

GENERAL NOTES

1. GENERAL:
- A. APPLICABLE STRUCTURAL CODE: NORTH CAROLINA BUILDING CODE - 2018 EDITION, IN CONJUNCTION WITH ASCE 7-10 AND LOCAL AMENDMENTS.
- B. THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. EXCEPT WHERE NOTED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER OF RECORD SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS).
- C. WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE REFERENCED EDITION AND/OR APPENDIX. ANY ENGINEERING DESIGN, PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW, SHALL BEAR THE SEAL OF A REGISTERED ENGINEER RECOGNIZED BY THE BUILDING CODE JURISDICTION OF THIS PROJECT.
- D. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.
- E. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, PLUMBING, FIRE SPRINKLER, AND ELECTRICAL DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL REQUIREMENTS INTO THE SHOP DRAWINGS AND CONSTRUCTION.
- F. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
- G. CONNECTIONS OF ALL ITEMS SUPPORTED BY THE STRUCTURE ARE THE RESPONSIBILITY OF THE DESIGNER WHO ARE MAKING THESE ATTACHMENTS. THESE ATTACHMENTS SHALL BE DESIGNED TO RESIST ALL GRAVITY, WIND, WIND UPLIFT, THERMAL LOADS, ETC.
- H. WHERE A DETAIL, TYPICAL SECTION, TYPICAL SECTION OR A NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS UNLESS OTHERWISE NOTED.
- I. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS & FIELD CONDITIONS. BUILDING DIMENSIONS AND ELEVATIONS, WHERE SHOWN, WERE PROVIDED BY THE ARCHITECT AND IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY AND COORDINATE ALL DIMENSIONS PRIOR TO PROCEEDING WITH THE WORK. ANY DISCREPANCIES SHALL BE RESOLVED THROUGH THE ARCHITECT. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL ITEMS WITH THE APPROPRIATE TRADE DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION.
- J. ENGINEER'S REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC. DOES NOT RELIEVE THE CONTRACTOR FROM MEETING THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY SPECIFIC DEVIATIONS AND OBTAIN ENGINEER'S WRITTEN APPROVAL FOR THE SPECIFIC DEVIATION.
- K. UNLESS NOTED, SUBMIT SHOP DRAWINGS OF ALL FABRICATED MATERIALS FOR REVIEW. DESIGN DRAWINGS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY WERE CHECKED, BEAR THE INITIAL OF THE CHECKER AND ARE STAMPED "APPROVED" BY THE GENERAL CONTRACTOR.
- L. ALL REQUIRED TESTING REPORTS SHALL BE AVAILABLE AT THE JOB SITE.
- M. NO SUPPORT PROVISIONS HAVE BEEN MADE FOR EQUIPMENT OVER 300 POUNDS OR PIPES OVER 4" DIAMETER EXCEPT AS SHOWN. UNLESS NOTED, EQUIPMENT SHALL BE SUPPORTED BY TWO (2) BEAMS OR TRUSSES WHERE EACH SUPPORTS HALF OF THE EQUIPMENT WEIGHT. ANY ADDITIONAL REQUIREMENTS SHOULD BE APPROVED BY THE ENGINEER. EQUIPMENT AND PIPES SHALL BE BRACED Laterally BY THE INSTALLER TO RESIST SEISMIC FORCES.
- N. EQUIPMENT WEIGHTS SHOWN ON THE STRUCTURAL DRAWINGS ARE MAXIMUM OPERATING WEIGHTS INCLUDING CURBS AND MISCELLANEOUS ITEMS. THE CONTRACTOR SHALL VERIFY AND BRING TO THE ATTENTION OF THE ENGINEER ANY DEVIATIONS.
- O. WHERE ROOF DRAINS ARE USED FOR ROOF DRAINAGE, AN EMERGENCY DRAINAGE SYSTEM SHALL BE PROVIDED TO MINIMIZE WATER ACCUMULATION (REF ARCHITECTURAL DRAWINGS).
- P. JOHNSTON BURKHOLDER ASSOCIATES, ITS EMPLOYEES, AND REPRESENTATIVES SHALL NOT BE RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS AND METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH CONSTRUCTION; NOR WILL THEY BE RESPONSIBLE FOR ANY FAILURE BY THE CONTRACTOR TO PERFORM OR COMPLETE CONSTRUCTION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. JOBSITE VISITS BY JOHNSTON BURKHOLDER ASSOCIATES SHALL NOT CONSTITUTE APPROVAL, AWARENESS, OR LIABILITY FOR ANY HAZARDOUS CONDITIONS.
2. DESIGN CRITERIA:
- A. DEAD LOADS
- REF FRAMING PLAN
- B. LIVE LOADS
- ROOF = 20 PSF
- C. RISK CATEGORY = II
- D. SNOW LOADS
1. GROUND SNOW LOAD = 15 PSF
 2. FLAT-ROOF SNOW LOAD = 10.5 PSF
 3. SNOW EXPOSURE FACTOR, Ce = 1.0
 4. THERMAL FACTOR, Ct = 1.0 (MAIN BUILDING)
 5. SNOW LOAD IMPORTANCE FACTOR, I = 1.0
 6. ALL APPLICABLE EFFECTS DUE TO SNOW DRIFTING
- E. WIND LOADS
1. CODE MINIMUM ULTIMATE WIND SPEED = 116 MPH
 2. DESIGN ULTIMATE WIND SPEED = 145 MPH
 3. WIND LOAD IMPORTANCE FACTOR = 1.0
 4. WIND EXPOSURE CATEGORY = C FOR MAIN WIND FORCE-RESISTING SYSTEM
 5. WIND EXPOSURE CATEGORY = C FOR COMPONENTS AND CLADDING
- F. SEISMIC LOADS
1. SEISMIC IMPORTANCE FACTOR, I = 1.0
 2. MAPPED SPECTRAL RESPONSE ACCELERATIONS
 - Ss = 0.172
 - S1 = 0.083 3. SOIL SITE CLASSIFICATION = D
 4. DESIGN SPECTRAL RESPONSE PARAMETERS
 - Sds = 0.184
 - Sd1 = 0.132 5. SEISMIC DESIGN CATEGORY = B
 6. BASIC SEISMIC-FORCE-RESISTING SYSTEM = LIGHT FRAME WOOD WALLS WITH STRUCTURAL WOOD SHEAR PANELS
 7. DESIGN BASE SHEAR, V = Cs*W
 8. SEISMIC RESPONSE COEFFICIENT, Cs = 0.0283
 9. RESPONSE MODIFICATION FACTOR, R = 6.5
 10. ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE ANALYSIS
- G. DESIGN LOAD COMBINATIONS (ASD)
1. D
 2. D + (Lr OR S OR R)
 3. D + (0.6W OR 0.7E)
 4. D + 0.75(Lr OR S OR R) + 0.75(0.6W OR 0.7E)
 5. 0.6D + 0.6W
 6. 0.6D + 0.7E
- STRESS INCREASES: INCREASES IN ALLOWABLE STRESSES SPECIFIED IN THE APPROPRIATE MATERIAL CHAPTER OR THE REFERENCED STANDARDS SHALL NOT BE USED WITH THE LOAD COMBINATIONS, EXCEPT THAT A DURATION OF LOAD INCREASE SHALL BE PERMITTED IN ACCORDANCE WITH CHAPTER 23.
3. SHALLOW FOUNDATIONS AND BUILDING PAD:
- A. THE FOUNDATION HAS BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS MADE IN THE GEOTECHNICAL REPORT PREPARED BY SUMMIT, A UNIVERSAL ENGINEERING SCIENCES COMPANY, DATED FEBRUARY 21, 2022.
- B. CONTINUOUS FOOTINGS AND SPREAD FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUSTAINING A NET ALLOWABLE BEARING PRESSURE OF 2500 PSF AT A DEPTH OF 12 INCHES.
- C. MAXIMUM TOTAL FOUNDATION AND SLAB SETTLEMENT 1" AND DIFFERENTIAL SETTLEMENT 1/2" OVER A DISTANCE OF 50 FEET. MINIMUM SUBGRADE MODULUS 100 PCL. PARAMETERS SHALL BE VERIFIED BY A QUALIFIED GEOTECHNICAL ENGINEER. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO REPORT ADVERSE CONDITIONS.

- D. CONTRACTOR TO PREPARE SUBGRADE FOR BUILDING PAD AND PAVING AREAS PER GEOTECHNICAL ENGINEER'S RECOMMENDATIONS. THE SITE SHALL BE PREPARED UNIFORMLY IN ACCORDANCE WITH CIVIL DRAWINGS.
- E. WHERE FILL IS REQUIRED IT SHALL BE PLACED IN ACCORDANCE WITH GEOTECHNICAL RECOMMENDATIONS AND UNDER THE OBSERVATION OF A QUALIFIED GEOTECHNICAL ENGINEER.
- F. ALL EXCAVATIONS AND BUILDING PADS SHALL BE OBSERVED BY A QUALIFIED GEOTECHNICAL ENGINEER TO VERIFY THE DESIGN CRITERIA AND REPORT ADVERSE CONDITIONS. (GEOTECHNICAL ENGINEER TO BE HIRED AND PAID BY THE OWNER) CERTIFICATION OF EXCAVATION SHALL BE PROVIDED TO BUILDING OFFICIAL. IN CASE OF CONFLICT BETWEEN THE GEOTECHNICAL REPORT, SPECIFICATIONS AND DRAWINGS, THE MORE STRINGENT REQUIREMENT SHALL APPLY.
- G. FOOTING ELEVATIONS GIVEN ARE FOR PURPOSE OF CONTRACT AND SHALL BE ADJUSTED (LOWERED) AT TIME OF EXCAVATION TO MEET SOIL CONDITIONS. ENGINEER SHOULD BE CONSULTED.
- H. GRANULAR BASE BELOW SLAB-ON-GRADE AND SPREAD FOOTINGS SHALL BE PLACED AND COMPACTED IN ACCORDANCE WITH THE RECOMMENDATIONS AND UNDER THE OBSERVATION OF A QUALIFIED GEOTECHNICAL ENGINEER.
- I. BACKFILL AGAINST WALLS SHALL BE DEPOSITED EVENLY AGAINST BOTH SIDES OF THE WALLS UNTIL THE LOWER GRADE IS REACHED.
- J. THE CONTRACTOR SHALL BRING TO THE ATTENTION OF THE ARCHITECT THE LOCATION OF EXISTING UTILITIES AND PROCEED WITH THE WORK ONLY AFTER WRITTEN APPROVAL FROM THE ARCHITECT.
- K. EXCAVATIONS FOR CONTINUOUS FOOTINGS SHALL BE MADE TRUE TO LINE AND GRADE. THE SIDES OF THE FOOTINGS SHALL BE FORMED. EXCEPT WHERE SOIL CONDITIONS ARE SUCH THAT THE SIDES OF THE EXCAVATION STAND FIRM AND SQUARE, EXCAVATIONS SHALL BE MADE TO FIRM, CLEAN BEARING SOIL.
- L. EXCAVATIONS FOR FOOTINGS AND FOUNDATIONS, WHICH ARE TO SERVE AS FORMS, SHALL BE THOROUGHLY WET PRIOR TO PLACING CONCRETE.
4. CONCRETE:
- A. CONCRETE SHALL CONFORM WITH ACI BUILDING CODE (ACI-318) AND SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH, AND DENSITY, AND WATER/CEMENT IN ACCORDANCE WITH THE FOLLOWING:
- | LOCATION | STRENGTH, PSI | DENSITY, PCF | MAX W/C RATIO |
|--------------------------|---------------|--------------|---------------|
| INTERIOR SLABS | 3000 | 145 | 0.55 |
| FOUNDATIONS | 3000 | 145 | 0.55 |
| ALL OTHER CONCRETE (UNO) | 3000 | 145 | 0.55 |
- NOTE: DESIGN STRENGTH BASED ON 2,500 PSI CONCRETE
- B. UNLESS NOTED, CEMENT SHALL CONFORM TO ASTM C150, TYPE I OR II, OR C595, TYPE II, AGGREGATES SHALL BE NORMAL WEIGHT CONFORMING TO ASTM C33. ADMIXTURES CONTAINING CHLORIDES SHALL NOT BE USED.
- C. CONCRETE EXPOSED TO WEATHER (I.E., EXPOSED EXTERIOR SLABS, CONCRETE WALLS, RETAINING WALLS, ETC.) SHALL BE AIR-ENTRAINED IN ACCORDANCE WITH ACI 318.
- D. UNLESS NOTED, MAXIMUM SLUMP FOR CONCRETE SHALL BE 4 INCHES (PLUS OR MINUS ONE INCH) AT POINT OF PLACEMENT, WHERE APPROVED BY THE ENGINEER OF RECORD. LARGER SLUMP IS PERMITTED WITH THE ADDITION OF HIGH RANGE WATER REDUCING ADMIXTURE.
- E. CONCRETE CURING SHALL COMPLY WITH ACI 308. CURING PROCESS SHALL START IMMEDIATELY FOLLOWING INITIAL SET. CONCRETE SHALL BE MAINTAINED IN MOIST CONDITION FOR AT LEAST 7 DAYS AFTER PLACEMENT UNLESS PROVISIONS OF ACI-318 ARE FOLLOWED.
- F. HOT WEATHER CONCRETING: SPECIAL PRECAUTIONS RECOMMENDED BY ACI 305 SHALL BE TAKEN WHEN THE AMBIENT TEMPERATURE IS ABOVE 80 DEGREES FAHRENHEIT OR WHEN MAXIMUM RATE OF EVAPORATION IN THE CONCRETE EXCEEDS 0.2 LB/SF/HR. THIS RATE IS AFFECTED BY AIR TEMPERATURE, RELATIVE HUMIDITY, CONCRETE TEMPERATURE AND WIND VELOCITY. AFTER FINISHING CONCRETE, USE LIGHT FOG SPRAY UNTIL CURING COMPOUND IS USED, OR WET CURING METHOD IS IMPLEMENTED.
- G. COLD WEATHER CONCRETING: WHEN, FOR MORE THAN THREE (3) CONSECUTIVE DAYS, THE MEAN DAILY TEMPERATURE DROPS BELOW 40 DEGREES FAHRENHEIT, SPECIAL MATERIALS AND PROCEDURES SHALL BE PROVIDED DURING PLACING AND CURING OF CONCRETE PER ACI 306.
5. REINFORCING STEEL:
- A. REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 (U.N.) AND GRADE 40 FOR #3 AND SMALLER BARS. GRADE 60 REINFORCING MAY NOT BE RE-BENT OR FIELD BENT WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER.
- B. WELDED WIRE REINFORCING SHALL CONFORM TO ASTM A-1064, MINIMUM LAP SHALL BE 8" U.N.O.
- C. REINFORCING THAT IS WELDED SHALL BE WELDABLE TYPE, ASTM A-706.
- D. NO TACK WELDING OF REINFORCING IN THE FIELD WILL BE PERMITTED.
- E. ALL DETAILING, FABRICATION AND PLACING OR REINFORCING BARS, UNLESS OTHERWISE NOTED, SHALL CONFORM TO ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" AND THE LATEST ACI MANUAL OF STANDARD PRACTICE FOR DETAIL REINFORCED CONCRETE STRUCTURES".
- F. MINIMUM LAP SPLICE AND TENSION DEVELOPMENT DIMENSION TABLE BASED ON 3000 PSI NORMAL WEIGHT CONCRETE, Fy = 60 KSI, NON-COMPACTION BARS, LAP CLASS B, WITH 3" MIN CL:
- | BAR | TOP BAR SPLICE | DEVELOPMENT | BAR | OTHER BAR SPLICE | DEVELOPMENT |
|-----|----------------|-------------|-----|------------------|-------------|
| #3 | 21" | 16" | #3 | 19" | 12" |
| #4 | 23" | 18" | #4 | 18" | 14" |
| #5 | 28" | 22" | #5 | 22" | 17" |
| #6 | 34" | 26" | #6 | 26" | 20" |
- WHEN LAPPING TWO DIFFERENT SIZE BARS, USE THE LAP DIMENSION OF THE SMALLER BAR OR THE ANCHORAGE DIMENSION OF THE LARGER BAR. USE WHICHEVER DIMENSION IS LARGER.
- G. MINIMUM CONCRETE COVER FOR REINFORCING CONCRETE REINFORCING SHALL BE PLACED PER THE TOLERANCES OF ACI 117 AND ACI 318
- | LOCATION | MINIMUM COVER |
|--|---------------|
| FOOTINGS, GRADE BEAMS AND SLABS | |
| CAST AGAINST & PERMANENTLY EXPOSED TO EARTH | 3" |
| SLABS ON GRADE | MID-DEPTH |
| INTERIOR SLABS AND STAIRS | 1" |
| EXTERIOR SLABS AND STAIRS #5 AND SMALLER | 1 1/2" |
| EXTERIOR SLABS AND STAIRS #6 AND LARGER | 2" |
| WALLS INTERIOR FACE | 1" |
| WALLS INTERIOR FACE #5 AND SMALLER | 1 1/2" |
| WALLS INTERIOR FACE #6 AND LARGER | 2" |
| *VERIFY W/ ENVIRONMENTAL CONDITIONS PER ACI 318, CHAPTER 19. | |
8. SLABS:
- A. MAXIMUM SLUMP WITHOUT A PLASTICIZER AT POINT OF PLACEMENT SHALL BE 5 INCHES. MIX DESIGNS SHALL TAKE CARE TO PROVIDE THE LARGEST POSSIBLE SIZE OF COARSE AGGREGATE WHILE MAINTAINING CONCRETE WORKABILITY. NOMINAL MAXIMUM AGGREGATE SIZE SHALL NOT BE LESS THAN 3/4 INCH NOR MORE THAN 1/3 THE DEPTH OF THE SLAB.
- B. FOR INTERIOR SLABS ON GRADE, PROVIDE CONCRETE WITH AN ULTIMATE SHRINKAGE LES THAN 0.05% AT 28 DAYS. LABORATORY TEST RESULTS SHALL BE SUBMITTED INDICATING THAT THE CONCRETE SLAB ON GRADE MIX DESIGN (OR COMPARABLE) MEETS THE ULTIMATE SHRINKAGE REQUIREMENTS. SHRINKAGE VALUES FOR CONCRETE SPECIMENS SHALL BE TESTED PER ASTM C157 AND THE PROCEDURES IN ACI 209R TO PREDICT THE ULTIMATE DRYING SHRINKAGE.
- C. CONCRETE SHALL BE MIXED, PLACED, FINISHED AND CURED PER REFERENCED EDITION OF ACI 302.1 FOR THE APPROPRIATE FLOOR CLASS TYPE. CURING COMPOUND SHALL BE COMPATIBLE WITH ARCHITECTURAL FLOOR FINISH. SLABS SHALL BE PLACED ON A FLAT, SMOOTH, FIRM, COMPACTED SUBGRADE.
- D. FOR SUBGRADE AND SUBBASE PREPARATION, SEE GEOTECHNICAL REPORT AND SPECIFICATIONS.
- E. IMMEDIATELY PRIOR TO POURING EACH SLAB, A QUALIFIED GEOTECHNICAL ENGINEER (TO BE PAID BY OWNER) SHALL INSPECT THE PAD AND PROVIDE A CERTIFICATE STATING THAT THE PAD MEETS THE PROJECT DESIGN REQUIREMENTS. IF WEATHER CONDITIONS CHANGE, RE-CERTIFICATION WILL BE REQUIRED.
- F. SLAB-ON-GRADE REINFORCING SHALL BE SUPPORTED ON CHAIRS AT MID-DEPTH OF SLAB WITH SAND PLATES OR PRECAST CONCRETE SUPPORTS (PRECAST CONCRETE SUPPORTS SHOULD BE AT LEAST 4" SQUARE AT THE BASE WITH MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI) OF SUFFICIENT HEIGHT TO INSURE THAT REINFORCING IS AT THE PROPER HEIGHT WITH MAXIMUM SPACING OF 3'-0" EACH WAY. FOR WELDED WIRE FABRIC, PLACEMENT OF REINFORCING USING THE PULL- UP METHOD IS NOT PERMITTED.

- G. VAPOR BARRIER, IF NOT SPECIFIED BY ARCHITECTURAL SPECIFICATION OR SOILS REPORT SHALL CONSIST OF A MINIMUM 15 MIL ASTM E1745 CLASS A MATERIAL LAPPED A MINIMUM OF 6" AND TAPED PER MANUFACTURER RECOMMENDATIONS. THE BARRIER SHALL BE PLACED ON TOP OF A SMOOTH AND COMPACTED SUBGRADE SURFACE. THE FLOOR SLAB SHALL BE PLACED OVER A 4" LAYER OF COMPACTED AGGREGATE BASE COURSE ON TOP OF THE VAPOR BARRIER. ANY DAMAGE TO VAPOR BARRIER SHALL BE REPAIRED PRIOR TO AGGREGATE COURSE PLACEMENT. CARE SHALL BE TAKEN TO KEEP MOISTURE AWAY FROM THE COMPACTED SUBBASE. SUBGRADE MUST BE ALLOWED TO DRY AFTER RAINS PRIOR TO SLAB PLACEMENT. FLOOD CURING IS NOT ALLOWED. SAND IS NOT AN ALTERNATIVE FOR THE SUB-BASE COURSE.
- H. UNLESS NOTED, FLOOR CONTRACTION JOINTS FOR SLAB-ON-GRADE SHALL BE SPACED EQUALLY. CONTRACTION JOINTS SPACING SHALL BE REDUCED AT HOT AND/OR DRY CLIMATE. SAW CUT SLAB (18" WIDE X SLAB THICKNESS/4 U.N.O.) AS SOON AS THE SURFACE IS FIRM ENOUGH SO THAT IT WILL NOT BE DAMAGED BY THE BLADE. JOINT FILLER SHALL BE PLACED IN CLEAN JOINTS, OVERFILLED AND SHAVED FLUSH. THE OPTIMUM TIME TO CUT THE SLAB WILL BE THE CONTRACTOR'S RESPONSIBILITY. SLABS WITH CRACKS AND/OR SLAB WHICH EXHIBIT "SLAB CURLING" WILL NOT BE ACCEPTED. SLAB REPLACEMENT SHALL BE AT OPTION OF THE OWNER AT CONTRACTOR'S EXPENSE.
- I. REF ARCHITECTURAL AND CIVIL DRAWINGS FOR SIDEWALKS, EXTERIOR SLABS, INTERIOR SLABS, SLAB DEPRESSIONS, AND ELEVATIONS.
- J. SLAB-ON-GRADE AT CORNERS OF OPENINGS, CUT OUTS AND PENETRATIONS SHALL BE REINFORCED WITH 2-#4 (3'-0" LONG).
- K. AT SLAB-ON-GRADE, SEAL COLUMN BOX-OUTS, CONSTRUCTION JOINTS AND CONTRACTION JOINTS TO PREVENT WATER PENETRATION UNDER SLAB DURING & AFTER CONSTRUCTION TO PREVENT SOILS FROM EXPANDING.
- L. PIPES, DUCTS, CONDUITS, ETC. SHALL NOT BE PLACED IN SLABS UNLESS APPROVED BY THE STRUCTURAL ENGINEER. (PLACE A MINIMUM 4" BELOW SLAB).
- M. THE GENERAL CONTRACTOR, INCLUDING SUB CONTRACTORS AND SUPPLIERS SHALL NOT DRIVE OR PARK HEAVY EQUIPMENT ON THE CONCRETE SLAB. ANY DAMAGE TO SLAB DUE TO HEAVY EQUIPMENT SHALL BE THE CONTRACTOR'S RESPONSIBILITY.
- N. COMBUSTION HEATERS SHALL NOT BE USED DURING CONCRETE PLACEMENT UNLESS PRECAUTIONS ARE TAKEN TO PREVENT EXPOSURE OF THE CONCRETE TO EXHAUST GASES.
- O. REF ARCH SPECIFICATIONS FOR ALL FLOOR SLAB FINISHES.
- P. PROVIDE NEW TROWEL BLADES FOR CONCRETE FINISHING OF FLOOR SLABS.
7. POST-INSTALLED ANCHORING SYSTEM:
- A. UNLESS NOTED, ALL ANCHORS IN CONCRETE SHALL BE CARBON STEEL HILTI KWIK HUS-EZ SCREW ANCHORS PER ICC ESR-3056 OR APPROVED ICC EQUIVALENT.
- B. WHERE NOTED ON THE DRAWINGS FOR CONCRETE ANCHORAGE, USE HILTI HIT HY-100 THREADED ROD AND SILENT ANCHORING SYSTEM (ICC ESR-3574) OR APPROVED EQUIVALENT. UNLESS NOTED, ANCHOR EMBEDMENT SHALL BE AS FOLLOWS: 3/8" DIA. - 3 3/8", 1/2" DIA. - 4 1/2", 5/8" DIA. - 5 5/8", 3/4" DIA. - 6 3/4".
- C. UNLESS NOTED, ANCHOR SPACING (AS) AND ANCHOR EDGE DISTANCE (ED) SHALL BE AS PUBLISHED BY THE MOST CURRENT APPROVED HILTI REPORT IN ORDER TO DEVELOP MAXIMUM WORKING LOAD.
- D. ALL ANCHORS SHALL BE INSTALLED AS PER MANUFACTURER'S INSTRUCTIONS AND UNDER MANUFACTURER'S SUPERVISION IN ORDER TO DEVELOP THE PUBLISHED WORKING LOADS.
- E. MINIMUM CONCRETE COMPRESSIVE STRENGTH REQUIRED AT TIME OF INSTALLATION OF ANY ANCHORS SHALL BE 2500 PSI.
8. WOOD FRAMING:
- A. ALL NAILING NOT NOTED SHALL BE PER TYPICAL DETAIL AND COMMON NAIL DIAMETER TABLE BELOW. ALL BOLTING SHALL BE PER STRUCTURAL STEEL SECTION ABOVE. WOOD CONNECTORS SHALL BE AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. OR OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT ICC APPROVAL. WHERE "TYPE" OF CONNECTOR IS INDICATED ON THE DRAWINGS, THE CONNECTOR AND ATTACHMENT SHALL BE PER THE MAXIMUM MODEL NUMBER BASED ON THE SIZE OF THE MEMBERS CONNECTED.
- B. WOOD FRAMING MEMBERS SHALL NOT BE NOTCHED OR DRILLED WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER THROUGH THE ARCHITECT.
- | NAIL TYPE | REQ'D DIA | REQ'D GA | LENGTH | NAIL TYPE | REQ'D DIA | REQ'D GA | LENGTH |
|-----------|-----------|----------|--------|-----------|-----------|----------|--------|
| 6d | 0.113" | 11 1/2 | 2" | 12d | 0.148" | 9 | 3 1/4" |
| 8d | 0.131" | 10 1/4 | 2 1/2" | 16d | 0.162" | 8 | 3 1/2" |
| 10d | 0.148" | 9 | 3" | 20d | 0.192" | 6 | 4" |
- C. WOOD STUDS AND TRUSSES SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 19% WHEN DELIVERED TO THE JOB SITE.
- D. REINFORCING-TREATED WOOD:
1. ALL SILL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESERVATIVE-TREATED WOOD. WOOD FRAMING MEMBERS ATTACHED DIRECTLY TO THE INTERIOR OF EXTERIOR MASONRY OR CONCRETE WALLS BELOW GRADE SHALL BE PRESERVATIVE-TREATED WOOD.
 2. ALL WOOD FRAMING MEMBERS, INCLUDING WOOD SHEATHING THAT ARE IN CONTACT WITH EXTERIOR WALLS AND ARE LESS THAN 8 INCHES FROM FINISHED GRADE SHALL BE PRESERVATIVE-TREATED WOOD.
 3. ALL FASTENERS INCLUDING NUTS AND WASHERS IN CONTACT WITH PRESERVATIVE-TREATED WOOD SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL OR STAINLESS STEEL. THE COATING WEIGHTS FOR ZINC-COATED FASTENERS SHALL BE PER ASTM A153. FASTENERS OTHER THAN NAILS, WOOD SCREWS AND LAG SCREWS ARE PERMITTED TO BE OF MECHANICALLY DEPOSITED ZINC-COATED STEEL WITH COATING WEIGHTS PER ASTM B695, CLASS 55 MINIMUM.
 4. ALL FASTENERS INCLUDING NUTS AND WASHERS IN CONTACT WITH PRESERVATIVE-TREATED WOOD SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL OR STAINLESS STEEL. THE COATING WEIGHTS FOR ZINC-COATED FASTENERS SHALL BE PER ASTM A153. FASTENERS OTHER THAN NAILS, WOOD SCREWS AND LAG SCREWS ARE PERMITTED TO BE OF MECHANICALLY DEPOSITED ZINC-COATED STEEL WITH COATING WEIGHTS PER ASTM B695, CLASS 55 MINIMUM.
- E. FRAMING LUMBER SHALL COMPLY WITH THE REFERENCED EDITION OF THE GRADING RULES OF THE WPPA OR THE WCLIB. ALL SAWN LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED LUMBER GRADING AGENCY AND SHALL HAVE MINIMUM PROPERTIES WHICH MEET OR EXCEED THE FOLLOWING WOOD TYPES:
1. EXTERIOR WALL STUDS: DOUGLAS FIR-LARCH (DF-L) No. 2 OR BETTER, OR SOUTHERN PINE No. 2 OR BETTER.
 2. HEADERS: DOUGLAS FIR-LARCH (DF-L) No. 2 OR BETTER, OR SOUTHERN PINE No. 2 OR BETTER.
 3. BEARING PLATES AND TOP PLATES: DOUGLAS FIR-LARCH (DF-L) No. 2 OR BETTER, OR SOUTHERN PINE No. 2 OR BETTER.
9. PREFABRICATED WOOD TRUSS FRAMING MEMBERS:
- A. FRAMING MEMBERS SHALL BE DESIGNED TO SUPPORT SELF WEIGHT PLUS LIVE LOAD AND SUPERIMPOSED DEAD LOADS STATED IN THE GENERAL STRUCTURAL NOTES OR AS LOCATED ON PLANS. BRIDGING SIZE AND SPACING BY FABRICATOR UNLESS NOTED OTHERWISE. ALL CONNECTORS SHALL HAVE CURRENT ICC APPROVAL. FRAMING MEMBERS SHALL BE AGENCIES STAMPED AND CONFORM TO THE REFERENCED BUILDING CODE AND ANSIP/PTA NATIONAL DESIGN STANDARD FOR METAL CONNECTED WOOD TRUSS CONSTRUCTION "CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, ERECTION DRAWINGS AND DESIGN CALCULATIONS SEALED BY A REGISTERED ENGINEER IN THE PROJECT LOCATION STATE FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. SAID SUBMITTAL, IN ADDITION TO LOADS SPECIFIED IN THE G.S.N. AND PLANS, SHALL INCLUDE, BUT NOT BE LIMITED TO, THE FOLLOWING:
1. DEFLECTION/CAMBER: ROOFS WITHOUT PLASTER OR GYPBOARD CEILING'S TOTAL LOAD MAXIMUM = L/180, LIVE LOAD MAXIMUM = L/240. ROOFS WITH PLASTER OR GYPBOARD CEILING'S TOTAL LOAD MAXIMUM = L/240, LIVE LOAD MAXIMUM = L/360. FABRICATOR SHALL DESIGN ROOF MEMBERS FOR PONDING WHERE ROOF SLOPES ARE LESS THAN 1/4" PER FOOT. FRAMING MEMBERS SHALL BE CAMBERED FOR 1.0 TIMES THE DEAD LOAD DEFLECTION. MAXIMUM TOTAL LOAD DEFLECTION OF MEMBERS SHALL BE 1"; FABRICATOR SHALL DESIGN ADJACENT MEMBERS FOR MAXIMUM OF 1/4" DIFFERENTIAL DEFLECTION.
 2. TOP CHORD MEMBER WOOD SPECIES SHALL HAVE A SPECIFIC GRAVITY OF 0.42 OR GREATER.
- B. VERIFY SIZE, WEIGHT AND LOCATION OF SUPPORTED EQUIPMENT WITH ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, SPRINKLER AND THEIR RELATED DRAWINGS. ADDITIONAL FRAMING MEMBERS SHALL BE SUPPLIED AS REQUIRED TO SUPPORT EQUIPMENT.
- C. FABRICATOR SHALL HAVE ICC APPROVAL OR BE APPROVED ACCORDING TO THE BUILDING JURISDICTION.
10. SPECIAL INSPECTIONS:
- A. SPECIAL INSPECTOR (SI) SHALL BE RETAINED AND PAID BY THE OWNER AND PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED UNDER SECTION 1704 OF THE IBC AS PER TABLE ON 50.3.

