

Job 18090552	Truss A1	Truss Type Piggyback Base Supported Gable	Qty 1	Ply 1	282Hybrid 2009
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC
 8.220 s Aug 13 2018 MTEK Industries, Inc. Thu Sep 13 11:04:06 2018 Page 1
 ID:WYU5CM90RsiDxYtJ5CQZAYe4Rl-RkllrVwv78CuUpr?TLb2VpK4TjqtZOPyRSUkye4MN

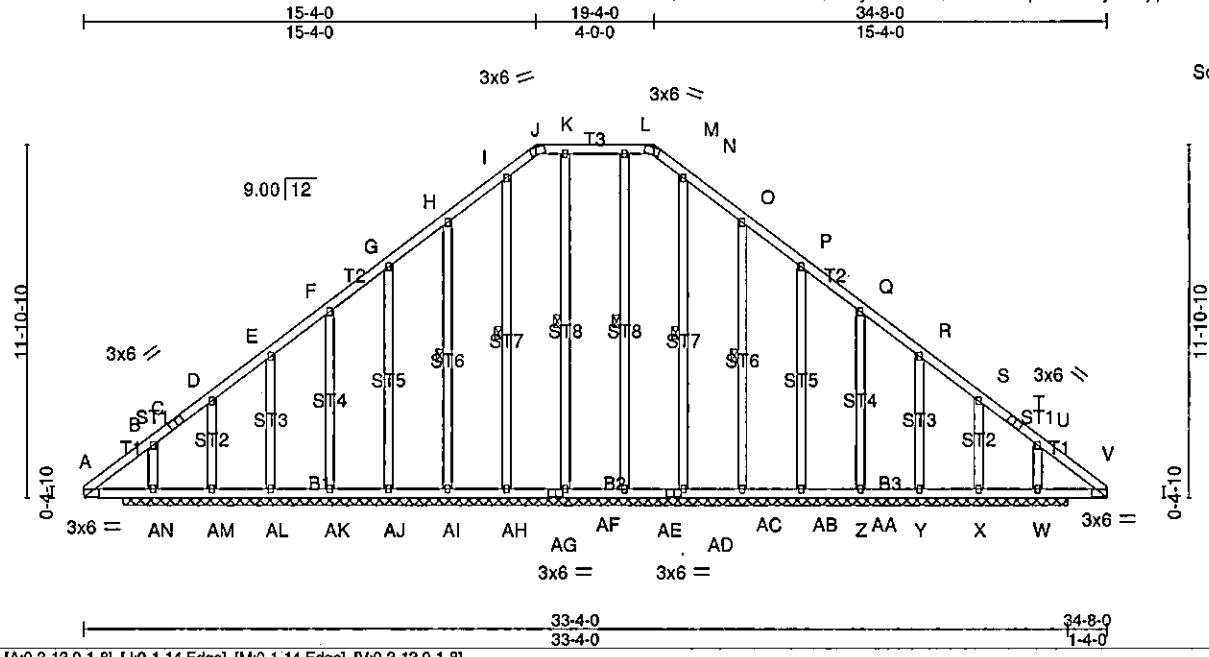


Plate Offsets (X,Y)-- [A:0-3-13,0-1-8], [J:0-1-14,Edge], [M:0-1-14,Edge], [V:0-3-13,0-1-8]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.23	Vert(LL)	n/a	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.34	Vert(TL)	n/a	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.14	Horz(TL)	0.01	W	n/a		
BCDL 10.0	Code IRC2009/TPI2007		Matrix-SH						
								Weight: 275 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except 2-0-0 oc purlins (10-0-0 max.): J-M.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt
 K-AF, I-AH, H-AI, L-AE, N-AC, O-AB

REACTIONS. (lb/size) AF=191/32-0-0 (min. 0-4-6), AH=194/32-0-0 (min. 0-4-6), AI=152/32-0-0 (min. 0-4-6), AJ=162/32-0-0 (min. 0-4-6), AK=157/32-0-0 (min. 0-4-6), AL=170/32-0-0 (min. 0-4-6), AM=120/32-0-0 (min. 0-4-6), AN=241/32-0-0 (min. 0-4-6), AE=191/32-0-0 (min. 0-4-6), AC=194/32-0-0 (min. 0-4-6), AB=152/32-0-0 (min. 0-4-6), AA=162/32-0-0 (min. 0-4-6), Z=157/32-0-0 (min. 0-4-6), Y=170/32-0-0 (min. 0-4-6), X=120/32-0-0 (min. 0-4-6), W=241/32-0-0 (min. 0-4-6)
 Max Horz AN=348(LC 4)
 Max Uplift AF=6(LC 4), AI=108(LC 5), AJ=71(LC 5), AK=83(LC 5), AL=54(LC 5), AM=283(LC 4), AN=266(LC 3), AE=1(LC 3), AB=108(LC 6), AA=71(LC 6), Z=82(LC 6), Y=56(LC 6), X=265(LC 3), W=242(LC 4)
 Max Grav AF=191(LC 1), AH=194(LC 1), AI=154(LC 9), AJ=162(LC 9), AK=157(LC 1), AL=177(LC 9), AM=288(LC 3), AN=287(LC 4), AE=191(LC 1), AC=194(LC 1), AB=154(LC 10), AA=162(LC 10), Z=157(LC 1), Y=177(LC 10), X=271(LC 4), W=278(LC 10)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=193/176, B-C=221/232, C-D=216/245, D-E=108/181, E-F=73/198, F-G=20/250, G-H=0/300, H-I=0/369, I-J=0/327, J-K=0/320, K-L=0/320, L-M=0/320, M-N=0/325, N-O=0/362, O-P=0/293, P-Q=5/244, Q-R=57/192, R-S=94/166, S-T=196/226, T-U=202/213, U-V=180/163
 BOT CHORD A-AN=162/199, AM-AN=149/186, AL-AM=149/186, AK-AL=149/186, AJ-AK=149/186, AI-AJ=149/186, AH-AI=149/186, AG-AH=149/186, AF-AG=149/186, AE-AF=149/186, AD-AE=149/186, AC-AD=149/186, AB-AC=149/186, AA-AB=149/186, Z-AA=149/186, Y-Z=149/186, X-Y=149/186, W-X=149/186, V-W=149/186
 WEBS K-AF=151/30, I-AH=154/12, H-AI=114/131, G-AJ=121/96, F-AK=119/103, E-AL=127/92, D-AM=130/164, B-AN=175/90, L-AE=151/25, N-AC=154/7, O-AB=114/132, P-AA=121/96, Q-Z=119/103, R-Y=127/92, S-X=123/156, U-W=175/86

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only.
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) All plates are 2x3 MT20 unless otherwise indicated.
 - 6) Gable studs spaced at 2-0-0 oc.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint AF, 108 lb uplift at joint AI, 71 lb uplift at joint AJ, 83 lb uplift at joint AK, 54 lb uplift at joint AL, 283 lb uplift at joint AM, 266 lb uplift at joint AN, 1 lb uplift at joint AE, 108 lb uplift at joint AB, 71 lb uplift at joint AA, 82 lb uplift at joint Z, 56 lb uplift at joint Y, 265 lb uplift at joint X and 242 lb uplift at joint W.
 - 10) Non Standard bearing condition. Review required.
 - 11) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

