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65W2-R

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LIFE SAFETY NOTES:

EXIT DOORS TO BE OPERABLE FROM THE INSIDE WITHOUT THE USE OF A KEY OR SPECIAL KNOWLEDGE OR EFFORT. EXCEPTION: MAIN EXIT DOOR HAS KEY-LOCKING HARDWARE. GENERAL CONTRACTOR TO PROVIDE A READABLE VISIBLE, DURABLE SIGN ON OR ADJACENT TO THE DOOR STATING "THIS DOOR TO REMAIN UNLOCKED WHEN BUILDING IS OCCUPIED". SIGN TO BE ONE INCH HIGH LETTERS ON CONTRASTING BACKGROUND.

EXIT ILLUMINATION:

EXITS TO BE ILLUMINATED AT ANY TIME THE BUILDING IS OCCUPIED WITH LIGHT HAVING INTENSITY OF NOT LESS THAN ONE FOOTCANDLE AT FLOOR LEVEL. THE POWER SUPPLY FOR EXIT ILLUMINATION HAS BATTERY BACKUP IN THE EVENT OF A POWER FAILURE.

EXIT SIGNS TO BE INSTALLED AT THE REQUIRED EXITS FROM THE ROOM OR AREA AND WHEN OTHERWISE NECESSARY TO CLEARLY INDICATE THE DIRECTION OF EGRESS. GRAPHICS. ILLUMINATION AND POWER SUPPLY SHALL COMPLY WITH CHAPTER 10 OF THE IBC.

FLAMMABLE AND COMBUSTIBLE MATERIALS:

FLAMMABLE AND COMBUSTIBLE MATERIALS WILL NOT EXCEED MAXIMUM QUANTITIES ALLOWED UNDER TABLE 3404.3.4.1 OF THE IFC OR TABLE 7902.5-B OF THE UFC. THERE WILL BE NO HIGH-PILED COMBUSTIBLE STORAGE

PORTABLE FIRE EXTINGUISHERS:

TYPE, SIZE, NUMBER AND LOCATION OF FIRE EXTINGUISHERS SHALL BE INSTALLED ACCORDING TO LOCAL REQUIREMENTS.

INTERIOR FINISHES:

INTERIOR FINISHES TO HAVE A FLAME-SPREAD RATING OF 25 OR LESS.

INSULATION:

WALL INSULATION TO HAVE A FLAME-SPREAD RATING OF NOT MORE THAN 25 AND A SMOKE DEVELOPMENT RATING OF NOT MORE THAN 450. ROOF INSULATION (FOAM PLASTIC) TO HAVE A FLAME-SPREAD RATING OF NOT MORE THAN 75 AND A SMOKE-DEVELOPED RATING OF NOT MORE THAN 450.

ROOFING:

ROOF COVERING SPECIFIED IS CLASS A.

SHELVING:

MAXIMUM HEIGHT OF SHELVING FIXTURES IS 12'-0" THERE IS NO "HIGH PILED STORAGE" IN THE STORE.

BUILDING CODE ANALYSIS

DESIGN CODES: 2018 NC BUILDING CODE OCCUPANCY CLASSIFICATION: GROUP M (MERCANTILE) TYPE OF CONSTRUCTION: VB UNPROTECTED NUMBER OF STORIES: 1 ALLOWED / 1 SHOWN BUILDING AREA: 9000 ALLOWED / 6446 SHOWN AREA INCREASE: NOT APPLICABLE

EXTERIOR WALL RATINGS:

EXTERIOR WALLS SHALL BE CONSTRUCTED WITH LIGHT WEIGHT CONCRETE BLOCKS CONFORMING TO NCMA TEK 7-1A 1999 STANDARDS WHICH SAY THAT 8" UNITS ARE RATED FOR 2 HOURS AND THAT IF THE CORES ARE EITHER GROUTED SOLID OR FILLED WITH VERMICULITE THEY ARE RATED FOR 4 HOURS.

OCCUPANT LOAD FOR EGRESS: MERCANTILE: 6446 / 60 = 215

EXITS:

2 EXITS REQUIRED FOR OCCUPANT LOAD > 50

EXIT WIDTH :107 OCCUPANTS X . 2 = 43 IN. REQUIRED MINIMUM = 72 IN. PROVIDED

EXIT SEPARATION: 110 = 55 FT. REQUIRED

100 FT. PROVIDED

** MAXIMUM OVERALL DIAGONAL DISTANCE

MAXIMUM TRAVEL DISTANCE: 200 / 88 SHOWN

AISLES:

MINIMUM WIDTH: 36 IN. REQUIRED / 39 IN. PROVIDED

(AREAS SERVING EMPLOYEES ONLY)

MINIMUM WIDTH: 44 IN. REQUIRED / 48 IN.(min) PROVIDED

(PUBLIC AREAS)

MAX. DEAD END LENGTH = 20 FT. / 15 FT. SHOWN

DEFERRED SUBMITTALS: (if required)

THE FOLLOWING ITEMS WILL BE DEFERRED SUBMITTALS IF REQUIRED: ROOF FRAMING LAYOUT PLAN & CALCS

FIXTURE ANCHORING PLAN & DETAILS STORE FRONT PLAN & CALCS

SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD, WHO SHALL REVIEW AND FORWARD THEM TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND FOUND TO BE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE

THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTED DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

SPECIAL INSPECTIONS: (if required)

IN ADDITION TO THE REGULAR INSPECTIONS, THE FOLLOWING CHECKED ITEMS WILL ALSO REQUIRE SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1704 OF THE INTERNATIONAL BUILDING CODE.

TEM	REQUIRED
SOIL COMPLIANCE PRIOR TO FOUNDATION INSPECTION STRUCTURAL CONCRETE OVER 2500 PSI EXPANSION AND/OR EPOXY ANCHORS STRUCTURAL MASONRY WELDING	YES YES YES YES YES

DESIGNERS OF RECORD:

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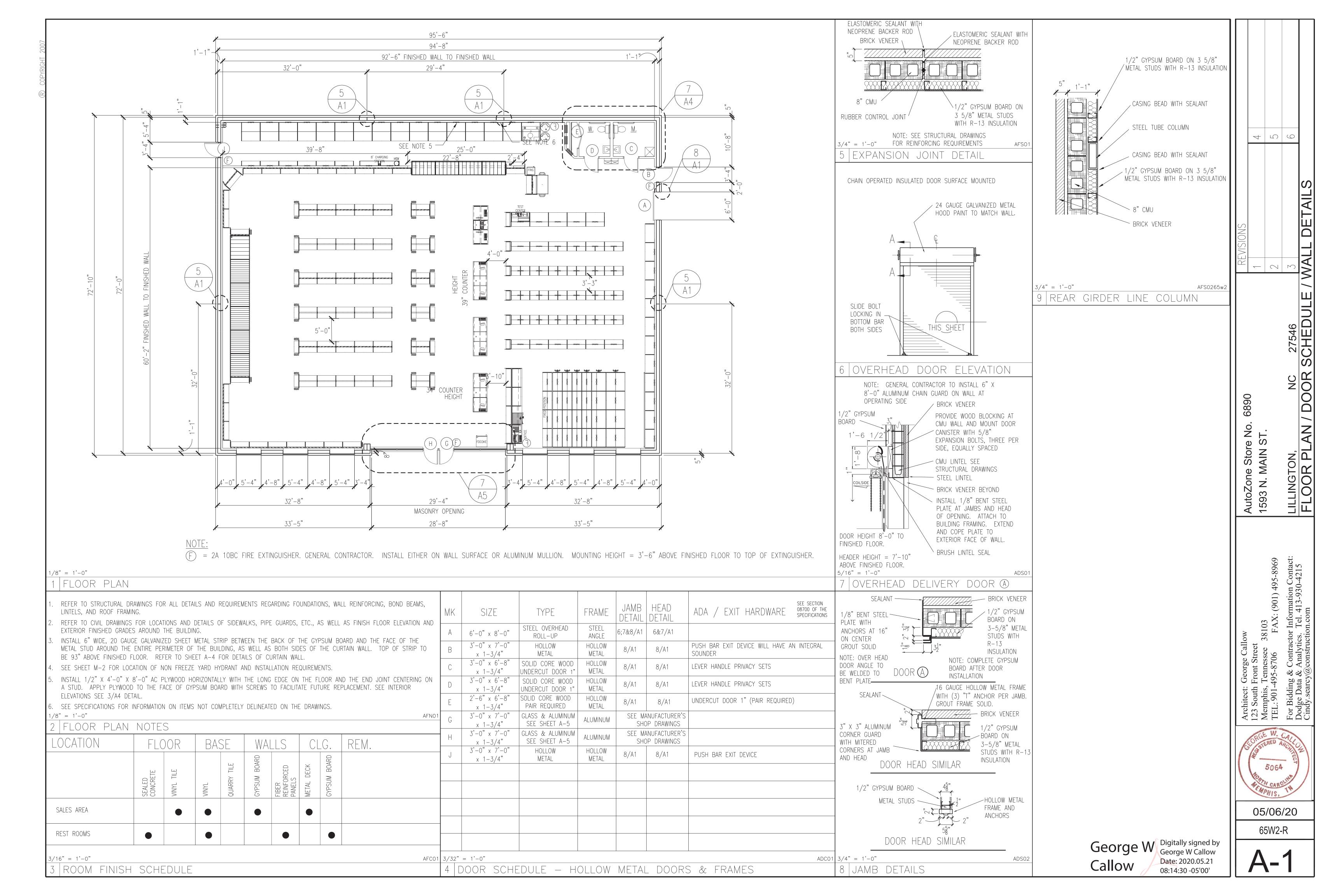
> George W Callow