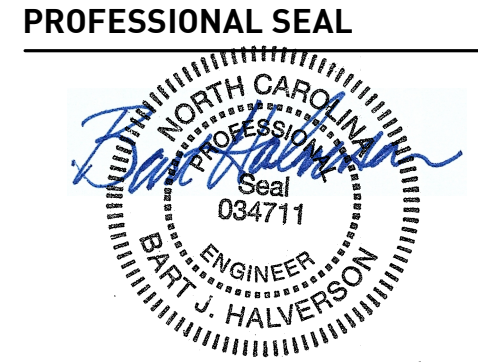
- B. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
- C. THE DUTIES OF THE SPECIAL INSPECTOR SHALL INCLUDE, BUT ARE NOT LIMITED TO, VERIFICATION OF CONSTRUCTION QUALITY CONTROL, TESTING, COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS, BUILDING CODE REQUIREMENTS, AND LOCAL BUILDING DEPARTMENT REQUIREMENTS.
- D. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE PROPER NOTIFICATION TO THE SPECIAL INSPECTOR AND PROCEED WITH THE CONSTRUCTION ONLY AFTER THE SPECIAL INSPECTOR'S APPROVAL.
- E. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
1. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS. THE INSPECTOR MAY NOT ALTER, MODIFY, ENLARGE OR WAIVE ANY OF THE REQUIREMENTS OF THE DOCUMENTS.
 2. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE CONSTRUCTION OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL, IN RESPONSIBLE CHARGE, THE CONTRACTOR AND THE OWNER. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR AND THE OWNER FOR CORRECTION, IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL, IN RESPONSIBLE CHARGE AND THE OWNER PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK.
 3. A FINAL REPORT OF INSPECTIONS DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PERIODICALLY AT A FREQUENCY AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK, WHERE SPECIAL INSPECTION REPORTS SHALL BE PLACED AT CONTRACTOR'S EXPENSE.
 4. QUALITY ASSURANCE TESTING, DUPLICATE INSPECTIONS SHALL NOT BE REQUIRED.
11. STRUCTURAL ENGINEER OF RECORD (SER) NOTIFICATION OF CONSTRUCTION PROGRESS AND SITE VISIT (WHEN APPLICABLE):
- A. CONTRACTOR SHALL NOTIFY ARCHITECT FIVE (5) WORKING DAYS PRIOR TO THE FOLLOWING CONSTRUCTION MILESTONES:
1. SPREAD FOOTING FOUNDATIONS: AFTER FOUNDATION REINFORCING IS PLACED AND BEFORE PLACING CONCRETE AT THE FOUNDATIONS.
 2. WOOD WALL CONSTRUCTION: AFTER WALL STUDS ARE ERECTED AND BEFORE INTERIOR SHEATHING IS PLACED.
 3. ROOF STRUCTURE: AFTER STRUCTURE IS ERECTED AND BEFORE INSULATION AND ROOFING IS INSTALLED. CONTRACTOR SHALL ARRANGE A PRE-ROOFING MEETING BEFORE ANY ROOFING WORK IS STARTED.
 4. PUNCH LIST: AFTER ALL STRUCTURAL ITEMS ARE COMPLETE.
- B. FAILURE TO NOTIFY ARCHITECT OF ANY CONSTRUCTION MILESTONE MAY RESULT IN CONTRACTOR HAVING TO REMOVE WORK FOR THE PURPOSE OF REVIEW AT CONTRACTOR'S EXPENSE.
- C. PREMATURE NOTIFICATION FOR SITE VISIT WILL RESULT IN AN ADDITIONAL SITE VISIT WITH ALL EXPENSES AND FEES PAID BY THE CONTRACTOR.
12. SIPS PANELS
- A. THE STRUCTURAL INSULATED PANEL (SIPS) SUPPLIER SHALL BE CERTIFIED BY THE STRUCTURAL INSULATED PANEL ASSOCIATION (SIPA) PRIOR TO THE START OF PRODUCTION OF PANELS AND SHALL HAVE A MINIMUM OF TWO YEARS OF EXPERIENCE PRODUCING PANELS.
- B. SIPS PANELS SHALL COMPLY WITH THE PERFORMANCE AND DESIGN CRITERIA IN THE CONTRACT DOCUMENTS. SIPS SUPPLIER SHALL ENGAGE A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED TO DESIGN ALL STRUCTURAL COMPONENTS AND CONNECTIONS. SIPS PANELS SHALL WITHSTAND THE EFFECTS OF ALL DESIGN LOADS WITHOUT EXCEEDING THE ALLOWABLE WORKING STRESSES. SIP PANEL DEFLECTIONS SHALL BE LIMITED TO A MAXIMUM DEFLECTION OF L/240 OF THE SPAN.
- C. THE SIPS SUPPLIER SHALL PRODUCE A COMPLETE SET OF SEALED SHOP DRAWINGS AND CALCULATIONS FOR REVIEW BY THE EOR. THE SHOP DRAWINGS SHALL INCLUDE ALL INFORMATION REQUIRED TO PROPERLY INSTALL THE PANELS, AND ANY DETAILS AND ALL DESIGN LOADINGS REQUIRED FOR INTERFACING WITH THE BUILDING FOUNDATION AND SLAB SYSTEM.
- D. THE PANELS SHALL BE BRACED DURING INSTALLATION. BRACING TO BE DESIGNED BY THE SIPS SUPPLIER AND SHALL REVIEW THE INSTALLATION PROCEDURES WITH THE GENERAL CONTRACTOR TO ENSURE PROPER INSTALLATION.
- E. SIP PANEL MANUFACTURER SHALL BE APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE OWNER OR THE OWNER'S AUTHORIZED AGENT FOR SUBMITTAL TO THE BUILDING OFFICIAL AS SPECIFIED IN SECTION 1704.5 STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

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CIRCLE K STORES, INC.

ANGIER, NC

9706 KENNEBEC CHURCH ROAD,
ANGIER, NC 27501

PROTOCOL # R1.1 11/21/22



CIRCLE K STORE INC.
PROJECT NUMBER: 22130

GENERAL NOTES

S0.1

ABBREVIATIONS

- AB ANCHOR BOLT
- ACI AMERICAN CONCRETE INSTITUTE
- AFF ABOVE FINISHED FLOOR
- AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION
- AISI AMERICAN IRON AND STEEL INSTITUTE
- ARCH ARCHITECTURAL
- ASTM AMERICAN SOCIETY FOR TESTING AND MATERIALS
- AWS AMERICAN WELDING SOCIETY
- BFF BELOW FINISHED FLOOR
- BL BLOCK LINTEL
- BM BEAM
- BO BOTTOM OF
- BOS BOTTOM OF STEEL
- BRG BEARING
- CJ CONTRACTION JOINT
- CL CENTER LINE
- CLR CLEAR
- CMU CONCRETE MASONRY UNIT
- COL COLUMN
- CONC CONCRETE
- COND CONDENSER UNIT
- CONST CONSTRUCTION
- CONT CONTINUOUS
- DBA DEFORMED BAR ANCHOR
- DIA DIAMETER
- EF EXHAUST FAN
- EIFS EXTERIOR INSULATION AND FINISH SYSTEM
- EJ EXPANSION JOINT
- EL ELEVATION
- ELEC ELECTRICAL
- EQ EQUAL
- EW EACH WAY
- FDN FOUNDATION
- FF FINISHED FLOOR
- FS FAR SIDE
- FTG FOOTING
- FV FIELD VERIFY
- GA GAUGE
- GC GENERAL CONTRACTOR
- GYP GYPSUM BOARD
- H HEIGHT
- HORIZ HORIZONTAL
- HSA HEADED STUD ANCHOR
- HSS HOLLOW STRUCTURAL SHAPE
- INFO INFORMATION
- ISO ISOLATION
- JBE JOIST BEARING ELEVATION
- JST JOIST
- JT JOINT
- KSI KIPS PER SQUARE INCH
- L LENGTH
- LB POUNDS
- LLH LONG LEG HORIZONTAL
- LLV LONG LEG VERTICAL
- LONG LONGITUDINAL
- MAX MAXIMUM
- MECH MECHANICAL
- MFR MANUFACTURER
- MIN MINIMUM
- MISC MISCELLANOUS
- MO MASONRY OPENING
- MTL METAL
- NIC NOT IN CONTRACT
- NO NUMBER
- NS NEAR SIDE
- NTS NOT TO SCALE
- OC ON CENTER
- OD OUTSIDE DIAMETER
- OFOP OUTSIDE FACE OF PANEL
- OH OPPOSITE HAND
- OSL OUTSTANDING LEG
- PAF POWER ACTUATED FASTENER
- PC PRECAST
- PCF POUNDS PER CUBIC FOOT
- PL PLATE
- PLF POUNDS PER LINEAR FOOT
- PLUMB PLUMBING
- PMEJ PREMOLDED EXPANSION JOINT
- PSF POUNDS PER SQUARE FOOT
- PSI POUNDS PER SQUARE INCH
- QTY QUANTITY
- ROU REFRIGERATION CONDENSING UNIT
- REF REFER TO
- REINF REINFORCING
- REQD REQUIRED
- REV REVERSE
- RLL ROOF LIVE LOAD
- RO ROUGH OPENING
- ROT ROTATE
- RTU ROOF TOP UNIT
- SCHED SCHEDULE
- SDI STEEL DECK INSTITUTE
- SIM SIMILAR
- SIP STRUCTURAL INSULATED PANEL
- SJI STEEL JOIST INSTITUTE
- SIPCS SPACES
- SPECS SPECIFICATIONS
- STRUC STRUCTURAL
- T&B TOP AND BOTTOM
- THK THICKNESS
- TL TOTAL LOAD
- TO TOP OF
- TOC TOP OF CONCRETE
- TOF TOP OF FOOTING
- TOGB TOP OF GRADE BEAM
- TOS TOP OF STEEL
- TOW TOP OF WALL
- TRANS TRANSVERSE
- TYP TYPICAL
- UNO UNLESS NOTED OTHERWISE
- VERT VERTICAL
- W WIDTH
- WP WORK POINT
- WWR WELDED WIRE REINFORCEMENT

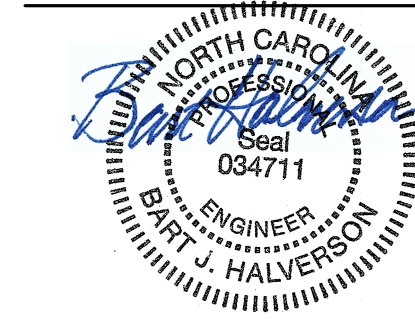
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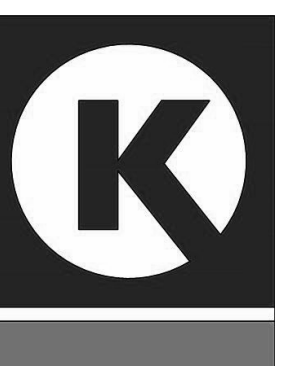
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ANGIER, NC

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PROTOCOL # R1.1 11/21/22

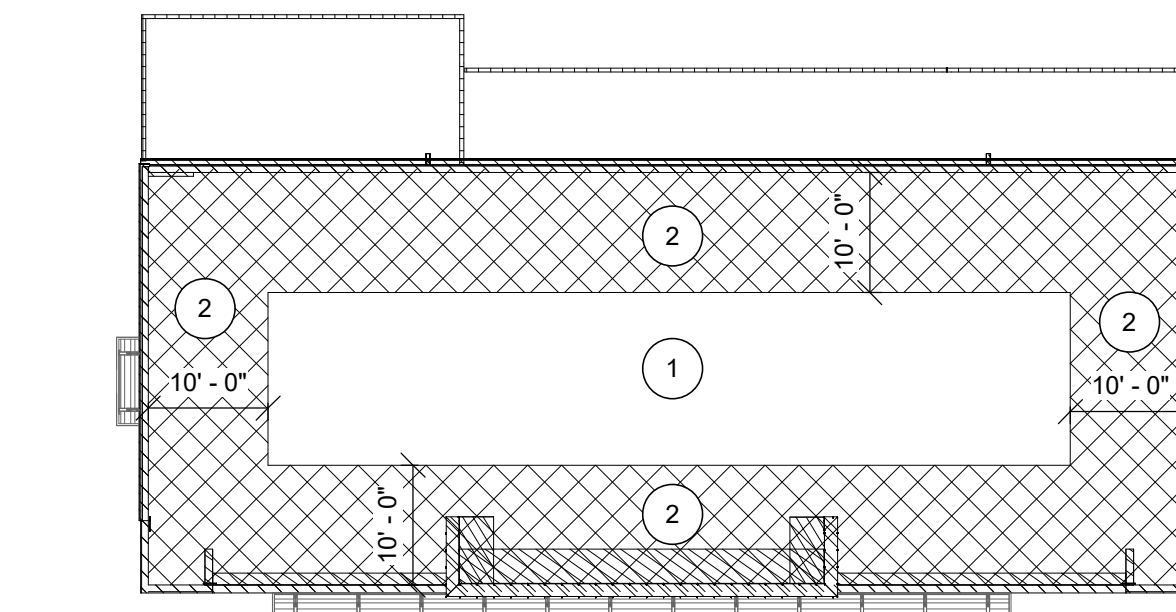


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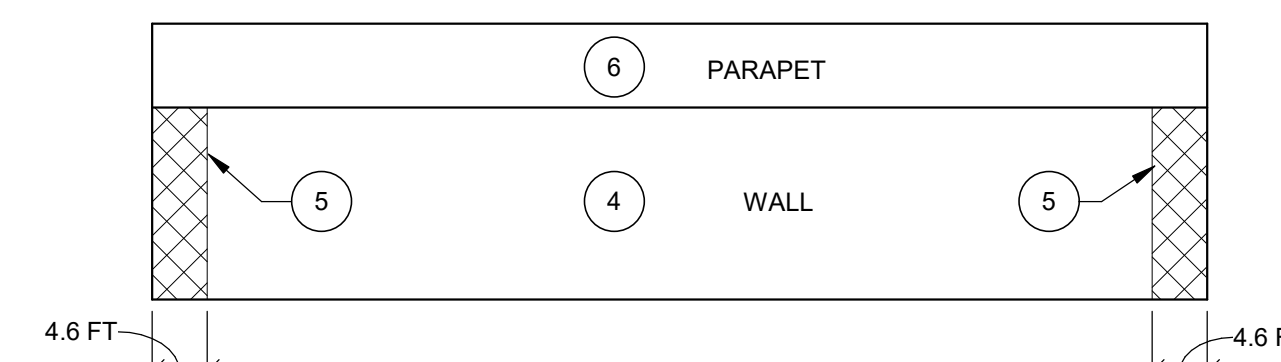
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DIAGRAMS AND SCHEDULES

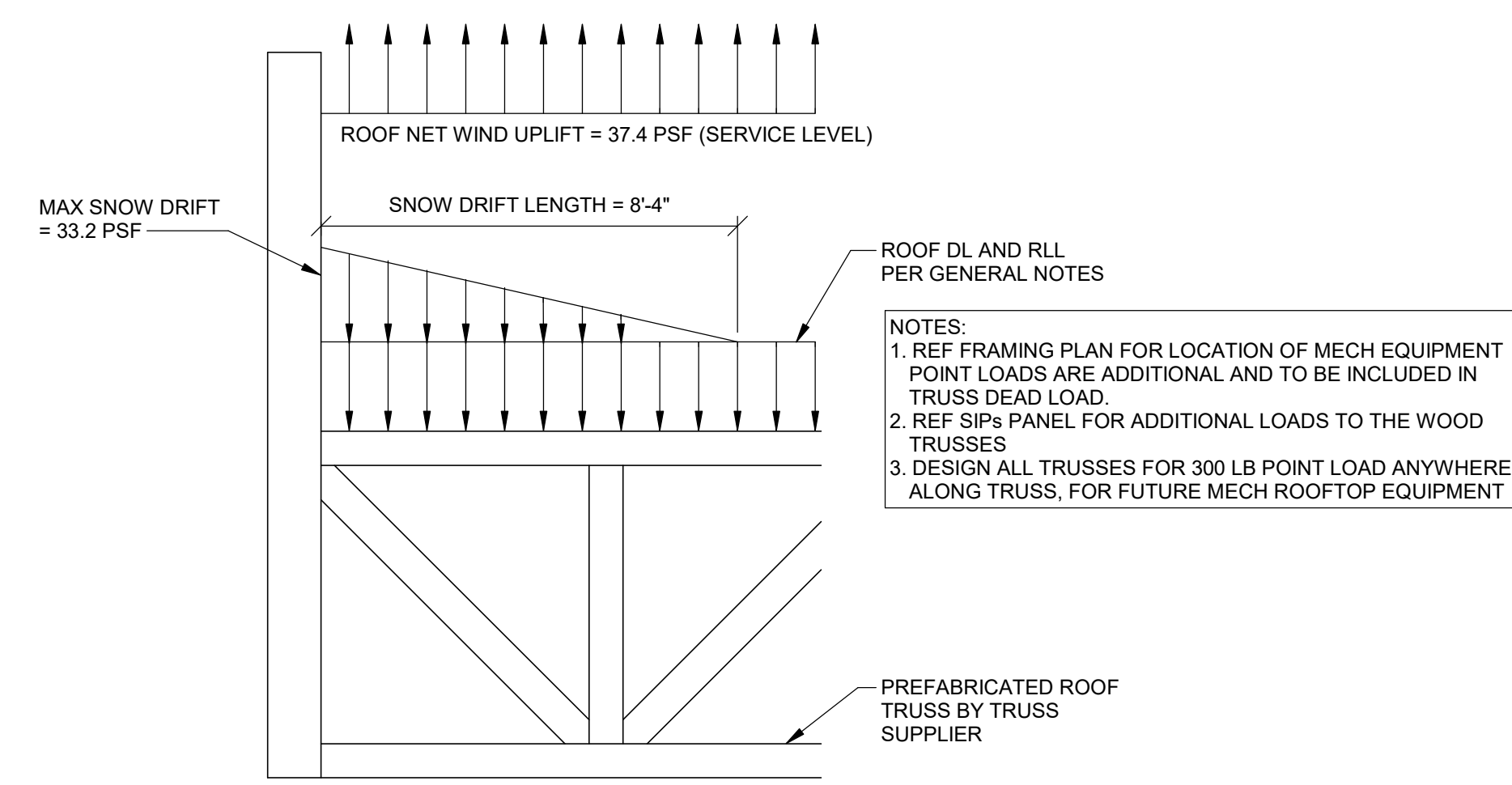
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COMPONENT AND CLADDING WIND PRESSURE DIAGRAM - ROOF



COMPONENT AND CLADDING WIND PRESSURE DIAGRAM - WALLS



TYPICAL WOOD TRUSS LOADING DIAGRAM | 2

3/4" = 1'-0"

C&C ZONE	EFFECTIVE WIND AREA (SF)	POSITIVE PRESSURE (PSF)	NEGATIVE PRESSURE (PSF)	C&C ZONE	EFFECTIVE WIND AREA (SF)	POSITIVE PRESSURE (PSF)	NEGATIVE PRESSURE (PSF)
1	10	+11.4	-44.7	4	10	+25.7	-27.8
	20	+10.7	-41.7		20	+24.5	-26.7
	50	+10.0	-37.8		50	+23.0	-25.2
	100	+10.0	-34.9		100	+21.9	-24.0
2	10	+11.4	-59.0	5	10	+25.7	-34.2
	20	+10.7	-55.2		20	+24.5	-32.0
	50	+10.0	-50.2		50	+23.0	-29.0
	100	+10.0	-46.4		100	+21.9	-26.7
3	10	+11.4	-80.4	6	10	+103.1	-54.3
	20	+10.7	-72.8		20	+93.8	-50.7
	50	+10.0	-62.8		50	+81.7	-45.9
	100	+10.0	-55.2		100	+72.4	-42.3

COMPONENT AND CLADDING WIND PRESSURE SCHEDULE (145 MPH)

COMPONENTS & CLADDING WIND PRESSURES | 1

NTS

LEGEND

- ◇ OFOW OUTSIDE FACE OF WALL - CONCRETE STEM WALL
- WFX FOOTING MARK; REF SCHED
- FX
- ROOF TOP UNIT (RTU)
- EXHAUST FAN (EF)
- APPROXIMATE LOCATION OF SHEAR WALL HOLD DOWN, REF SIPS PANEL DRAWINGS FOR HOLD DOWN DETAILS AND EXACT LOCATIONS
- ◇ OFOP OUTSIDE FACE OF PANEL - SIP OR INSULATED WALL

STRUCTURAL SPECIAL INSPECTIONS (2015 IBC)

SOIL BEARING (2015 IBC 1705.6)		
VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	PERIODIC	-
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	PERIODIC	-
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	PERIODIC	-
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	CONTINUOUS	-
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	PERIODIC	-

CONCRETE CONSTRUCTION (2015 IBC 1705.3 AND ACI 318-14)		
VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD
1. INSPECTION OF REINFORCING STEEL AND PLACEMENT	PERIODIC	IBC 1908.4; ACI 318: Ch. 20, 25.2, 25.3, 26.6.1-26.6.3
2. NOT USED.	-	-
3. INSPECTION OF ANCHORS CAST IN CONCRETE INCLUDING, SIZE, LENGTH, PROJECTION, AND LOCATION.	PERIODIC	ACI 318: 17.8.2
4A. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS - ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. INSPECTIONS SHALL INCLUDE HOLE SIZE AND DEPTH, CLEANING PROCEDURE, MATERIALS, AND LOCATION.	CONTINUOUS	ACI 318: 17.8.2.4
4B. INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS - MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 3A. INSPECTIONS SHALL INCLUDE HOLE SIZE AND DEPTH, CLEANING PROCEDURE, MATERIALS, AND LOCATION.	PERIODIC	ACI 318: 17.8.2
5. INSPECTION TO VERIFY USE OF REQUIRED DESIGN MIX.	PERIODIC	IBC 1904.1, 1904.2, 1908.2, 1908.3; ACI 318: CH. 19, 26.4.3, 26.4.4
6. INSPECTION PRIOR TO CONCRETE PLACEMENT, AND AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS, FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. REF TO GENERAL NOTES, CAST-IN-PLACE CONCRETE EXECUTION FOR TESTING REQUIREMENTS.	CONTINUOUS	IBC 1908.10; ASTM C172, ASTM C31; ACI 318: 26.4.5, 26.12
7. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	CONTINUOUS	IBC:1908.6, 1908.7, 1908.8; ACI 318: 26.4.5
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	PERIODIC	IBC:1908.9; ACI 318: 26.4.7-26.4.9
9. NOT USED.	-	-
10. INSPECT ERECTION OF PRECAST (OR SITE CAST) CONCRETE MEMBERS.	PERIODIC	ACI 318: CH. 26.8
11. NOT USED.	-	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	PERIODIC	ACI 318: 26.11.1.2(b)
EXCEPTIONS TO CONCRETE SPECIAL INSPECTION:		
1. INSPECTION IS NOT REQUIRED FOR PLACEMENT OF SLAB-ON-GRADE CONCRETE. INSPECTION OF SLAB-ON-GRADE REINFORCEMENT IS REQUIRED PRIOR TO PLACEMENT OF CONCRETE.		
2. INSPECTION IS NOT REQUIRED FOR FOR PLACEMENT OF FOUNDATION CONCRETE (DESIGN f_c OF 2,500 PSI). INSPECTION OF FOUNDATION REINFORCEMENT IS REQUIRED PRIOR TO PLACEMENT OF CONCRETE.		