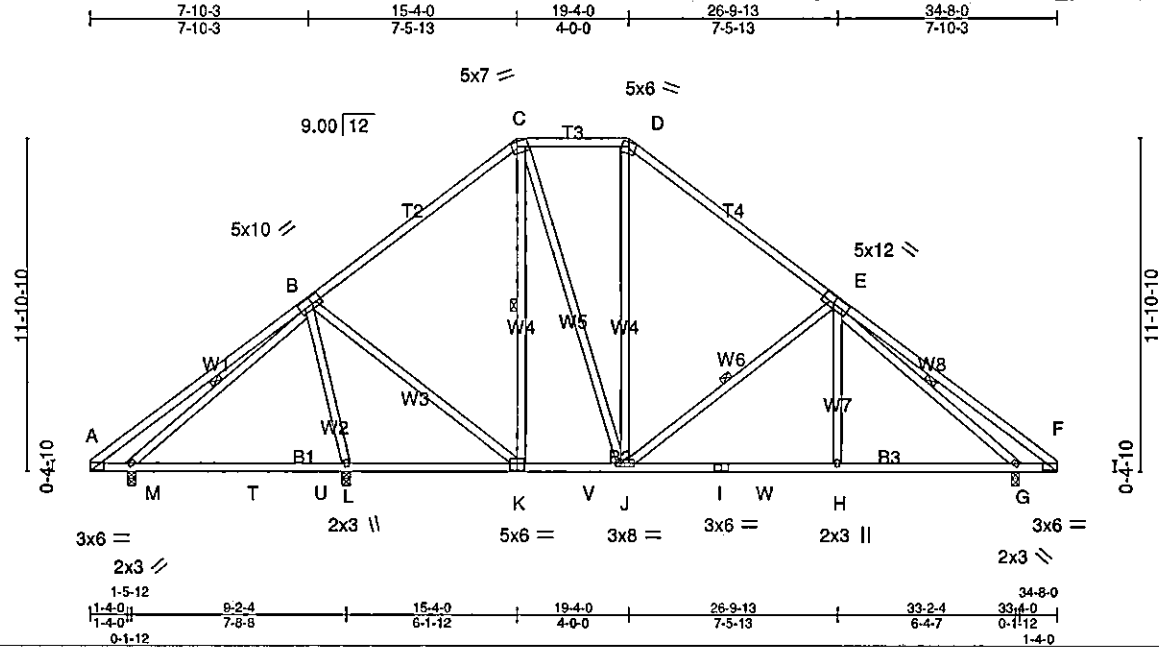


Plate Offsets (X,Y)-- [A:0-6-0,0-0-6], [B:0-4-0,0-3-0], [C:0-2-4,0-2-0], [E:0-5-4,0-3-0], [F:0-6-0,0-0-6], [K:0-3-0,0-3-0]

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2009/TPI2007	CSI. TC 0.73 BC 0.54 WB 0.83 Matrix-MSH	DEFL. in (loc) l/defl L/d Vert(LL) -0.09 H-J >999 240 Vert(TL) -0.23 H-J >999 180 Horz(TL) 0.03 G n/a n/a	PLATES GRIP MT20 244/190 Weight: 236 lb FT = 20%
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LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3	BRACING- TOP CHORD Structural wood sheathing directly applied or 4-5-2 oc purlins, except 2-0-0 oc purlins (6-0-0 max.); C-D. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: K-L WEBS 1 Row at midpt C-K, E-J, B-M, E-G
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REACTIONS. (lb/size) L=1350/0-3-8 (min. 0-2-2), M=531/0-3-8 (min. 0-1-8), G=1199/0-3-8 (min. 0-1-14)
 Max Horz M=-346(LC 3)
 Max Uplift L=-198(LC 5), M=-71(LC 6), G=-200(LC 6)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-260/37, B-C=-753/247, C-D=-599/272, D-E=-899/244, E-F=-288/74
 BOT CHORD A-M=0/134, M-T=-161/232, T-U=-161/232, L-U=-161/232, K-L=-131/193, K-V=-68/479, J-V=-68/479, I-J=-10/976, I-W=-10/976, H-W=-10/976, G-H=-9/978, F-G=0/144
 WEBS B-L=-1148/299, B-K=0/685, C-K=-311/25, C-J=-104/436, D-J=-36/190, E-J=-487/252, E-H=0/294, B-M=-124/183, E-G=-1152/122

- NOTES-
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint L, 71 lb uplift at joint M and 200 lb uplift at joint G.
 - 7) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job 18090552	Truss A3	Truss Type Piggyback Base	Qty 1	Ply 1	282Hybrid 2009
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC
 Job Reference (optional)
 8/22/2018 8:22:05 AM Aug 13 2018 MiTek Industries, Inc. Thu Sep 13 11:04:13 2018 Page 1
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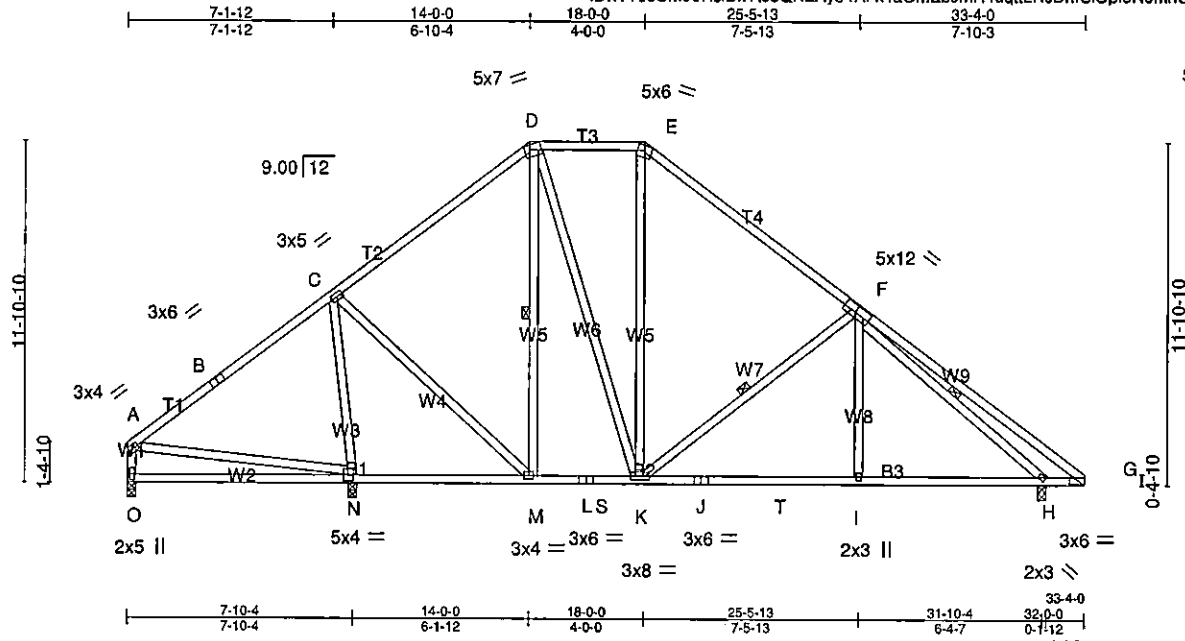


Plate Offsets (X,Y)-- [A:Edge,0-1-8], [D:0-2-4,0-2-0], [F:0-5-4,0-3-0], [G:0-6-0,0-0-6], [O:0-2-8,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.71	in (loc) I/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.54	Vert(LL) -0.09 I-K >999 240		
BCLL 0.0 *	Lumber DOL 1.15	WB 0.95	Vert(TL) -0.23 I-K >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MSH	Horz(TL) 0.03 H n/a n/a		
	Code IRC2009/TPI2007			Weight: 232 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.2

