

5.) POURED CONCRETE PAVING UNDER THE CANOPY TO BE EXCLUSIVELY FOR WORK SPACE

7.) FORM, SET, AND POUR FOUNDATIONS PER McGEE'S SITE SPECIFIC APPROVED FOUNDATION

PLAN. ALL FORMS SHALL BE REMOVED PRIOR TO McGEE'S ARRIVAL. ALL THREADS

9.) PROVIDE TEMPORARY POWER SOURCE (110 VOLTS) WITHIN 100 FEET OF THE STRUCTURE

11.) VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS. ANY DEVIATIONS FROM THESE

10.) OBTAIN ALL REQUIRED PERMITS FROM LOCAL AUTHORITIES AND ARRANGE ALL LOCAL INSPECTIONS.

DRAWINGS DUE TO FIELD CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE

SHALL BE FREE FROM DEBRIS AND DUST AND SHALL BE ACCESSIBLE.

8.) INSTALL ALL ANCHOR BOLTS W/ NUTS. SET AT PROPER ELEVATIONS WITH

AND STORAGE OF MATERIALS.

6.) REMOVE ALL OVERHEAD OBSTRUCTIONS.

NO MORE THAN 1/4" TOLERANCE.

PROJECT ENGINEER FOR MODIFICATIONS.

FOR INSTALLERS USE.



CANOPY FOUNDATION INSTALLATION CONTRACTOR SHALL DETERMINE WHICH FINISHED GRADE ELEVATION AT EACH CANOPY COLUMN IS THE LOWEST AND ESTABLISH ALL FOUNDATION LOCATIONS IN RELATION TO THAT ELEVATION. CONTRACTOR MUST VERIFY FUEL CONTAINMENT BOX SIZE AND LOCATION TO ENSURE

FOUNDATION DOES NOT INTERFERE WITH BOX INSTALLATION. TOP OF FOUNDATION DEPTH MAY BE GREATER THAT BUT NOT LESS THAN 12" BELOW THE PREVIOUSLY DETERMINED LOWEST FINISHED GRADE ELEVATION

2) ALLOWABLE SOIL BEARING CAPACITY OF 2,500 PSF PROVIDED BY THE OWNER PER THE GEOTECHNICAL REPORT BY SUMMIT DATED FEBRUARY 21, 2022, SUMMIT PROJECT No

3) FOUNDATIONS (WHERE SHOWN) HAVE BEEN SIZED FOR GIVEN LOADS AND ALLOWABLE SOIL PRESSURE. THEIR DESIGN ASSUMES THAT THERE ARE NO BURIED TANKS OR OTHER NEARBY OBSTRUCTIONS THAT WOULD BE DETRIMENTAL TO THEIR PROPER FUNCTION. THE ENGINEER OF RECORD SHALL BE NOTIFIED PRIOR TO CONSTRUCTION OF FOUNDATIONS FOR THE RESOLUTION OF ANY CONFLICT. WHERE A FOUNDATION DETAIL IS NOT SHOWN, McGEE CORPORATION AND THEIR ENGINEERS TAKE NO RESPONSIBILITY FOR THE FOUNDATION DESIGN.

4) ASTM F1554 GRADE 36 ANCHOR BOLTS & WOOD TEMPLATES

5) ALL CONCRETE WORK SHALL BE PERFORMED IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI): "BUILDING CODE REQUIREMENTS FOR REINFORCED

CONCRETE" (ACI 318-14) "SPECIFICATIONS FOR STRUCTURAL CONCRETE"

"HOT WEATHER CONCRETING"

APPROVED AS SUBMITTED

REQUESTED DELIVERY DATE:_

ELEVATION FORMS FORWARDED TO GENERAL CONTRACTOR

NOTE: SIGNED SALES ORDER, APPROVAL DRAWINGS, AND A COMPLETED

ELEVATION FORM MUST BE RECEIVED AT LEAST 3 WEEKS PRIOR TO DELIVERY

APPROVED WITH NOTED CHANGES

NUMBER APPROVED AS SUBMITTED

LAYOUT APPROVED AS SUBMITTED

APPROVED WITH NOTED CHANGES

DECALS

LIGHTS

APPROVED BY: __

OF ANY CANOPY MATERIALS.

6) ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI AND A MINIMUM UNIT WEIGHT OF 145 PCF. REINFORCING STEEL SHALL BE NEW BILLET STEEL DEFORMED BARS CONFORMING TO ASTM A615,

Wide Flange Beams-ASTM A992, Grade 50, Fy = 50 KSI Angle and Channel - ASTM A36, Fy = 36 KSI Plate - ASTM A36, Fy = 36 KSI HSS - ASTM A500 SHAPED, Grade C, Fy = 50 KSI ASTM A500 ROUND, Grade C, Fy = 46 KSI

- GENERAL NOTES 8) ALL WELDED CONNECTIONS SHALL BE MADE IN
 - E7OXX ELECTRODES. ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER. 9) BOLTS SHALL CONFORM TO ASTM A325-N FOR STRUCTURAL STEEL BEARING AND TENSION CONNECTIONS. "SNUG TIGHT
 - BOLTS PER AISC & RCSC SPECIFICATIONS. 10) ERECTION OF STEEL STRUCTURE SHALL BE PERFORMED PER ALL AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)

ACCORDANCE WITH LATEST AWS SPECIFICATIONS, USING

-CL (4)-2"ø HOLES

-PL 1 1/4" x 22" x 1'-10" LG.

_PL 1/2 x 5 x 5

(USE 1 1/2"x36" LONG A.B.)

- ERECTION PROVISIONS. 11) STRUCTURAL AND MISCELLANEOUS STEEL SUBJECTED TO EXTERIOR EXPOSURE HAS BEEN PRIMED COATED ONLY.
- FIELD TOUCH-UP, FINISH PAINTING AND MAINTENANCE ARE THE RESPONSIBILITY OF THE OWNER. 12) LIGHT GAUGE COLD FORMED SHAPES SHALL CONFORM TO
- ASTM A653 AND ASTM C-955. ALL MEMBERS SHALL BE FORMED FROM MATERIAL HAVING A 50 KSI MINIMUM YIELD STRENGTH.

13) STRUCTURAL DESIGN CRITERIA: Governing Codes = 2018 NCSBC (2015 IBC) AND ASCE 7-10

Importance Factor-I = 1.0

Risk Category = II Roof Live Load = 20 PSF Roof Snow Load = 32.3 PSF (Flat Roof + Drifting) Roof Snow Design (ASCE 7-10): Ground Snow Load-Pg = 15 PSF Flat roof Snow Load-Pf = 17.6 PSF Exposure Factor-Ce = 1.0

Thermal Factor-Ct = 1.2Wind Design (ASCE 7-10): Basic Wind Speed (3 Sec. Gust) - Vult = 116 MPH Importance Factor-I = 1.0

Exposure - "C" Earthquake Design (ASCE 7-10): Importance Factor - I = 1.0Site Class - D Spectral Response Coefficients -

Ss = 0.171 g Fa = 1.6 Sds = 0.182 g SI = 0.082 g Fv = 2.4 SdI = 0.131 g Seismic Design Category - B Basic Seismic - Force - Resisting System -

Ordinary Cantilevered Column System Response Modification Coefficient - R = 1 1/4System Overstrength Factor - $\Omega o = 1 \frac{1}{4}$ Deflection Amplification Factor - Cd = 1 1/4 Analysis - Equivalent Lateral Force Procedure Seismic Base Shear (V) = 6.71 k

> A rp E ngineering CONSULTING ENGINEERS

202 EAST FRANKLIN STREET, SUITE A PO BOX 587 · MONROE · NC 28111 (704) 225-0079

			NC	COA C2424
MCGEE	PR. JOB NO.	FINAL JOB NO. 62690	DRAWING NO. P062690	Jul
12701 East Independence Blvd. P.O. Box 1375 Matthews, NC 28106-1375 Phone: (704) 882-1500 Watts: (800) 526-5589	CIRCLE K FUEL CANOPY 9706 KENNEBEC CHURCH RD ANGIER, NC (HARNETT) SCALE: 1/8"=1'-0" IN ACCORDANCE DATE: 12/22/22 WITH REV. LETTER: CHK'D BY:			in ON Million
These prints are the property of M ^C Gee Corp Reproduction or reuse is prohibited without written permission.	METAL CANOPY 24'-0" x 191'-8"			
	FOUNDATIO	ON PLAN	SHEET NO	