WOOD LATERAL FORCE RESISTING SYSTEM (2015 IBC 1705.11 AND 1705.12):			
VERIFICATION AND INSPECTION	FREQUENCY	REFERENCED STANDARD	
1. WIND-RESISTING SYSTEM (IBC 1705.11.1) AND SEISMIC RESISTING SYSTEM (IBC 1705.12.2)			
A. VERIFY NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITH IN THE MAIN LATERAL FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLD-DOWN WHERE NAIL SPACING OF WOOD DIAPHRAGMS AND WOOD SHEAR WALLS IS 4" OR LESS.	PERIODIC	-	
B. INSPECTION OF FIELD-GLUING OPERATION OF ELEMENTS OF THE MAIN WIND FORCE-RESISTING SYSTEM WHERE NAIL SPACING OF WOOD DIAPHRAGMS AND WOOD SHEAR WALLS IS 4" OR LESS.	CONTINUOUS	-	
2. WIND-RESISTING COMPONENTS (IBC 1705.11.3)			
A. INSPECTION IS REQUIRED FOR THE FASTENING OF THE FOLLOWING SYSTEMS AND COMPONENTS:			
i. ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS.	PERIODIC	-	
ii. EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING.	PERIODIC	-	

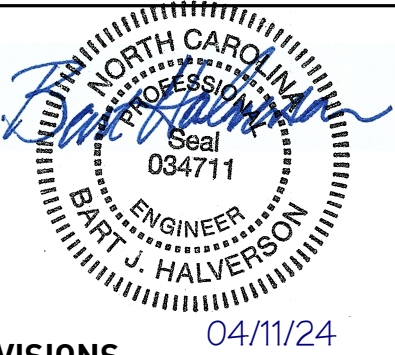
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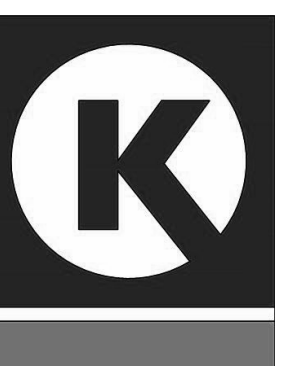
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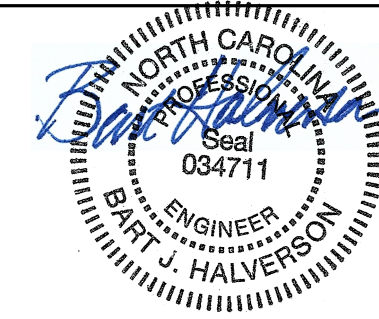
TABLE OF SPECIAL INSPECTIONS

S0.3

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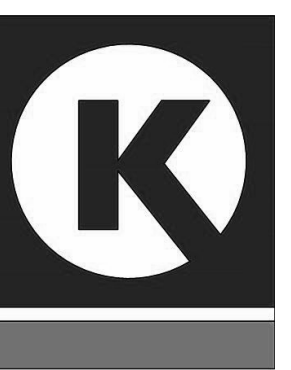
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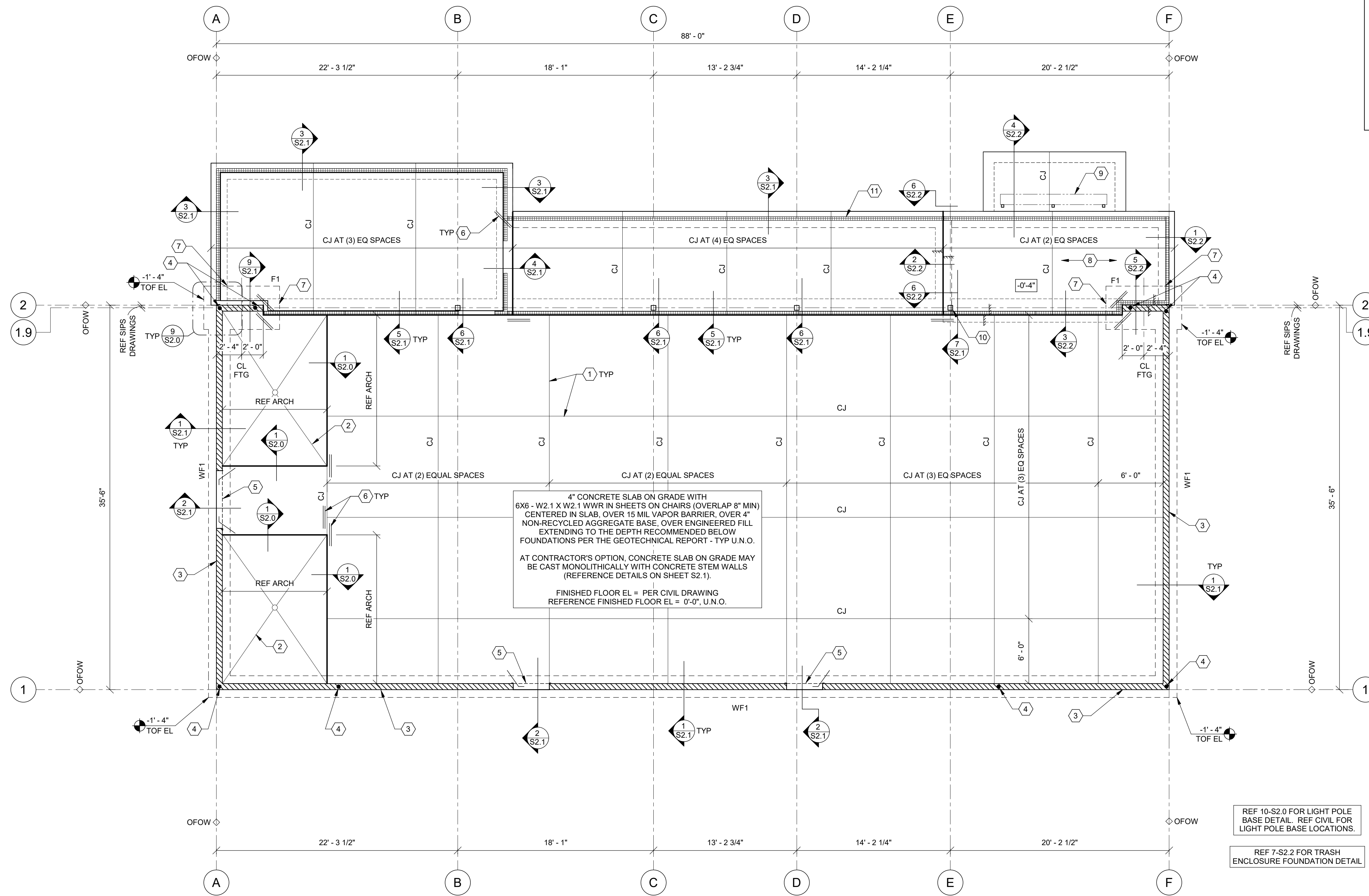
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FOUNDATION AND SLAB PLAN

S1.0

WALL LEGEND	
	STRUCTURAL INSULATED PANELS (SIPs)
	PRE-MANUFACTURED COOLER / FREEZER WALLS BY SUPPLIER
	APPROXIMATE LOCATION OF SHEAR WALL HOLD DOWN. REF SIPs PANEL DRAWINGS FOR HOLD DOWN DETAILS AND EXACT LOCATIONS
	FINISH FLOOR ELEVATION



REF 10-S2.0 FOR LIGHT POLE BASE DETAIL. REF CIVIL FOR LIGHT POLE BASE LOCATIONS.

REF 7-S2.2 FOR TRASH ENCLOSURE FOUNDATION DETAIL.

FOUNDATION AND SLAB PLAN | 1

3/16" = 1'-0"

FOUNDATION PLAN NOTES	
NUMBER	PLAN NOTE
GENERAL PLAN NOTES	
A.	REF SHEET S0.1, S0.2 AND S0.3 FOR GENERAL NOTES, LEGEND AND SPECIAL INSPECTIONS REQUIREMENTS.
B.	REFER TO SHEET S1.1 AND ARCH DRAWINGS FOR LOCATIONS, DIMENSIONS AND ELEVATIONS OF EXTERIOR SIP WALL AND INSULATION METAL PANEL WALLS.
C.	ALL ELEVATIONS REFERENCED FROM REFERENCE FINISH FLOOR ELEVATION OF 0'-0".
D.	TOP OF FOOTING ELEVATION SHALL BE ASSUMED TO BE CONSTANT BETWEEN ELEVATIONS SHOWN ON PLAN U.N.O. (FOUNDATION REQUIREMENTS ENSCRIBED ON S0.1 PER GEOTECHNICAL ENGINEER RECOMMENDATIONS SHALL BE FOLLOWED AS GRADING CONDITIONS VARY, REF CIVIL).
E.	FOR TYPICAL REINFORCING BAR BEND DETAILS, REF 4-S2.0.
F.	FOR PIPING RUNNING PARALLEL AND PERPENDICULAR TO THE FOUNDATION REF 6-S2.0 & 7-S2.0.
G.	THE GENERAL CONTRACTOR, INCLUDING SUBCONTRACTORS AND SUPPLIERS, SHALL NOT ALLOW ANY VEHICULAR TRAFFIC ON THE BUILDING PAD AFTER THE PAD CERTIFICATION OR THE CONCRETE SLAB ONCE PLACED. ANY DAMAGE TO THE PAD DUE TO THE VEHICULAR TRAFFIC SHALL BE REPAIRED/REPLACED AT THE EXPENSE OF THE CONTRACTOR.
H.	FOUNDATION CONTRACTOR NEEDS TO CONTACT THE ELECTRICAL CONTRACTOR SO THEY CAN INSTALL THE UFER GROUNDING ELECTRODE BEFORE THE FOUNDATION IS INSPECTED.

FOUNDATION PLAN NOTES	
NUMBER	PLAN NOTE
REFERENCED PLAN NOTES	
1	FOR TYPICAL FLOOR CONSTRUCTION JOINT, REF 1-S2.0; FOR TYPICAL FLOOR CONTRACTION JOINT, REF 2-S2.0.
2	COORDINATE SLAB SLOPES AT DRAIN LOCATIONS WITH THE ARCH DRAWINGS.
3	CONCRETE STEM WALL: REF DETAIL 1-S2.1 FOR DETAILS AND FACE OF WALL LOCATION. REF DETAILS 13, 14, AND 15 ON SHEET S2.0 FOR STEM WALL DETAILS.
4	INDICATES SHEAR WALL HOLDDOWN; REF SIPs PANEL DRAWINGS FOR DETAILS AND EXACT LOCATIONS. ALL HOLDDOWN ANCHORS REQUIRING ANCHOR BOLTS SHALL BE EMBEDDED INTO THE CONTINUOUS FOOTING THE SPECIFIED AMOUNT SHOWN IN DETAIL 12-S2.0.
5	REF ARCH FOR ALL WALL OPENING SIZES AND LOCATIONS WITH THE ARCH DRAWINGS. REF 11-S2.0 FOR RE-ENTRANT BARS AT OPENINGS.
6	REF 5-S2.0 FOR SLAB REINFORCEMENT AT RE-ENTRANT CORNERS.
7	DOWEL CONTINUOUS REINFORCEMENT IN GRADE BEAM; DOWEL BOTTOM BARS INTO FOOTING, AND TOP BARS INTO STEM WALL / SLAB TURNDOWN WHERE APPLICABLE.
8	GC TO COORDINATE LOCATION OF RECESSED SLAB WITH FREEZER BOX VENDOR AND ARCH DRAWINGS.
9	EXTERIOR ELECTRICAL EQUIPMENT, COORDINATE LOCATION WITH ELEC DRAWINGS; REF 4-S2.2 FOR LATERAL SUPPORT POST (3 PLACES).
10	FREEZER VENDOR TO DESIGN FREEZER TO RESIST COLUMN REACTIONS PER SIPs PANEL MANUFACTURER. REFER TO SIPs PANEL AND FREEZER DRAWINGS FOR COLUMN BEARING CONDITION AND ADDITIONAL INFORMATION.
11	PRE-MANUFACTURED INSULATED METAL PANEL FREEZER/COOLER WALL PANELS DESIGNED AND ANCHORED TO SUPPORT ALL BUILDING CODE REQUIRED LOADS. REF ARCH FOR SIZES AND LOCATION.

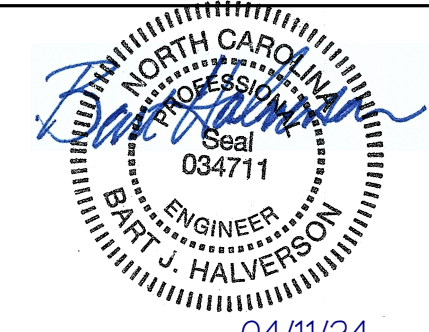
WALL FOOTING SCHEDULE					
MARK	WIDTH	THICKNESS	REINF (TRANS)	REINF (LONG)	REINF LOCATION
WF1	2'-0"	1'-0"	#5 AT 24" OC	(3) #5	BOT

COLUMN FOOTING SCHEDULE						
MARK	WIDTH	LENGTH	THICKNESS	REINF (TRANS)	REINF (LONG)	REINF LOCATION
F1	4'-0"	7'-0"	1'-6"	(9) #5	(5) #5	T&B

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PROFESSIONAL SEAL



04/11/24

REVISIONS

ISSUE	DATE
OTF	02/15/23
OTB SET	01/05/24

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NORTH CAROLINA COA #0-9672 EXPIRES 06/30/23
JBA PROJECT #: 23023

PROFESSIONAL IN CHARGE

BJH

PROJECT MANAGER

RB

QUALITY CONTROL

SMK

DRAWN BY

JBA

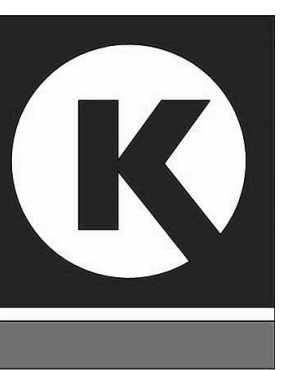
PROJECT NAME

CIRCLE K STORES, INC.

ANGIER, NC

9706 KENNEBEC CHURCH ROAD,
ANGIER, NC 27501

PROTOCOL # R1.1 11/21/22

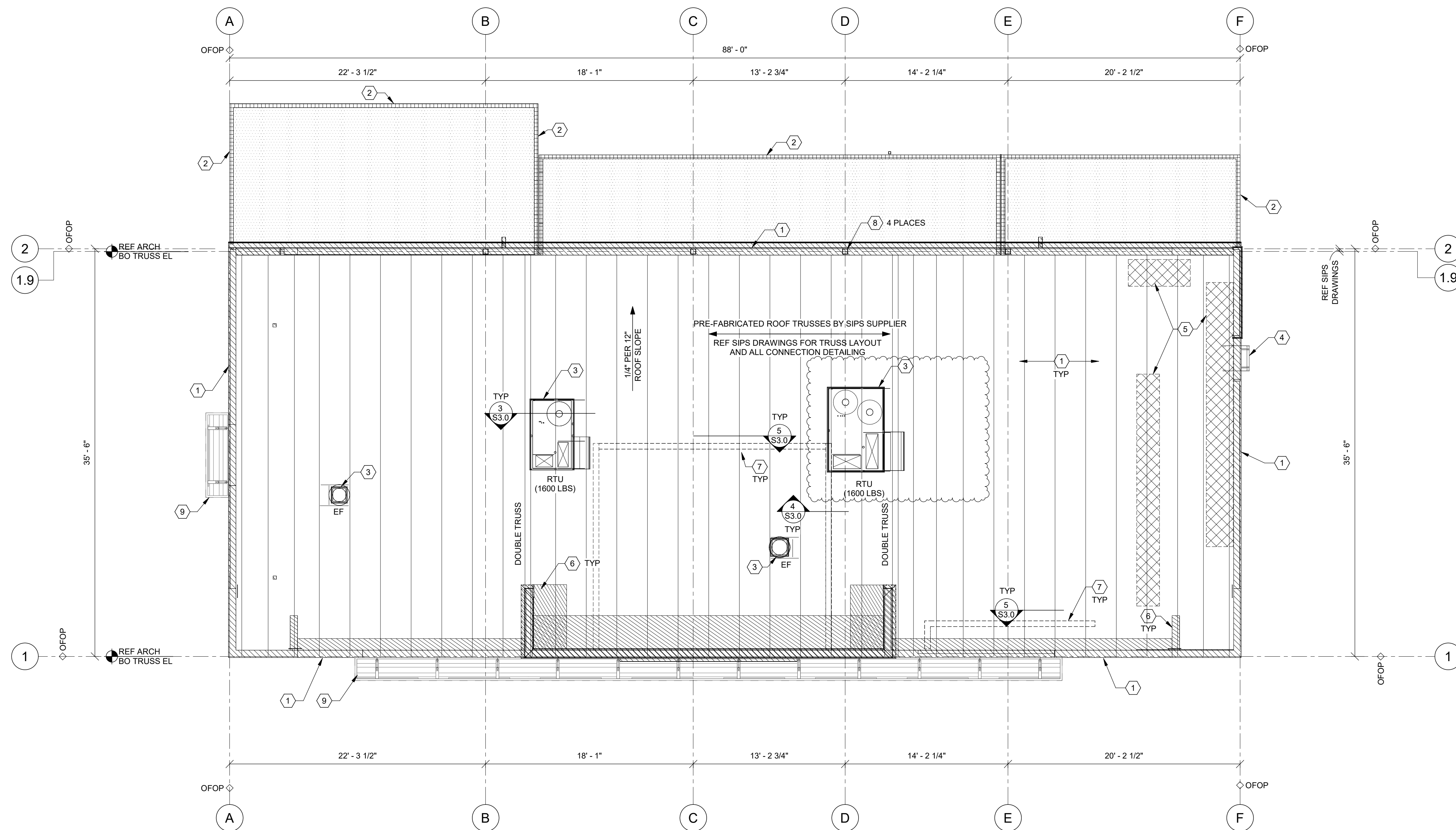


CIRCLE K STORE INC.

PROJECT NUMBER: 22130

ROOF FRAMING PLAN

S1.1



ROOF FRAMING PLAN | 1

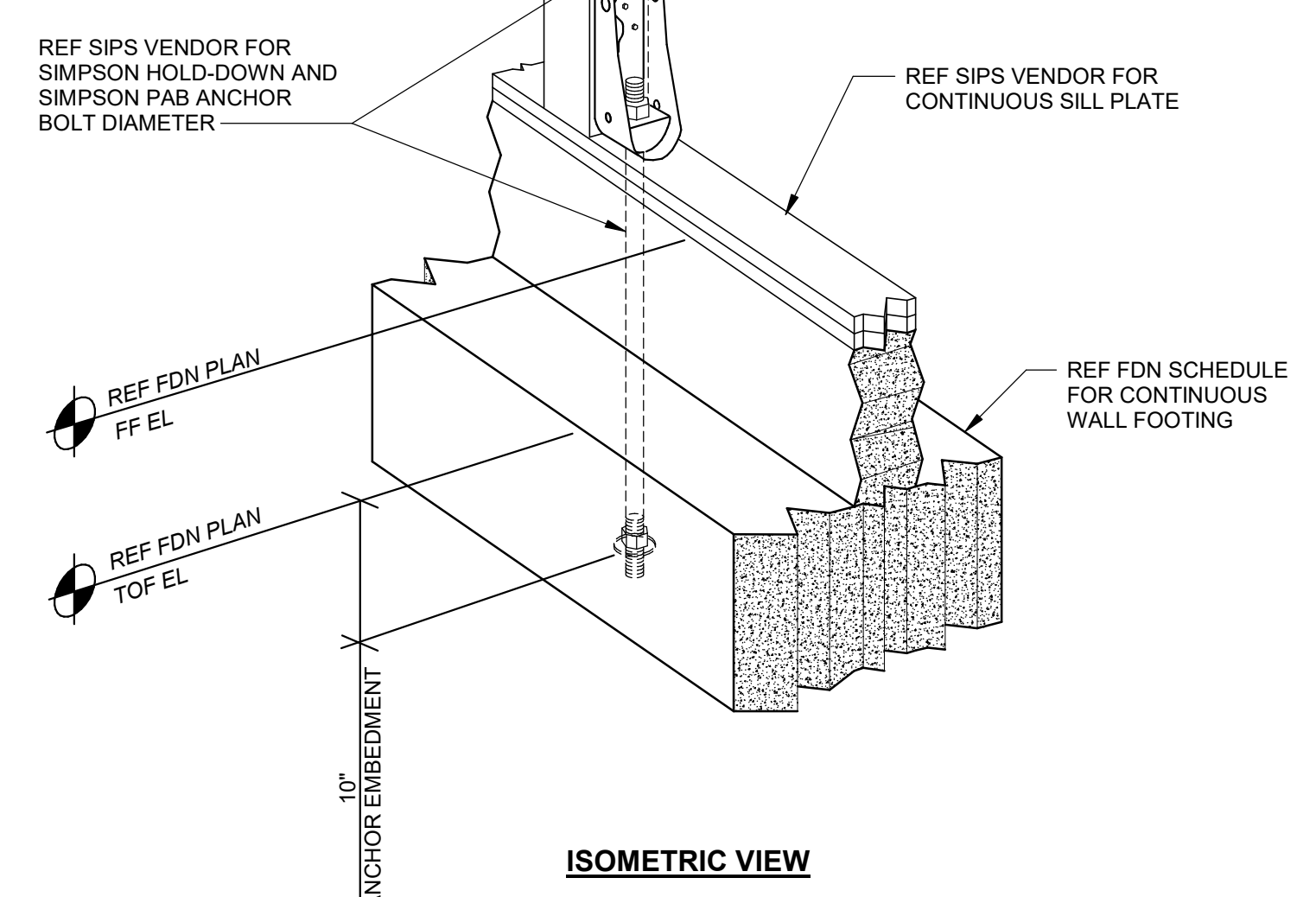
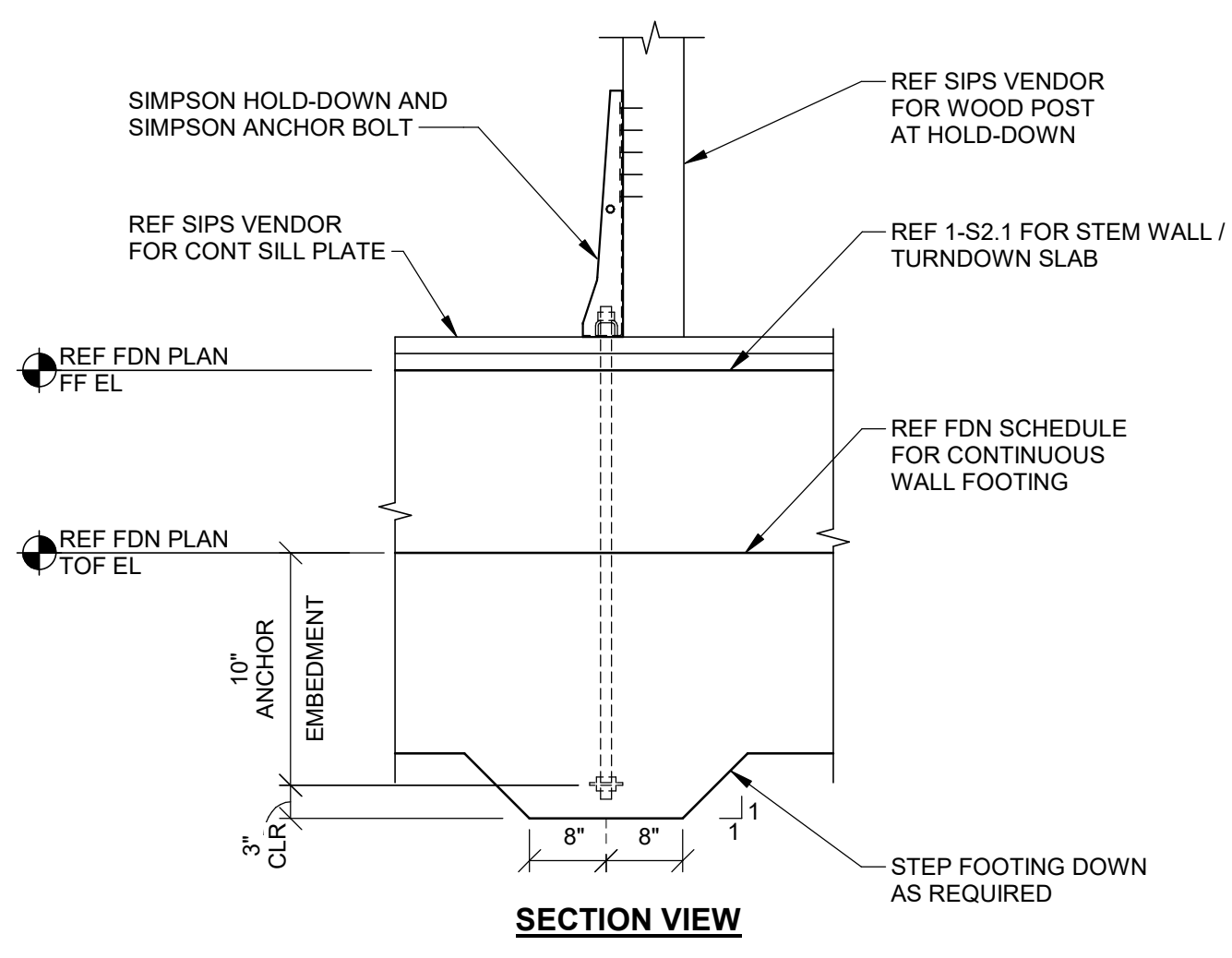
3/16" = 1'-0"

ROOF FRAMING PLAN NOTES	
NUMBER	PLAN NOTE
GENERAL PLAN NOTES	
A.	REF SHEET S0.1, S0.2 AND S0.3 FOR GENERAL NOTES AND SPECIAL INSPECTIONS REQUIREMENTS.
B.	REFER TO ARCH DRAWINGS FOR LOCATIONS, DIMENSIONS AND ELEVATIONS OF EXTERIOR SIP WALLS AND INSULATION METAL PANEL WALLS.
C.	REF ARCH DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS.
D.	ALL ITEMS SUCH AS MECHANICAL EQUIPMENT, DUCT WORK, PIPES, CEILINGS, FIXTURES, ETC. THAT ARE TO BE SUPPORTED OR HUNG FROM THE WOOD TRUSSES SHALL BE FRAMED WITH AUXILIARY FRAMING TO THE PANEL POINTS OF THE TRUSS. METHODS OF FRAMING THAT INDUCE BENDING TO THE TRUSS CHORD OR WEB MEMBERS WILL NOT BE PERMITTED. COORDINATE BRIDGING LOCATIONS SO AS NOT TO INTERFERE WITH ANY MECHANICAL EQUIPMENT.
E.	APPROXIMATE WEIGHTS OF ROOF TOP EQUIPMENT ARE SHOWN ON PLAN. VERIFY ALL LOADS, LOCATIONS, CURB SIZES AND OPENING SIZES WITH MECHANICAL CONTRACTOR PRIOR TO FABRICATION. ADJUST SPACING OF WOOD TRUSSES AS REQUIRED FOR MECHANICAL OPENINGS. REF TYPICAL RTU SUPPORT DETAIL 3-S3.0 FOR FRAMING.
F.	RTUS AND RCUS MUST BE PLACED ON OR BETWEEN TRUSSES DESIGNED FOR THE ADDITIONAL LOAD AS SHOWN. REF MECHANICAL FOR EXACT LOCATIONS.
G.	PLACEMENT OF SPRINKLER CONTRACTOR'S PIPING AND TRUSS BRIDGING SHALL BE COORDINATED BY THE GENERAL CONTRACTOR. THE CONTRACTOR SHALL MAKE PROVISIONS FOR THE TRUSS SUPPLIER TO ADJUST/RELOCATE BRIDGING OR SPRINKLER CONTRACTOR TO ADJUST PIPING VERTICALLY WHERE CONFLICTS OCCUR. ALL CHANGES REQUIRED OF THE TRUSS BRIDGING SHALL BE DESIGNED BY TRUSS MANUFACTURER.