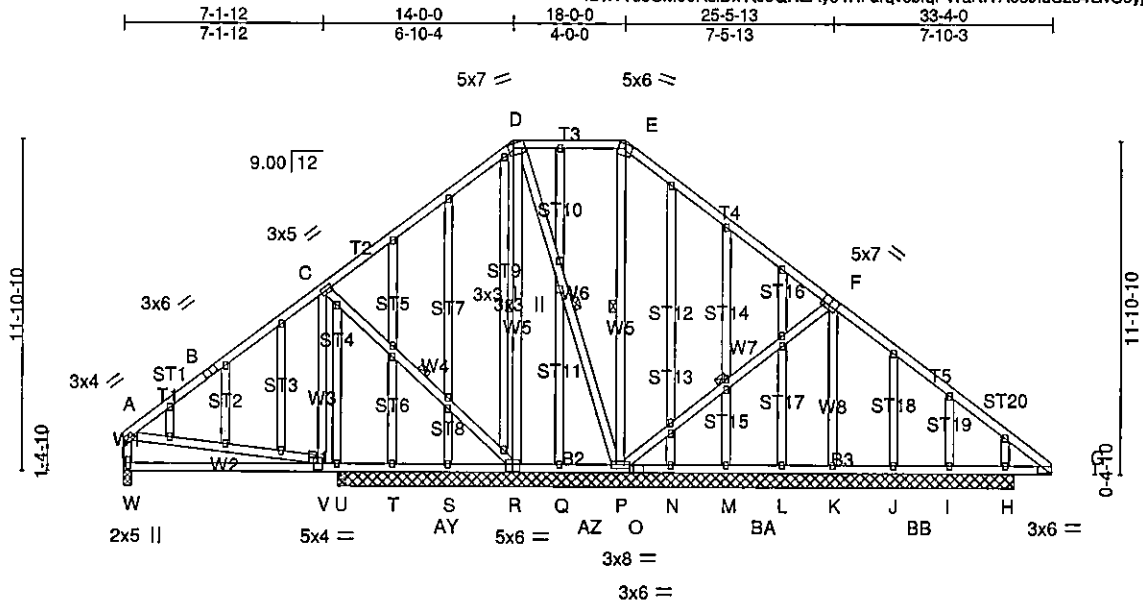
BRACING-
 TOP CHORD Structural wood sheathing directly applied or 4-6-1 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.); D-E.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt D-M, F-K, F-H

REACTIONS. (lb/size) O=341/0-3-8 (min. 0-1-8), N=1330/0-3-8 (min. 0-2-1), H=1189/0-3-8 (min. 0-1-14)
 Max Horz O=365(LC 3)
 Max Uplift O=45(LC 6), N=190(LC 5), H=195(LC 6)
 Max Grav O=347(LC 8), N=1330(LC 1), H=1189(LC 1)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-272/100, B-C=-164/129, C-D=-730/243, D-E=-585/266, E-F=-883/236, F-G=-286/73, A-O=-278/86
 BOT CHORD N-O=344/395, M-N=-183/242, L-M=-88/464, L-S=-88/464, K-S=-88/464, J-K=-4/965, J-T=-4/965, I-T=-4/965, H-I=-3/967, G-H=0/141
 WEBS C-N=-1176/265, C-M=-14/640, D-M=-301/53, D-K=-113/441, E-K=-39/177, F-K=-489/252, F-I=0/295, A-N=-142/155, F-H=-1140/115

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph; TC DL=6.0psf; BC DL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BC DL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint O, 190 lb uplift at joint N and 195 lb uplift at joint H.
 - This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



Scale = 1:79.1

Plate Offsets (X,Y)-- [A:Edge,0-1-8], [D:0-2-4,0-2-0], [F:0-3-8,0-3-4], [G:0-6-0,0-0-6], [O:0-1-13,0-1-8], [R:0-3-0,0-3-0], [W:0-2-8,0-1-0]

LOADING (psf)	SPACING-	CSI.	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.75	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.42	Vert(LL) -0.06 V-W >999 240		
BCLL 0.0	Lumber DOL 1.15	WB 0.31	Vert(TL) -0.16 V-W >568 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MSH	Horz(TL) 0.01 H n/a n/a		
	Code IRC2009/TPI2007			Weight: 348 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.); D-E.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt C-R, D-R, D-P, E-P, F-P

REACTIONS. (lb/size) W=436/0-3-8 (min. 0-1-8), R=538/24-3-8 (min. 0-4-3), P=603/24-3-8 (min. 0-4-3), K=540/24-3-8 (min. 0-4-3), Q=107/24-3-8 (min. 0-4-3), S=55/24-3-8 (min. 0-4-3), T=23/24-3-8 (min. 0-4-3), U=202/24-3-8 (min. 0-4-3), N=34/24-3-8 (min. 0-4-3), M=48/24-3-8 (min. 0-4-3), L=104/24-3-8 (min. 0-4-3), J=130/24-3-8 (min. 0-4-3), I=32/24-3-8 (min. 0-4-3), H=305/24-3-8 (min. 0-4-3)
 Max Horz W=-365(LC 3)
 Max Uplift W=-28(LC 6), R=-143(LC 5), P=-157(LC 6), K=-128(LC 6), T=-3(LC 3), U=-103(LC 5), I=-35(LC 10), H=-154(LC 6)
 Max Grav W=454(LC 9), R=542(LC 9), P=603(LC 1), K=577(LC 10), Q=107(LC 9), S=106(LC 2), T=54(LC 10), U=211(LC 9), U=202(LC 1), N=73(LC 2), M=83(LC 2), L=104(LC 10), J=131(LC 10), I=59(LC 6), H=314(LC 10)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=417/48, B-C=308/77, C-D=104/181, D-E=0/191, E-F=72/237, F-G=73/208, A-W=385/65
 BOT CHORD V-W=350/395, U-V=177/315, T-U=177/315, T-AY=177/315, S-AY=177/315, R-S=177/315, Q-R=98/272, P-AZ=98/272,
 O-P=48/98, N-O=48/98, M-N=48/98, M-BA=48/98, L-BA=48/98, K-L=48/98, J-K=52/99, J-BB=52/99, I-BB=52/99, H-I=52/99, G-H=52/99
 WEBS C-V=6/249, C-R=390/218, D-R=213/54, D-P=136/81, E-P=391/82, F-P=74/216, F-K=456/155, A-V=81/176

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only.
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) All plates are 2x3 MT20 unless otherwise indicated.
 - 6) Gable studs spaced at 2-0-0 oc.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint W, 143 lb uplift at joint R, 157 lb uplift at joint P, 128 lb uplift at joint K, 3 lb uplift at joint T, 103 lb uplift at joint U, 35 lb uplift at joint I and 154 lb uplift at joint H.
 - 10) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job 18090552	Truss A5	Truss Type Piggyback Base	Qty 11	Ply 1	282Hybrid 2009
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC
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 8/22/2018 8:22:05 AM MTEK Industries, Inc. Thu Sep 13 11:04:23 2018 Page 1

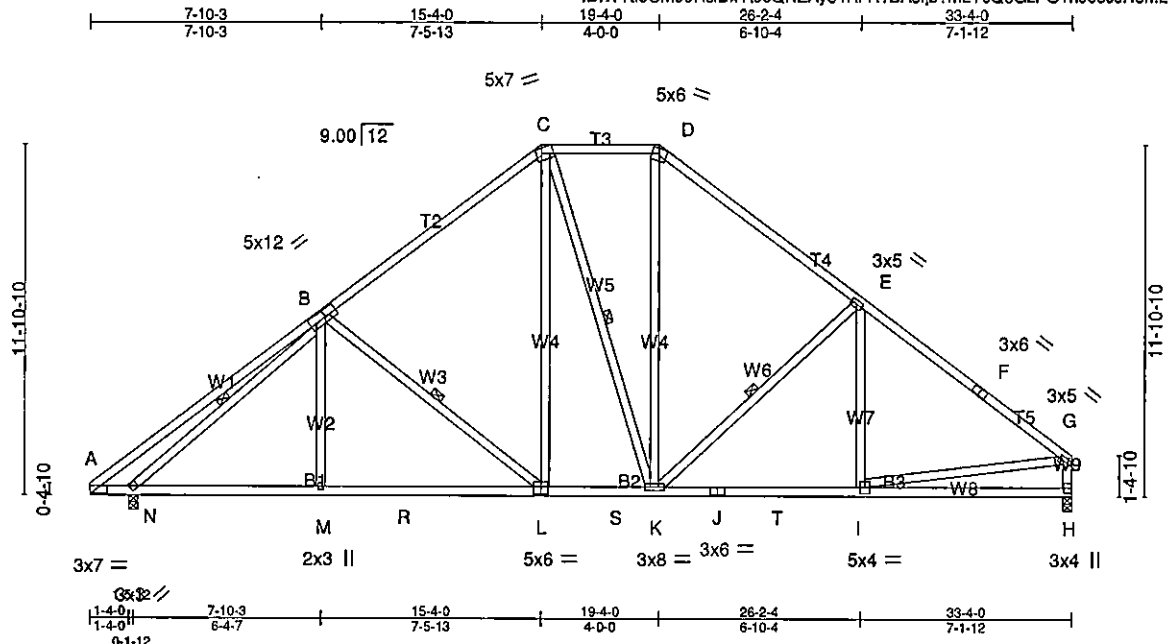


Plate Offsets (X,Y)-- [A:0-7-0,0-0-6], [B:0-5-0,0-3-0], [C:0-2-4,0-2-0], [G:Edge,0-1-8], [L:0-3-0,0-3-0]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.78	Vert(LL) -0.12	L-M	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.64	Vert(TL) -0.29	L-M	>999	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.65	Horz(TL) 0.07	H	n/a	n/a		
BCDL 10.0	Code IRC2009/TPI2007	Matrix-MSH					Weight: 231 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (5-7-9 max.); C-D.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt B-L, C-K, E-K, B-N