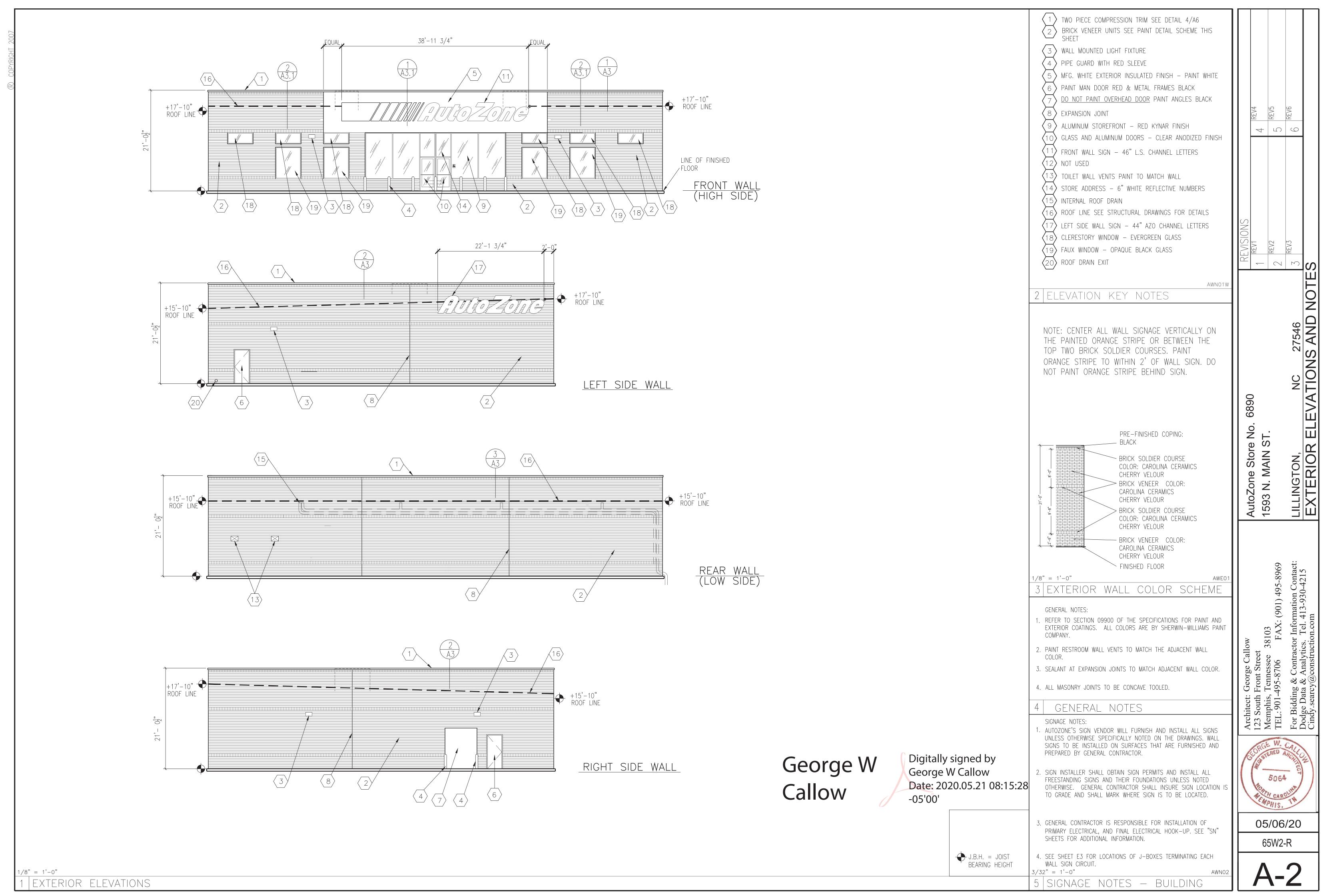
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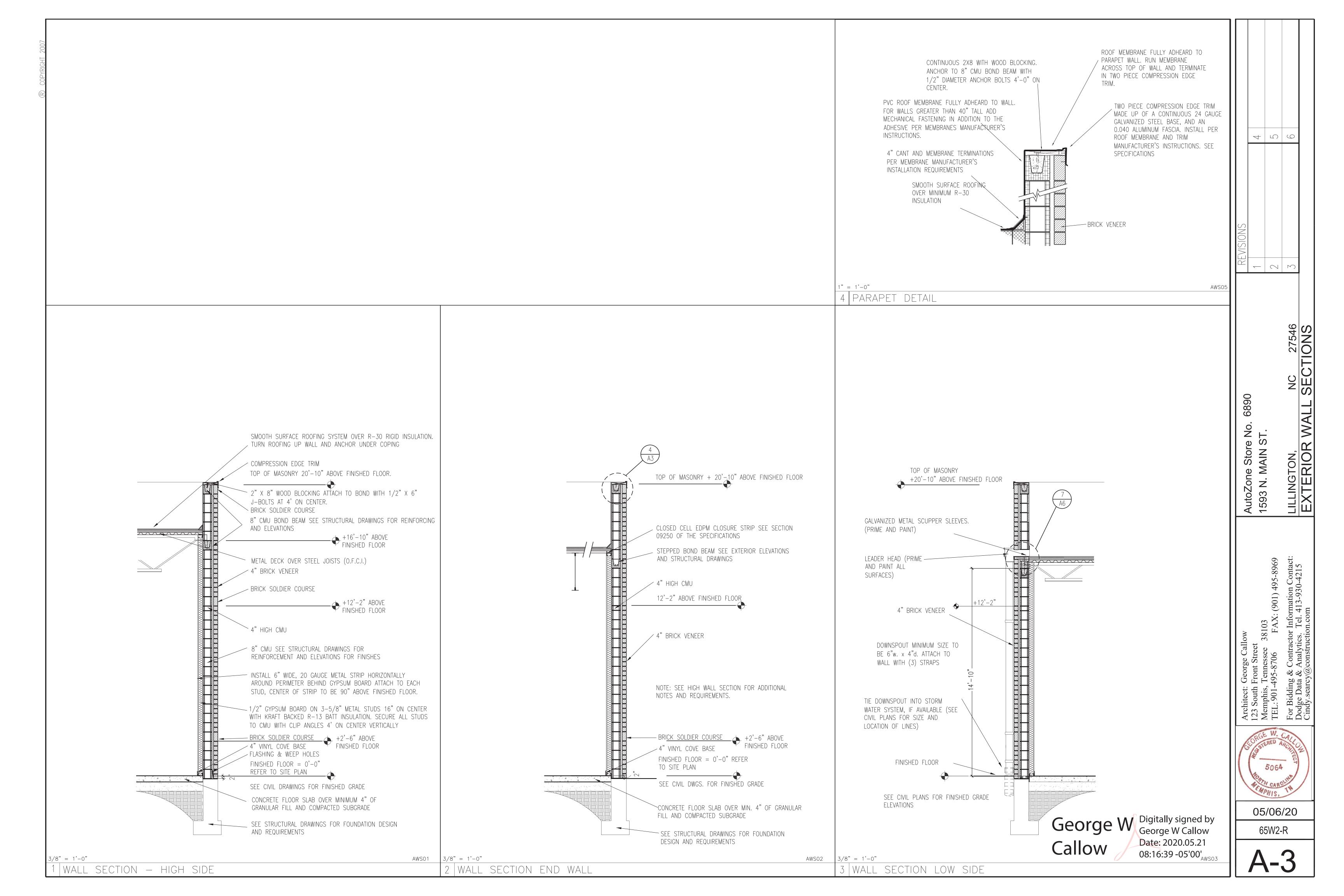
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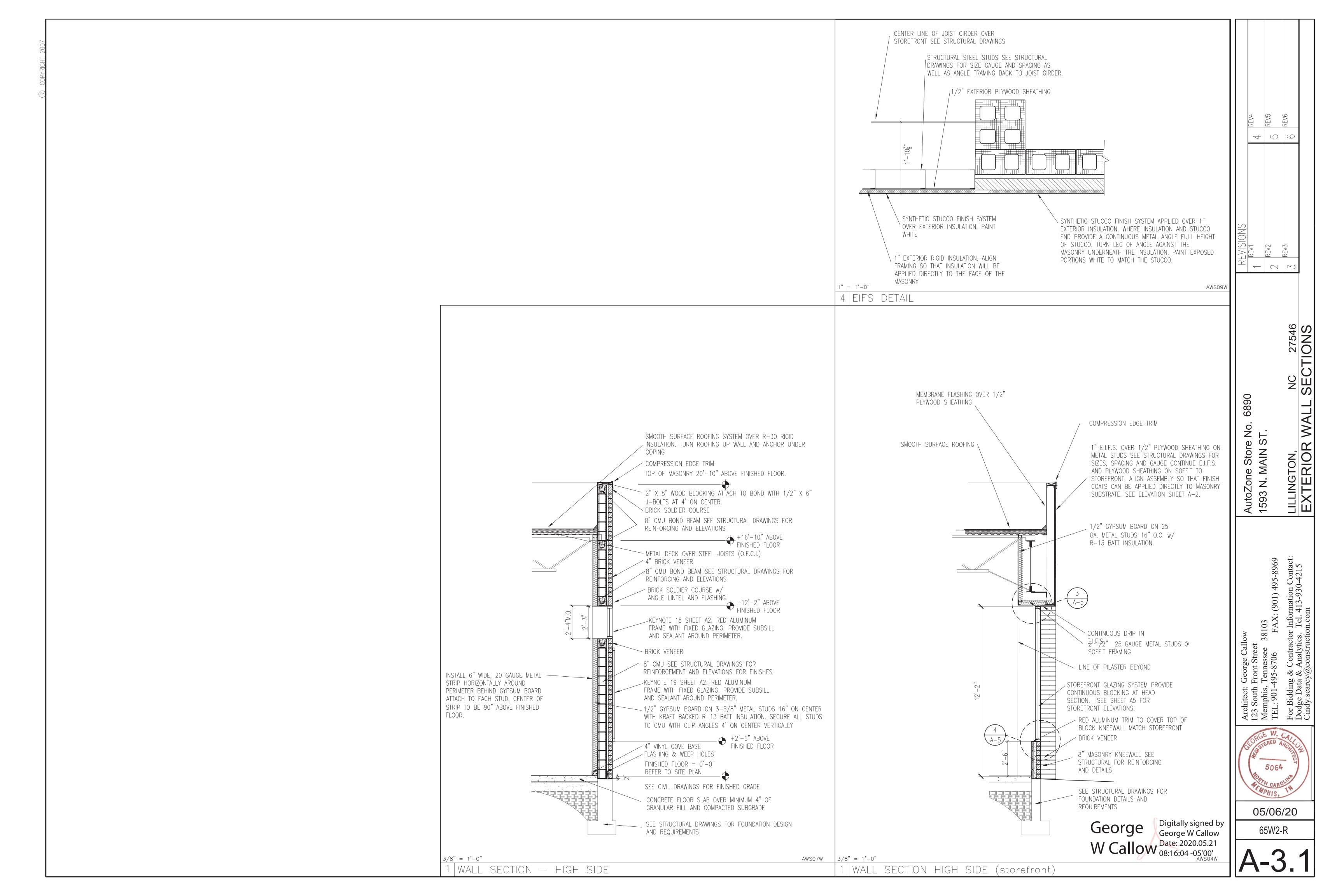
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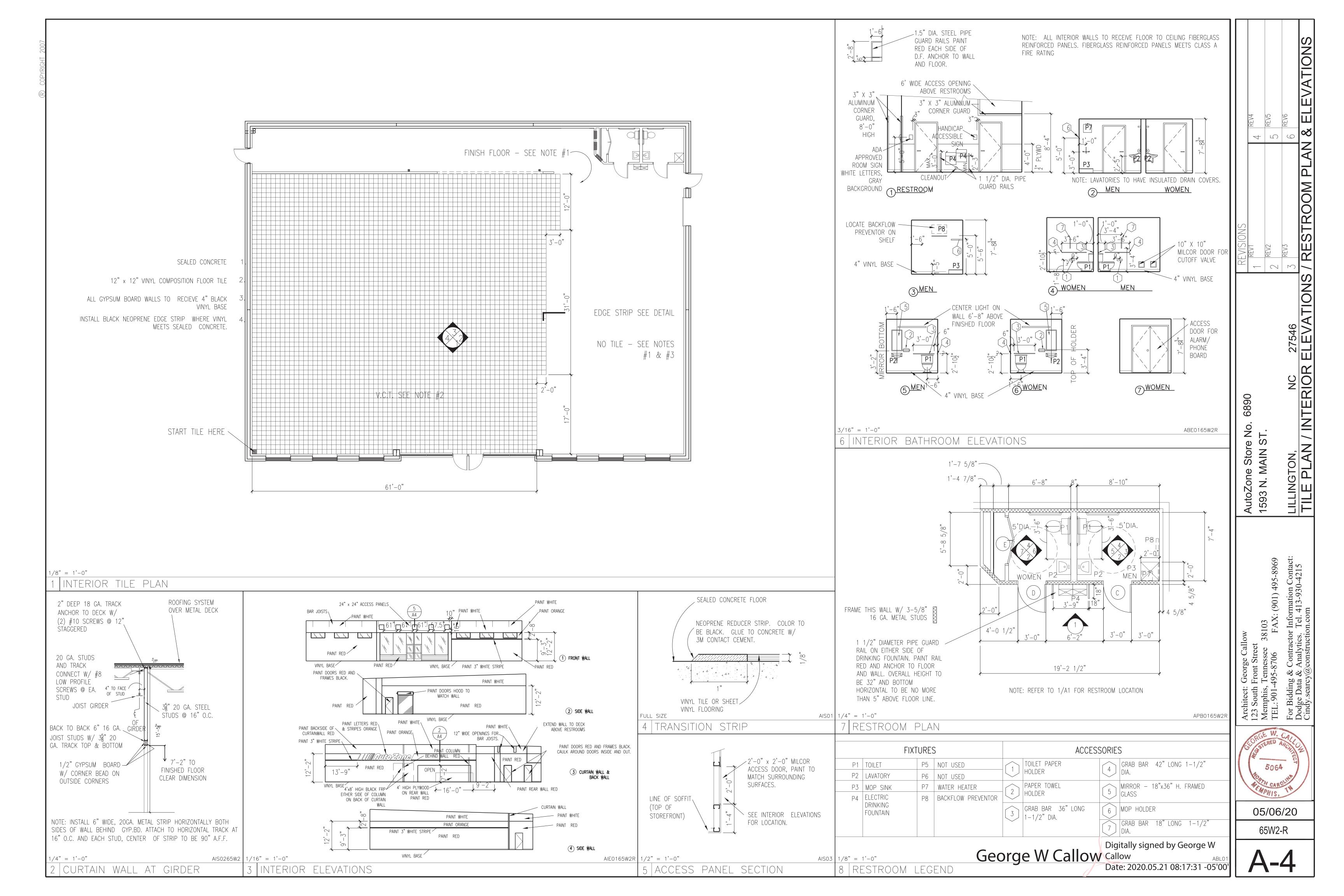
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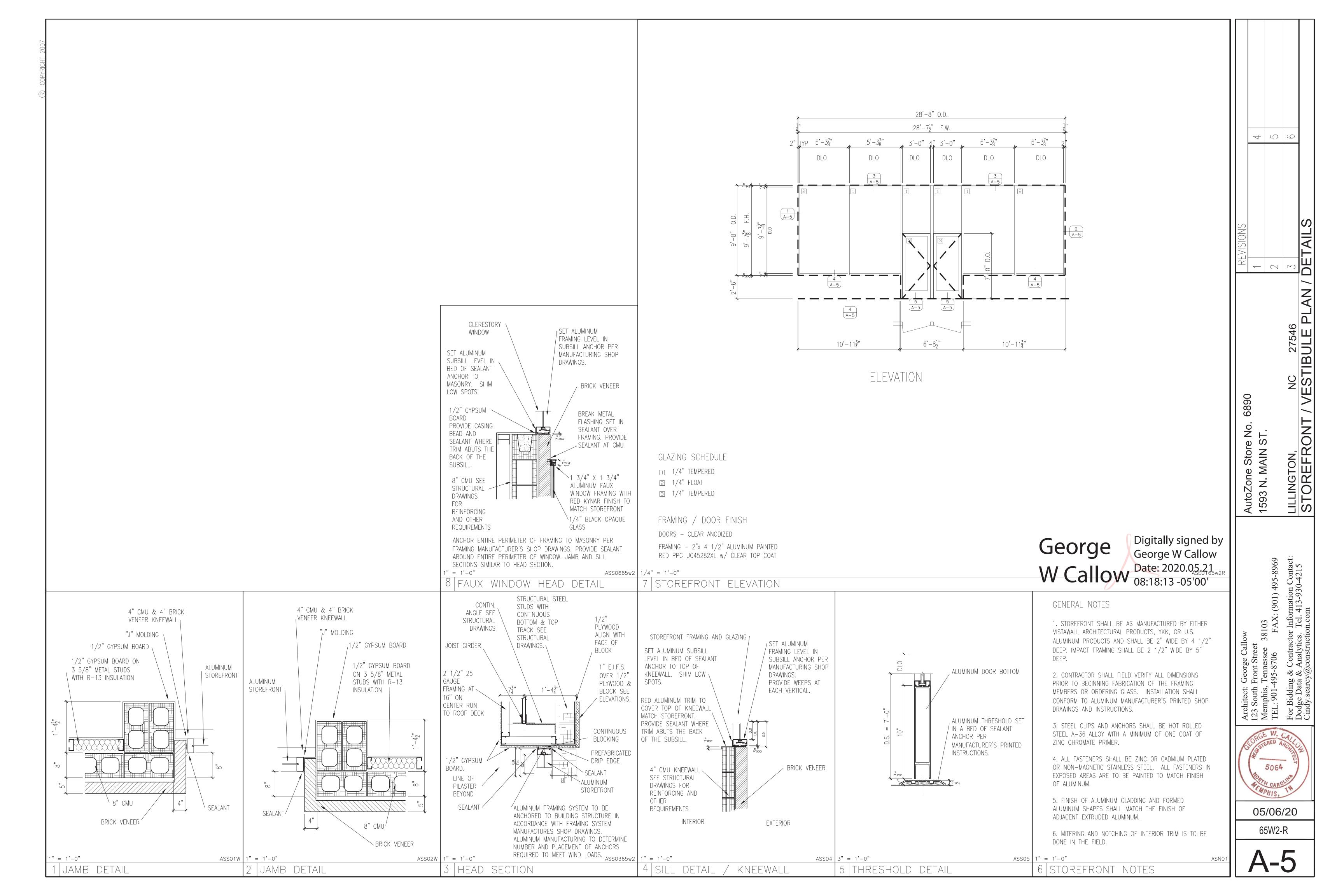