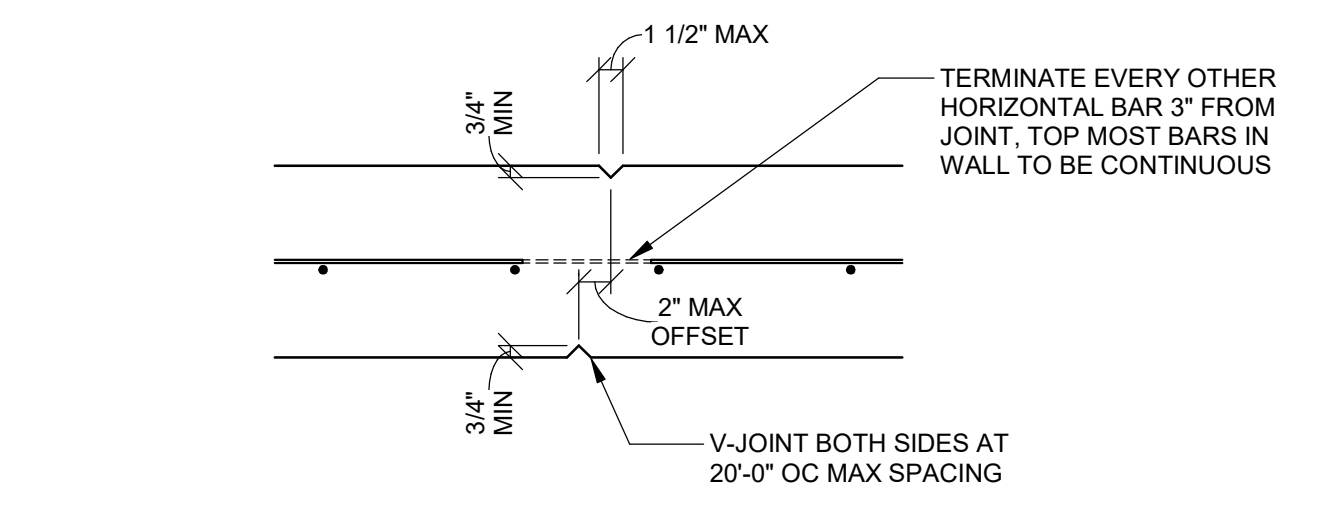
ROOF FRAMING PLAN NOTES	
NUMBER	PLAN NOTE
REFERENCED PLAN NOTES	
1	SIP PANEL ROOF DESIGNED TO SUPPORT ALL BUILDING CODE REQUIRED LOADS AND DESIGNED AS A DIAPHRAGM TO DISTRIBUTE WIND/SEISMIC LOADS TO SHEAR WALLS. REF 1-S3.0 FOR ADDITIONAL FRAMING AT OPENINGS. REF ARCH FOR TOP OF SIP PANEL ELEVATIONS.
2	PRE-MANUFACTURED INSULATED METAL PANEL FREEZER/COOLER WALL PANELS DESIGNED AND ANCHORED TO SUPPORT ALL BUILDING CODE REQUIRED LOADS. REF ARCH FOR SIZES AND LOCATION.
3	REF 1-S3.0 FOR RTU DUCT OPENING AND ROOF OPENING FRAMES. GC TO VERIFY LOCATION, SIZE AND WEIGHT OF MECHANICAL UNITS. OPERATING WEIGHT SHALL NOT EXCEED THE WEIGHT NOTED ON THE PLAN.
4	EXTERIOR ROOF ACCESS LADDER, REF ARCH FOR LOCATION. SIP PANEL MANUFACTURER TO COORDINATE CONNECTION.
5	DESIGN TRUSSES FOR AN ADDITIONAL 50 PSF STORAGE SHELF LOAD (RL), TYP AT THIS STYLE HATCH; REF ARCH FOR LOCATIONS AND DETAILS.
6	PARAPET BRACING DESIGNED BY SIPS SUPPLIER.
7	INTERIOR SOFFIT FRAMING, REF ARCH FOR LOCATIONS AND REF APPLICABLE STRUC FRAMING SECTIONS FOR SUPPORT AND ADDITIONAL LOADS TO TRUSSES.
8	COLUMN BEARING CONDITION TO BE COORDINATED WITH SIPS PANEL AND COOLER / FREEZER DRAWINGS; COOLER / FREEZER BOXES TO BE DESIGNED TO TRANSFER COLUMN DOWNWARD AND UPLIFT REACTIONS PER SIPS PANEL VENDOR. REFER TO SIPS PANEL AND FREEZER DRAWINGS FOR COLUMN BEARING CONDITION AND ADDITIONAL INFORMATION.
9	PRE-MANUFACTURED CANOPY BY OTHERS, REF ARCH. SIP PANEL MANUFACTURER COORDINATE, CONNECTION WITH CANOPY MANUFACTURER.

WALL LEGEND	
	STRUCTURAL INSULATED PANELS (SIP)
	FREEZER / COOLER WALL PANEL
	FREEZER / COOLER CEILING PANEL BELOW

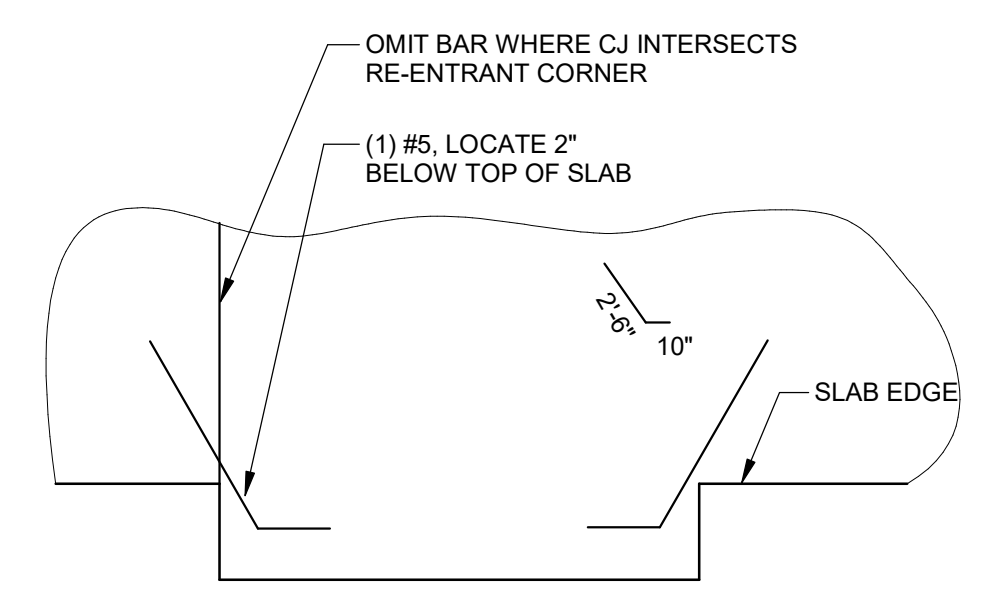
ROOF DEAD LOADS	
DEAD LOADS	
MEMBRANE (UNBALASTED)	= 1.0 PSF
SIPS PANEL	= 2.0 PSF
WOOD TRUSSES	= 3.0 PSF
MECH/ELEC/FIRE SPRINK	= 3.0 PSF
MISC	= 4.0 PSF
TOTAL	= 15.0 PSF



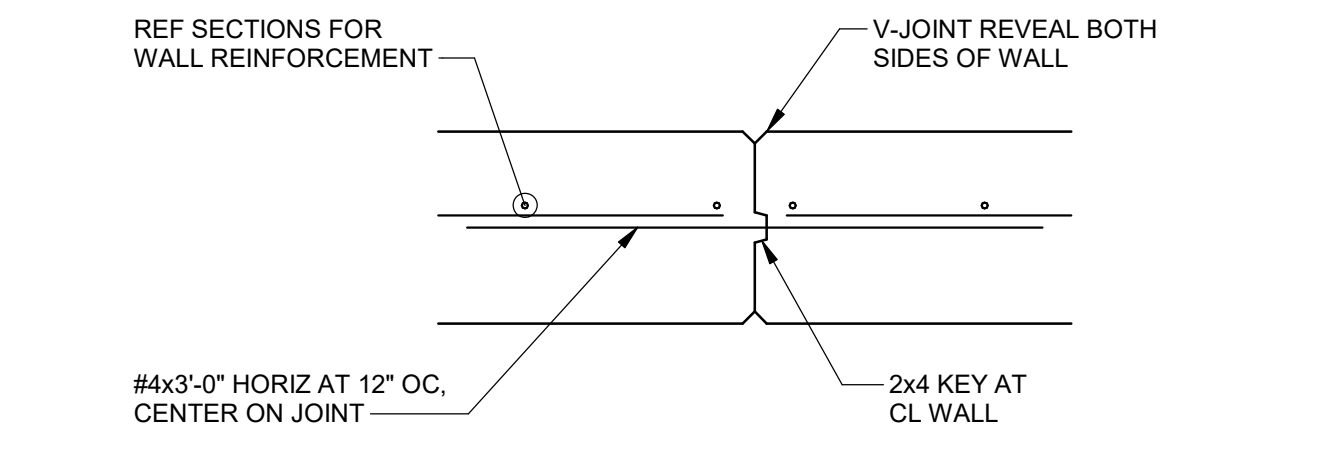
SHEAR WALL HOLD DOWN | 12
3/4" = 1'-0"



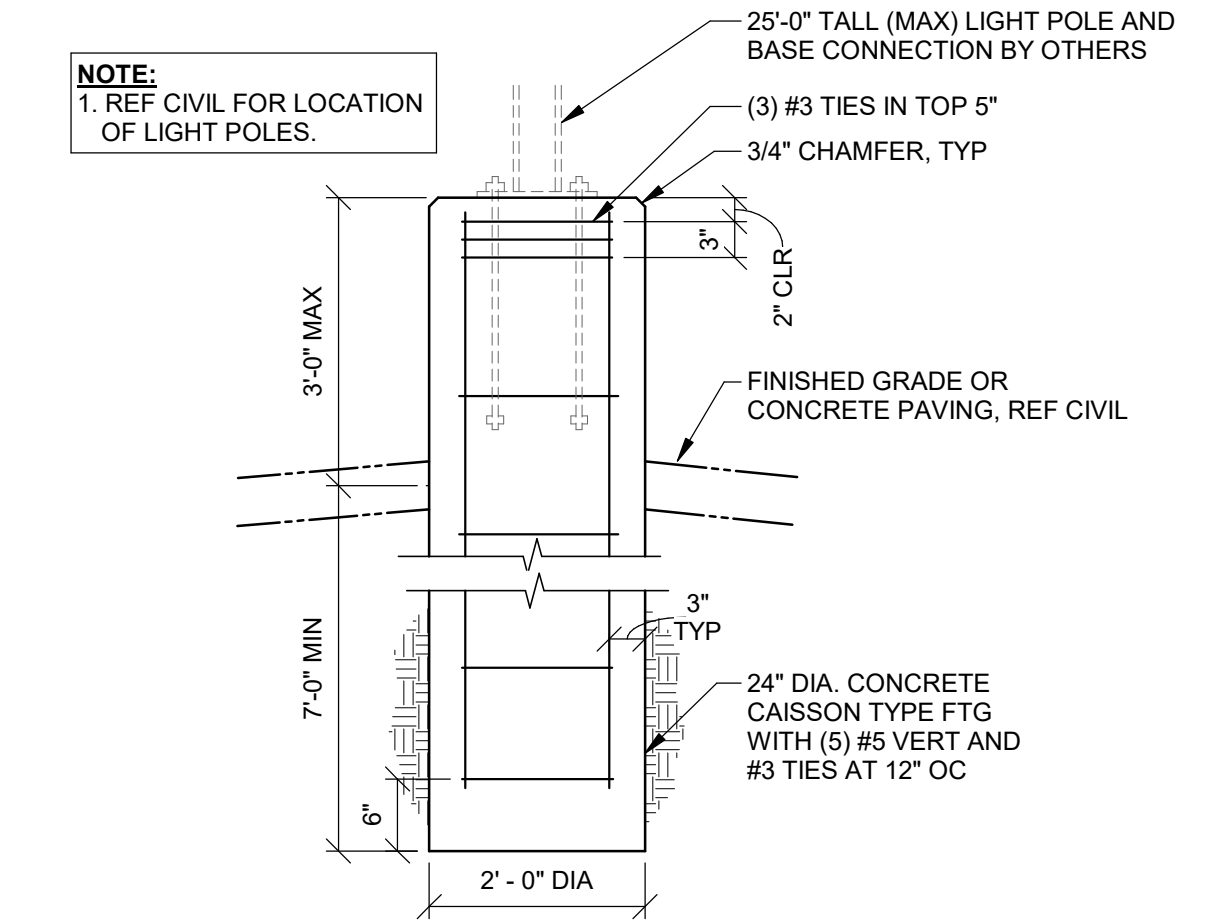
CONCRETE WALL CONTROL JOINT | 15
1" = 1'-0"



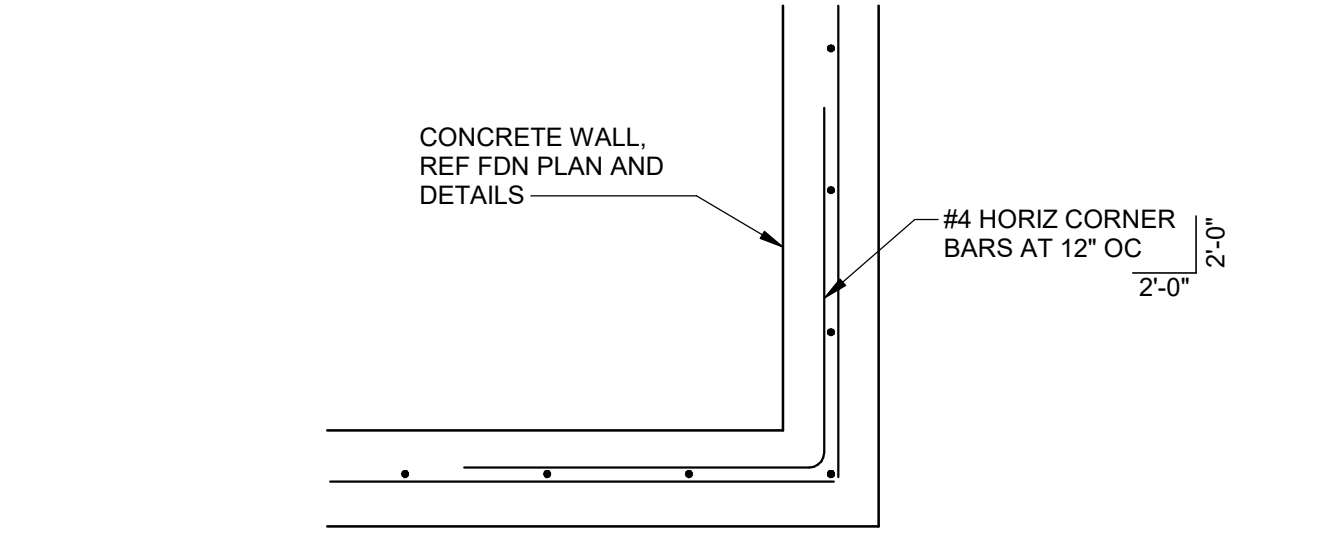
SLAB REINFORCEMENT AT DOOR OPENINGS | 11
3/4" = 1'-0"



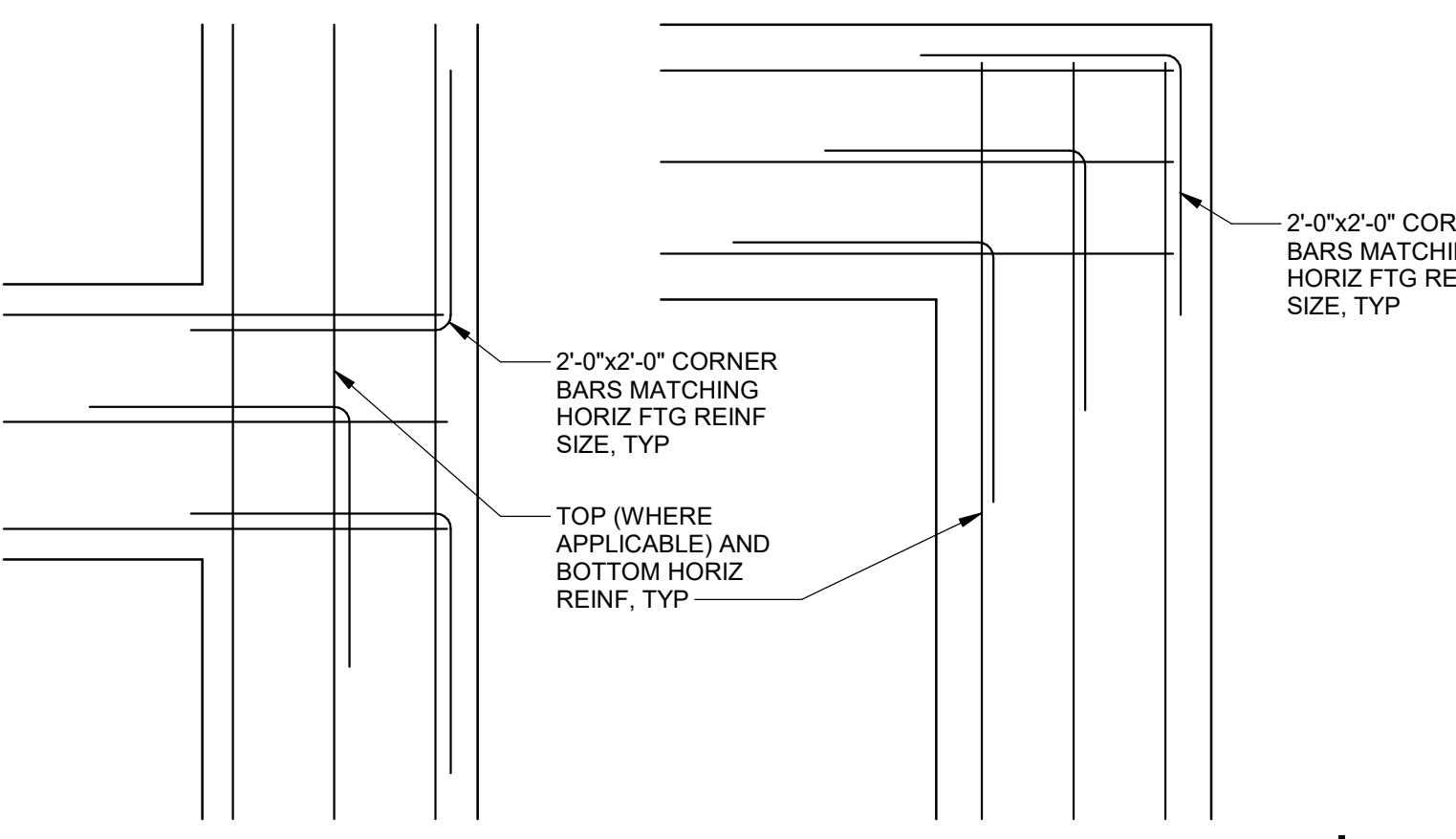
CONCRETE WALL CONSTRUCTION JOINT | 14
1" = 1'-0"



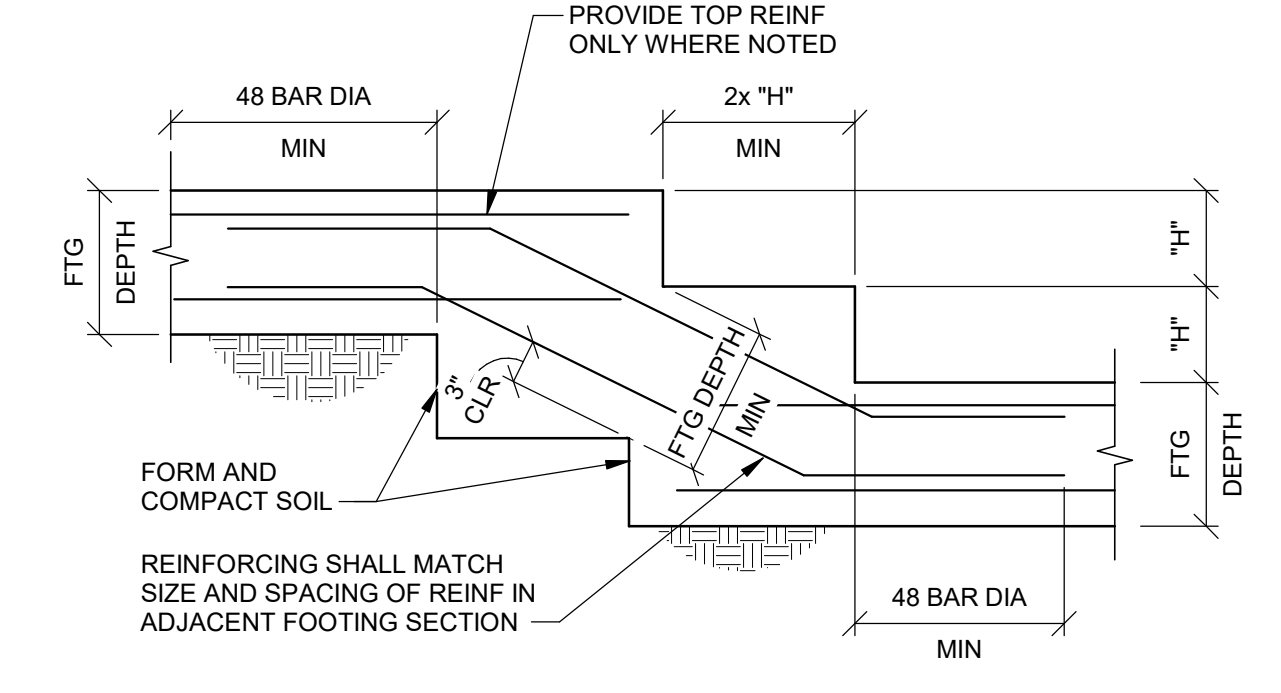
TYP LIGHT POLE FTG | 10
3/4" = 1'-0"



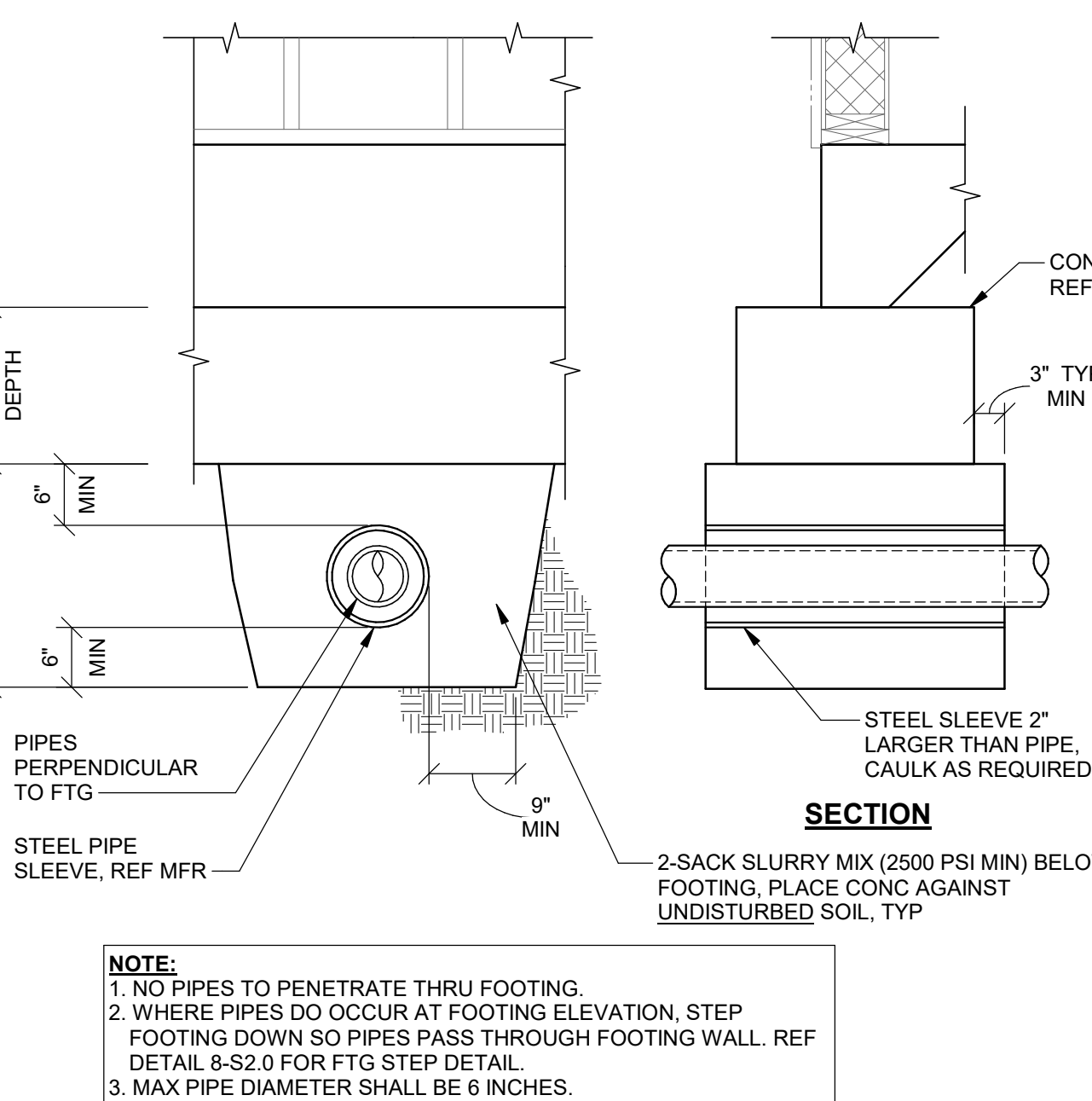
CONCRETE WALL CORNER DETAIL | 13
3/4" = 1'-0"