REACTIONS. (lb/size) H=1433/0-3-8 (min. 0-2-4), N=1546/0-3-8 (min. 0-2-7)
 Max Horz N=365(LC 4)
 Max Uplift H=164(LC 6), N=219(LC 5)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-346/76, B-C=-1432/283, C-D=-1014/297, D-E=-1406/286, E-F=-1655/223, F-G=-1766/194, G-H=-1375/199
 BOT CHORD A-N=0/172, M-N=205/1362, M-R=206/1359, L-R=206/1359, L-S=101/1023, K-S=101/1023, J-K=50/1324, J-T=50/1324, I-T=50/1324, H-I=48/139
 WEBS B-M=0/290, B-L=-448/248, C-L=-88/487, C-K=-183/178, D-K=-98/472, E-K=-446/251, E-I=0/184, B-N=-1609/150, G-I=-31/1202

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCCL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCCL = 10.0psf.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint H and 219 lb uplift at joint N.
 - 7) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job 18090552	Truss A6	Truss Type Piggyback Base	Qty 5	Ply 1	282Hybrid 2009
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC 8.220 s Aug 13 2018 MiTek Industries, Inc. Thu Sep 13 11:04:28 2018 Page 1
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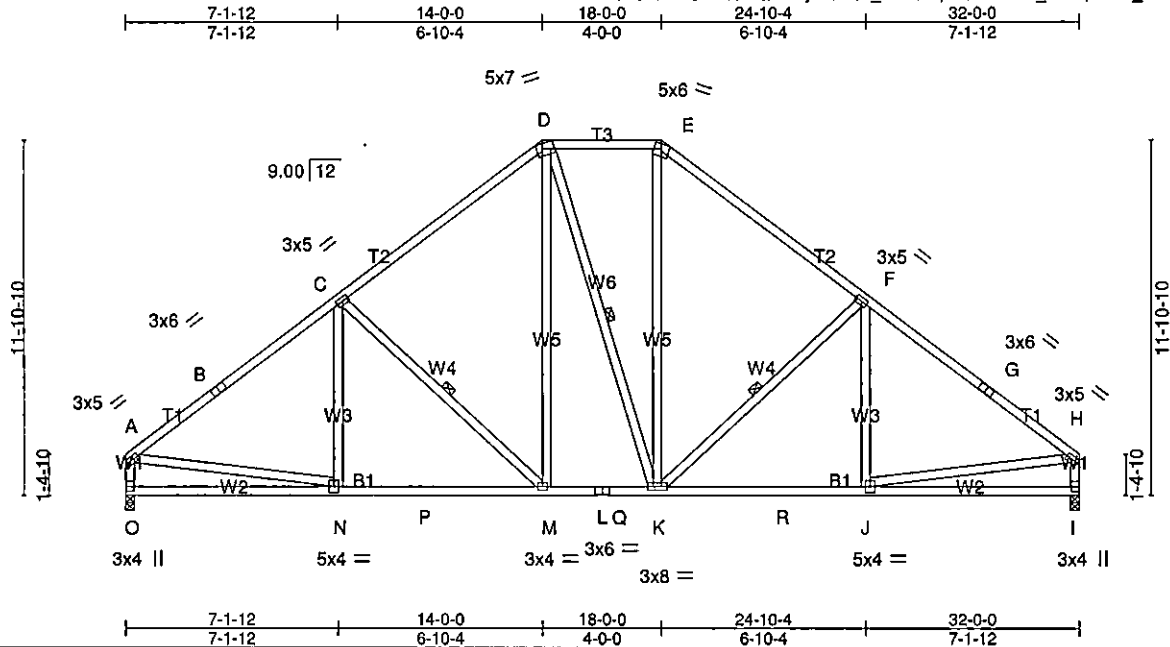


Plate Offsets (X,Y) - [A:Edge,0-1-8], [D:0-2-4,0-2-0], [H:Edge,0-1-8]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.68	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.15	BC 0.60	Vert(LL) -0.11 M-N >999 240		
BCLL 0.0	Lumber DOL 1.15	WB 0.50	Vert(TL) -0.24 M-N >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-MSH	Horz(TL) 0.05 n/a n/a		
	Code IRC2009/TPI2007			Weight: 226 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (5-8-12 max.): D-E.
 BOT CHORD Rigid ceiling directly applied or 9-7-6 oc bracing.
 WEBS 1 Row at midpt C-M, D-K, F-K

REACTIONS. (lb/size) O=1431/0-3-8 (min. 0-2-4), I=1427/0-3-8 (min. 0-2-4)
 Max Horz O=364(LC 4)
 Max Uplift O=165(LC 5), I=165(LC 6)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-1764/195, B-C=-1654/225, C-D=-1403/290, D-E=-1008/297, E-F=-1398/290, F-G=-1648/225, G-H=-1758/195, A-O=-1374/200, H-I=-1369/200
 BOT CHORD N-O=-370/393, N-P=-206/1323, M-P=-206/1323, L-M=-105/1012, L-Q=-105/1012, K-Q=-105/1012, K-R=-53/1318, J-R=-53/1318, I-J=-48/139
 WEBS C-N=0/185, C-M=-447/252, D-M=-110/489, D-K=-178/179, E-K=-92/460, F-K=-446/251, F-J=0/184, A-N=-32/1201, H-J=-34/1195

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint O and 165 lb uplift at joint I.
 - 7) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANS/TPI 1.
 - 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job 18090552	Truss A7	Truss Type Piggyback Base	Qty 9	Ply 1	282Hybrid 2009
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC
 ID: WYti5CM90RsiDxYU5QNZAYe4RI-9woyz3rseRb3DyPBYYWaNSPXv5LyPiyIMdFTNWgYe4LY
 8.220 s Aug 13 2018 MiTek Industries, Inc. Thu Sep 13 11:04:33 2018 Page 1