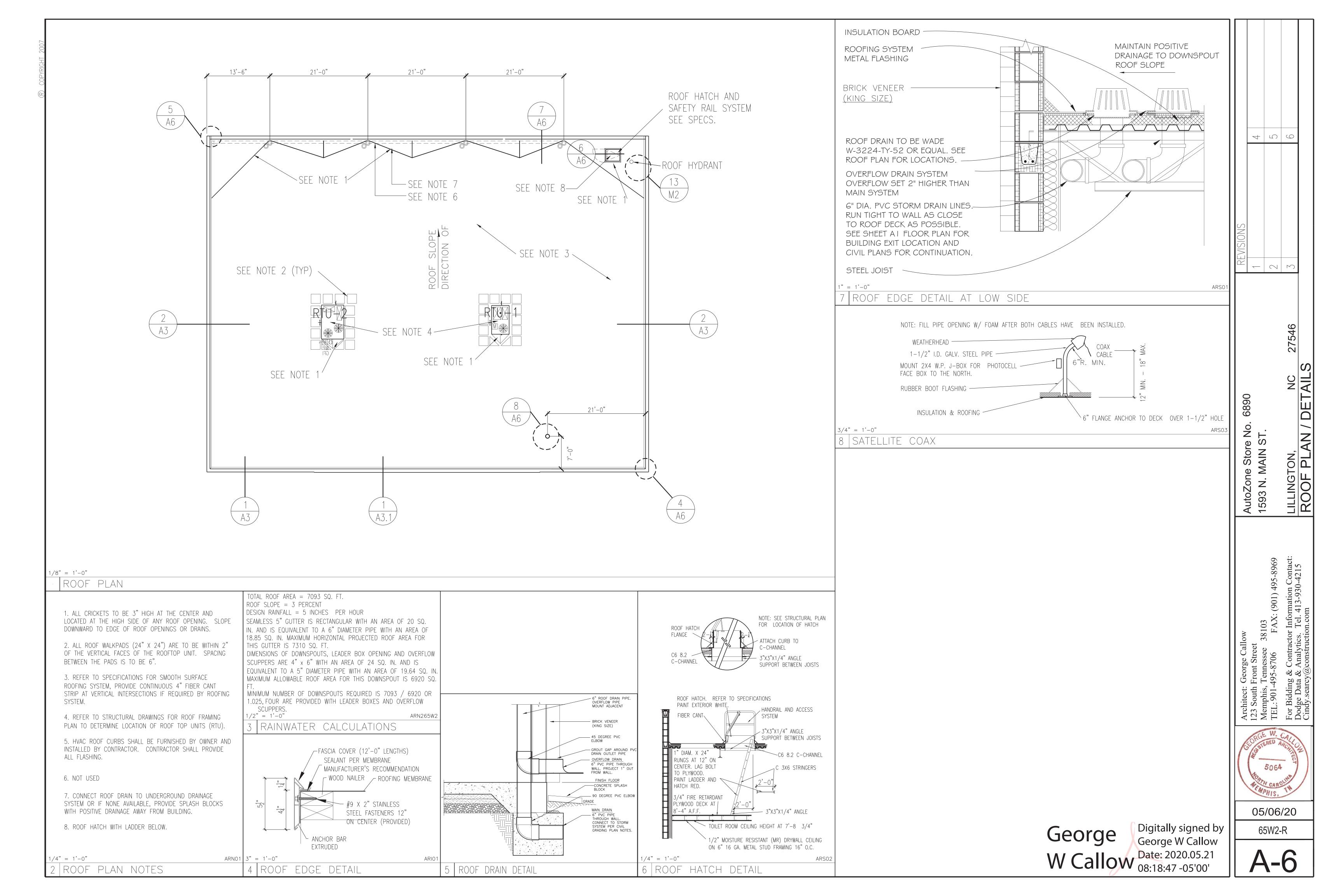












	AISI S211	-NORTH AMERICAN STANDARD F DESIGN, 2012 EDITION, INCLUD		
SIG	<u> SN LOADS</u>			
	a. ROOF	CIBLE PER GOVERNING CODE)AB ON GRADE)		CONCENTRATED (LBS) 300 2,000
	b. FLAT ROOF SNOVc. SNOW EXPOSURId. SNOW LOAD IMPO	LOAD, Pg W LOAD, Pf E FACTOR, Ce DRTANCE FACTOR, Is PR, Ct	10.5 PSF 1.0 1.0	
	b. RISK CATEGORYc. WIND EXPOSUREd. DESIGN WIND PR COMPUTED PER	N WIND SPEED (3-SECOND GUST), MPH ESSURE FOR COMPONENTS AND CLAD GOVERNING BUILDING CODE USING EX SURE COEFFICIENT (ENCLOSED)	II C DING SHALL BE POSUREC (SEE DIAGRA	AM ON SHEET S0.1)
	EARTHOLIAKE DESIG	IN DATA:		

EΑ	RTHQUAKE DESIGN DATA:
a.	OCCUPANCY RISK CATEGOR
h	CEICNIC IMPORTANCE EACT

4.	EARTHQUAKE DESIGN DATA:	
	a. OCCUPANCY RISK CATEGORY	
	b. SEISMIC IMPORTANCE FACTOR, Ie	1.0
	c. MAPPED SPECTRAL RESPONSE ACCELERATIONS	S _s = 0.181g
		$S_1 = 0.085g$
	d. SITE CLASS	
	e. DESIGN SPECTRAL RESPONSE ACCELERATIONS	$S_{DS} = 0.193q$
		$S_{D1} = 0.137g$
	f. SEISMIC DESIGN CATEGORY	
	g. BASIC SEISMIC REINFORCING SYSTEM	
	h. DESIGN BASE SHEAR	
	i. SEISMIC RESPONSE COEFFICIENT	CS = 0.055
	i RESPONSE MODIFICATION COFFFICIENT	

ED MASONRY . EQUIVALENT LATERAL FORCE k. ANALYSIS PROCEDURE USED .

FOUNDATIONS

- 1. FOUNDATION DESIGN IS BASED ON RECOMMENDATIONS IN THE GEOTECHNICAL REPORT NO. B1803403, PREPARED BY BRAUN INTERTEC CORP, DATED MAY 4, 2018. CONTRACTOR SHALL REVIEW GEOTECHNICAL REPORT PRIOR TO
- 2. FOUNDATIONS ARE DESIGNED TO BEAR ON UNDISTURBED NATURAL SOILS OR PROPERLY COMPACTED ENGINEERED FILL WITH A NET ALLOWABLE BEARING CAPACITY OF 2,000 PSF. (SEE GEOTECHNICAL REPORT.)
- 3. TOPSOIL, FILL, AND/OR OTHER DELETERIOUS MATERIALS ENCOUNTERED DURING THE SITE PREPARATION MUST THE SPECIFIED DESIGN BEARING CAPACITY. (SEE GEOTECH REPORT FOR MORE INFORMATION).
- OWNER OR CONTRACTOR SHALL EMPLOY A SOILS TESTING LABORATORY APPROVED BY THE ENGINEER TO PERFORM TESTING SERVICES AS REQUIRED BY THE SPECIFICATIONS AND TO INSPECT ALL BEARING SURFACES OF SLABS AND FOUNDATIONS.
- 5. NOTIFY ENGINEER IF FOUNDATION CONDITIONS ENCOUNTERED DIFFER FROM SOILS EXPLORATION INFORMATION MADE AVAILABLE TO THE CONTRACTOR.
- REMOVE ALL EXISTING PAVEMENT, STRUCTURES, FOUNDATIONS, TOPSOIL, UNSUITABLE FILLS, AND ORGANIC SOILS ENCOUNTERED WITHIN AND BELOW THE AREA TO BE OCCUPIED BY SLABS ON GRADE AND FOUNDATIONS. THESE MATERIALS SHALL NOT BE USED FOR FILL WITHIN OR ADJACENT TO THE BUILDING.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR AND SHALL PROVIDE TEMPORARY SHORING, BRACING, UNDERPINNING, AND OTHER MEASURES NECESSARY TO ENSURE STABILITY AND SAFETY DURING ERECTION AND CONSTRUCTION

AND TO PREVENT MOVEMENT OF SOIL THAT COULD DAMAGE EXISTING STRUCTURES, PAVEMENT, UTILITIES, ETC.

- AFTER EXCAVATING FOR SLABS ON GRADE, THE EXPOSED NATURAL SOIL SHALL BE THOROUGHLY COMPACTED PRIOR TO PLACING THE GRANULAR MATERIAL.
- 9. CENTER FOOTINGS UNDER COLUMNS OR WALLS UNLESS NOTED.
- 10. UNLESS NOTED OTHERWISE ON THE CIVIL/SITE DRAWINGS, PROVIDE A MINIMUM 2% GRADE WITHIN 10-FEET OF THE PERIMETER OF THE FOUNDATION SYSTEM TO ALLOW SURFACE WATER TO DRAIN AWAY.
- 11. DO NOT PLACE FILL OR CONCRETE ON FROZEN GROUND.