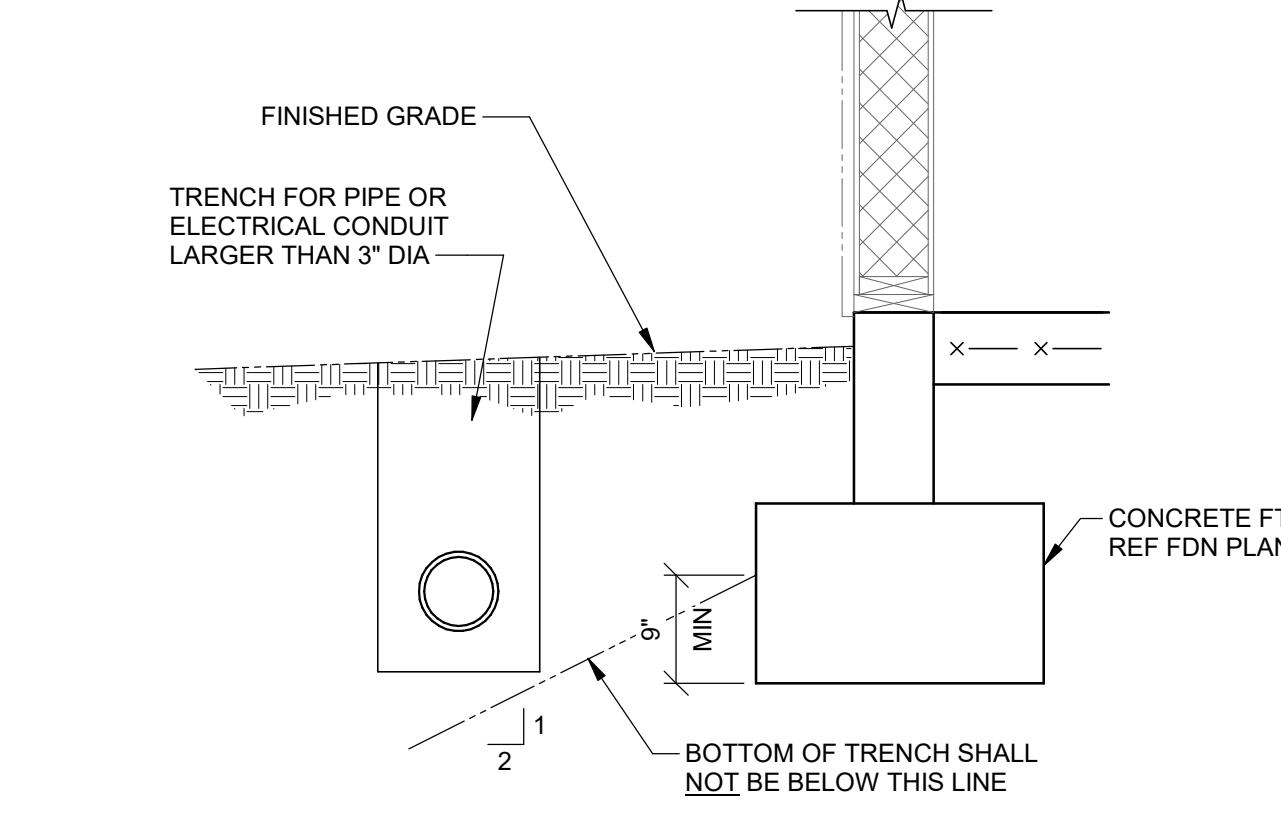
CONCRETE FOOTING CORNER BAR REINFORCEMENT | 9
1" = 1'-0"



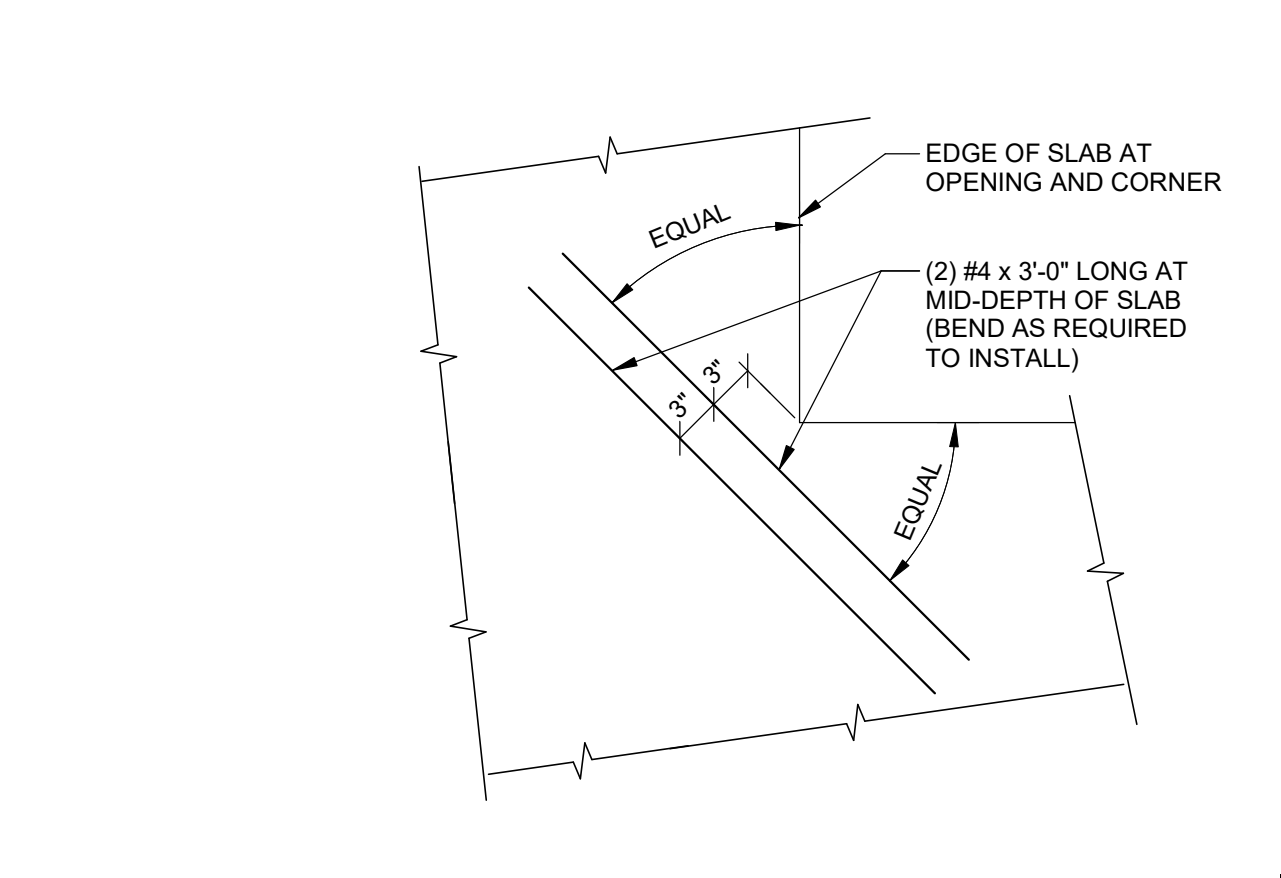
DETAIL AT STEPPED FOOTING | 8
3/4" = 1'-0"



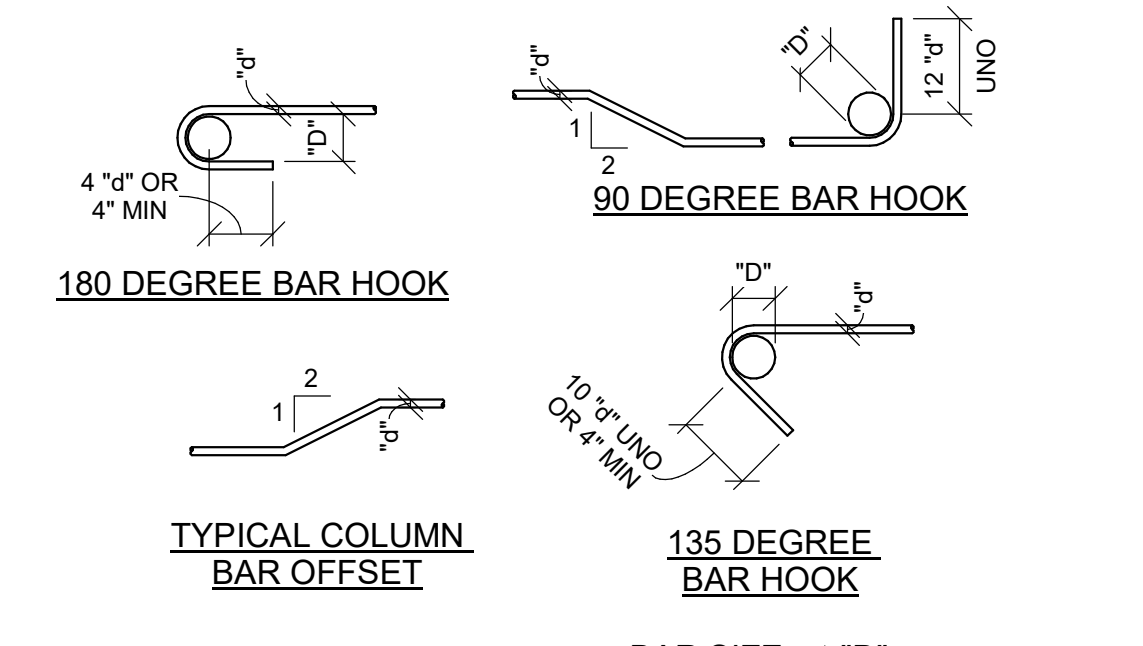
UTILITY LINE PERPENDICULAR TO FDN | 7
3/4" = 1'-0"



UTILITY LINE PARALLEL TO FDN | 6
3/4" = 1'-0"



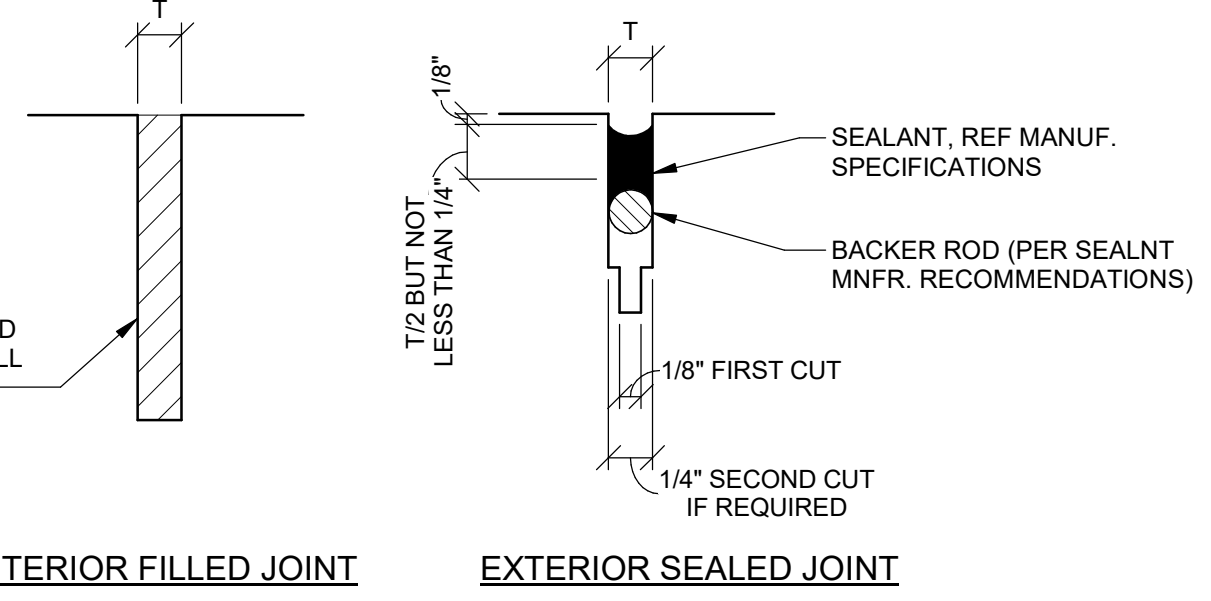
TYPICAL REINFORCEMENT AT SLAB OPENING | 5
3/4" = 1'-0"



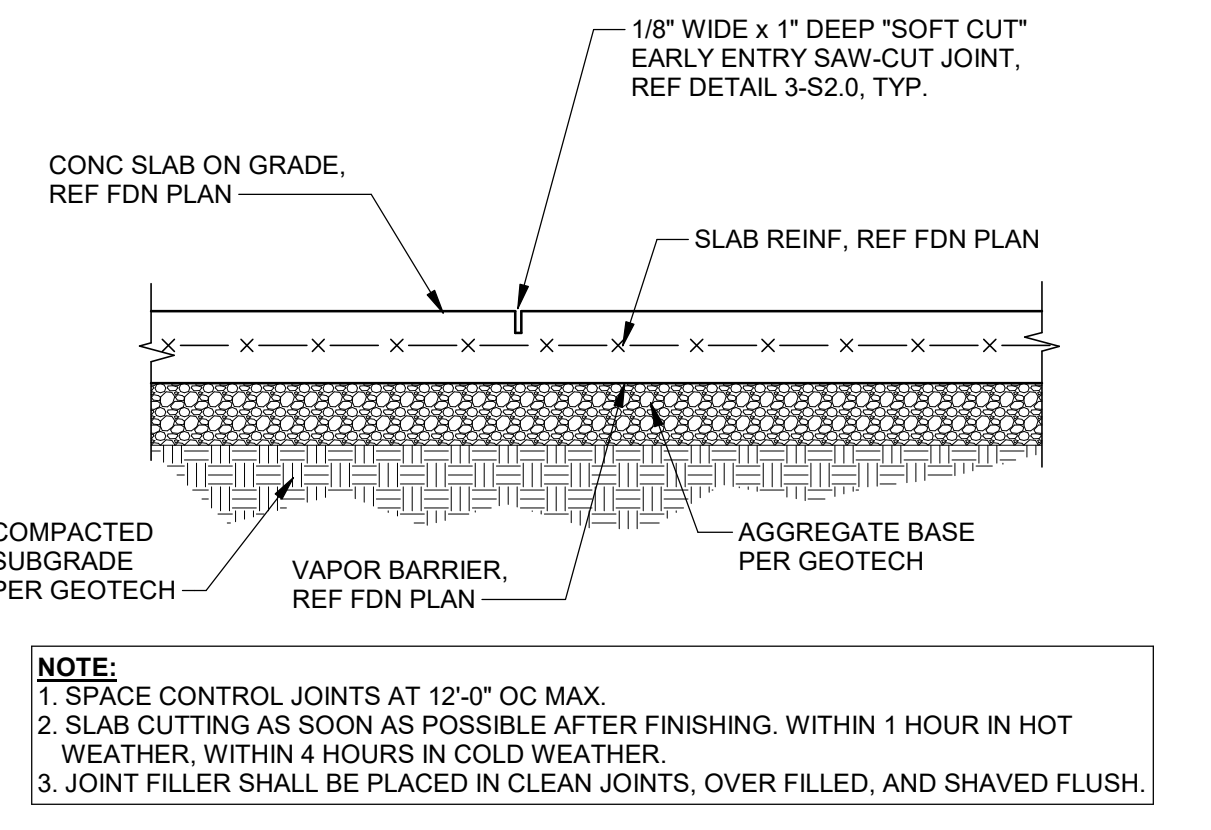
NOTE: 1. PROVIDE THESE HOOKS THROUGHOUT WORK AS INDICATED ON DRAWINGS.

BAR SIZE	"D"
#3 THRU #8	6 "d"
#9 THRU #11	8 "d"
#14 THRU #18	10 "d"

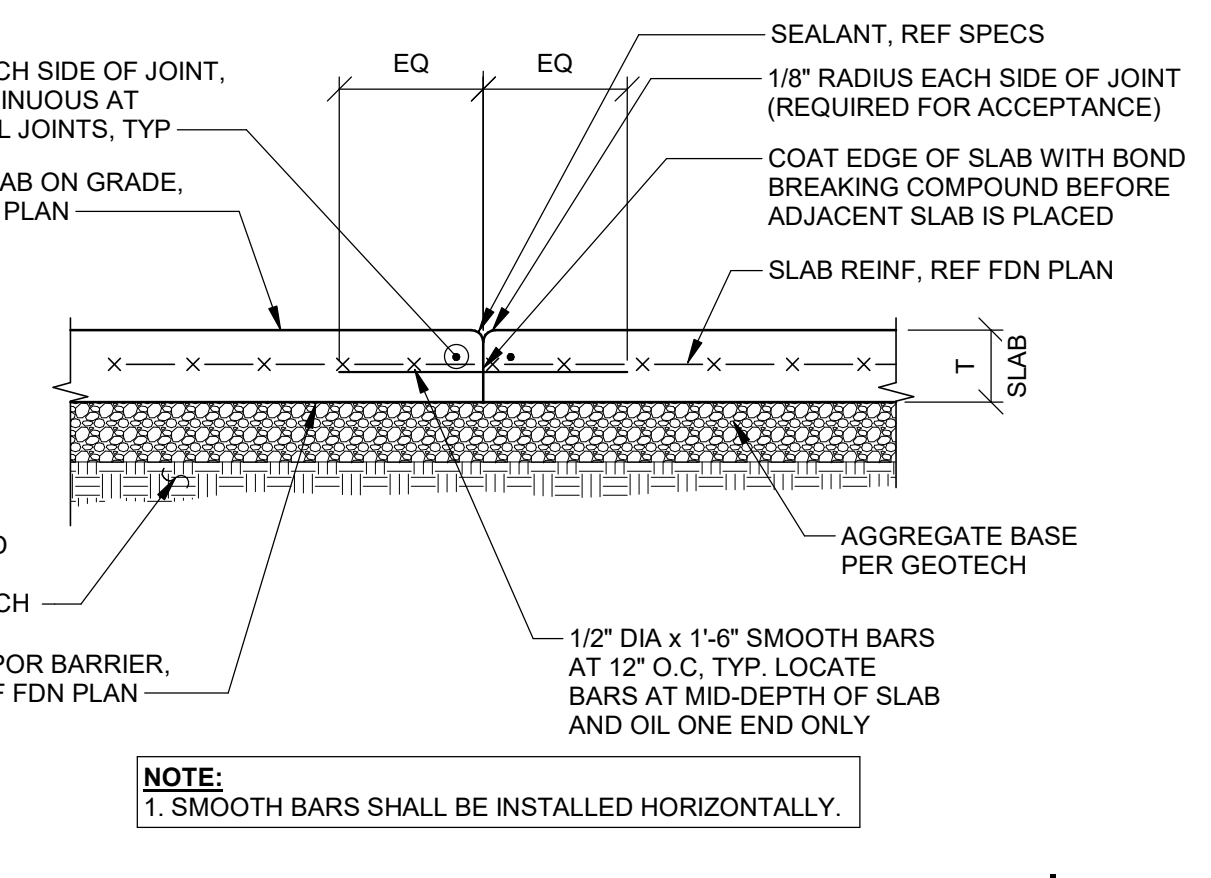
TYPICAL REINFORCING BARS BENDS DETAIL | 4
3/4" = 1'-0"



TYPICAL CONSTRUCTION JOINT | 1
3/4" = 1'-0"



TYPICAL CONTRACTION JOINT | 2
3/4" = 1'-0"



TYPICAL CONSTRUCTION JOINT | 1
3/4" = 1'-0"

rdc.

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ENGINEER
BRIAN J. HALVERSON
034711

REVISIONS

ISSUE	DATE
OTF	02/15/23
OTB SET	01/05/24

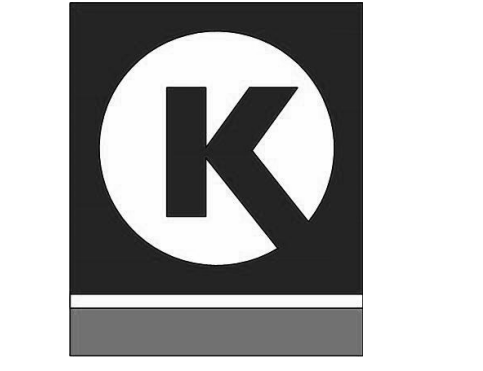
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NORTH CAROLINA COA #0-9612 EXPRES 06/30/23
JBA PROJECT #: 23023

PROFESSIONAL IN CHARGE
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RB
QUALITY CONTROL
SMK
DRAWN BY
JBA
PROJECT NAME

CIRCLE K STORES, INC.

ANGIER, NC
9706 KENNEBEC CHURCH ROAD,
ANGIER, NC 27501

PROTOCOL # R1.1 11/21/22



CIRCLE K STORE INC.
PROJECT NUMBER: 22130

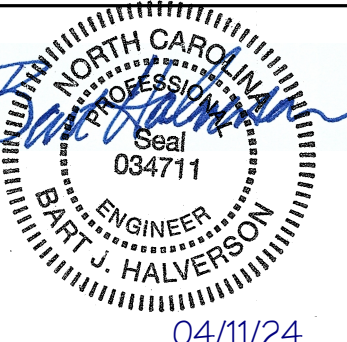
FOUNDATION DETAILS

S2.0

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NORTH CAROLINA CO. #0612 - EXPIRES 06/30/23
JBA PROJECT # - 23023

PROFESSIONAL IN CHARGE

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PROJECT MANAGER

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QUALITY CONTROL

SMK

DRAWN BY

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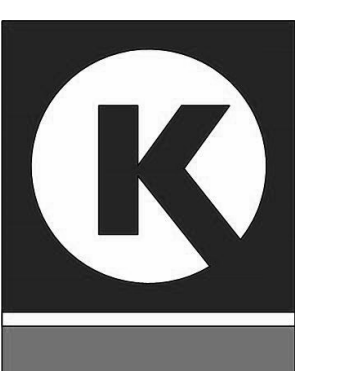
PROJECT NAME

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ANGIER, NC

9706 KENNEBEC CHURCH ROAD,
ANGIER, NC 27501

PROTOCOL # R1.1 11/21/22

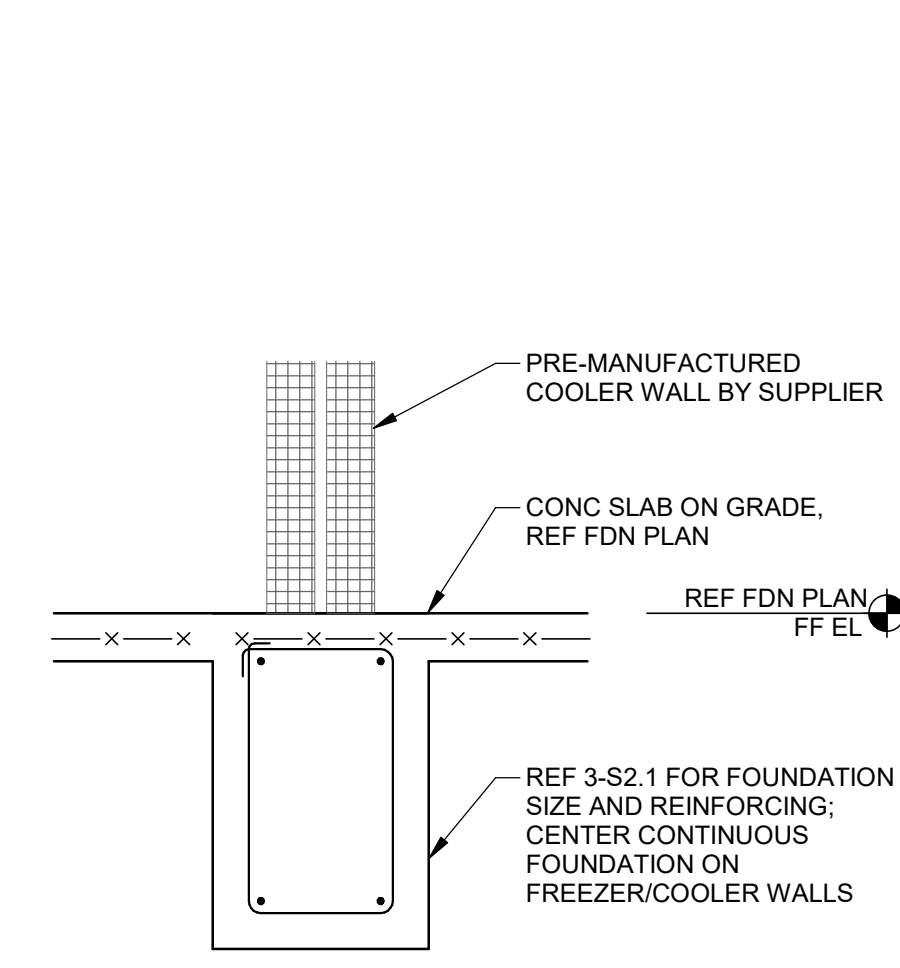


CIRCLE K STORE INC.