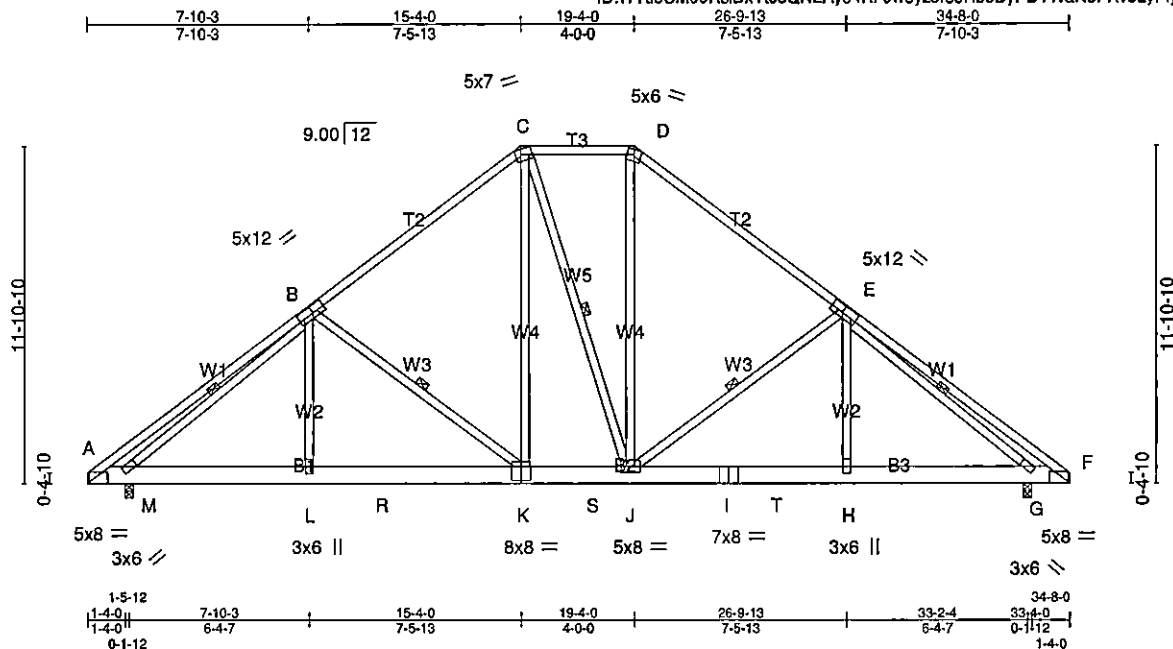


Plate Offsets (X,Y) -- [A:0-4-0,0-1-6], [B:0-5-4,0-3-0], [C:0-2-4,0-2-0], [E:0-5-4,0-3-0], [F:0-4-0,0-1-6], [K:0-4-0,0-6-0]

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.37	Vert(LL) -0.05 K-L >999 240	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.26	Vert(TL) -0.11 K-L >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.31	Horz(TL) 0.04 G n/a n/a	
BCDL 10.0	Code IRC2009/TPI2007	Matrix-MSH		Weight: 288 lb FT = 20%

LUMBER-
 TOP CHORD 2x4 SP SS
 BOT CHORD 2x8 SP No.1
 WEBS 2x4 SP SS

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 4-10-14 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): C-D.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt B-K, C-J, E-J, B-M, E-G

REACTIONS. (lb/size) M=1538/0-3-8 (min. 0-2-7), G=1533/0-3-8 (min. 0-2-6)
 Max Horz M=-343(LC 3)
 Max Uplift M=-216(LC 5), G=-216(LC 6)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-715/193, B-C=-1448/285, C-D=-1036/299, D-E=-1440/285, E-F=-716/193
 BOT CHORD A-M=-13/474, L-M=-191/1382, L-R=-192/1380, K-R=-192/1380, K-S=-80/1036, J-S=-80/1036, I-J=-58/1373, I-T=-58/1373, H-T=-58/1373, G-H=-58/1375, F-G=-14/474
 WEBS B-L=0/259, B-K=-454/259, C-K=-91/471, C-J=-173/181, D-J=-84/459, E-J=-451/259, E-H=0/253, B-M=-1206/61, E-G=-1197/58

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint M and 216 lb uplift at joint G.
 - This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

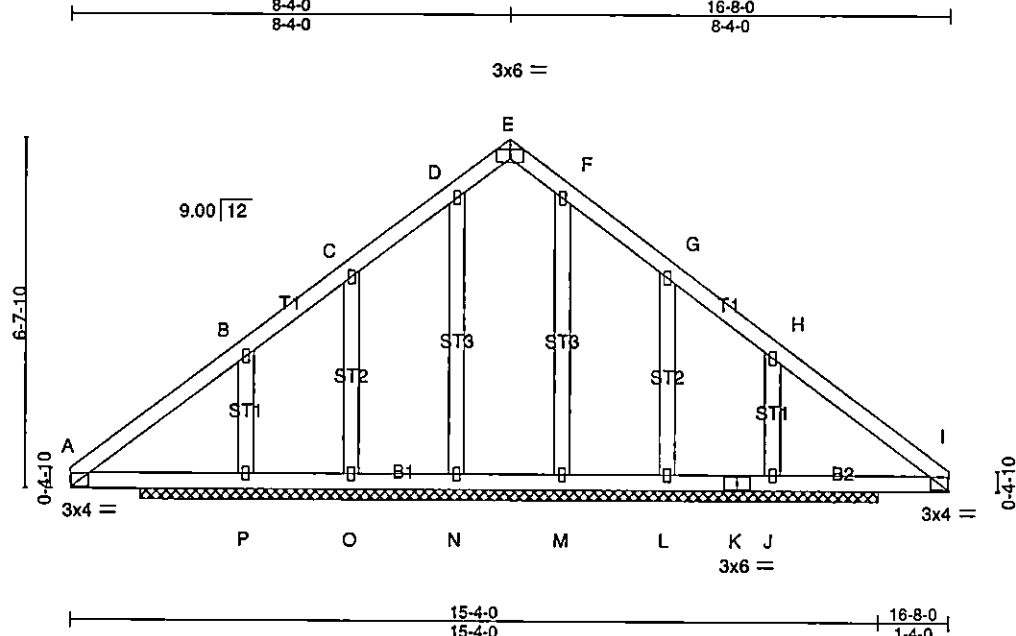


Plate Offsets (X,Y)-- [E=0-3-0,Edge] 15'-4" 16'-8" 1'-4"

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.25	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.23	Vert(TL) n/a - n/a 999	
BCLL 0.0	Rep Stress Incr YES	WB 0.15	Horz(TL) -0.00 J n/a n/a	
BCDL 10.0	Code IRC2009/TP12007	Matrix-SH		Weight: 91 lb FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 OTHERS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purfins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (lb/size) N=294/14-0-0 (min. 0-2-1), O=71/14-0-0 (min. 0-2-1), P=302/14-0-0 (min. 0-2-1), M=294/14-0-0 (min. 0-2-1), L=71/14-0-0 (min. 0-2-1), J=302/14-0-0 (min. 0-2-1)
 Max Horz P=189(LC 4)
 Max Uplift O=159(LC 4), P=99(LC 6), L=157(LC 3), J=98(LC 5)
 Max Grav N=294(LC 1), O=129(LC 3), P=378(LC 9), M=294(LC 1), L=127(LC 4), J=378(LC 10)

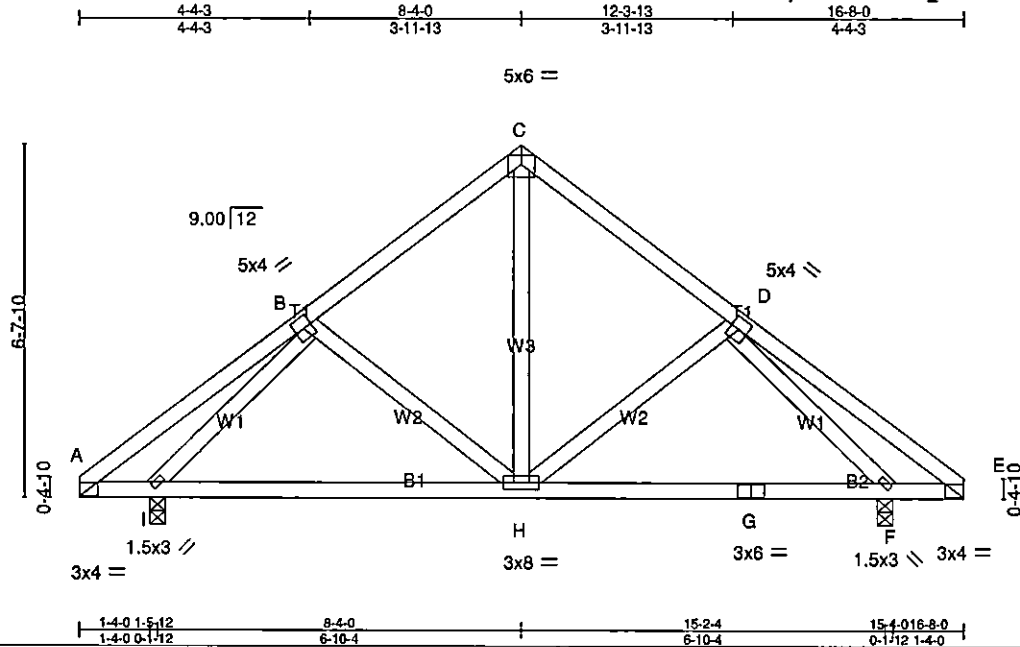
FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-70/234, B-C=-29/175, C-D=0/218, D-E=0/126, E-F=0/126, F-G=0/218, G-H=-26/175, H-I=-69/234
 BOT CHORD A-P=-132/97, O-P=-132/97, N-O=-132/97, M-N=-132/97, L-M=-132/97, K-L=-132/97, J-K=-132/97, I-J=-132/97
 WEBS D-N=-248/0, C-O=60/139, B-P=-236/118, F-M=-248/0, G-L=-60/138, H-J=-236/118