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH ACI 318.
- 2. CONCRETE SHALL HAVE THE FOLLOWING 28-DAY COMPRESSIVE STRENGTHS: CAST-IN-PLACE CONCRETE 4,000 PSI
- .. 1,500 PSI 3. USE 6 ± 1.5%, ENTRAINED AIR PER ASTM C260 FOR ALL CONCRETE EXPOSED TO WEATHER.
- 4. WATER CEMENT RATIO SHALL BE AS FOLLOWS: ALL INTERIOR SLABS-ON-GRADE 0.45 (MAX) CONCRETE WITH ENTRAINED AIR 0.45 (MAX) CONCRETE WITHOUT ENTRAINED AIR . . . 0.48 (MAX)
- 5. FIBER REINFORCING SHALL CONFORM TO ASTM C1116. FIBER REINFORCEMENT SHALL BE MACRO FIBER UNIFORMLY DISPERSED IN THE CONCRETE MIXTURE PER THE MANUFACTURER'S RECOMMENDATION, BUT NOT LESS THAN A RATE OF 4.0 lb/Cu Yd AND 1.5 INCHES LONG.
- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60. ALL REINFORCING TO BE WELDED SHALL CONFORM TO ASTM A706.
- 7. ALL WELDED WIRE REINFORCING SHALL CONFORM TO ASTM A1064 PROVIDED IN FLAT SHEETS OR ROLLS.
- 8. ADMIXTURES SHALL CONTAIN NO MORE THAN 0.05% CHLORIDE IONS BY WEIGHT OF CEMENT WHEN TESTED IN ACCORDANCE WITH AASHTO T260.
- 9. CONTRACTOR SHALL KEEP A COPY OF "FIELD REFERENCE MANUAL: STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE ACI 301 WITH SELECTED ACI REFERENCES", (ACI PUBLICATION SP-15) AT THE PROJECT FIELD OFFICE.
- 10. ALL REINFORCING DETAILS SHALL CONFORM TO "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" ACI 315, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.
- 11. SUBMIT FOR APPROVAL CONCRETE MIX DESIGN AND CERTIFICATION OF CONFORMITY OF CONCRETE MATERIALS. 12. THE OWNER OR CONTRACTOR SHALL EMPLOY A TESTING LABORATORY APPROVED BY THE ENGINEER/ARCHITECT
- PERFORM THE TESTING SPECIFIED PER PARAGRAPH 1.6.4 OF ACI 301. THE TESTING LABORATORY SHALL MEET THE REQUIREMENTS OF ASTM E329. TESTING SHALL BE MADE BY AN ACI CONCRETE FIELD TESTING TECHNICIAN GRADE 1 OR APPROVED EQUIVALENT. A TECHNICIAN GRADE 1 SHALL BE PRESENT DURING ALL CONCRETE
- 13. SUBMIT SHOP DRAWINGS FOR REVIEW. THESE DRAWINGS SHALL SHOW ALL CONCRETE MEMBER DIMENSIONS AND DOWELS FOR MASONRY WALLS.
- -STANDARD SPECIFICATION, LOAD TABLES, AND WEIGHT TABLES FOR STEEL JOIST AND 14. PROVIDE DOWELS FROM FOUNDATIONS TO MATCH COLUMN, PIER AND WALL VERTICAL REINFORCING.
 - 15. PROVIDE ADEQUATE BOLSTERS, HI-CHAIRS, SUPPORT BARS, ETC., TO MAINTAIN SPECIFIED CLEARANCES FOR THE ENTIRE LENGTH OF ALL REINFORCING BARS. SUPPORTS THAT BEAR DIRECTLY ON EXPOSED SURFACES SHALL BE
 - 16. ALL SLABS SHALL BE POURED MONOLITHICALLY, EXCEPT FOR THE REQUIRED CONSTRUCTION JOINTS.
 - 17. PROVIDE PERIMETER INSULATION AGAINST EXTERIOR FOUNDATION WALLS AND UNDER THE SLAB ADJACENT TO THE EXTERIOR OF THE BUILDING AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
 - 18. PROVIDE 3/4-INCH CHAMFER ON ALL EXPOSED CORNERS OF SLABS, COLUMNS AND WALLS UNLESS OTHERWISE INDICATED ON THE ARCHITECTURAL DRAWINGS. MINIMUM CLEARANCES FOR REINFORCING STEEL SHALL BE
 - 19. CURE ALL CONCRETE FOR A MINIMUM 7-DAYS. APPLY CURING COMPOUND AT THE MAXIMUM COVERAGE RATE OF 300 SQUARE FEET PER GALLON. USE PRODUCT IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SEE SPECIFICATIONS.
 - 20. ALL CONSTRUCTION JOINTS SHALL BE KEYED. PROVIDE KEYWAYS AT MEMBER CENTERLINE WITH A DEPTH OF 1-1/2 INCH AND HEIGHT EQUAL TO ONE-THIRD OF THE MEMBER'S DEPTH/THICKNESS.
 - 21. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS OF CONSTRUCTION JOINTS NOT INDICATED ON THE DRAWINGS FOR REVIEW BY THE ENGINEER/ARCHITECT.
 - 22. ALL ALUMINUM IN CONTACT WITH CONCRETE OR DISSIMILAR METALS SHALL BE COATED WITH GRAY EPOXY PRIMER, APPROVED BY THE ENGINEER.
 - 23. FORMWORK, FOR ALL CONCRETE THAT WILL BE EXPOSED IN THE COMPLETED STRUCTURE, SHALL BE CONSTRUCTED FROM A METAL OR SUITABLE SURFACE PLYWOOD THAT WILL PRODUCE AN ACCEPTABLY SMOOTH SURFACE. SEE SPECIFICATIONS.
 - 24. PITCH CONCRETE SLABS TO FLOOR DRAINS SHOWN ON MECHANICAL OR ARCHITECTURAL DRAWINGS.
 - 25. CONCRETE PROTECTION (CLEAR COVER) FOR REINFORCEMENT BARS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE: a. FOOTINGS:
 - 3 INCHES, BOTTOM AND UNFORMED EDGES 2 INCHES, FORMED EDGES
 - 2 INCHES, EXPOSED TO EARTH, WATER OR WEATHER
 - b. SLABS: 3/4 INCHES TO REINFORCEMENT
 - c. COLUMNS, PIERS:
 - 1-1/2 INCH TO TIES 2 INCH FOR VERTICAL REINFORCEMENT
 - 26. LAP SPLICE WELDED WIRE FABRIC ONE SPACE PLUS 2 INCHES AT EDGES AND ENDS AND PROVIDE ADDITIONAL REINFORCING WHERE SHOWN ON DRAWINGS. PLACE MESH 2 INCHES FROM TOP OF SLAB FOR SLABS ON GROUND AND 1 INCH FROM TOP OF SUPPORTED SLABS UNLESS NOTED OTHERWISE.
 - 27. ALL HOOKS SHALL BE ACI STANDARD HOOKS UNLESS DIMENSIONED OTHERWISE.

CONCRETE MASONRY

- MASONRY IS SUPPORTED IN THE COMPLETED CONSTRUCTION. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR SUPPORTING THE MASONRY DURING CONSTRUCTION IN CONFORMANCE WITH LOCAL, STATE AND NATIONAL LAWS AND AS REQUIRED.
- 2. MASONRY CONSTRUCTION AND MATERIAL SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6) EXCEPT AS MODIFIED IN THE SPECIFICATIONS AND BELOW. A COPY OF ACI 530.1/ASCE 6 SHALL BE ON THE JOB SITE AT ALL TIMES THAT MASONRY WORK IS BEING PERFORMED.
- BE REMOVED AND REPLACED WITH SELECT ENGINEERED FILL COMPACTED TO 90% PER ASTM D1557 AND MEETING 3. SUBMIT FOR REVIEW, PRIOR TO CONSTRUCTION, SHOP DRAWINGS SHOWING A PLAN AND ELEVATION VIEW OF ALL CMU WALL, AND A PLAN THAT SHOWS ALL DOWELS REQUIRED FOR VERTICAL CMU REINFORCING THAT EXTEND OUT OF CONCRETE. SHOW WALL THICKNESS, AND DIMENSION WALL LENGTH AND LOCATION. SHOWING TOP ELEVATIONS OF WALLS, BOND BEAMS AND GROUT POURS. SHOW LOCATION OF CONTROL JOINT LOCATIONS SOLID UNITS, CELLS TO BE GROUT FILLED, OPENING, LINTEL, JOINT REINFORCEMENT, REINFORCING BAR AND
 - 4. SUBMIT FOR REVIEW, PRIOR TO CONSTRUCTION, DOCUMENTATION FOR THE BLOCK, MORTAR, GROUT, ADMIXTURES, REINFORCING, BAR POSITIONER AND OTHER ACCESSORIES PROPOSED FOR USE. SUBMIT A WRITTEN DESCRIPTION OF THE METHOD OF REINFORCEMENT AND GROUT, AND OF GROUT CONSOLIDATION.
 - CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, NORMAL WEIGHT
 - 6. CONCRETE MASONRY UNITS WHICH CONTAIN VERTICAL REINFORCEMENT SHALL BE TWO CORE UNITS AND WITH CORES AND WEBS VERTICALLY ALIGNED.
 - 7. MORTAR FOR CONCRETE MASONRY UNITS SHALL BE NON-AIR ENTRAINED PORTLAND CEMENT-LIME CONFORMING TO ASTM C270, TYPE S. CEMENT IN MORTAR SHALL BE LOW-ALKALI AND NON-STAINING. TYPE N MORTAR AND MASONRY CEMENT SHALL NOT BE USED FOR CMU CONSTRUCTION.
 - 8. ADMIXTURES SHALL NOT BE USED IN THE MORTAR OR GROUT. ANTIFREEZE AND CALCIUM CHLORIDE SHALL NOT BE USED.
 - 9. MINIMUM NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS SHALL BE: STANDARD BLOCK = 1,900 PSI (F'M = 1,500 PSI)
 - 10. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI.
 - 11. ALL CONCRETE BLOCK WALLS SHALL BE REINFORCED VERTICALLY WITH 1-#5 BAR AT 24 INCHES ON CENTER. UNLESS NOTED OTHERWISE.
 - 12. PROVIDE 1-#5 VERTICAL BAR IN FIRST CORE AT EACH CORNER, END OF WALL, AND ADJACENT TO OPENINGS AND
 - 13. VERTICAL REINFORCEMENT SHALL EXTEND THROUGH BOND BEAMS AND TO WITHIN 2 INCHES OF THE TOP OF

