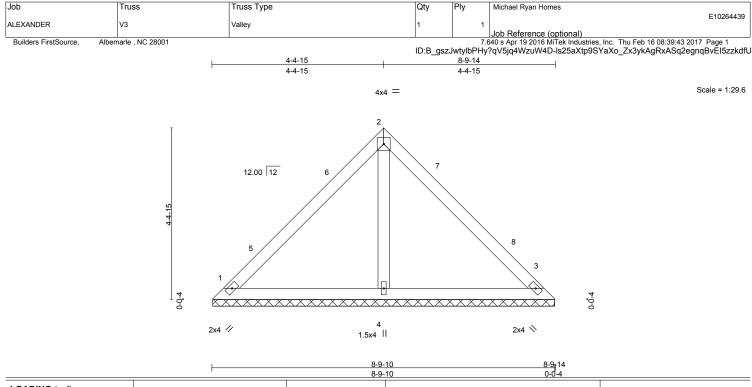
Edenton, NC 27932

(minin)



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	I	8-9-10			()-0-4		
TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 *	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2009/TPI2007	CSI. TC 0.23 BC 0.17 WB 0.06 (Matrix)	DEFL. Vert(LL) Vert(TL) Horz(TL)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 36 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (lb/size) 1=182/8-9-6, 3=182/8-9-6, 4=285/8-9-6 Max Horz 1=-93(LC 8) Max Uplift 1=-10(LC 11), 3=-10(LC 11)

FORCES. (Ib) - Maximum Compression/Maximum Tension

- TOP CHORD 1-5=-149/41, 5-6=-69/48, 2-6=-30/67, 2-7=-22/67, 7-8=-69/48, 3-8=-149/41
- BOT CHORD 1-4=-20/58, 3-4=-20/58
- WEBS 2-4=-163/47

NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) 0-4-4 to 3-4-4, Interior(1) 3-4-4 to 4-4-15, Exterior(2) 4-4-15 to 7-4-15 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 3. This connection is for uplift only and does not consider lateral forces.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

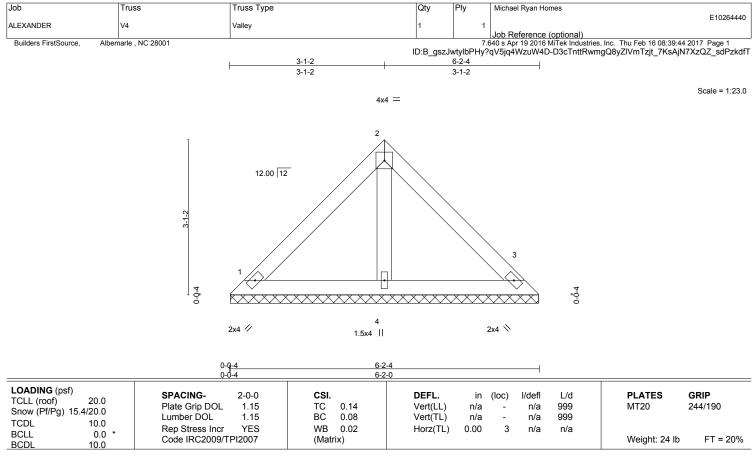


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



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LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 OTHERS

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=132/6-1-12, 3=132/6-1-12, 4=174/6-1-12 Max Horz 1=-63(LC 8) Max Uplift 1=-13(LC 11), 3=-13(LC 11)

FORCES. (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-88/43, 2-3=-88/43

BOT CHORD 1-4=-14/39, 3-4=-14/39

WEBS 2-4=-105/33

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed (C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

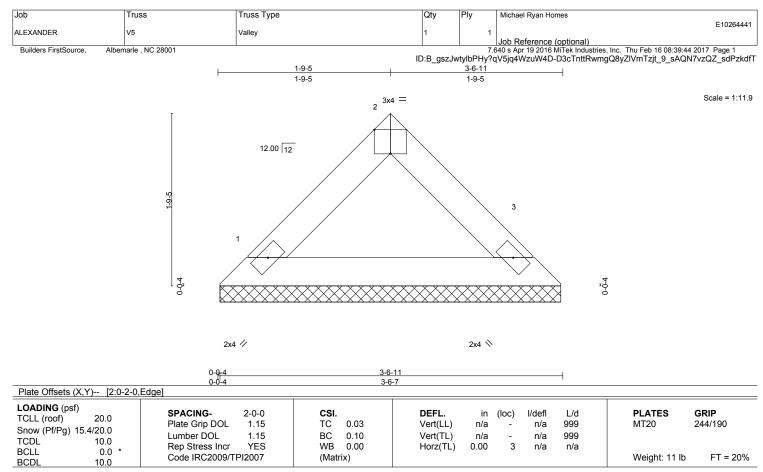
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BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (lb/size) 1=114/3-6-3, 3=114/3-6-3 Max Horz 1=-33(LC 6)

FORCES. (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-85/35, 2-3=-85/35

BOT CHORD 1-3=-8/43

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1; Min. flat roof snow load governs.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Structural wood sheathing directly applied or 3-6-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