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only.
 - 4) All plates are 1.5x3 MT20 unless otherwise indicated.
 - 5) Gable studs spaced at 2-0-0 oc.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint O, 99 lb uplift at joint P, 157 lb uplift at joint L and 98 lb uplift at joint J.
 - 9) Non Standard bearing condition. Review required.
 - 10) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1.

LOAD CASE(S) Standard

Job 18090552	Truss B2	Truss Type Common	Qty 5	Ply 1	282Hybrid 2009
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC
 8.220 s Aug 13 2018 MTEK Industries, Inc. Thu Sep 13 11:04:35 2018 Page 1
 ID:WYti5CM90RsiDxYUj5QNZAye4Rl-ZVT5b5tlxM_d4Q7mDe74j29T9Zx_vKjpJdi1X7ye4Lw



LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL) -0.04 F-H >999 240	MT20	244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.40	Vert(TL) -0.09 F-H >999 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.22	Horz(TL) 0.01 F n/a n/a		
BCDL 10.0	Code IRC2009/TPI2007	Matrix-MSH		Weight: 92 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purfins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) I=667/0-3-8 (min. 0-1-8), F=667/0-3-8 (min. 0-1-8)
 Max Horz I=-187(LC 3)
 Max Uplift I=-125(LC 5), F=-125(LC 6)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-147/69, B-C=-478/131, C-D=-478/131, D-E=-147/69
 BOT CHORD A-I=0/117, H-I=93/391, G-H=-24/391, F-G=-24/391, E-F=0/116
 WEBS C-H=-49/306, D-H=-115/148, B-H=-115/147, B-I=-576/178, D-F=-576/176

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 100mph; TCCL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint I and 125 lb uplift at joint F.
 - This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

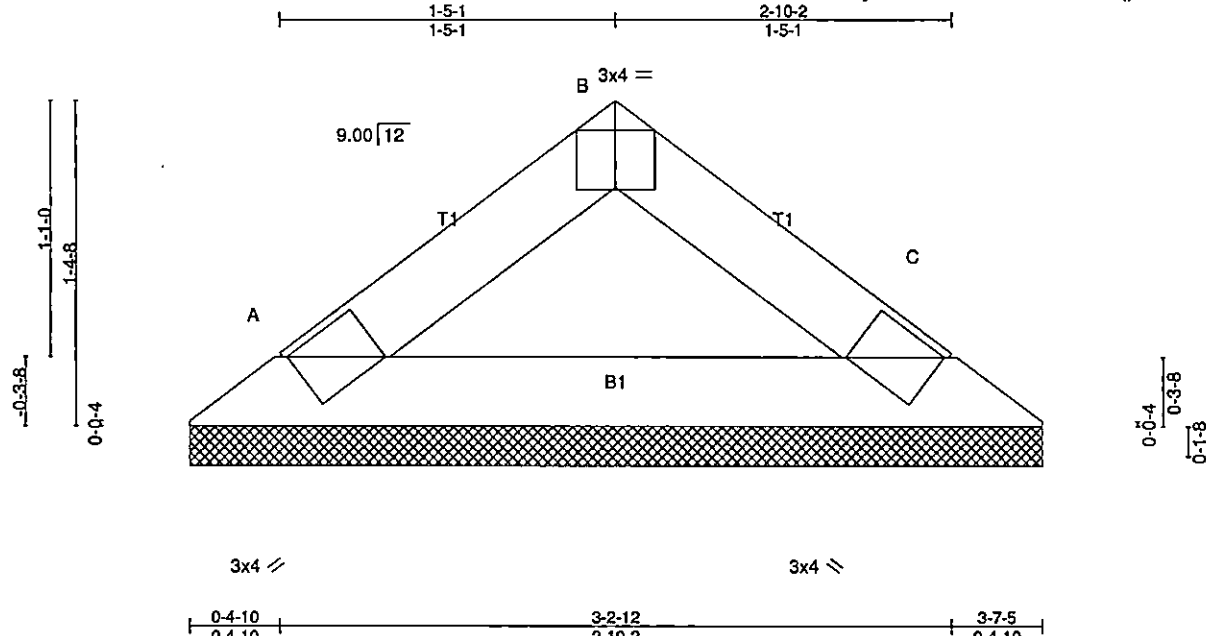


Plate Offsets (X,Y) -- (B:0-2-0,Edge)

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.15	TC 0.03	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.09	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	C	n/a	n/a		
BCDL 10.0	Code IRC2009/TPI2007		Matrix-P						Weight: 11 lb	FT = 20%

LUMBER-
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

BRACING-
 TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

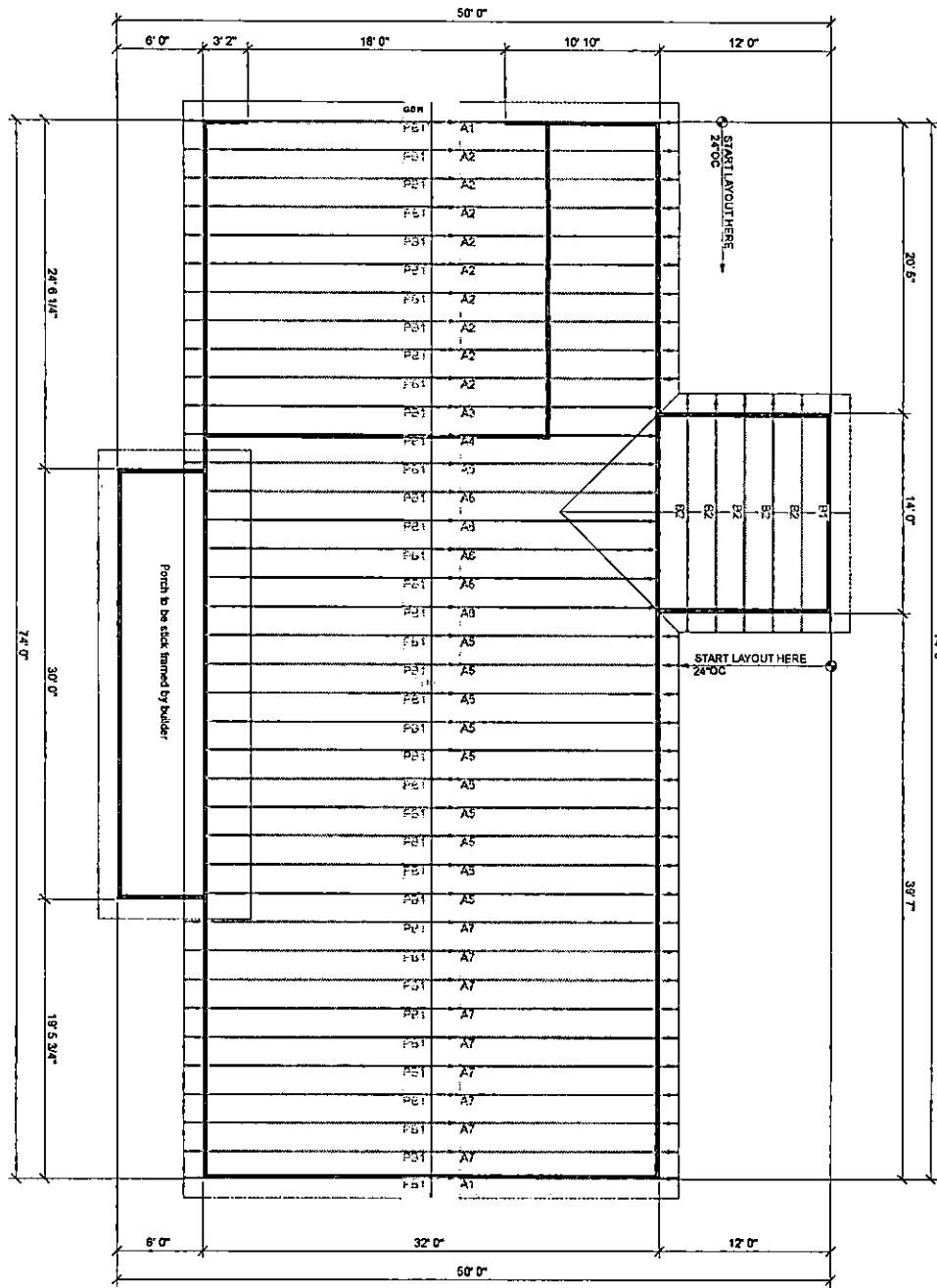
REACTIONS. (lb/size) A=112/3-7-5 (min. 0-1-8), C=112/3-7-5 (min. 0-1-8)
 Max Horz A=-31(LC 3)
 Max Uplift A=15(LC 5), C=15(LC 6)