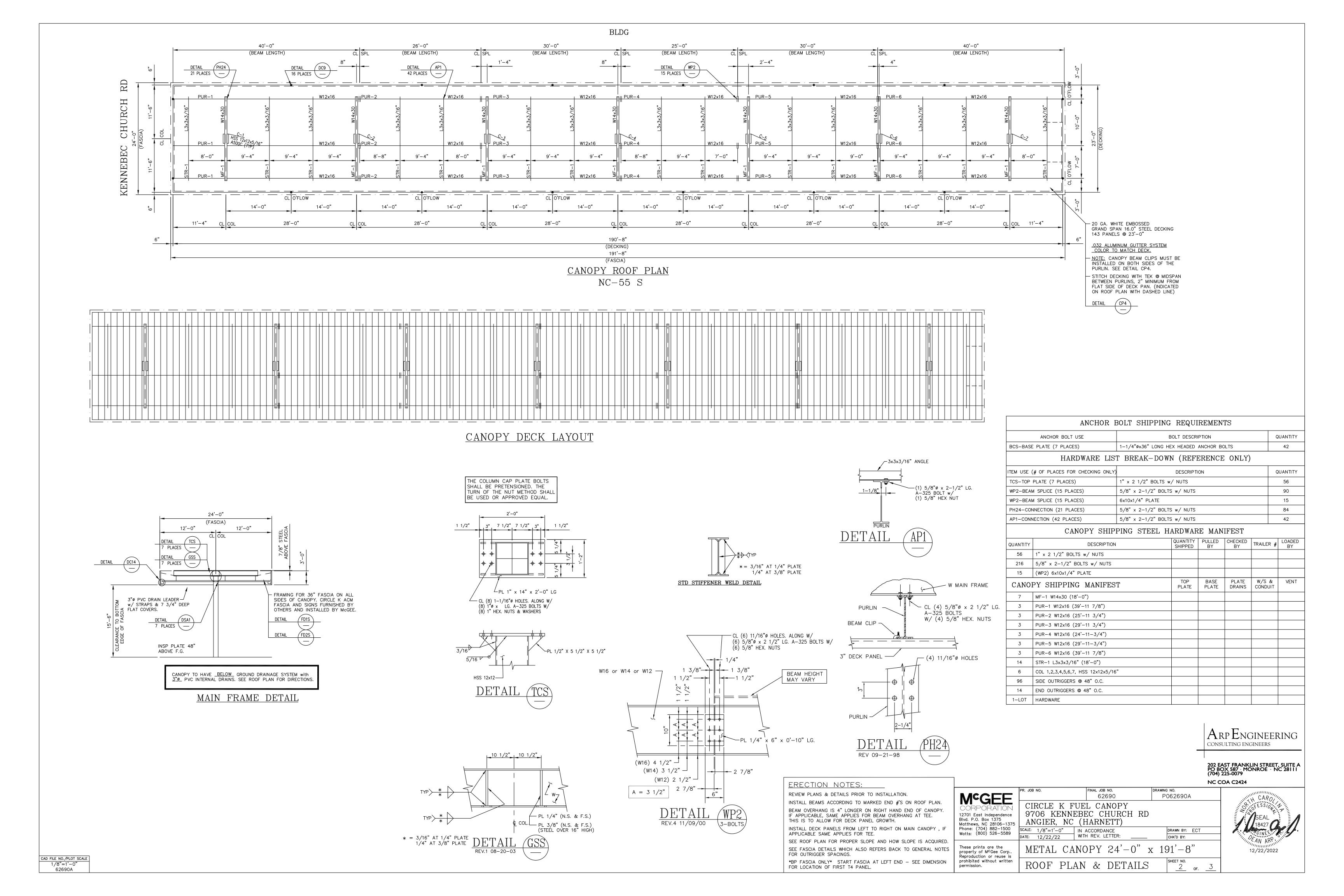


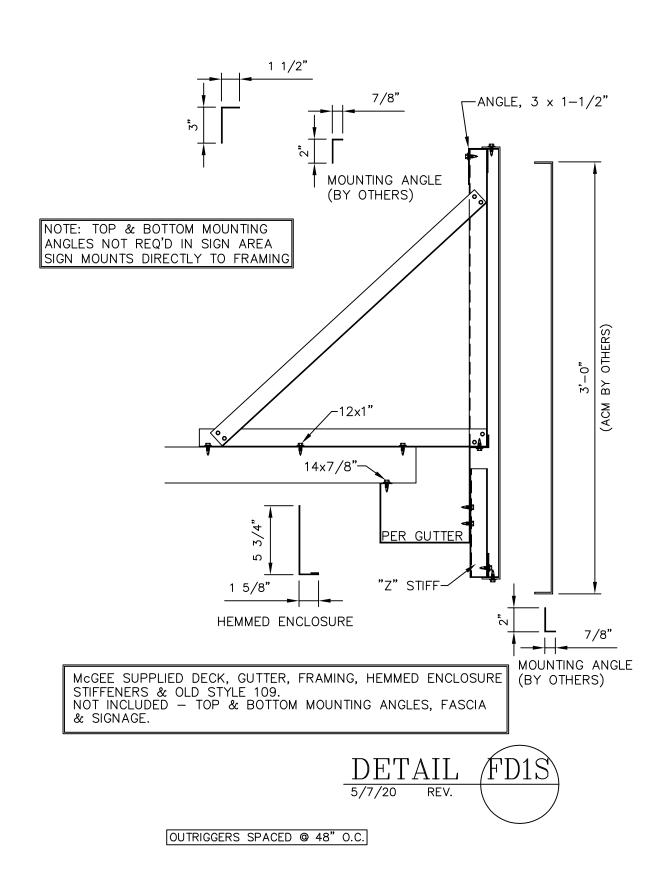
CAD FILE NO./PLOT SCALE 1/8"=1'-0" 62690

<u>C-4</u>

C-5

<u>C-6</u>





W MAIN FRAME

NOTE: MUST CLIP PANELS ON BOTH

SIDES OF BEAM AS SHOWN.

DECKING BELOW

MATERIAL-

55 COATING.

CLASS 55 COATING.