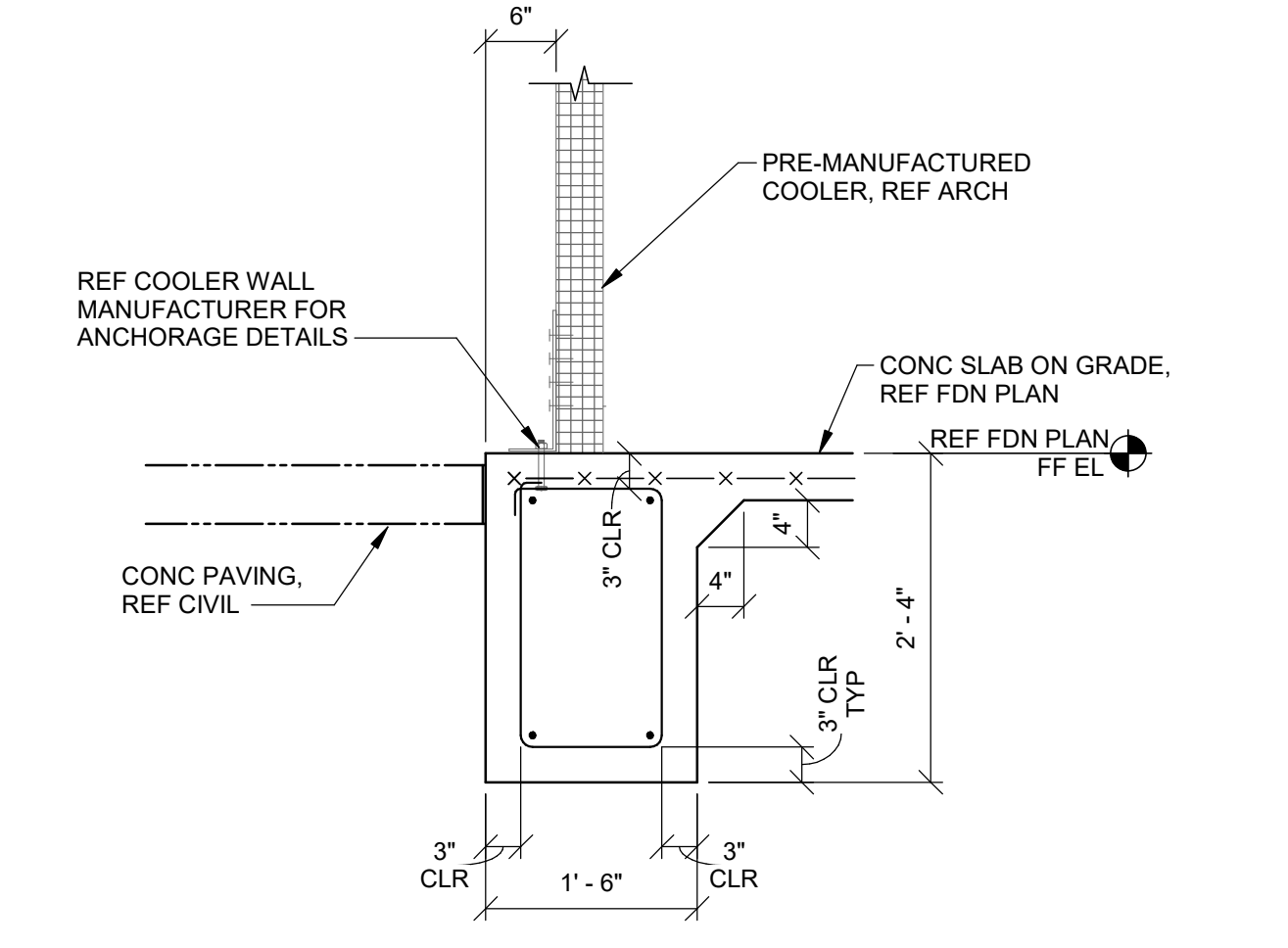
PROJECT NUMBER: 22130

FOUNDATION DETAILS

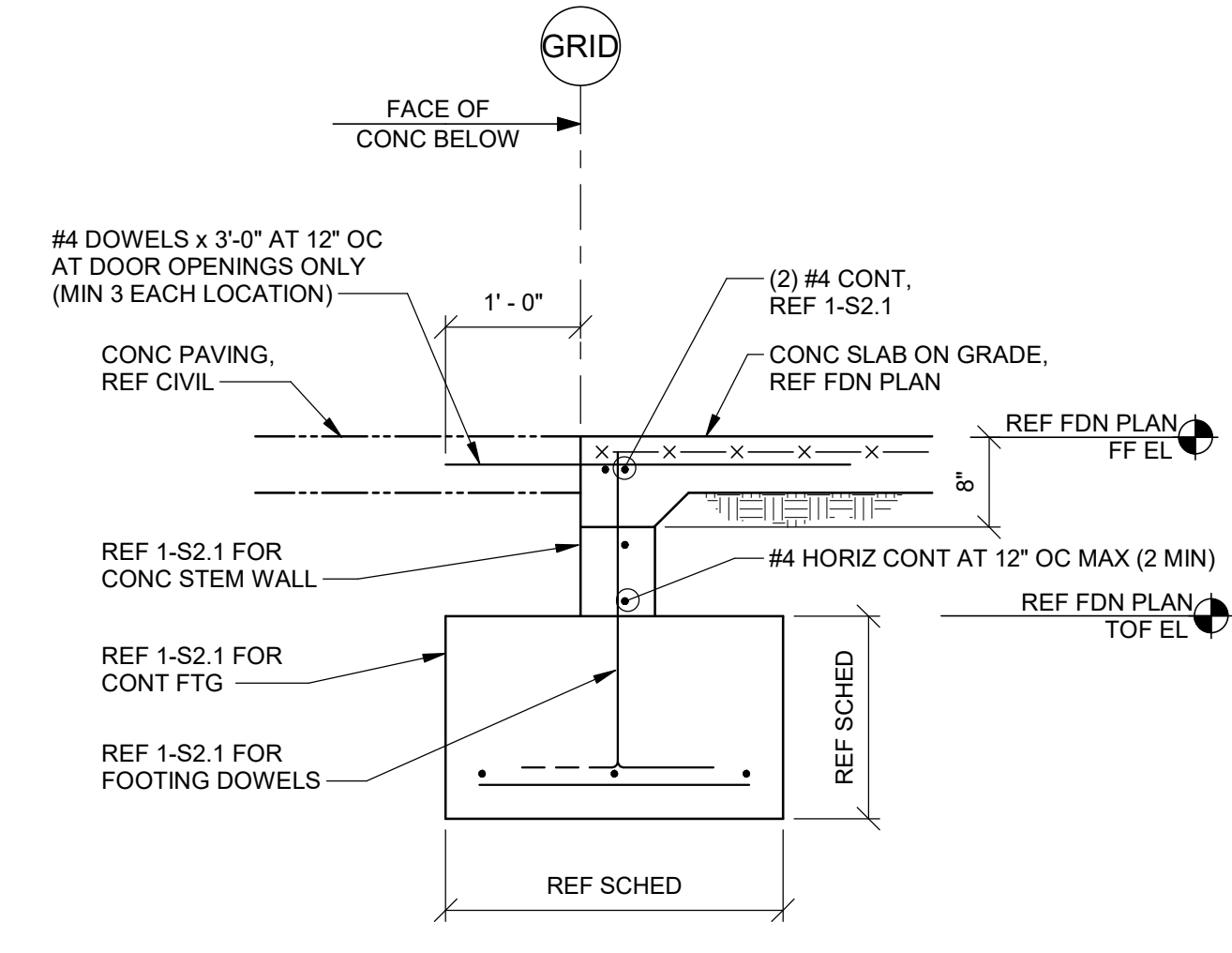
S2.1



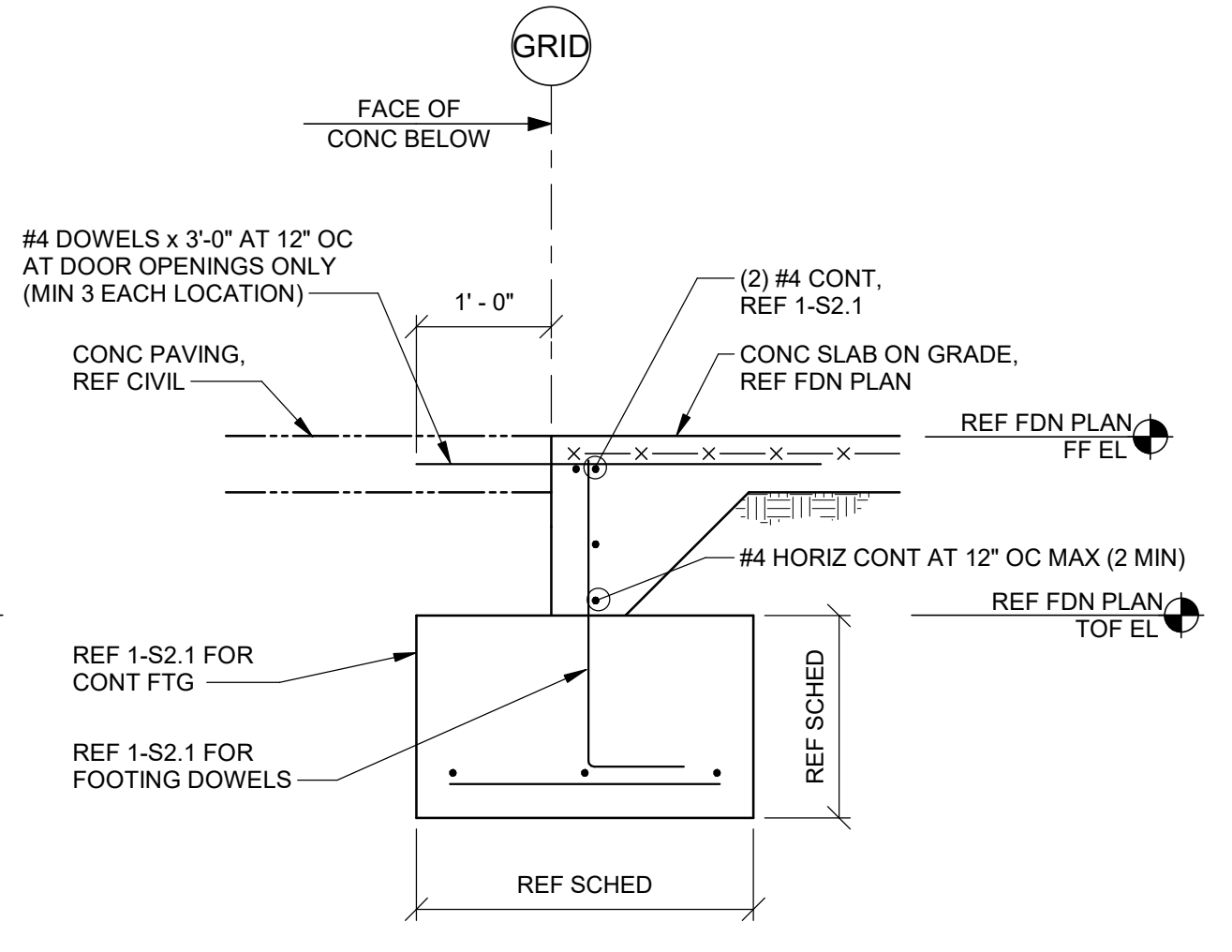
INTERIOR COOLER WALL SECTION | 4
3/4" = 1'-0"



EXTERNAL COOLER WALL SECTION | 3
3/4" = 1'-0"

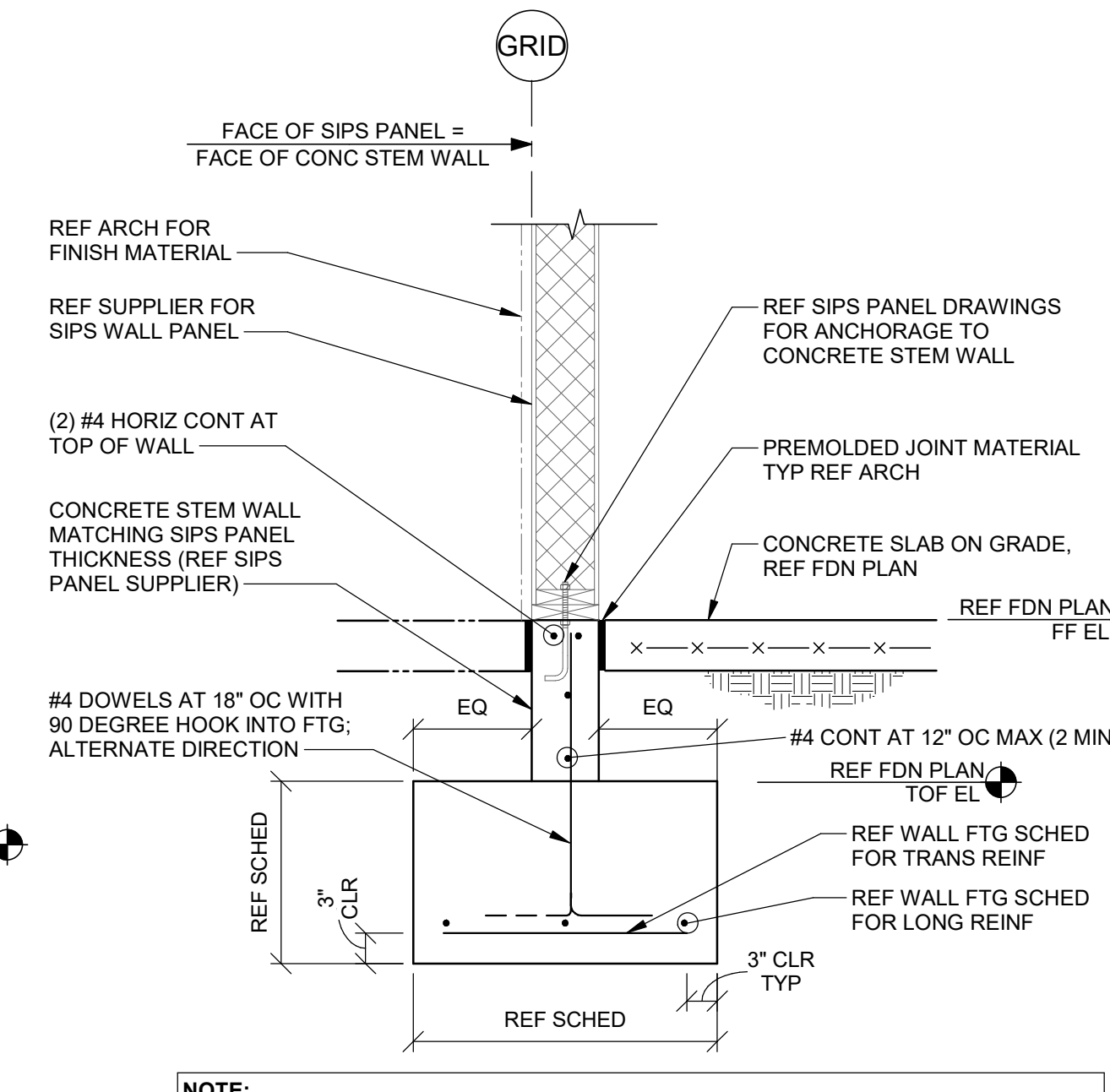


NOTE:
1. FOR ADDITIONAL INFORMATION NOT SHOWN, REFER TO DETAIL 1-S2.1.



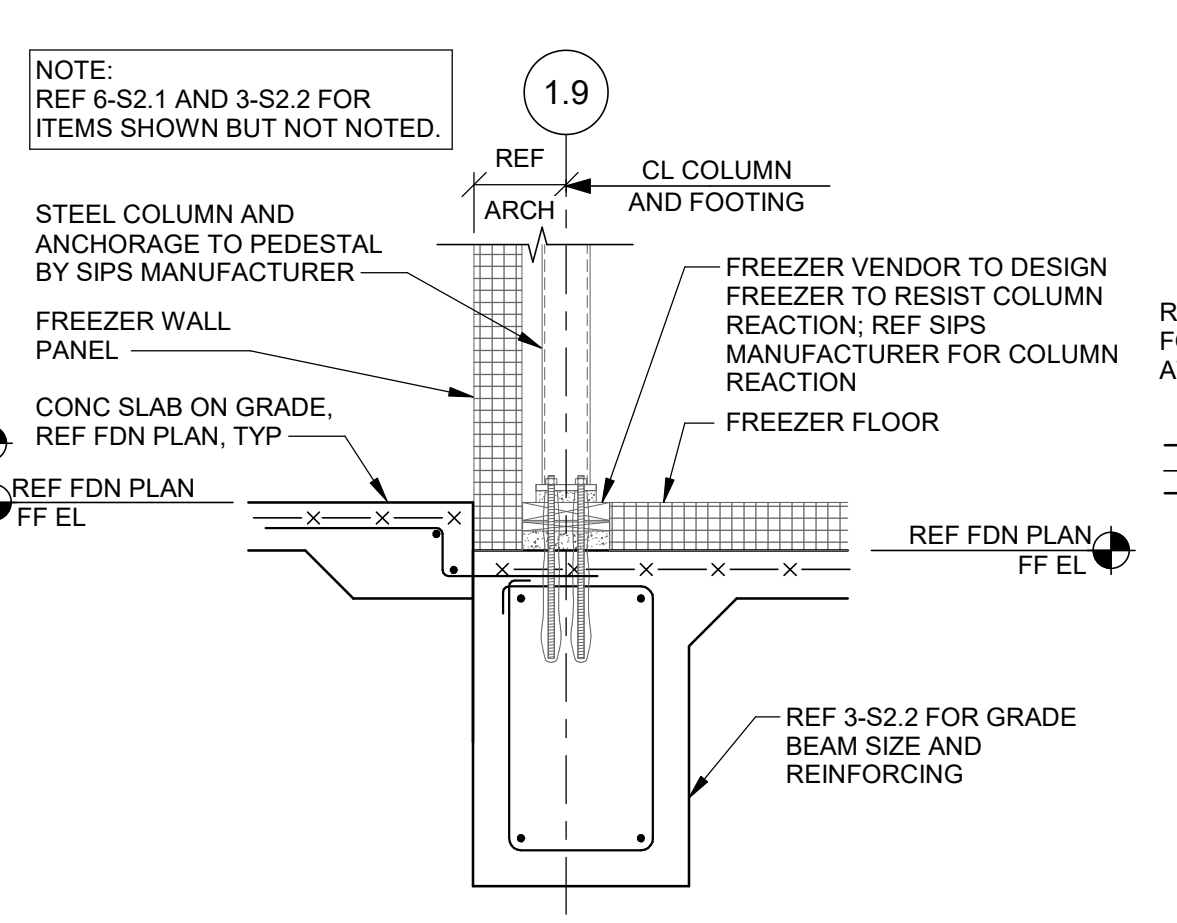
NOTE:
1. FOR ADDITIONAL INFORMATION NOT SHOWN, REFER TO DETAIL 1-S2.1.

CONTRACTOR'S OPTION
CONCRETE SLAB AT OPENING | 2
3/4" = 1'-0"

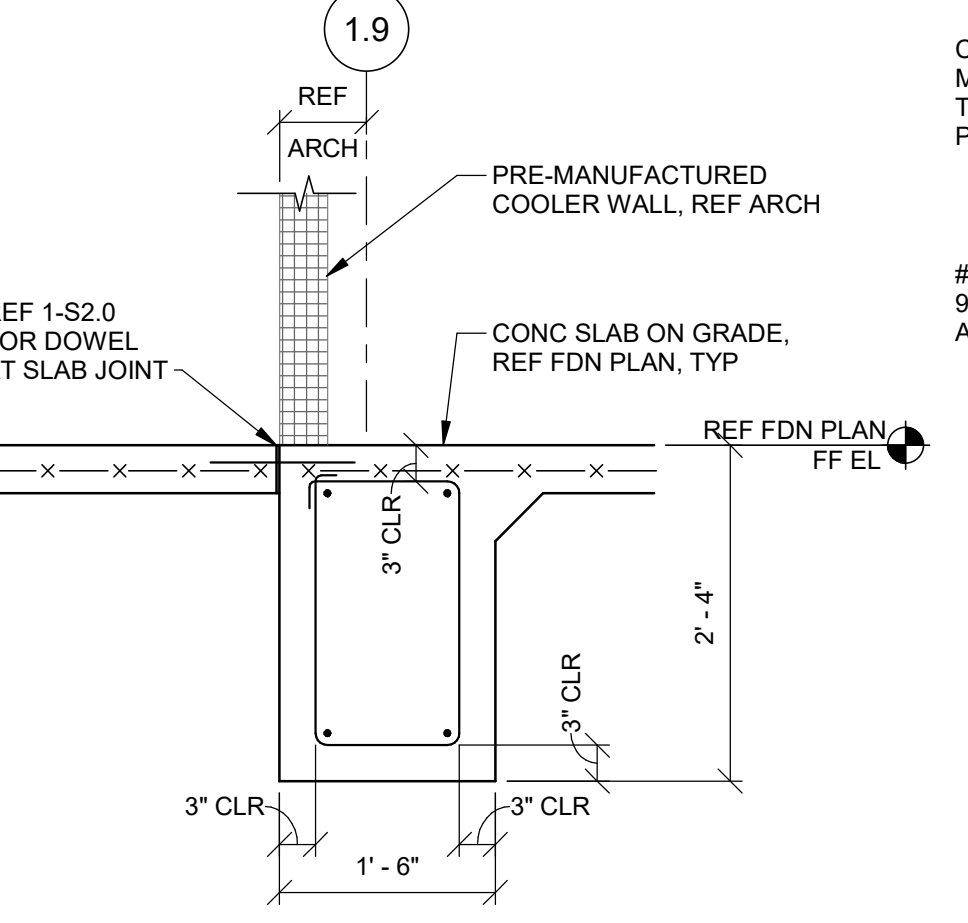


NOT USED | 8
3/4" = 1'-0"

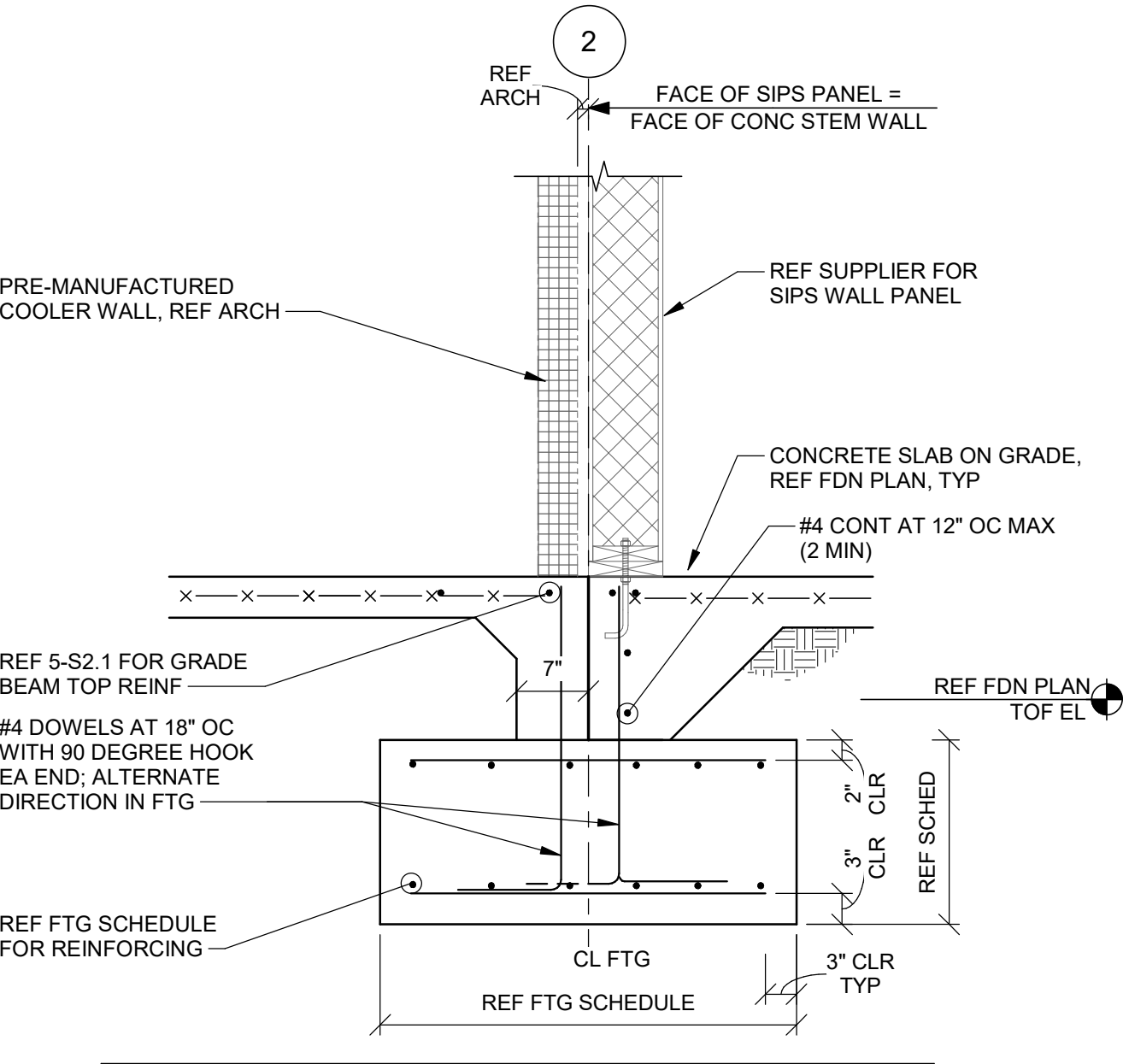
COLUMN AT COOLER | 6
3/4" = 1'-0"



COLUMN AT FREEZER | 7
3/4" = 1'-0"

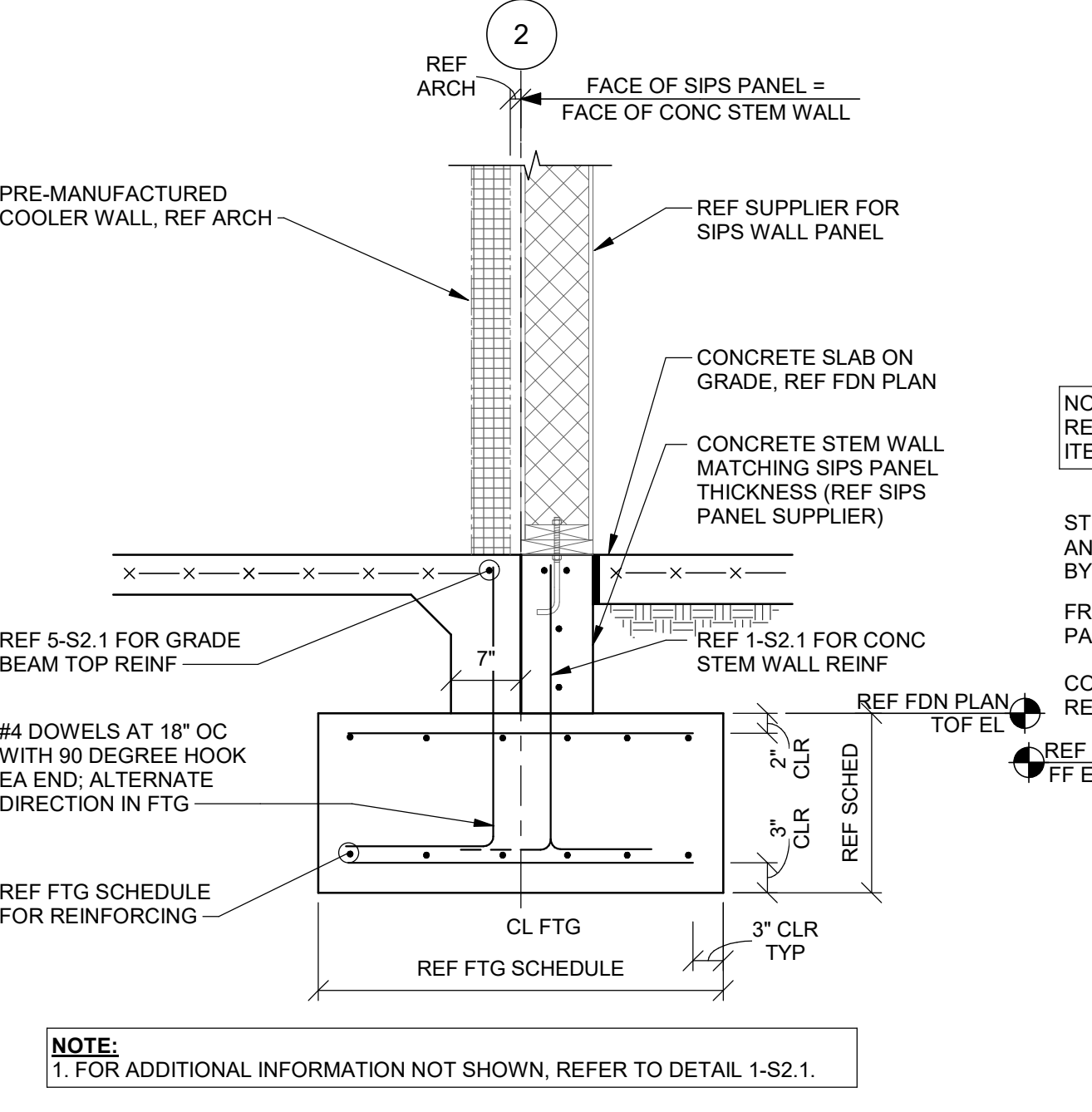


INTERIOR COOLER WALL SECTION | 5
3/4" = 1'-0"



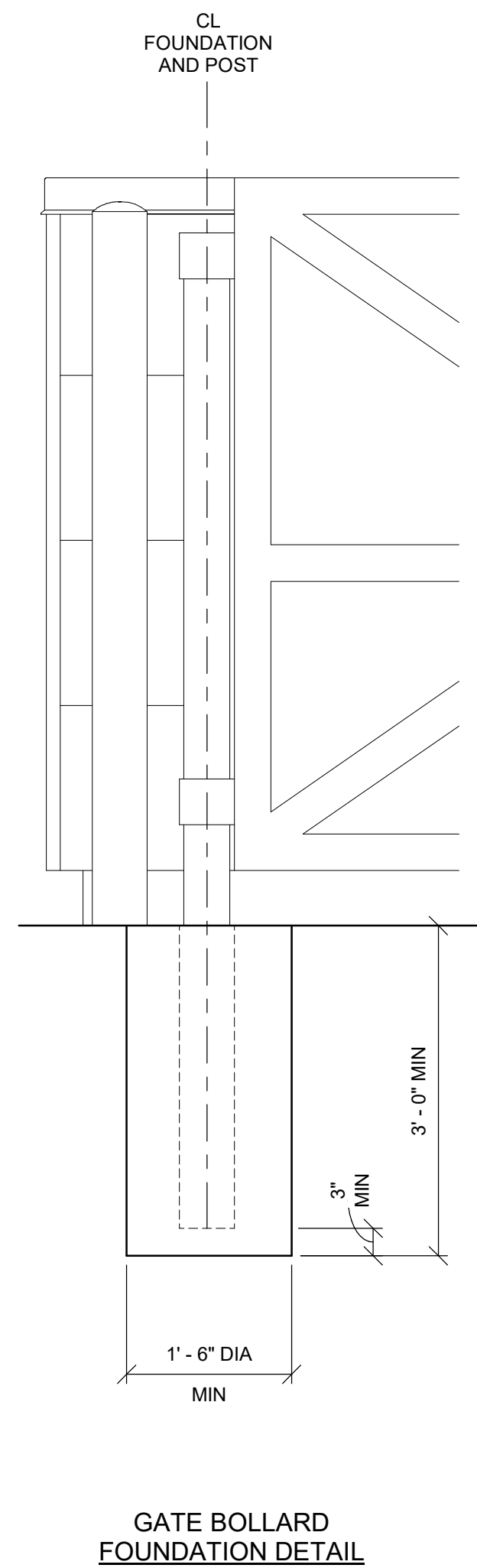
NOTE:
1. FOR ADDITIONAL INFORMATION NOT SHOWN, REFER TO DETAIL 1-S2.1.