FORCES. (lb) - Maximum Compression/Maximum Tension
 TOP CHORD A-B=-90/33, B-C=-90/33
 BOT CHORD A-C=-9/56

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 100mph; TC DL=6.0psf; BC DL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Gable requires continuous bottom chord bearing.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint A and 15 lb uplift at joint C.
 - 7) This truss is designed in accordance with the 2009 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 8) See standard piggyback truss connection detail for connection to base truss.

LOAD CASE(S) Standard

TRUSS TO WALL CONNECTIONS, IF SHOWN, ARE FOR LIFT ONLY AND DO NOT CONSIDER LATERAL LOADS. ALL CONNECTORS ON THIS PROJECT ARE TO BE INSTALLED PER THE CONNECTOR MANUFACTURER'S SPECIFICATIONS. ALL CONNECTORS SHOWN THAT ARE NOT "TRUSS TO TRUSS" ARE SUGGESTIONS ONLY AND ARE TO BE VERIFIED BY THE BUILDING DESIGNER OR ENGINEER OF RECORD FOR SUITABILITY TO THIS PARTICULAR PROJECT. UFP MID-ATLANTIC, LLC ACCEPTS NO RESPONSIBILITY FOR THE SPECIFIC APPLICATION OR SUITABILITY OF ANY CONNECTOR THAT IS NOT "TRUSS TO TRUSS" AS THEY APPLY TO THIS SPECIFIC STRUCTURE.



Scale: 1/8" = 1'-0"
 Date: 11/13/2015
 Project: 15090552

ROOF AREA: 3939.38 _ RIDGE LINE: 97 _ VALLEY LINES: 26.68 _ HIP LINES: 0 _ \triangle Indicates Left End of Truss

15090552

Customer: **CUF Greg Gage**
 Job Name: **Yates**
 Scale: 1/8" = 1'-0"
 Revision: 01/13/2015
 Revision Date: _____

NOTES: THIS DRAWING IS THE PROPERTY OF UFP MID-ATLANTIC, LLC AND IS NOT TO BE REPRODUCED OR COPIED IN ANY MANNER WITHOUT THE WRITTEN PERMISSION OF UFP MID-ATLANTIC, LLC. THIS DRAWING IS FOR INFORMATION ONLY AND IS NOT TO BE USED FOR CONSTRUCTION. UFP MID-ATLANTIC, LLC AND ITS SUBSIDIARIES SHALL NOT BE RESPONSIBLE FOR ANY ERRORS OR OMISSIONS IN THIS DRAWING. THE USER OF THIS DRAWING SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS PRIOR TO CONSTRUCTION. UFP MID-ATLANTIC, LLC SHALL NOT BE RESPONSIBLE FOR ANY DAMAGES OR INJURIES RESULTING FROM THE USE OF THIS DRAWING. UFP MID-ATLANTIC, LLC SHALL NOT BE RESPONSIBLE FOR ANY DAMAGES OR INJURIES RESULTING FROM THE USE OF THIS DRAWING. UFP MID-ATLANTIC, LLC SHALL NOT BE RESPONSIBLE FOR ANY DAMAGES OR INJURIES RESULTING FROM THE USE OF THIS DRAWING.



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 LOCUST, NC PHONE (704) 388-2028
 LIBERTY, NC PHONE (800) 648-4038
 PEANSBORO, VA PHONE (800) 397-8571

1. TEMPORARY BRACING TO BE INSTALLED W/T.P.I. STANDARD DCSI-B1.
2. SEE ENGINEERED DRAWING FOR PERMANENT BRACING MINIMUM REQUIREMENTS.
3. FRAMER TO VERIFY ALL DIMENSIONS, DROP, & RISE LOCATIONS PRIOR TO TRUSS PLACEMENT.
4. BLDG FRAMER RESPONSIBLE FOR ADJUSTMENT OF TRUSS SPACING TO MISS PLUMBING GRIPS, UNLESS NOTED OTHERWISE.

This layout is not an engineered drawing. This drawing was created to establish truss placement only. It is the responsibility of the builder to provide adequate support for all the elements shown in this drawing.

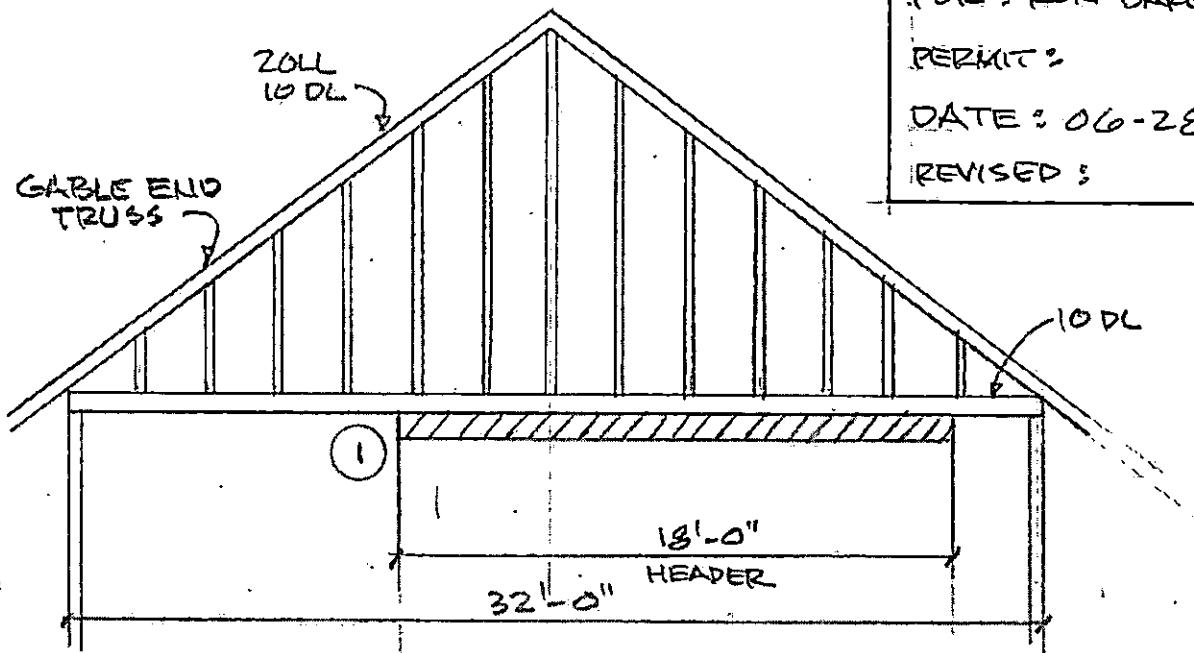
YATES Plan No. 2

FOR: RON BAREFOOT

PERMIT:

DATE: 06-28-18

REVISED:



① $W = (2'-0'')(18'-0'')(40 \text{ PSF})$

$W = 1440 \text{ LBS}$

$w = 80 \text{ PLF}$

Choose (2) $1\frac{3}{4}'' \times 11\frac{1}{4}''$ LVL with (2) 2x4 studs each end
(see attached)

GANG LAM LVL BY LOUISIANA PACIFIC 2950 F_b-2.0'E

GANG-LAM LVL 2950 1 1/2" O.E. MAXIMUM UNIFORM LOAD (PLF)

ALLOWABLE FLOOR LOADS (PLF) 100%

Beam Span (ft)	1 Ply 1 1/4 x 7 1/4			1 Ply 1 1/4 x 9 1/4			1 Ply 1 1/4 x 9 1/2			1 Ply 1 1/4 x 11 1/4			1 Ply 1 1/4 x 11 3/4			1 Ply 1 1/4 x 14			1 Ply 1 1/4 x 16 * Refer To Note 4			1 Ply 1 1/4 x 18 * Refer To Note 4		
	Live Load Deflection		Total Load	Live Load Deflection		Total Load	Live Load Deflection		Total Load	Live Load Deflection		Total Load	Live Load Deflection		Total Load	Live Load Deflection		Total Load	Live Load Deflection		Total Load	Live Load Deflection		Total Load
	L/360	L/480	L/240	L/360	L/480	L/240	L/360	L/480	L/240	L/360	L/480	L/240	L/360	L/480	L/240	L/360	L/480	L/240	L/360	L/480	L/240	L/360	L/480	L/240
6	681	522	777	1048	1016	1046	1082	1082	1082	1348	1348	1348	1450	1450	1450	1827	1827	1827	2233	2233	2233	2698	2698	2698
7	443	337	639	864	669	864	893	720	893	1102	1102	1102	1181	1181	1181	1470	1470	1470	1772	1772	1772	2110	2110	2110
8	303	229	441	603	461	736	649	497	760	932	794	932	996	918	996	1229	1229	1229	1469	1469	1469	1732	1732	1732
9	215	163	315	434	330	607	467	356	637	748	574	807	861	667	861	1056	1041	1056	1254	1254	1254	1468	1468	1468
10	158	120	231	321	244	467	347	263	504	559	427	704	649	497	758	925	784	925	1094	1094	1094	1274	1274	1274
11	120	90	174	244	185	355	263	199	384	428	325	584	498	380	644	785	603	823	969	870	969	1125	1125	1125
12	93	70	134	189	143	276	205	155	298	334	253	484	389	296	543	618	473	732	870	686	870	1007	945	1007
13	73	55	105	150	113	218	162	122	235	265	201	385	310	235	449	495	377	625	717	550	790	911	761	911
14	59	44	84	121	91	175	130	96	189	214	162	310	250	189	363	401	305	541	584	446	689	807	621	832
15	48	36	68	98	74	142	106	80	154	175	132	253	205	155	297	329	250	472	481	367	601	668	512	744
16	40	-	55	81	61	117	88	66	126	145	109	209	170	128	245	274	207	396	401	305	529	559	427	656
17	33	-	46	68	51	97	74	55	105	121	91	174	142	107	205	230	174	332	337	256	469	472	359	582
18	-	-	38	58	43	81	62	47	88	102	77	147	120	91	172	194	147	281	286	217	413	401	305	520
19	-	-	32	49	37	68	53	40	74	87	66	124	102	77	146	166	125	239	245	185	353	344	261	467
20	-	-	-	42	32	58	46	34	63	75	57	106	88	66	125	143	108	205	211	160	304	297	225	421
21	-	-	-	37	-	50	39	-	54	65	49	91	76	57	108	124	93	177	183	138	263	258	195	371
22	-	-	-	32	-	43	34	-	47	57	43	79	66	50	93	108	81	154	160	121	229	225	170	324
23	-	-	-	-	-	37	-	-	40	50	37	68	58	44	81	95	71	134	140	106	200	198	150	284
24	-	-	-	-	-	32	-	-	35	44	33	60	51	39	71	84	63	117	124	93	176	175	132	250
25	-	-	-	-	-	-	-	-	39	-	-	52	46	34	62	74	56	103	110	83	155	155	117	221
26	-	-	-	-	-	-	-	-	35	-	-	46	41	31	55	66	50	91	98	74	138	138	104	196
27	-	-	-	-	-	-	-	-	31	-	-	41	36	-	48	59	45	81	88	66	122	124	93	175
28	-	-	-	-	-	-	-	-	-	-	-	36	33	-	43	53	40	72	79	59	109	111	84	156
29	-	-	-	-	-	-	-	-	-	-	-	-	-	-	38	48	36	64	71	53	98	100	76	140
30	-	-	-	-	-	-	-	-	-	-	-	-	-	-	34	43	33	57	64	48	88	91	68	126

How to use maximum uniform load tables:

1. Select the correct table for the beam application you need.
2. Choose the required beam span in the left column.
3. Select a beam depth from the tables that satisfies BOTH the live and total load PLF on the beam.
4. Check the bearing requirements as shown on page 8.

Example: Floor live load 480 PLF, L/360 deflection limit.
 Floor total load 660 PLF, L/240 deflection limit.
 Beam span 14' - 0"

Solution: Try 2 plies 1 1/4" x 11 3/4", which can carry:
 • Live load 2 x 250 = 500 > 480 PLF ✓ OK
 • Total load 2 x 363 = 726 > 660 PLF ✓ OK

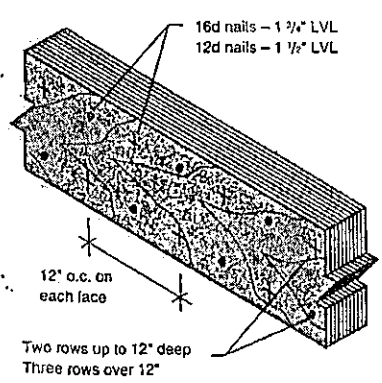
Notes (for page 6 and 7)

1. Beam spans are defined as follows: Simple span dimensions are measured from inside face of supports. Multiple span dimensions are measured from inside face of exterior supports to center line of interior supports.
2. These tables are for simple spans (with a support at each end) or for continuous (multiple span) beams if spans are equal.
3. PLF values are for a single ply of 1 1/4" Gang-Lam LVL.
 - Double the values for two plies or 3/4" thickness.
 - Triple the values for three plies or 5/4" thickness.
- * 4. For 1 1/4" x 16" beams and deeper, two plies (minimum) are required.
5. More than three plies may require special design. Contact your L-P engineered products distributor.

CONNECTION OF MULTIPLE PLY BEAMS

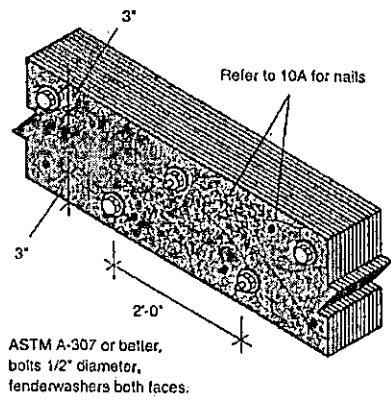
10A TOP LOADED (3 PLYS MAXIMUM)

Framing is applied on top of the beam so that each ply carries an equal load.



10B TOP LOADED 4 PLYS

Framing is applied on top of the beam so that each ply carries an equal load.



10C SIDE LOADED

The same framing is used on each side of the beam so the same load is carried on each face.