 - UNLESS NOTED OTHERWISE.
 - 15. ANCHORAGE OF REINFORCING STEEL INTO CONCRETE SHALL BE 36 BAR DIAMETERS BUT NO LESS THAN 12
 - 16. HORIZONTAL JOINT REINFORCING SHALL BE, UNLESS SHOWN OTHERWISE, STANDARD 9 GAGE, LADDER TYPE CONFORMING TO ASTM A951, SPACED VERTICALLY AT 8-INCH ON CENTERS ABOVE AND BELOW OPENINGS FOR THREE CONSECUTIVE COURSES AND AT 16 INCHES ON CENTERS ELSEWHERE. EXTEND REINFORCEMENT 2 FEET BEYOND EACH SIDE OF OPENINGS BUT DO NOT EXTEND THROUGH CONTROL JOINTS. PROVIDE FACTORY FABRICATED "T" AND "L" SHAPED PIECES AT INTERSECTIONS AND CORNERS.
 - 17. JOINT REINFORCEMENT SHALL BE SPLICED BY LAPPING THE LONGITUDINAL WIRES AT LEAST 12 INCHES; THE CROSS-WIRES WITHIN THE LAP SHALL BE REMOVED SO THAT THE LONGITUDINAL WIRES ARE SIDE BY SIDE. ALTERNATELY WHERE JOINT REINFORCING IS NOT REQUIRED IN BETWEEN EACH COURSE, SPLICES MAY BE MADE BY ABUTTING THE ADJACENT SECTIONS OF JOINT REINFORCING AND CENTERING A 48 INCH LENGTH OF JOINT REINFORCING IN THE BED JOINT IMMEDIATELY ABOVE OR BELOW THE BUTT JOINT. SPLICE WITH "T" AND "L" SHAPED PIECES AT INTERSECTIONS AND CORNERS.
 - 18. BOND BEAMS SHALL BE PROVIDED IN EACH WALL AT EACH FLOOR LEVEL, ROOF LEVEL, AND AT TOP OF WALL. FILL STEEL JOISTS AND JOIST GIRDERS BOND BEAMS WITH GROUT. REINFORCE BOND BEAMS WITH 2-# 5 UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS WITH 2'-0" LEGS AND BAR SUPPORTS TO OBTAIN THE REQUIRED CLEARANCE.
 - 19. BOND BEAMS AT THE TOP OF SLOPED WALLS SHALL FOLLOW THE SLOPE OF THE WALL. CUT OUT WEB OF BLOCKS SO THAT THE REINFORCING BAR(S) ARE 4 INCHES CLEAR OF THE TOP OF THE WALL. A MINIMUM OF 16 INCHES VERTICALLY SHALL BE GROUT FILLED.
 - 20. BOND BEAM REINFORCEMENT AND GROUT AT WALL CONTROL JOINTS SHALL BE CONTINUOUS. PROVIDE A DUMM CONTROL JOINT IN BOTH FACES OF BOND BEAM ALIGNED WITH WALL CONTROL JOINTS. THE BLOCK FACE SHELLS AT DUMMY CONTROL JOINTS SHALL BE FREE OF MORTAR AND GROUT. THE DUMMY CONTROL JOINT IN EXPOSED FACES SHALL HAVE BACKING ROD AND CAULK SEAL AS REQUIRED FOR THE CONTROL JOINT.
 - 21. VERTICAL CONTROL JOINTS IN CONCRETE MASONRY WALLS (OTHER THAN BASEMENT WALLS) SHALL BE PROVIDED WHERE SHOWN ON THE PLANS AND AS GIVEN BELOW:
 - a. AT 25 FEET OR LESS ON CENTERS BUT NOT MORE THAN 1 1/2 TIMES THE WALL HEIGHT
 - b. AT A DISTANCE NOT OVER ONE-HALF THE ABOVE SPACING FROM BONDED INTERSECTIONS OR CORNERS. c. AT ONE END OF A LINTEL FOR WALL OPENINGS SIX FEET OR LESS IN WIDTH
 - d. AT BOTH ENDS OF LINTELS FOR OPENINGS MORE THAN SIX FEET WIDE
 - e. ALL ABRUPT CHANGES IN WALL HEIGHT. f. AT ALL CHANGES IN WALL THICKNESS, SUCH AS THOSE AT PIPE AND DUCT CHASES AND THOSE ADJACENT TO
 - COLUMNS OR PILASTERS. g. ABOVE JOINTS IN FOUNDATIONS AND FLOORS.
 - h. BELOW JOINTS IN ROOFS AND FLOORS THAT BEAR IN THE WALL.
 - 22. CONTROL JOINTS SHALL NOT OCCUR AT WALL CORNERS, INTERSECTIONS, ENDS, WITHIN 2'-0" OF CONCENTRATED 8. LH JOISTS SHALL BE WELDED TO SUPPORTING STEEL WITH TWO 1/4-INCH FILLET WELDS 2-INCH LONG. MINIMUM. POINTS OF BEARING, OR JAMBS OVER OPENINGS UNLESS SPECIFICALLY SHOWN ON THE STRUCTURAL DRAWINGS.
 - 23. ALL MASONRY BELOW GRADE SHALL BE GROUTED SOLID.
 - 24. MECHANICALLY VIBRATE GROUT IN VERTICAL SPACES IMMEDIATELY AFTER POURING AND AGAIN MINUTES LATER.
 - 25. PROVIDE CLEANOUTS IF GROUT LIFT EXCEEDS 4'-0" IN BLOCK WALLS. MAXIMUM GROUT LIFT SHALL BE 8'-0".

PRECAST CONCRETE LINTELS

- PRECAST CONCRETE LINTELS SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH THE PRECAST MANUFACTURER'S SPECIFICATIONS.
- 2. PROVIDE LINTELS OVER ALL MASONRY OPENINGS AND OVER RECESSES WIDER THAN 12 INCHES IN ACCORDANCE
- WITH THE ACCOMPANYING LINTEL SCHEDULE ON SHEET S0.2, UNLESS NOTED OTHERWISE ON DRAWINGS.
- 3. PRECAST CONCRETE LINTELS SHALL BE MANUFACTURED UTILIZING: a. MINIMUM CONCRETE COMPRESSIVE STRENGTH, F'_C = 3,500 PSI
- b. REINFORCING YIELD STRENGTH, $F_Y = ASTM A615$, GRADE 60
- c. MINIMUM GROUT COMPRESSIVE STRENGTH, ASTM C476, F'G = 3,000 PSI
- 4. PRECAST CONCRETE LINTELS SHALL HAVE A MINIMUM BEARING LENGTH OF 8-INCHES AT EACH END.
- 5. BELOW EACH BEARING POINT OF LINTEL, GROUT FILL CELLS FOR A MINIMUM OF 16" BEYOND EDGE OF OPENING AND A MINIMUM OF 16" BELOW LINTEL BEARING.
- WHERE CONTROL JOINTS ARE AT ENDS OF LINTELS, PROVIDE 15 POUND FELT BOND BREAKER UNDER LINTEL BEARING AND DUMMY CONTROL JOINT ON EXPOSED FACES. NO MORTAR OR GROUT SHALL BE IN THE HEAD JOINT OF DUMMY CONTROL JOINTS OPPOSITE THE BLOCK SHELL. PROVIDE A POSITIVE MEANS OF PREVENTING GROUT FROM ENTERING DUMMY JOINT OPPOSITE THE BLOCK SHELL

STRUCTURAL STEEL

- STEEL SHALL BE FABRICATED BY A FABRICATOR HAVING AN AISC QUALITY CERTIFICATION CATEGORY: "STANDARD FOR STEEL BUILDING STRUCTURES (STD)."
- 2. STRUCTURAL STEEL WORK SHALL CONFORM TO THE "STEEL CONSTRUCTION MANUAL, AISC 360."
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF MEMBERS AND CONNECTIONS FOR ANY PORTION OF THE STRUCTURE NOT INDICATED ON THE PLANS. ALL SPECIAL CONDITIONS AND CONNECTIONS SHALL BE CAREFULLY AND COMPLETELY DETAILED AND SUBMITTED FOR APPROVAL.
- CONTRACTOR SHALL VERIFY THE EXACT LOCATION AND SIZE OF ALL OPENINGS FOR MECHANICAL EQUIPMENT WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION OF MATERIALS.
- ANY STEEL SHOWN ON DRAWINGS FOR SUPPORTING OR CONNECTING MECHANICAL, ELECTRICAL, OR PLUMBING EQUIPMENT IS FOR BID PURPOSES ONLY. CONTRACTOR SHALL COORDINATE EXACT SIZE AND LOCATION PRIOR
- 6. UNLESS SHOWN ON STRUCTURAL DRAWINGS, CONTRACTOR SHALL NOT CUT ANY HOLES IN STRUCTURAL STEEL MEMBERS WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.
- 7. ALL STEEL BEAMS SHALL BE FABRICATED AND ERECTED WITH THE NATURAL CAMBER (WITHIN THE MILL TOLERANCE) LOCATED ABOVE THE HORIZONTAL CENTERLINE BETWEEN THE END CONNECTIONS.
- 8. STEEL SHALL CONFORM TO THE FOLLOWING UNLESS OTHERWISE NOTED:
- a. ANGLES, PLATES, ETC: ASTM A36 b. STRUCTURAL TUBING:

SQUARE & RECTANGULAR, ASTM A500, GRADE B, 46 KSI

- c. ANCHOR RODS: ASTM F1554, GRADE 36 WELDED CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF THE AMERICAN WELDING SOCIETY, AWS
- D1.1. WELDING ELECTRODE MATERIAL SHALL BE E70XX. 10. MINIMUM WELDS, WHERE NOT SHOWN ON DRAWINGS, SHALL BE 3/16 INCH FILLET WELD, ALL AROUND.
- 11. ALL CONNECTIONS SHALL BE MADE WITH 3/4-INCH ASTM A325 BOLTS TIGHTENED TO SNUG-TIGHT CONDITION UNLESS OTHERWISE NOTED.
- 12. ALL CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR UTILIZING THE REQUIREMENTS IN AISC 360, AND THE CONTRACT DOCUMENTS. THE FABRICATOR SHALL USE ALLOWABLE STRESS DESIGN METHODOLOGY TO COMPLETE ALL CONNECTION DESIGNS INCLUDING THE
- FOLLOWING GUIDELINES. a. DETAIL ALL BOLTED CONNECTIONS AS BEARING TYPE CONNECTIONS WITH THREADS IN THE SHEAR PLANE,
- EXCEPT THE FOLLOWING CONNECTIONS, WHICH SHALL BE DESIGNED AS SLIP-CRITICAL CONNECTIONS: ALL CONNECTIONS IN DIRECT TENSION.
- ALL BEAM OR GIRDER CONNECTIONS USING OVERSIZED HOLES OR LONG SLOTS. ANY CONNECTION NOTED ON THE CONTRACT DRAWINGS AS SLIP-CRITICAL CONNECTION.
- 14. REINFORCING STEEL SPLICES SHALL BE LAPPED A MINIMUM OF 48 BAR DIAMETERS BUT NO LESS THAN 12 INCHES, 13. ALL SHELF ANGLES AND LINTELS IN EXTERIOR WALLS, INCLUDING BEARING PLATES AND ANCHOR RODS, SHALL BE GALVANIZED AFTER FABRICATION.
 - 14. ALL STEEL AND CORRESPONDING CONNECTIONS EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123 AND A153, RESPECTIVELY.
 - 15. ALL STEEL, AND ANCHOR RODS THAT WILL BE GALVANIZED, ENCASED IN CONCRETE, OR RECEIVE SPRAYED ON FIREPROOFING SHALL NOT BE PAINTED.
 - 16. PROVIDE 3/8-INCH DIAMETER WEEP HOLES AT BASE OF HSS AND PIPE COLUMNS AND IN BOTTOM OF CAPPED HSS
 - 17. PROVIDE 1/4" MIN CLOSURE PLATES TO ALL HOLLOW STRUCTURAL SECTIONS WITH A 1/4" FILLET WELD ALL