CONTRACTOR'S OPTION



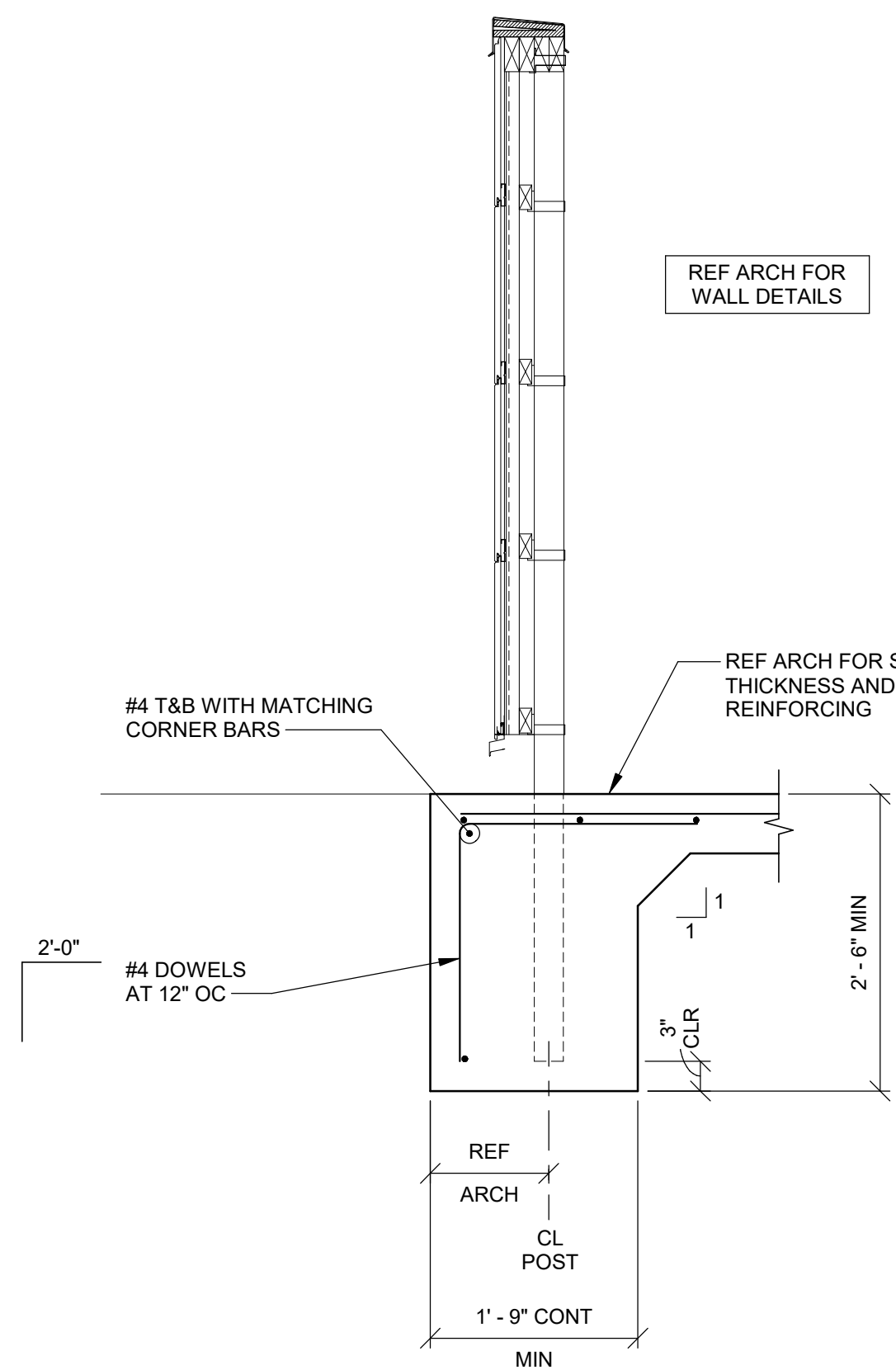
NOTE:
1. FOR ADDITIONAL INFORMATION NOT SHOWN, REFER TO DETAIL 1-S2.1.

INTERIOR COOLER WALL SECTION | 9
3/4" = 1'-0"



GATE BOLLARD FOUNDATION DETAIL

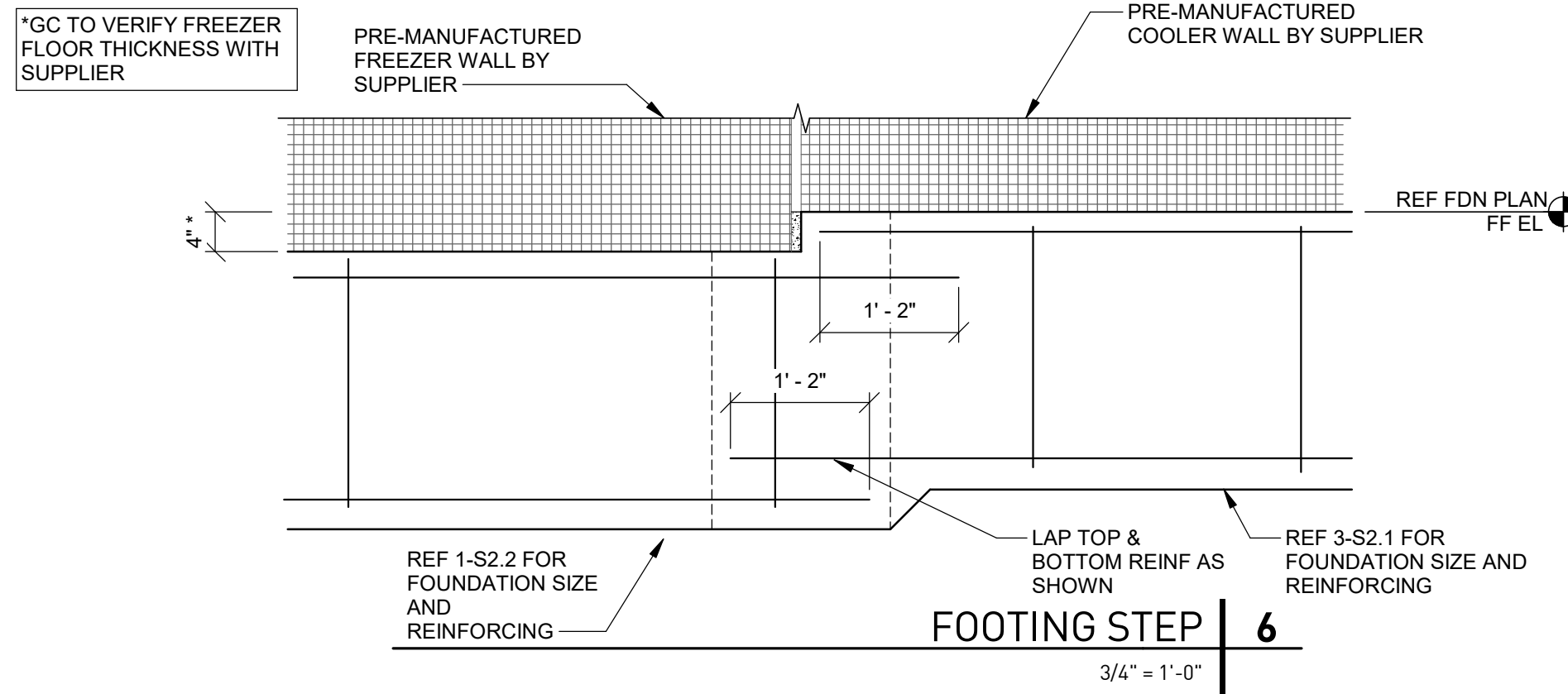
REF ARCH FOR GATE DETAILS



TRASH ENCLOSURE FOUNDATION WALL SECTION

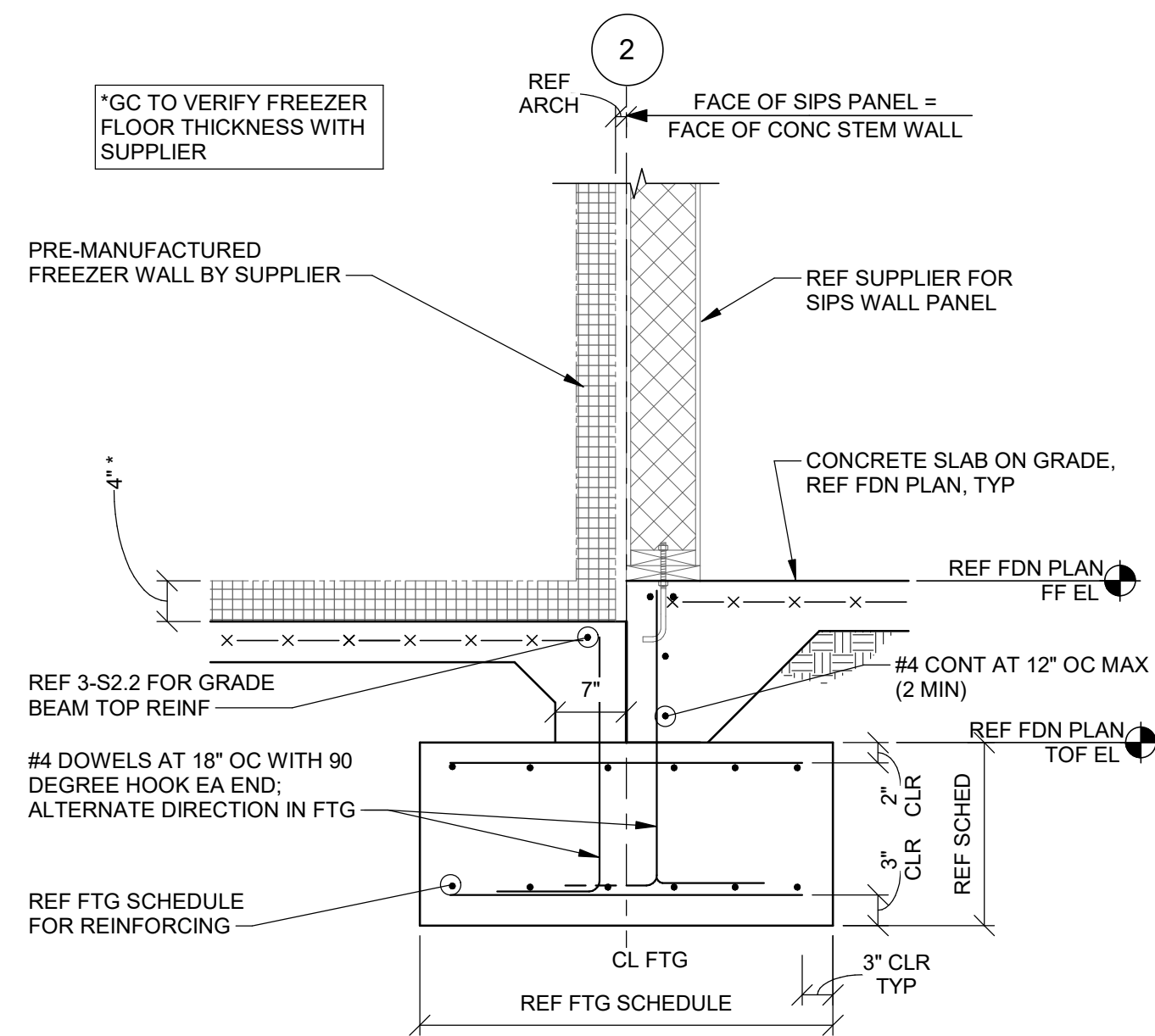
TRASH ENCLOSURE FOUNDATION DETAILS | 7

3/4" = 1'-0"



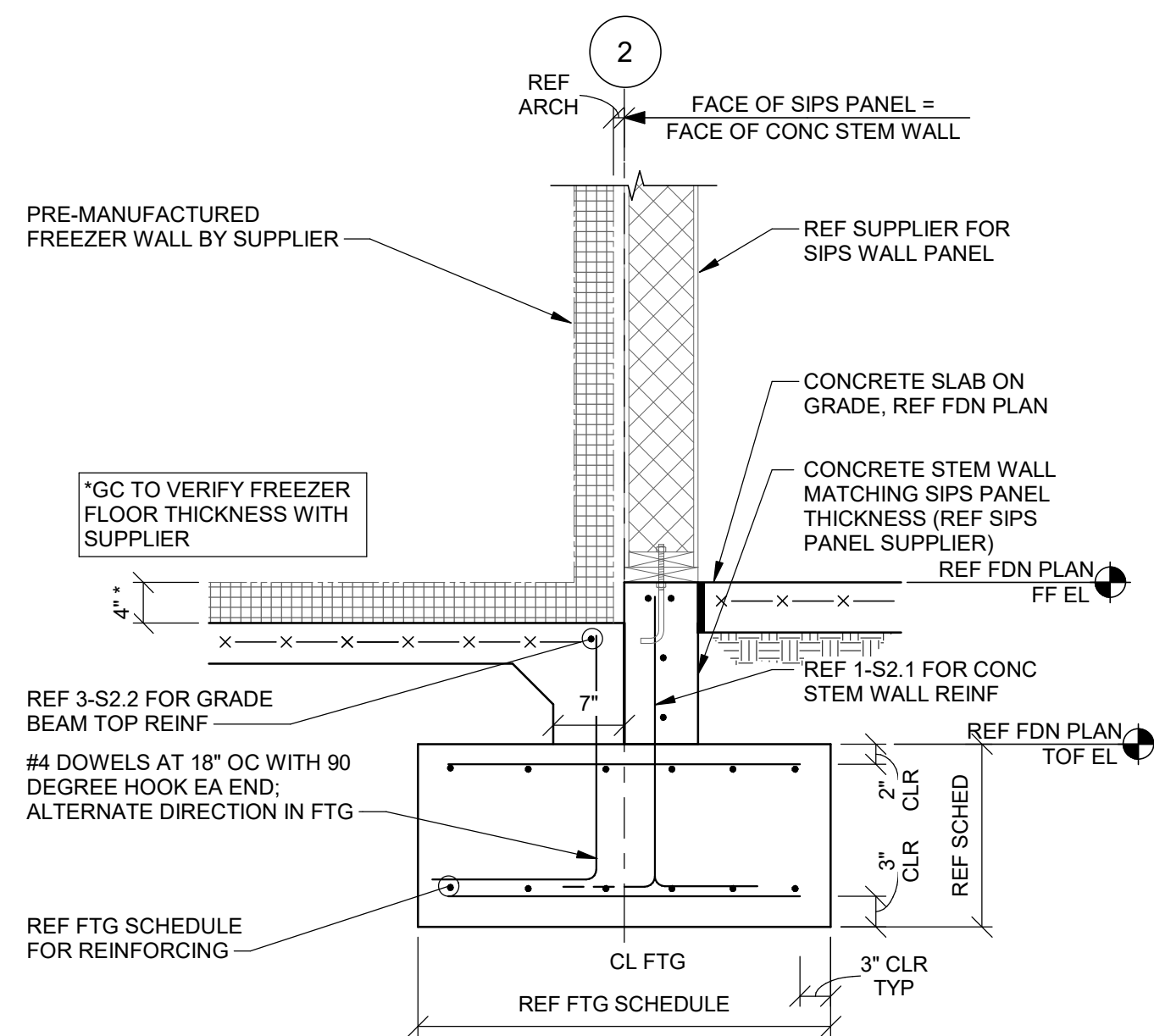
FOOTING STEP | 6

3/4" = 1'-0"



CONTRACTOR'S OPTION

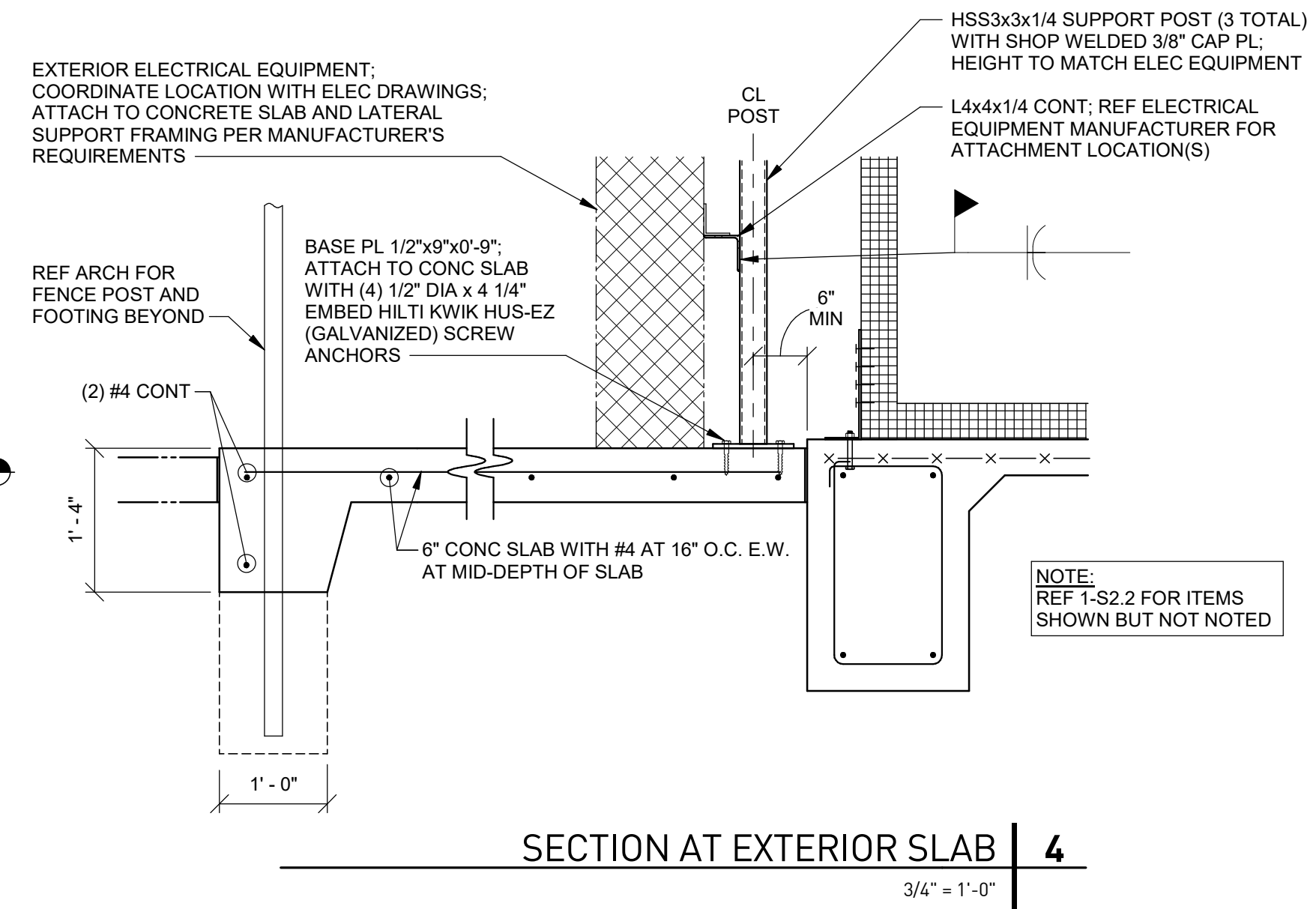
NOTE: 1. FOR ADDITIONAL INFORMATION NOT SHOWN, REFER TO DETAIL 1-S2.1.



INTERIOR FREEZER WALL SECTION | 5

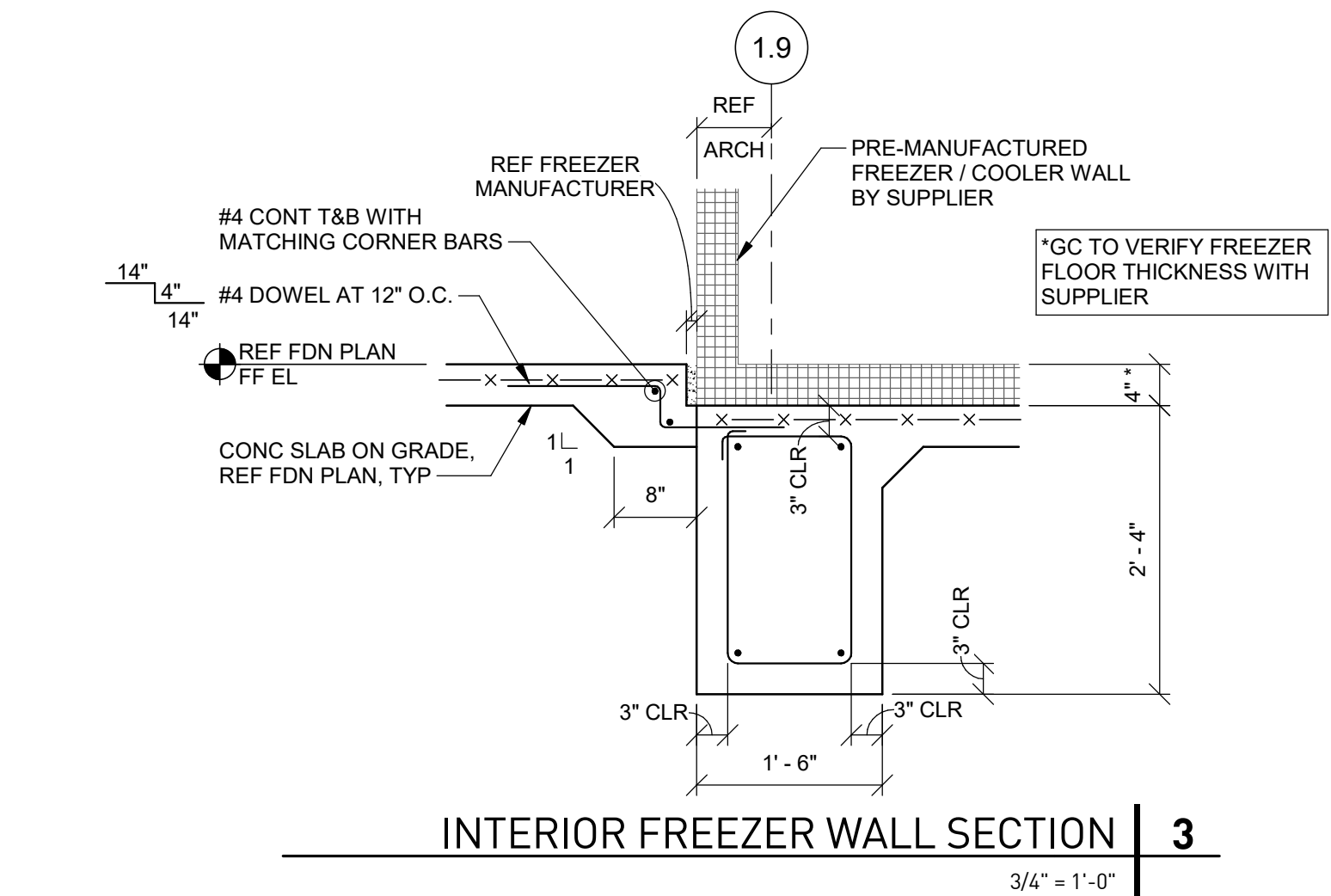
3/4" = 1'-0"

NOTE: 1. FOR ADDITIONAL INFORMATION NOT SHOWN, REFER TO DETAIL 1-S2.1.