W PURLIN-

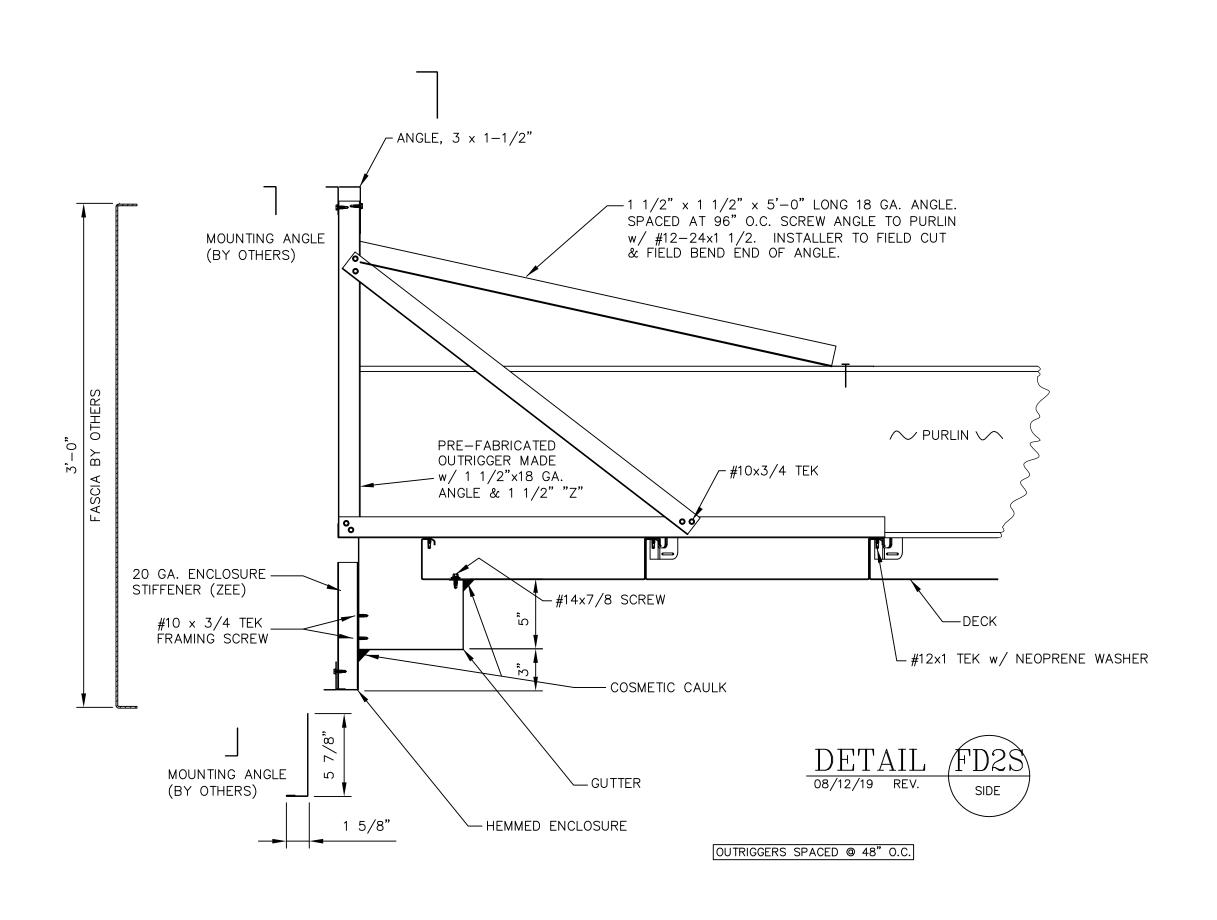
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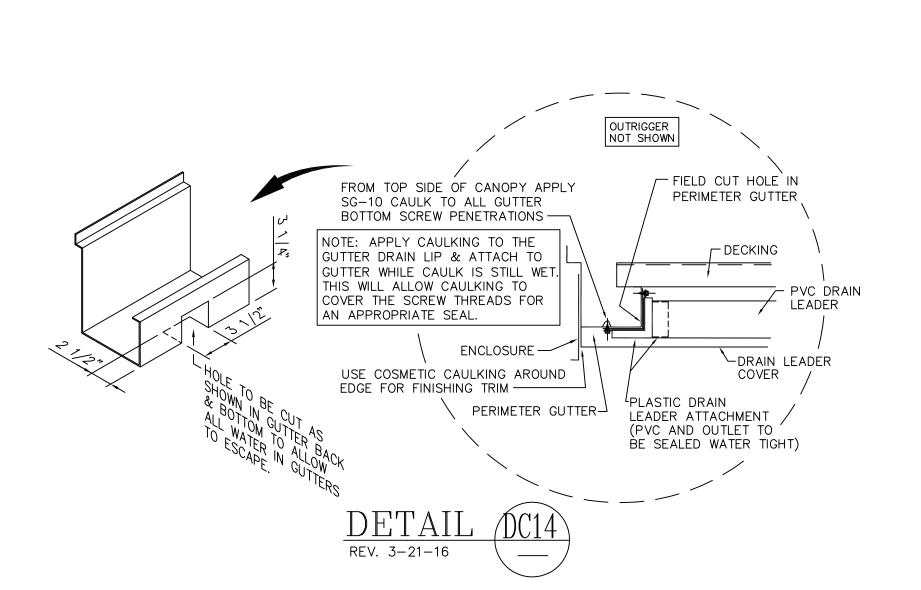
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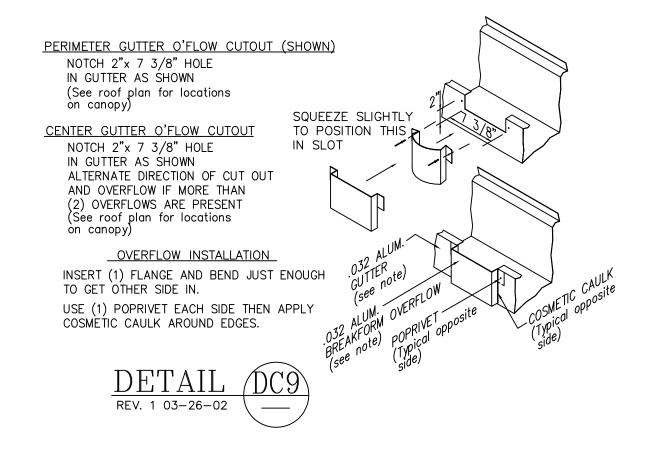
3" DECK PANEL -