18. SET COLUMN BASE PLATES UPON NON-METALLIC, SHRINK RESISTANT GROUT CONFORMING TO ASTM C1107.

19. PROVIDE HARDENED STEEL WASHERS CONFORMING TO ASTM F436 AND HEAVY HEX NUTS ON ANCHOR RODS.

- 1. ALL STEEL JOISTS, INCLUDING ANCHORAGE AND BRIDGING, SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE "STANDARD SPECIFICATIONS, LOAD TABLES AND WEIGHT TABLES
- 2. JOIST SIZES INDICATED ON THE PLANS ARE THE MINIMUMS. JOIST AND GIRDER LOADS SHOWN ON THE PLANS ARE SUPERIMPOSED AND DO NOT INCLUDE SELF-WEIGHT.
- JOIST MANUFACTURER SHALL DESIGN AND SUBMIT CALCULATIONS BY A REGISTERED ENGINEER FOR ALL SPECIALTY JOISTS, EXCEPT PARALLEL CHORD JOISTS WITH UNIFORM LOADS AND CONTINUOUSLY SUPPORTED COMPRESSION CHORDS PER SJI STANDARD LOAD TABLES.
- 4. ALL JOIST CALCULATIONS SHALL INCLUDE DEFLECTION AND CAMBER REQUIREMENTS. LIVE LOAD DEFLECTIONS SHALL BE LIMITED TO SPAN/360 AND TOTAL LOAD SHALL BE LIMITED TO SPAN/240.
- 5. ALL JOISTS SHALL BE CAMBERED FOR THE DESIGN DEAD LOADS.
- 6. JOIST TOP CHORD AND BOTTOM CHORD EXTENSIONS SHALL BE PROVIDED WHERE INDICATED ON ARCHITECTURAL OR STRUCTURAL DRAWINGS.
- 7. CAMBER OF JOIST ADJACENT TO BEAMS AND WALLS TO WHICH THE METAL DECK IS TO BE ATTACHED SHALL BE SUCH THAT THE METAL DECK CAN BE ATTACHED TO THE WALL OR BEAM WITHOUT DAMAGING THE METAL DECK. COORDINATE JOIST CAMBER WITH BEAM CAMBER.
- UNLESS NOTED OTHERWISE ON PLAN.
- 9. K-SERIES JOISTS SHALL BE WELDED TO SUPPORTING STEEL WITH TWO 1/8 INCH FILLET WELDS 2 1/2 INCH LONG, MINIMUM, UNLESS NOTED OTHERWISE ON PLAN.
- 10. JOIST BRIDGING (SPACING, TYPE, SIZE AND INSTALLATION) SHALL BE AS SPECIFIED IN THE SJI SPECIFICATIONS AND SHALL BE THE RESPONSIBILITY OF THE JOIST MANUFACTURER. BRIDGING HAS NOT BEEN SHOWN ON THE
- 11. ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED TO WALLS PER TYPICAL DETAILS.
- 12. FIELD DRILLING OR BURNING HOLES IN JOIST AND JOIST GIRDER MEMBERS IS NOT PERMITTED.
- 13. THE JOISTS AND JOIST GIRDERS HAVE BEEN SELECTED FOR THE DESIGN DEAD AND LIVE LOAD ONLY. THE JOIST MANUFACTURER SHALL PROVIDE JOISTS AND JOIST GIRDERS THAT ARE DESIGNED FOR THE LOADS SHOWN ON
- 14. MANUFACTURER SHALL ADD ADDITIONAL WEB MEMBERS AS REQUIRED AND ADJUST CHORD AND WEB SIZES ACCORDINGLY. DEPTHS OF JOIST GIRDERS SHALL NOT BE ALTERED.
- 15. DESIGN CALCULATIONS SHALL INCLUDE SUPERIMPOSED LOADS FOR FRAMING SUPPORTED EQUIPMENT. VERIFY SIZE, WEIGHT AND LOCATION WITH ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS AND SUBMITTED SHOP DRAWINGS.
- 16. THE JOISTS AND THE ERECTION OF THE JOISTS SHALL CONFORM TO THE REQUIREMENTS OF OSHA.
- 17. IF POSSIBLE, HANGERS SUPPORTING MECHANICAL EQUIPMENT, ETC. SHALL BE LOCATED AT THE JOIST PANEL POINTS. IF HANGERS ARE LOCATED BETWEEN JOIST PANEL POINTS, PROVIDE JOIST STIFFENERS AS INDICATED IN TYPICAL DETAILS. ALL HANGERS TO BE HUNG OFF BOTTOM CHORD CENTERLINE.
- 18. PROVIDE HEADERS SIZED FOR THE REACTION OF JOIST BEARING ON THE HEADER PLUS WEIGHT OF WALL BEARING ON HEADER.
- 19. MANUFACTURER SHALL DESIGN JOIST AND JOIST GIRDERS IN ACCORDANCE WITH THE UL DESIGN REQUIREMENTS IN ORDER TO ACHIEVE THE FIRE RATING SPECIFIED IN THE ARCHITECTURAL DRAWINGS/SPECIFICATIONS.
- 20. JOIST MANUFACTURER SHALL ALIGN WEB MEMBERS OF ADJACENT JOISTS WITH THE SAME DEPTH TO PERMIT MECHANICAL, ELECTRICAL, AND PLUMBING APPURTENANCES TO PASS THROUGH THE JOIST.

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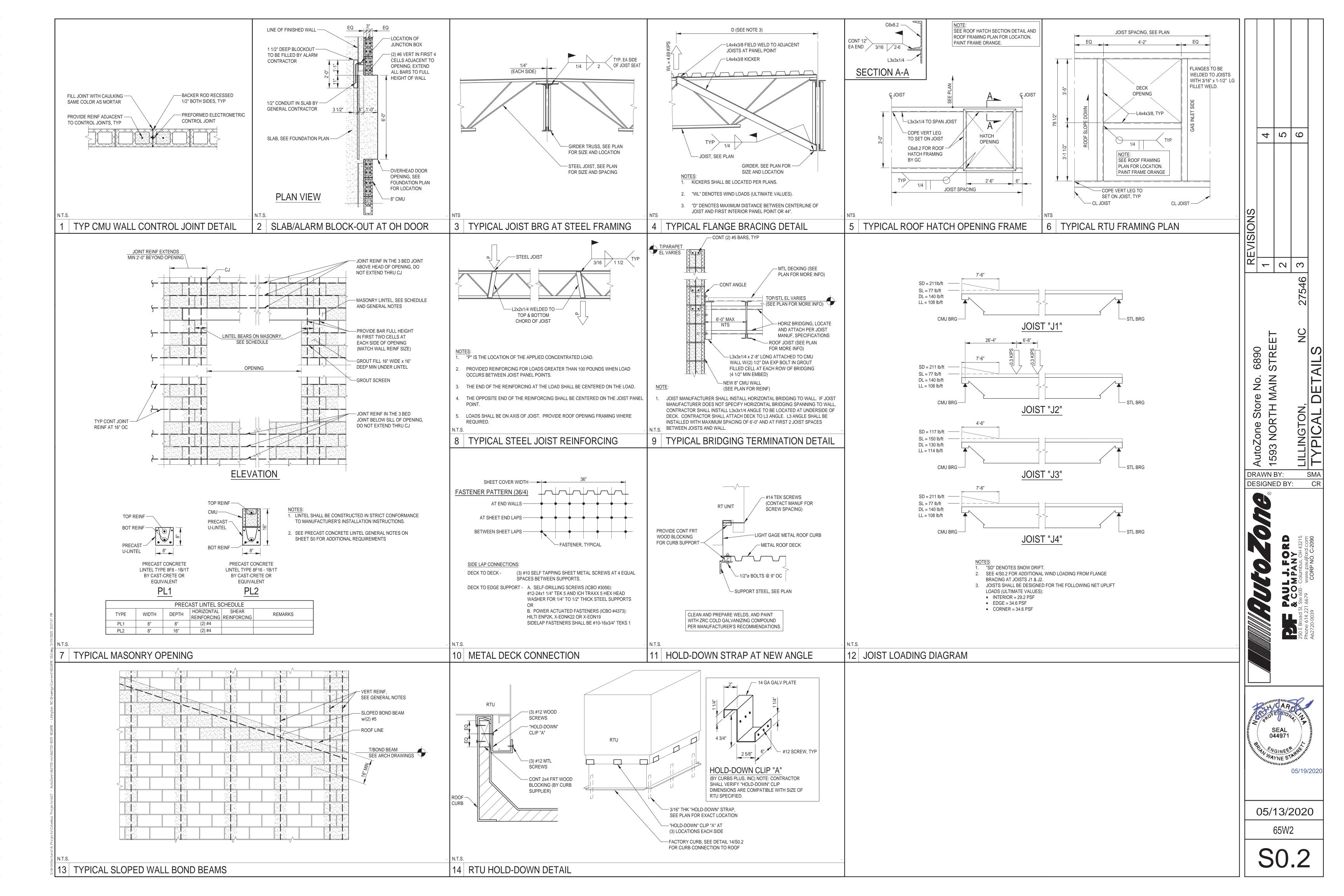