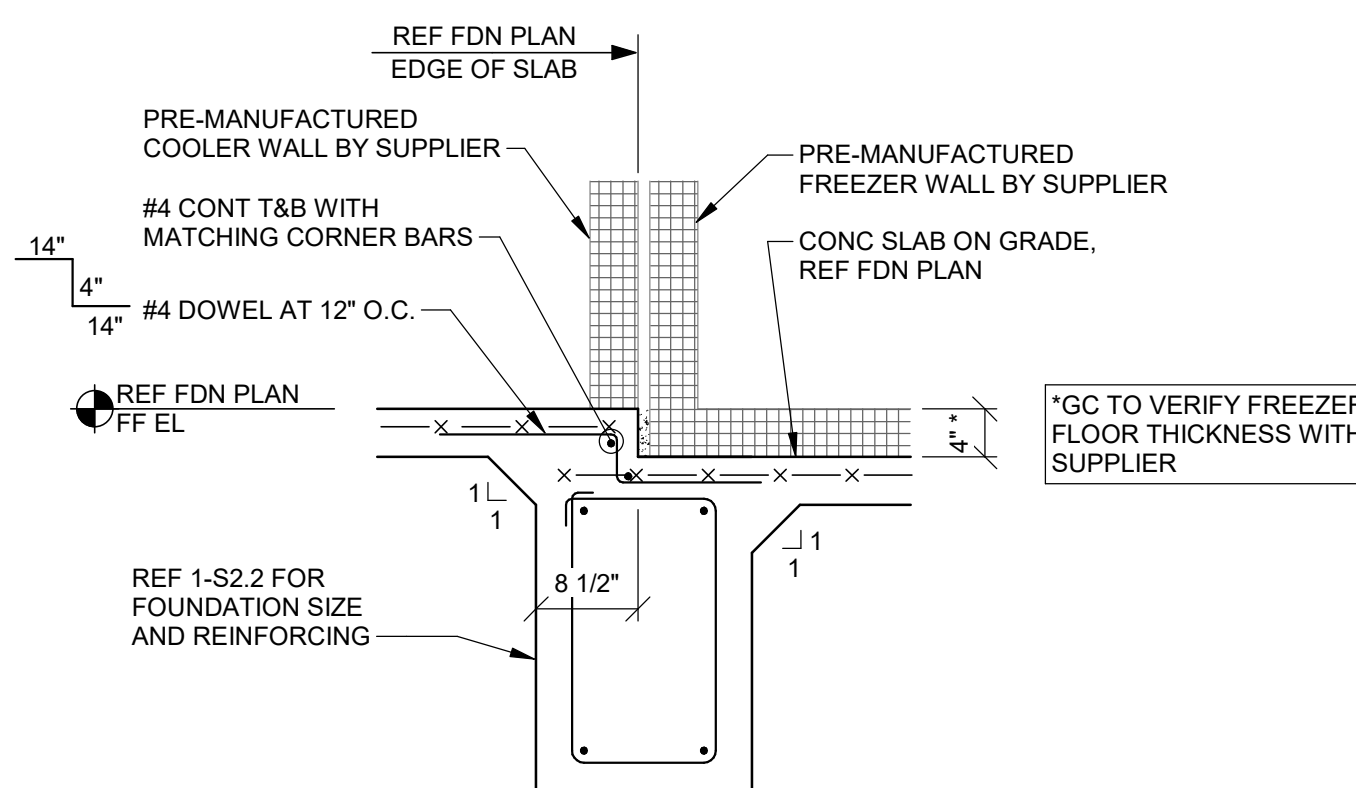
SECTION AT EXTERIOR SLAB | 4

3/4" = 1'-0"



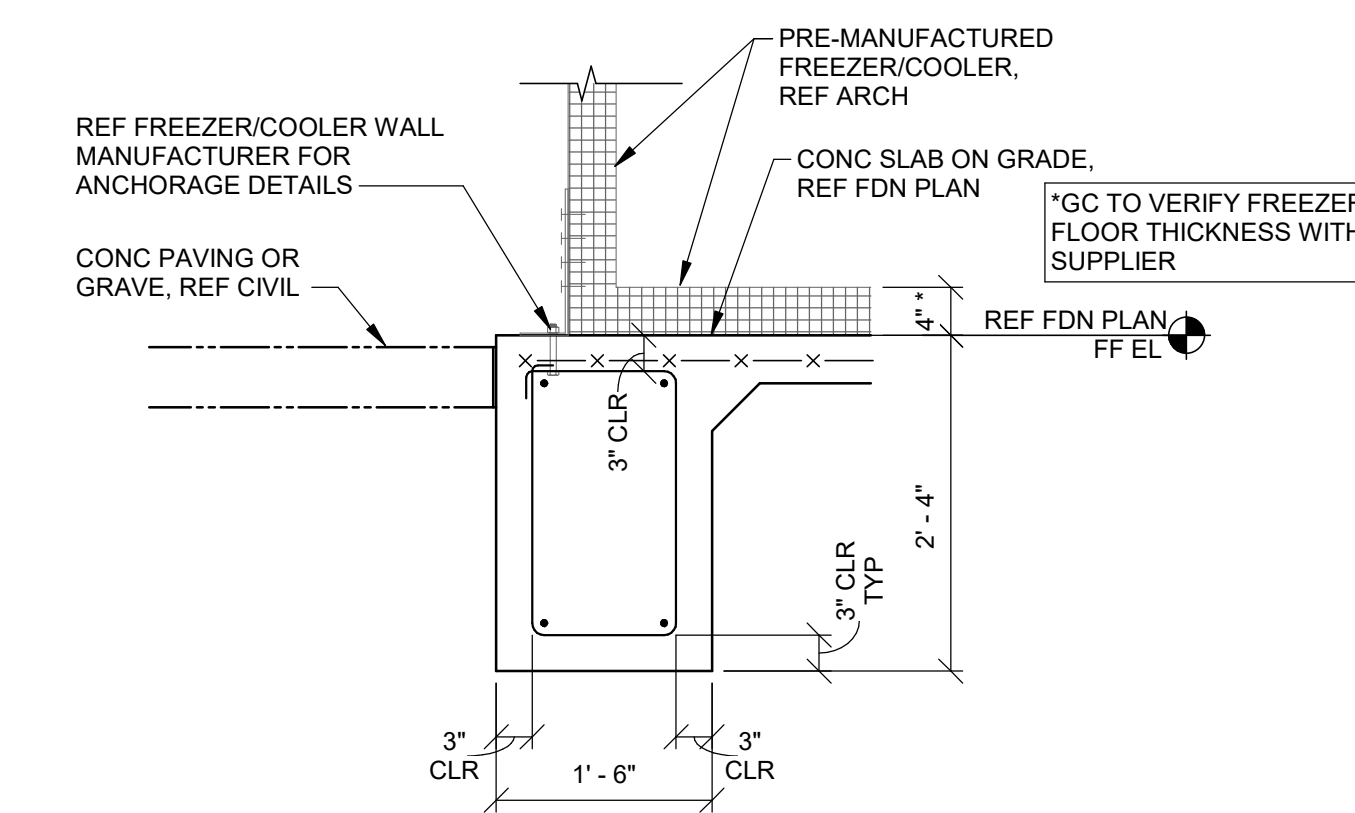
INTERIOR FREEZER WALL SECTION | 3

3/4" = 1'-0"



FREEZER / COOLER WALL SECTION | 2

3/4" = 1'-0"



EXTERNAL FREEZER WALL SECTION | 1

3/4" = 1'-0"

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OTB SET	01/05/24

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JBA PROJECT #: 23023

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QUALITY CONTROL

SMK

DRAWN BY

JBA

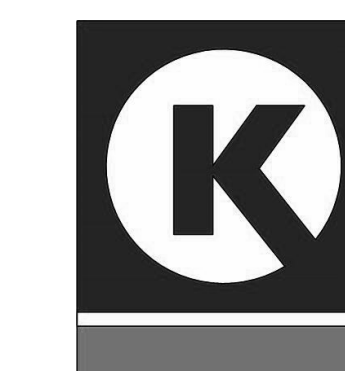
PROJECT NAME

CIRCLE K STORES, INC.

ANGIER, NC

9706 KENNEBEC CHURCH ROAD,
ANGIER, NC 27501

PROTOCOL # R1.1 11/21/22

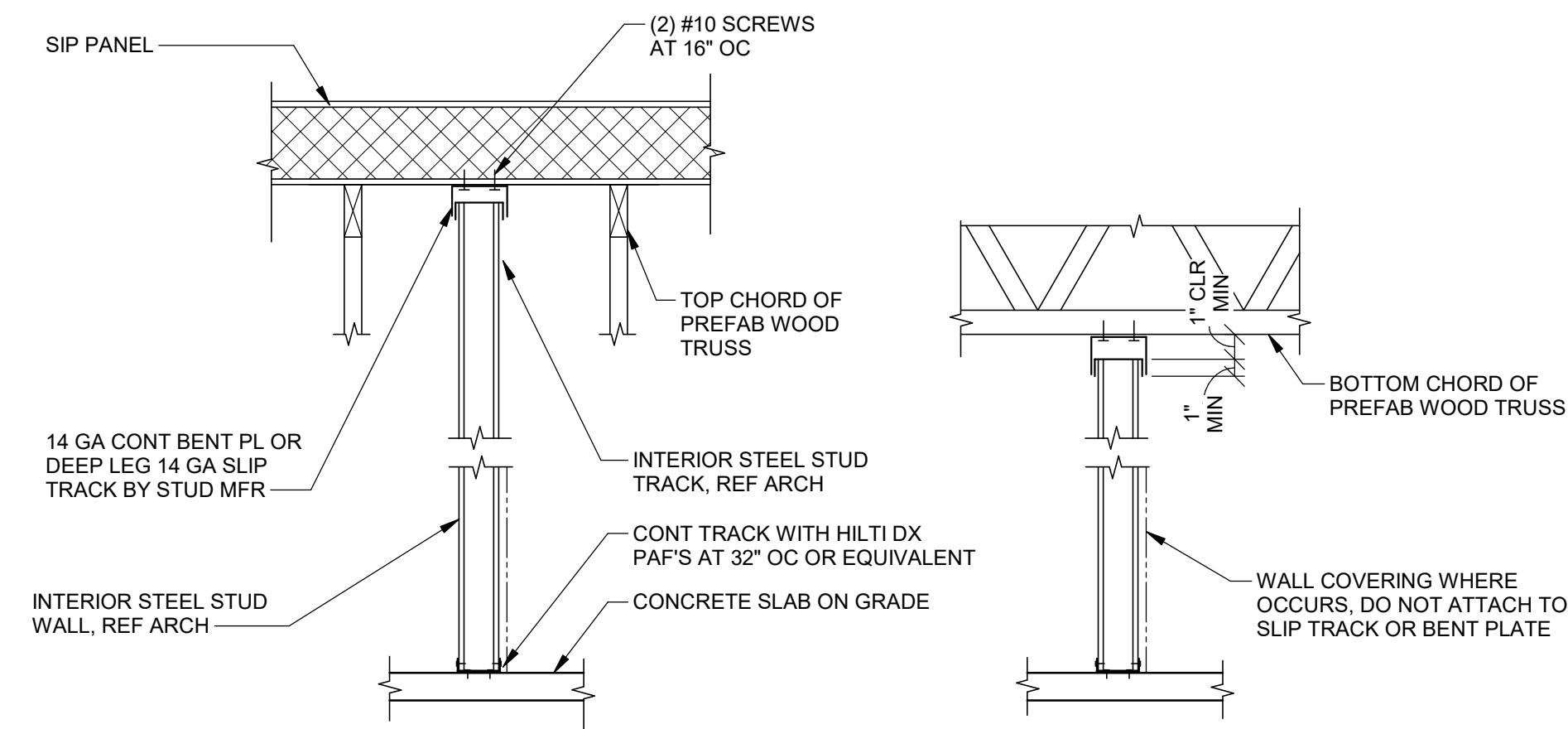


CIRCLE K STORE INC.

PROJECT NUMBER: 22130

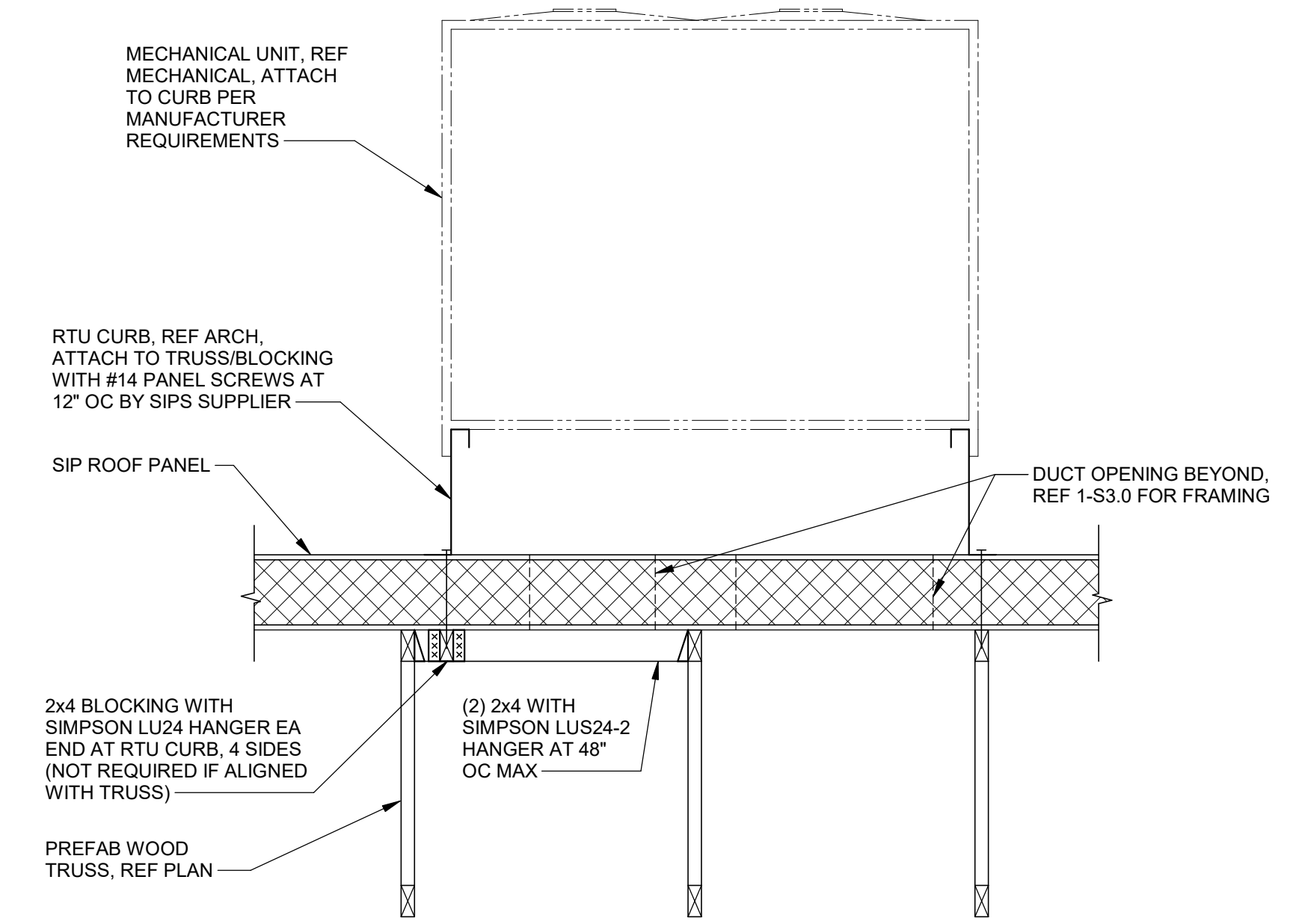
FOUNDATION DETAILS

S2.2



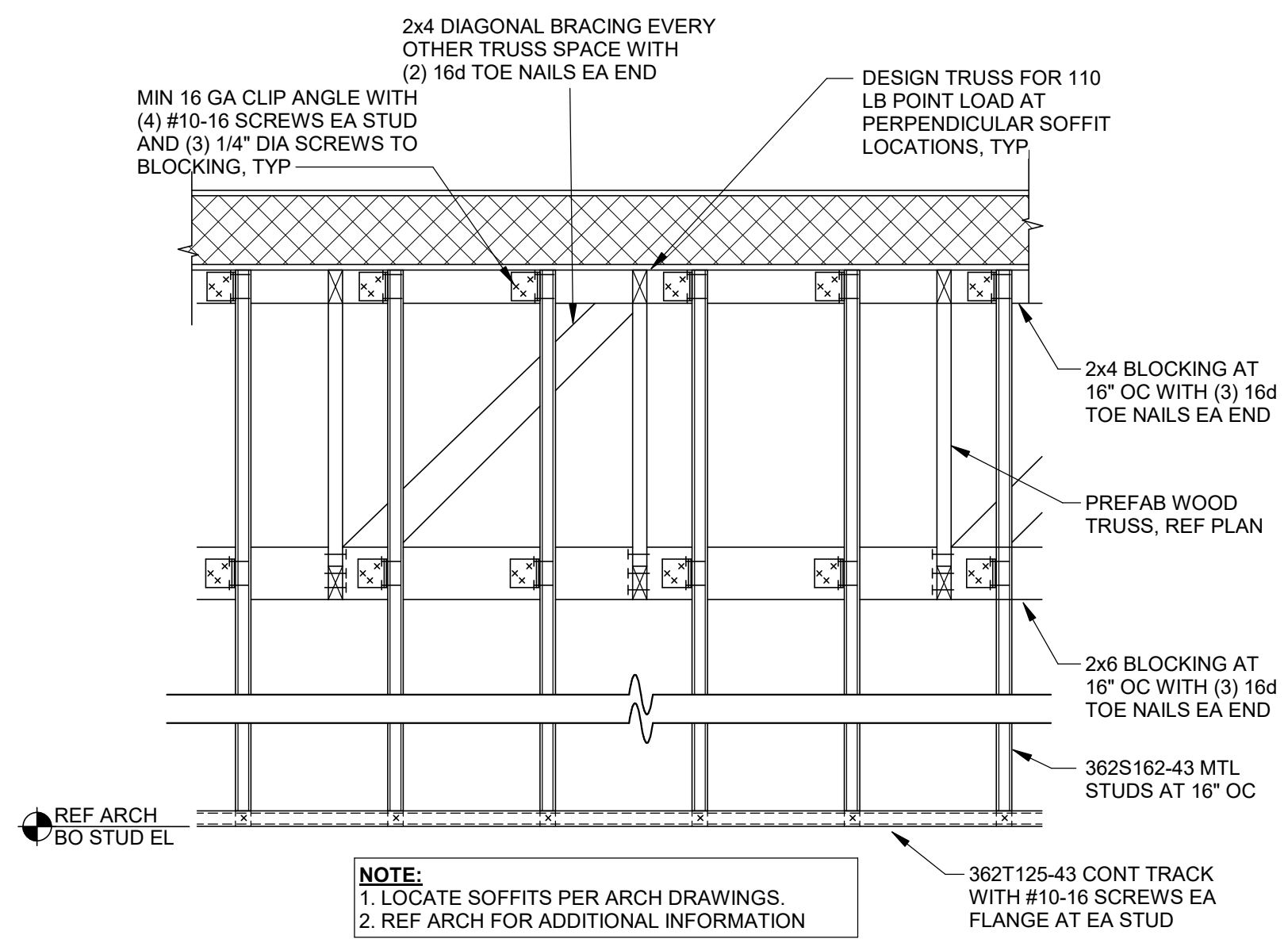
TYP NON-BEARING STEEL STUD WALL TO PREFAB WOOD TRUSS | 6

3/4" = 1'-0"



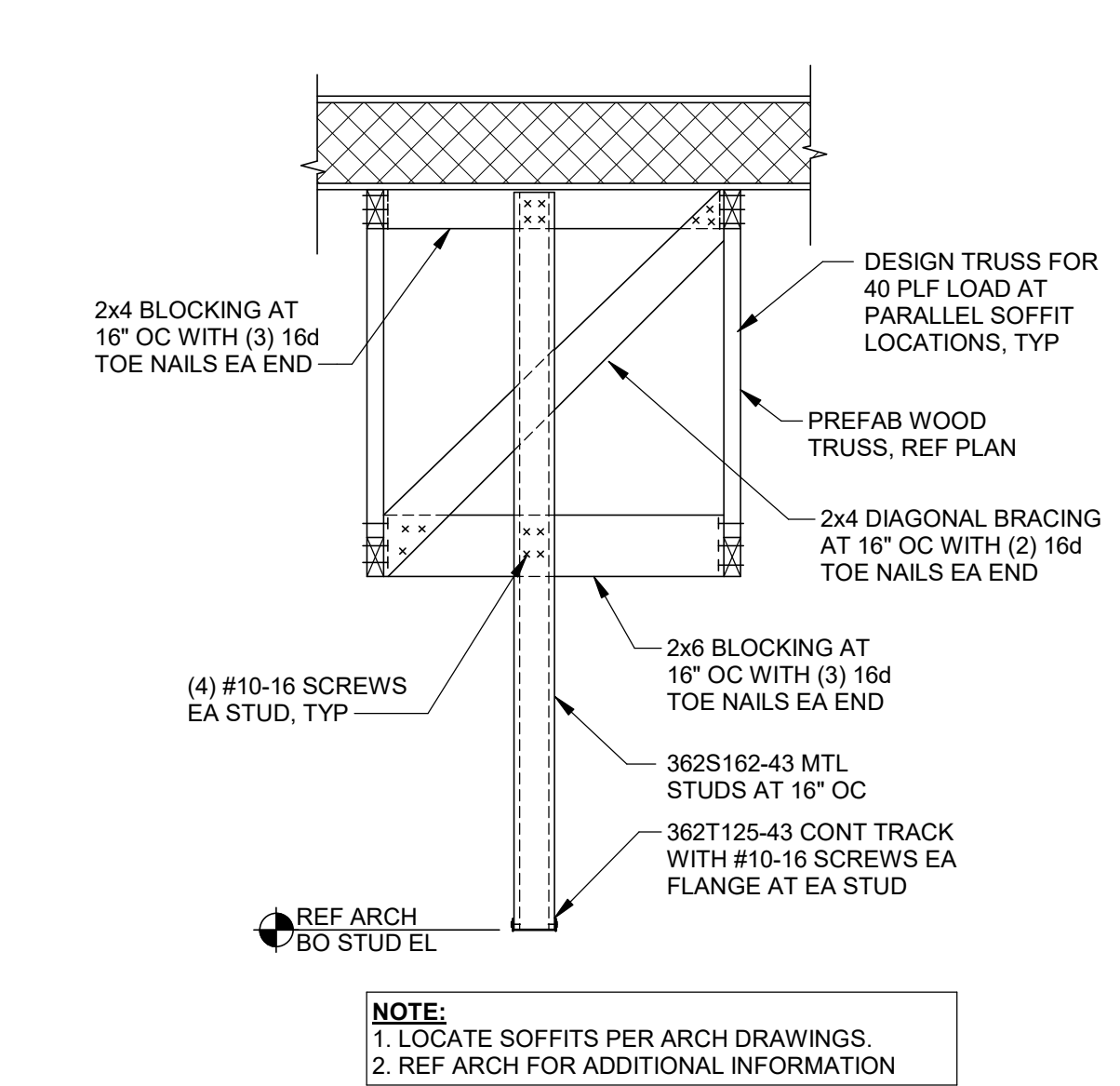
RTU SUPPORT | 3

3/4" = 1'-0"



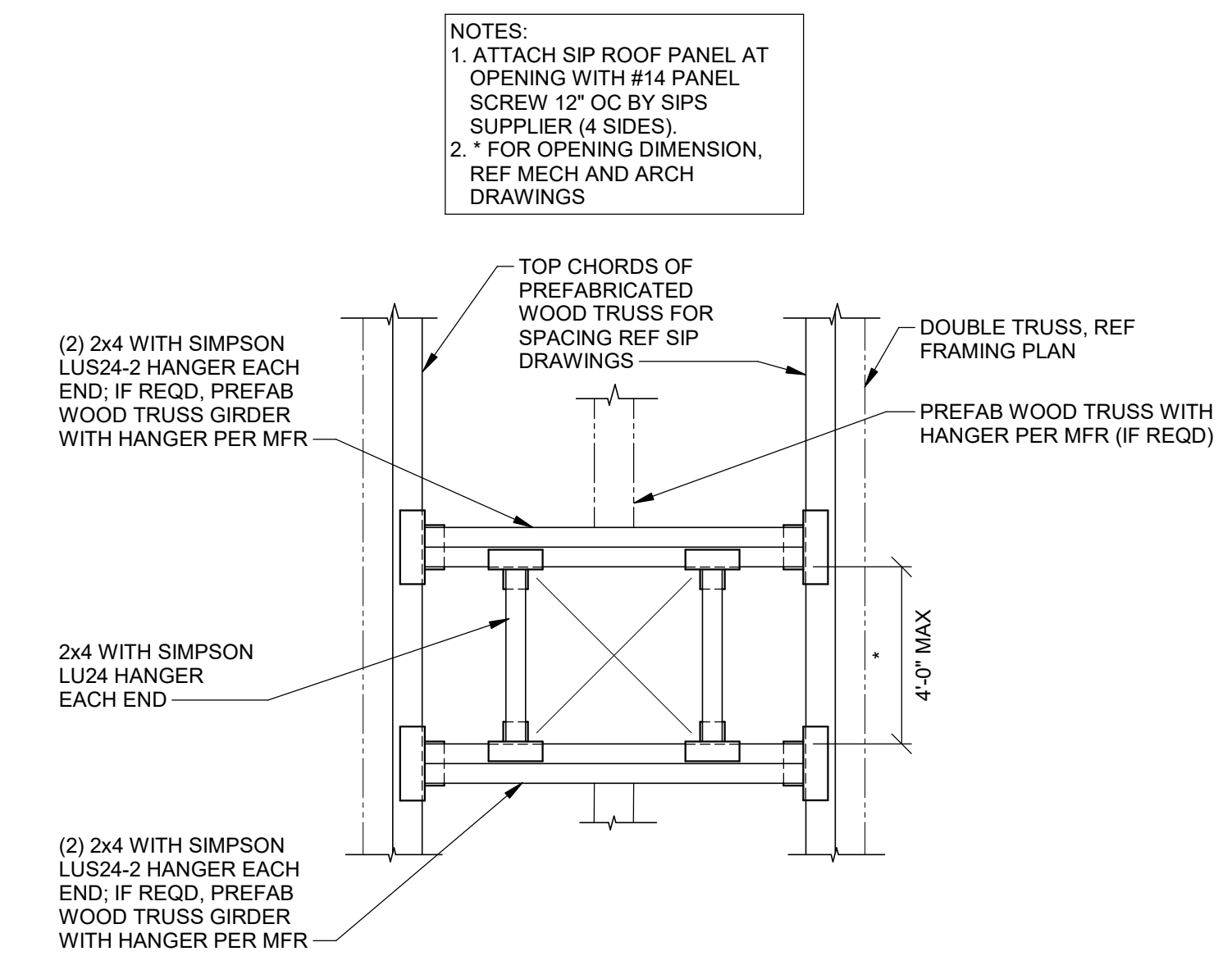
SOFFIT DETAIL PERPENDICULAR TO TRUSS | 5

3/4" = 1'-0"



SOFFIT DETAIL PARALLEL TO TRUSS | 4

3/4" = 1'-0"



TYP FRAMING AROUND SMALL ROOF OPNG | 1

NTS

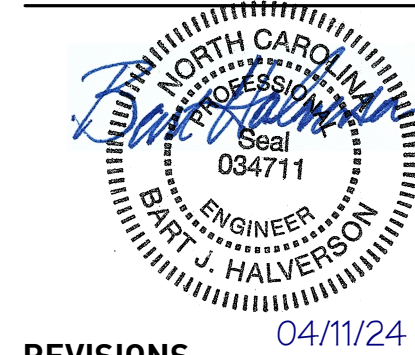
rdc.

11921 Freedom Drive #1110
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PROFESSIONAL SEAL



04/11/24

REVISIONS

ISSUE	DATE
OTP	02/15/23
OTB SET	01/05/24

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NORTH CAROLINA COA #0-9672 EXPRES: 06/30/23
JBA PROJECT #: 23023

PROFESSIONAL IN CHARGE

BJH

PROJECT MANAGER

RB

QUALITY CONTROL

SMK

DRAWN BY

JBA

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CIRCLE K STORES, INC.

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ROOF FRAMING DETAILS

S3.0