BEAM CLIP (TYP) -

SEE DETAIL TO THE RIGHT







#12x1 TEK W/WASHER

(Std.) 36" Long Steel Angle

Step 1 Attach a support bracket on each side of the column to support the cut rib of deck. Keep bracket at least 12" away from

column to allow room to seal around column. (Typ. all columns)

Step 2 Use McGee beam clip and TEK screws as shown to secure and support.



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					()
					NC COA C24
	MCCEE	PR. JOB NO.	FINAL JOB NO. 62690	drawing no. P062690B	
	12701 East Independence Blvd. P.O. Box 1375 Matthews, NC 28106-1375 Phone: (704) 882-1500 Watts: (800) 526-5589	CIRCLE K FUE 9706 KENNEBI ANGIER, NC (F	EC CHURCH RD		Thu HILLIAN
			ACCORDANCE H REV. LETTER:	DRAWN BY: EC-	T T
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INDICATORS AND RIVETS"

McGEE BEAM CLIP INSTALLATION PROCEDURE: SET BEAM CLIP WITH BOLT ON TOP OF BEAM FLANGE AND CLAMPING SURFACE UNDER DECK RIB. PUSH CLIP AGAINST DECK AND BEAM FLANGE WITH BOLT AS FAR ONTO BEAM FLANGE AS POSSIBLE. WHILE KEEPING BEAM CLIP VERTICAL, TURN BOLT TO SNUG TIGHT WITHOUT BURROWING INTO STEEL BEAM FLANGE. THEN PROCEED TO TURN BOLT 3/4 TURN (270°). TIGHTEN LOCK NUT MAKING SURE THAT BEAM CLIP REMAINS IN POSITION.

(GALVANIZED G90) (MIN YIELD STRENGTH = 36 ksi)

— 11 GA. STEEL

__LOCK NUT

— CLAMPING SURFACE

TOP & BOTTOM VIEW

2 3/8" SIDE VIEW

McGEE BEAM CLIP DETAIL

BOLT: 3/8" - 16 CLASS 3A X 2.25" STEEL FULLY THREADED HX HD M/S WITH CUP POINT. SAE J429, GR 8 W/ MIN TENSILE STRENGTH OF 150 KSI, CASE HARDENED & HEAT TREATED TO MIN/MAX MID-RADIUS CORE HARDNESS OF HRC 33-39. ZINC PLATED PER ASTM B695 WITH

CLIP BODY MATERIAL: 11ga (0.115") ASTM A653 FS TYPE B (A526 CQ)

NUTS: 3/8-16 3B HEX HEAD NUT AND SQUARE NUT PER SAE J995 GR 8 W/ MIN TENSILE STRENGTH OF 150 KSI, HEAT TREATED TO MIN/MAX

HARDNESS OF HRC 33-39. ZINC PLATED PER ASTM B695 WITH CLASS

METHODS FOR DETERMINING MECHANICAL PROPERTIES OF EXTERNALLY

AND INTERNALLY THREADED FASTENERS, WASHERS, DIRECT TENSION

PERFORMANCE TESTING PER ASTM F606/F606M -16 - "STANDARD TEST

3/8"x2-1/4" HEX HEAD — 📮



62690B