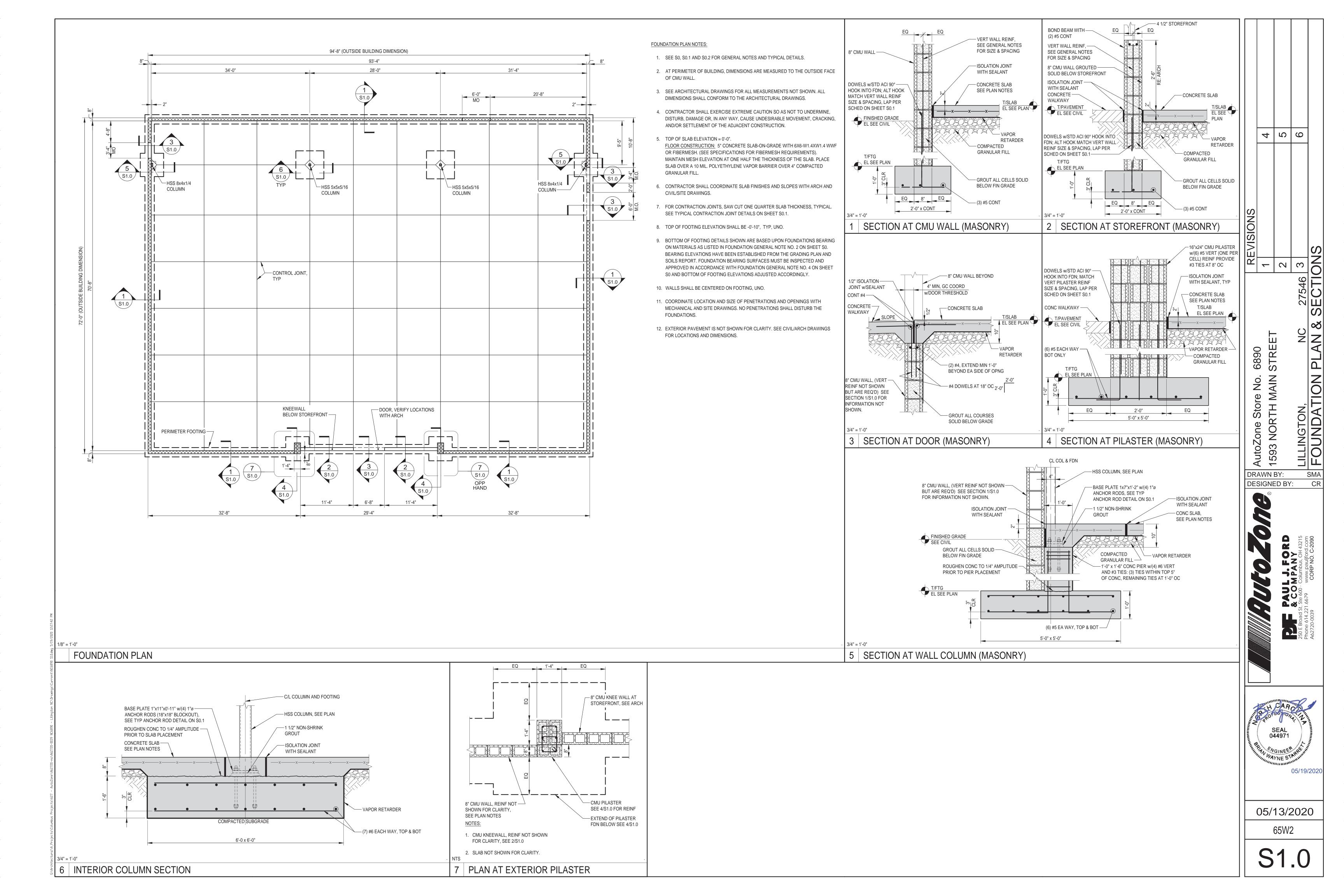
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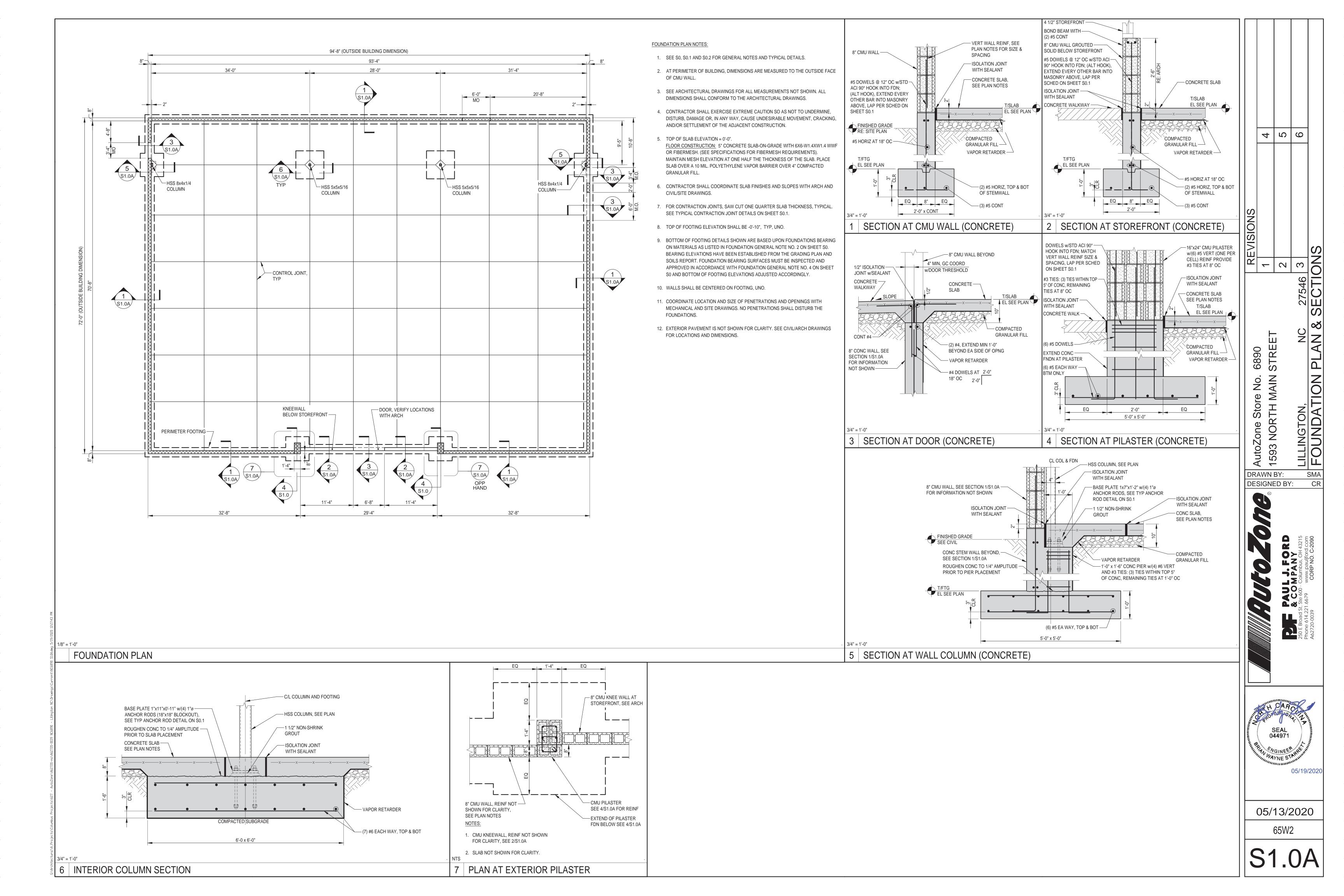
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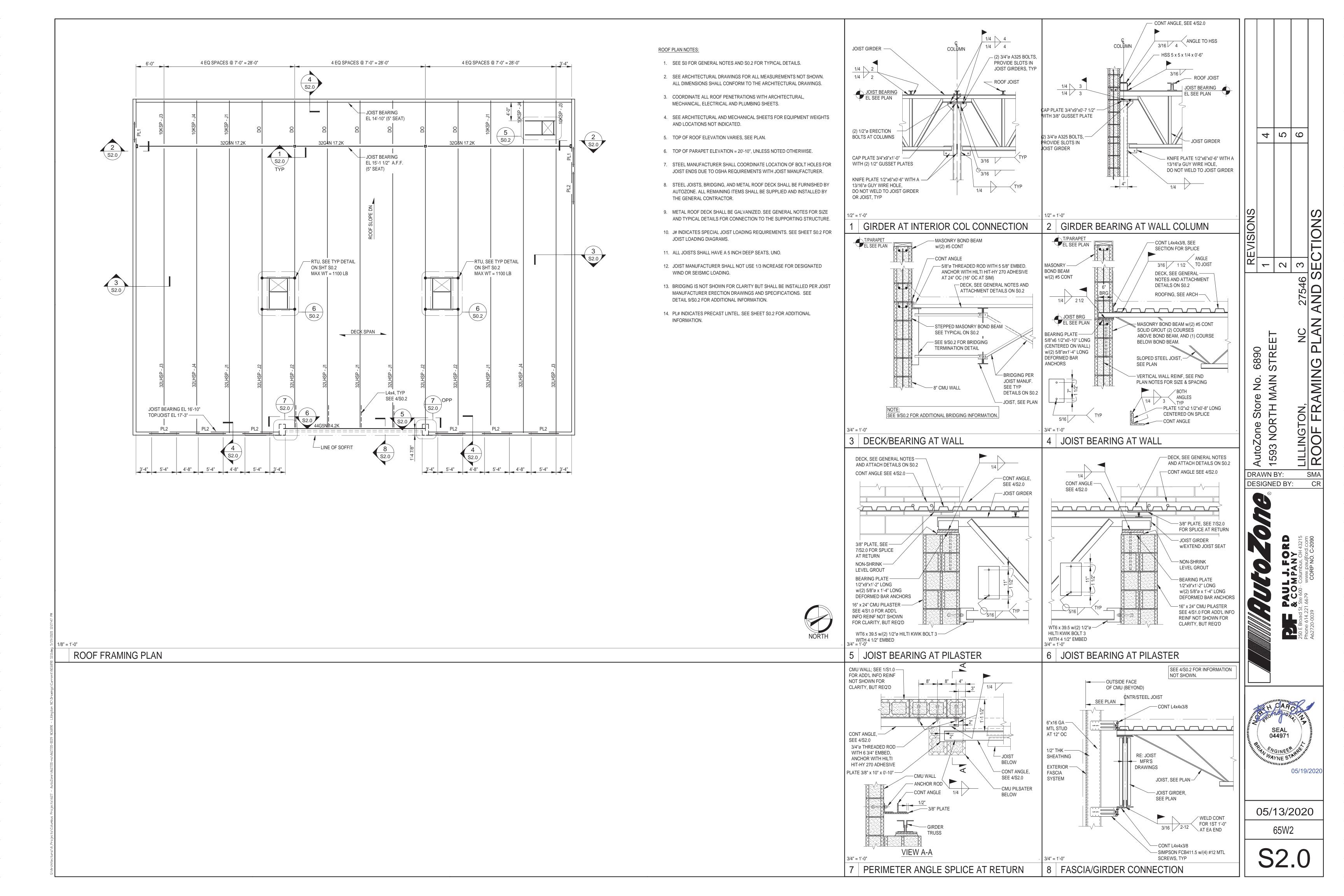
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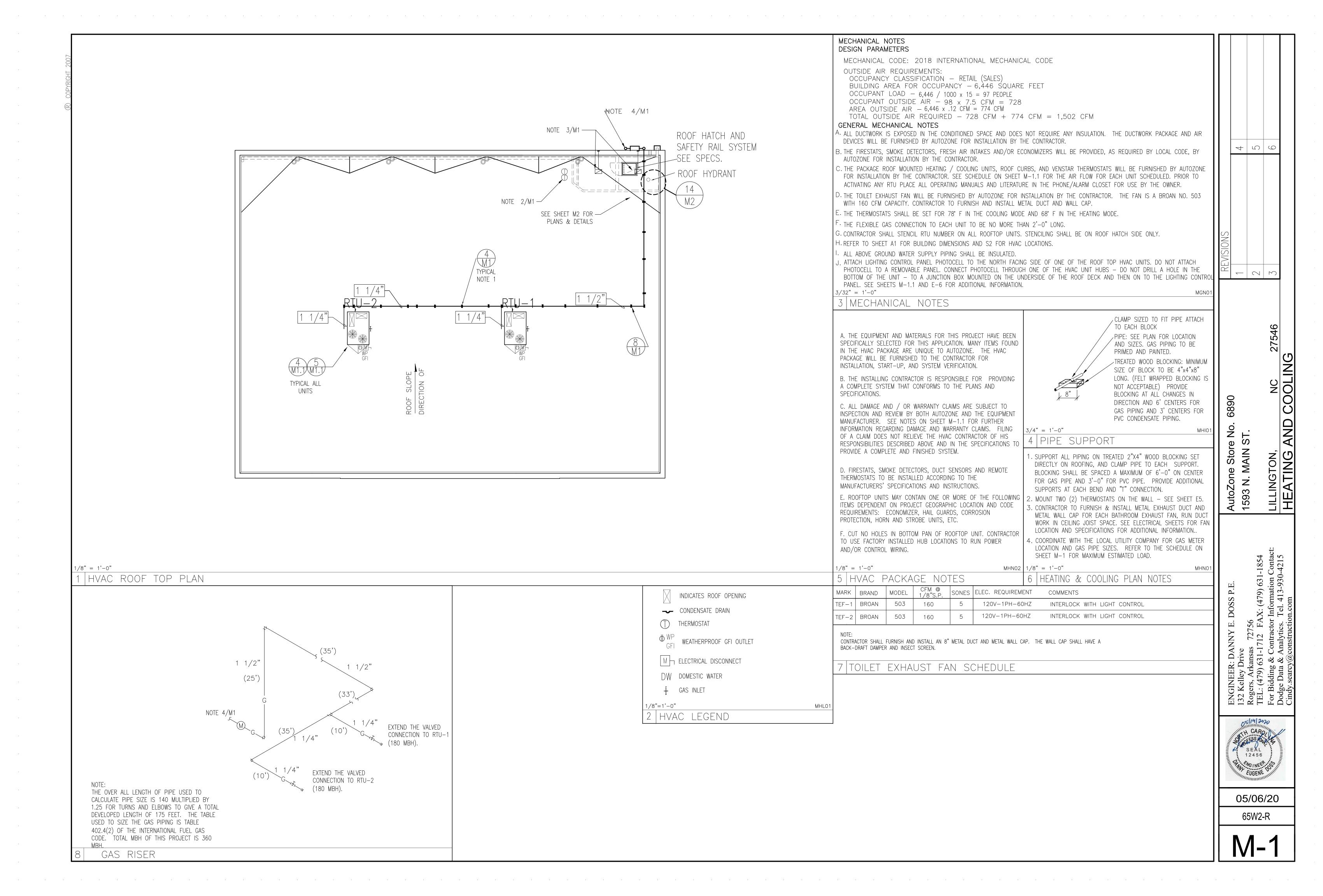
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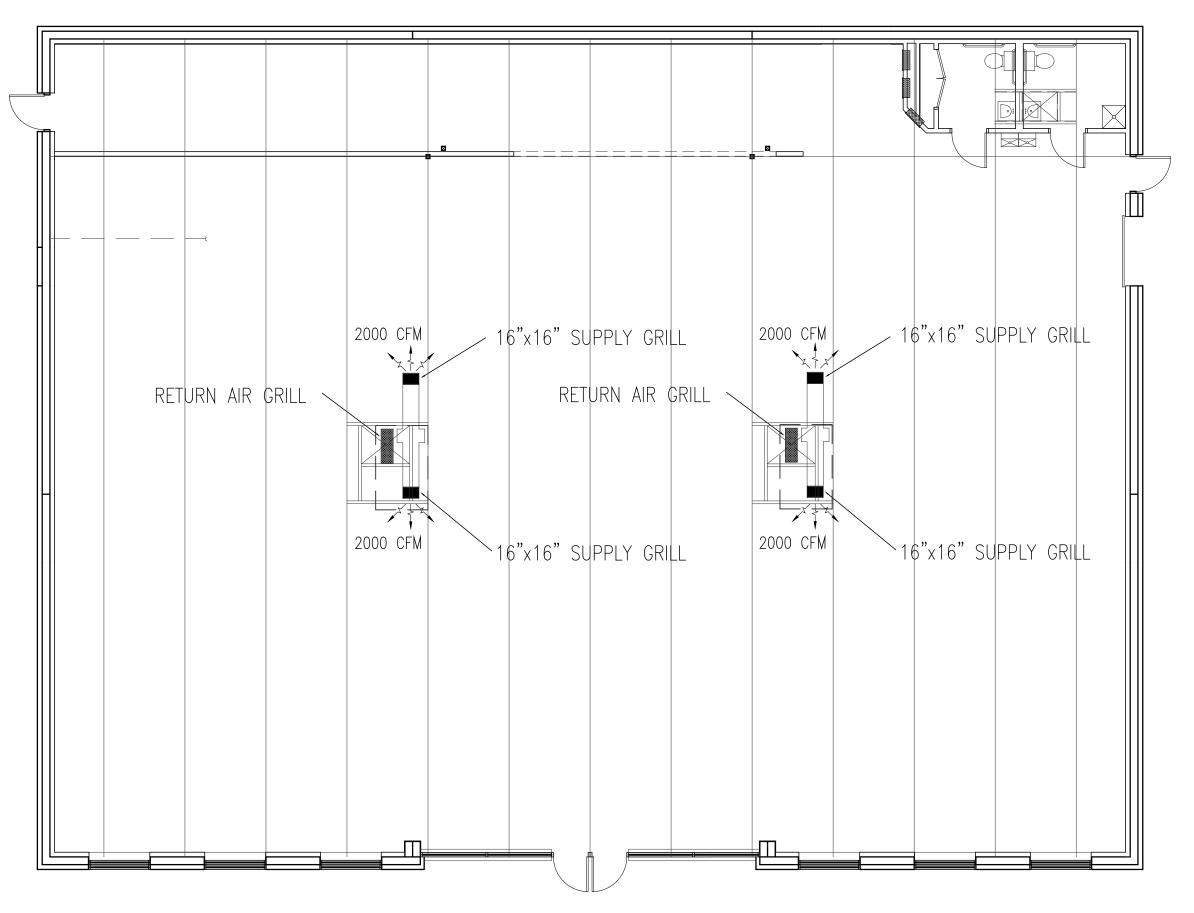












PACKAGE DUCTWORK NOTES:

- 1. PACKAGE DUCTWORK IS FURNISHED BY AUTOZONE AND INSTALLED
- BY CONTRACTOR.
- 2. THE BOTTOM OF THE DUCTS ARE TO BE NO LOWER THAN THE BOTTOM OF THE ROOF JOISTS.
- 3. DUCTWORK IS 22ga. STEEL INTERNALLY INSULATED.
- 4. DUCTWORK IS SHIPPED K/D DISASSEMBLED, CONTRACTOR TO
- ASSEMBLE IN THE FIELD.
- 5. SUPPLY REGISTERS ARE SINGLE DEFLECTION WITH CURVED
- ADJUSTABLE BLADES.
- 6. FACE BARS ON GRILLES ARE ADJUSTABLE TO CUSTOMIZE THE
- THROW AND PROFILE. 7. GRILLES ARE DESIGNED TO THROW 24 FEET AT 450 CFM

MHN04

.03 STATIC PRESSURE DROP (ALL GRILLES). 9. RETURN GRILL IS 1/2" STANDARD EXPANDED METAL WITH GASKET.

8. DUCT PACKAGE IS DESIGNED TO HANDLE 1600 - 2400 CFM WITH

BRACKET ON BOTTOM OF RETURN AIR DUCT CLOSE TO SUPPLY SIDE. EXTEND WIRE FROM SENSOR THRU DUCT AND INSERT THRU TUBE AND LEAVE 1/4" STICKING BELOW SMOKE DETECTOR IS FACTORY BOTTOM OF TUBE. MOUNTED IN THE RETURN AIR SECTION OF THE UNIT. 1 PAIR SENSOR TOBE DRAWINGS 18 GAUGE, SHIELDED CABLE BETWEEN SMOKE DETECTOR AND RTU CONTROLS J-BOX AT DECK FOR RTU POWER & CONTROLS AND LCP PHOTOCELL SEE NOTE: CONTROL WIRING FOR E-1 & 6 FOR ADDITIONAL INFO. EACH RTU MUST BE RUN IN SEPARATE CONDUIT. SEE MAKE CONNECTIONS THRU HUB PROVIDED SHEET E-6 FOR ADDITIONAL IN BOTTOM OF THE RTU. DO NOT CUT INFORMATION. PAN TO GAIN ACCESS. ALL CONNECTIONS 18 GAUGE 4 CONDUCTOR UNSHEILDED BETWEEN INSIDE OF RTU TO BE WEATHERPROOF. RTU CONTROLS AND T-STAT. 18 GAUGE 3 CONDUCTOR BETWEEN TEMP SENSOR AND RTU CONTROLS. RUN ALL CABLE IN 1/2" CONDUIT. No Scale 5 ROOF CURB

POWER PLAN

/8" = 1'-0"

1. YORK ROOF TOP UNITS WILL BE FURNISHED TO THE JOB SITE ON A SCHEDULED DELIVERY BASIS. THE CURBS, DUCT DROP AND DUCT PACKAGES WILL COME SEPARTELY FROM A THIRD PARTY YORK VENDOR. ALL ITEMS MUST BE REQUESTED WITH A NEEDED DELIVERY DATE IN THE AFI SCHEDULE REQUEST FORM AND SENT TO THE VENDOR AS SOON AS THE JOB NTP'S.

2. ONCE CONSTRUCTION HAS COMMENCED IT WILL BE THE GENERAL CONTRACTOR'S RESPONSIBILITY TO STAY IN TOUCH WITH THE YORK CONTACT AND TO SCHEDULE THE ACTUAL DATE THE ROOF TOP UNIT ARE TO BE DELIVERED TO THE JOB SITE. 3. IT IS EXPECTED THAT WHEN UNITS ARE DELIVERED THEY WILL GO FROM TRUCK TO ROOF AND NOT BE LEFT ON THE GROUND. IF THE JOB SCHEDULE WILL NOT ACCOMMODATE THE ORIGINAL

REQUESTED DELIVERY DATE THEN THE GENERAL CONTRACTOR MUST CALL YORK AND RESCHEDULE. UNITS ARE NOT TO BE LEFT ON THE GROUND WHERE THEY MAY BE SUBJECT TO DAMAGE.

4. WHEN UNITS ARRIVE THEY SHOULD BE INSPECTED FOR DAMAGE, AS THE AUTOZONE POLICY ONLY ALLOWS THE GENERAL CONTRACTOR 72 HOURS TO REPORT DAMAGE OR ANY PROBLEM, AFTER WHICH THE ISSUE BECOMES THE CONTRACTORS TO RESOLVE. IF THE DAMAGE IS SUPERFICIAL OR COSMETIC THEN THE UNITS SHOULD BE ACCEPTED, INSTALLED AND REPAIRED IN THE FIELD. IF THE DAMAGE AFFECTS THE OPERATION OF THE UNIT THEN IT SHOULD BE NOTED ON THE BILL OF LADING, NOTIFY YORK TO SCHEDULE A REPLACEMENT AND PICK-UP, AND NOTIFY THE AUTOZONE AFI MANAGER. SEE AUTOZONE AFI POLICY FOR

ADDITIONAL INFORMATION. 5. CONTACTS:

TO SCHEDULE DELIVERY: SHAWNA STEPHENS 405-419-6272 TO REPORT DAMAGE: SHAWNA STEPHENS 405-419-6272 FOR TECHNICAL SUPPORT: LEROY BERRY 800-481-9738 OPTION 4 AUTOZONE AFI MANAGER: LEW ELLIS 901-495-8707

1/8" = 1'-0"2 YORK HVAC NOTES

1. DOA (Dead on Arrival): LABOR ALLOWANCES APPLY ONLY TO THOSE SITUATIONS WHERE DEFECTS HAVE OCCURRED DIRECTLY AS A RESULT OF FACTORY WORKMANSHIP AND/OR MATERIALS. REIMBURSEMENT WILL NOT BE ALLOWED WHERE DAMAGE HAS BEEN SUSTAINED AS A RESULT OF MISAPPLICATION OR IMPROPER INSTALLATION.

2. FOR ALL DOA AND ANY OTHER WARRANTY ISSUES THE CONTRACTOR SHALL CALL THE YORK WARRANTY GROUP FOR INSTRUCTIONS ON HOW TO PROCEED. YORK WILL RESPOND TO ALL WARRANTY REQUESTS OR ISSUES WITHIN 24 HOURS. PROCEEDING WITH WORK IN ADVANCE OF NOTIFYING YORK MAY RESULT IN CONTRACTOR NOT BEING REIMBURSED FOR ALL OF HIS COSTS.

3. AS YORK HAS PRE ASSIGNED LABOR VALUES TO ALL PARTS OF THE ROOF TOP UNIT, CONTRACTOR MUST GET YORK'S CONCURRANCE IN THE CONTRACTOR'S ESTIMATED LABOR CHARGE TO DO THE WORK PRIOR TO COMMENCING, OR CONTRACTOR MUST AGREE TO YORK'S LABOR ALLOWANCE. PARTS WILL BE FURNISHED AT NO COST BY YORK, THEY MAY ASK CONTRACTOR TO RETURN THE DEFECTIVE PART AND CONTRACTOR MUST DO SO.

4. FOR EXPEDITED SERVICE CONTRACTOR SHOULD HAVE THE MODEL AND SERIAL NUMBER OF THE UNIT IN QUESTION PRIOR TO MAKING A CALL FOR WARRANTY SERVICE.

5. CONTACTS: FOR WARRANTY ISSUES: JESSICA RIBBLE 800-481-9738 OPTION 4 FOR TECHNICAL SUPPORT: LEROY BERRY 800-481-9739 OPTION 4

MHN03 1/8" = 1'-0" 3 YORK WARRANTY NOTES

						COOLING	DATA			HEAT INPL	JT	ELECTRICAL REQ.		E00N0N17ED	0011	LINUT
MARK	MANUFACTURER MODEL NO.	SUPPLY AIR	MIN. O.S.A.	E.S.P.	ENT. AIF D.B. W.I		SENSIBLE	EER	ENT. AIR	INPUT	OUTPUT	UNIT M.C.A. 208/3ø/60Hz	BREAKER	ECONOMIZER	COIL COATING	UNIT WEIGHT
	MODEL NO.	AIIV	U.S.A.		ן .ט.ט. אין	D.			D.B.			200/ 34/ 60112				
RTU-1	YORK ZYG12D2	4000 CFM	750 CFM	0.25 IN.	80 67	7 131,300 BTU	94,100 BTU	11.5	70	180,000 BTU	144,000 BTU	51.4	60	Required	No	1005
RTU-2	YORK	4000 CFM	755 CFM	0.25 IN.	80 67	7 131,300 BTU	94,100 BTU	11.5	70	180,000 BTU	144,000 BTU	51.4	60	Required	No	1005

EXTERIOR TEMP. SENSOR

ECONOMIZER

SENSOR TUBE

4 ROOFTOP UNIT - VIEW OF RTU

FLASHING AND FIBER CANT STRIP AT

BY RTU SUPPLIER)

METAL STRAP .

3"x3"x1/4" ANGLE RTU

FRAME SEE? STRUCTURAL

DRAWINGS

TO CONTROLS IN RTU

REMOTE TEMP SENSOR IS FACTORY \

INSTALLED AND CAN BE FOUND IN THE FILTER SECTION. MOUNT

MECHANICAL ROOF CURB (ROOF CURB FURNISHED CONVENIENCE OUTLET BY ELECTRICAL

ROOFING AND INSULATION ON METAL DECK.

REFER TO STRUCTURAL PLANS

FOR PROPER ROOF CURB

/ STEEL JOIST

SUPPLY AIR DIFFUSER ADJUSTED IN THE FIELD

,WEATHERPROOF J-BOX MOUNT

TO NORTH FACING SIDE OF

TO J-BOX AND WIRE THRU

UNIT TO J-BOX AT DECK

CONDENSATE DRAIN

COUNTER- FLASHING BY

GENERAL CONTRACTOR

RTU. MOUNT LCP PHOTOCELL

MOUNTING DETAILS.

HORN/STROBE UNIT

OF THE UNIT.

FACTORY MOUNTED AND WIRED

NON-FUSED DISCONNECT

CONTRACTOR SUPPLIED

OUTLET (ONE PER RTU)

WEATHER-PROOF GFI CONVENIENCE

CANT STRIP

ROOF DECK

LOCATION IF REQUIRED

WIRE MOUNTED REMOTE TEMPERATURE SENSOR PROBE INSIDE RETURN AIR DUCT, EXTEND

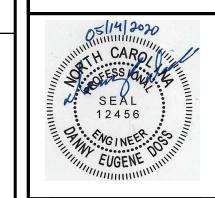
WIRE THROUGH SENSOR TUBE. LET WIRE HANG 1/4" BELOW BOTTOM OF TUBE.

JUNCTION BOX AND CONDUIT MOUNTED TO DECK PER NEC

SMOKE DETECTOR IS FACTORY MOUNTED IN THE RETURN AIR SECTION

CONTRACTOR. SEE SHEET E1.

3/32" = 1'-0"6 ROOFTOP UNIT HEATING AND COOLING SCHEDULE



MHC01

ENGINEER: DANNY E. D 132 Kelley Drive Rogers, Arkansas 72756 TEL: (479) 631-1712 FAX For Bidding & Contractor In Dodge Data & Analytics. T

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