iRooFA™

Instant Roof Framing Analysis

www.iroofa.solar

tel: 540.313.5317 - email: info@iRooFA.solar

STRUCTURAL ANALYSIS

for the

ROOFTOP PV SOLAR INSTALLATION

Project: Gordon Oleson, 365Robeson St, Spring Lake, NC 28390

Prepared for:

Top Tier

1530 Center Park Dr - Charlotte, NC 28217

Calculation Report Index

Project No: 66.420460, Rev. 0

Report Date: 08/04/2025

Report Prepared by:

NORTH CAROLINA PROFESSIONAL SEAL 043326 ENGINEER RICHARD PANTEL

Richard Pantel, P.E.
NC License No. 43326
Sealed 08/04/2025

Richard Pantel Digitally signed by Richard Pantel
Date: 2025.08.04 05:02:26 -05'00'

Loading Summary

Exposure and Occupancy Categories		
B		*Exposure Category (ASCE 7-10 Table 26.7.3, Page 274)*
II		*Building Use Occupancy / Risk Category (ASCE 7-10 Table 1.5-1, Page 5)*

Wind Loading:			
v	120	*mph*	*Over-ridden per client request. Original data from Municipality provided wind / snow loadings.*
qz	21.93	*psf*	*Velocity qz, calculated at height z [ASD]*

Snow Loading			
pg	15.00	*psf*	*Ground Snow Load pg (Over-ridden per client request. Original data from Municipality provided wind / snow loadings.)*
Total Snow Load			
ps	15.00	*psf*	*Effective snow load on roof and modules*

Module Data			
JA Solar: JAM54S31-405/MR			
Dimensions	*mm*	*ft*	*in*
Length	1,722	5.65	67.80
Width	1,134	3.72	44.65
Area (m^2, ft^2)	2.0	21.02	
Weight	kg	lb	
Module	21.50	47.40	

Roof Panel (Cladding) Loading Summary		Module Loading Summary			
Support Point Loads		*Upward*	*Upward*	*Upward*	*Downward*
Roof Zones		1	2	3	All
Net load per module	*lb*	-41	-68	-68	276

Positive values indicate net downward force

Primary Stanchion: IronRidge HALO ULTRAGRIP - (QM-HUG-01-B1)

StanchionFastener Pull-out and Spacing Calculations				
Framing spacing		*ft*	2.00	
Rails / Module		*ea*	2	
Max proposed stanchion span		*ft*	6.00	
# fasteners per stanchion			2	
Bolt thread embedment depth		*in*	1.75	
Safety Factor			1.10	
Pull-out for #14 threaded fasteners		*lb/in*	134	*lb per inch of embedment*
Factored max fastener uplift capacity		*lb*	425	
Fastener details	*Material* Stainless	*Size*	#14	*Predrill hole 0.12" dia or use self tapping*
Max stanchion uplift capacity		*lb*	1100	
Max support point uplift capacity		*lb*	425	

Roof Zones		*1*	*2*	*3*
Net lift per module	*lb*	41	68	68
Min tot bolt thread embedment depth rq'd	*in*	0.17	0.28	0.28
Net uplift pressure 7. 0.6D - 0.6W	*psf*	-2.42	-4.00	-4.00
Allowable lift area / support point	*sf*	175.38	106.12	106.12
Max rail span per support spacing	*ft*	6.00	6.00	6.00
Landscape Modules				
Length along rafter	*ft*	3.72		
Lift calc'ed max stanchion EW spacing	*ft*	> 6	> 6	> 6
Max stanchion EW spacing	*ft*	6.00	6.00	6.00
Maximum module area / support point	*sf*	11.16	11.16	11.16
Factored lift per support point	*lb*	-27	-45	-45
Portrait Modules				
Length along rafter	*ft*	5.65		
Lift calc'ed max stanchion EW spacing	*ft*	> 6	> 6	> 6
Max stanchion EW spacing	*ft*	6.00	6.00	6.00
Maximum module area / support point	*sf*	16.95	16.95	16.95
Factored lift per support point	*lb*	-41	-68	-68

Alternate Stanchion Fastener Pull-out and Spacing Calculations IronRidge HALO ULTRAGRIP - (QM-HUG-01-B1) - 6 screws				
Framing spacing		*ft*	2.00	
Rails / Module		*ea*	2	
Max proposed stanchion span		*ft*	6.00	
# fasteners per stanchion			6	
Bolt thread embedment depth		*in*	0.50	
Safety Factor			1.10	
Pull-out for 1/4 threaded fasteners		*lb/in*	134	
Factored max fastener uplift capacity		*lb*	364	
Fastener details	*Material* Stainless	*Size*	1/4	*Predrill hole 0.12" dia or use self tapping*
Max stanchion uplift capacity		*lb*	1100	
Max support point uplift capacity		*lb*	364	

Roof Zones		*1*	*2*	*3*
Net lift per module	*lb*	41	68	68
Min tot bolt thread embedment depth rq'd	*in*	0.06	0.09	0.09
Net uplift pressure 7. 0.6D - 0.6W	*psf*	-2.42	-4.00	-4.00
Allowable lift area / support point	*sf*	150.33	90.96	90.96
Max rail span per framing spacing	*ft*	6.00	6.00	6.00
Landscape Modules				
Length along rafter	*ft*	3.72		
Lift calc'ed max stanchion EW spacing	*ft*	> 6	> 6	> 6
Max stanchion EW spacing	*ft*	6.00	6.00	6.00

Maximum module area / support point	*sf*	11.16	11.16	11.16
Factored lift per support point	*lb*	-27	-45	-45
Portrait Modules				
Length along rafter	*ft*	5.65		
Lift calc'ed max stanchion EW spacing	*ft*	> 6	> 6	> 6
Max stanchion EW spacing	*ft*	6.00	6.00	6.00
Maximum module area / support point	*sf*	16.95	16.95	16.95
Factored lift per support point	*lb*	-41	-68	-68

Stanchion support threaded fastener sizes are indicated in the Module Loading Summary table above. Lift forces were determined from GCp and other coefficients contained in the ASCE nomographs

Conclusions

We were asked to review the roof of Gordon Oleson, located at 365Robeson St, Spring Lake, NC, by Top Tier, to determine its suitability to support a PV solar system installation.

The referenced building's roof structure was field measured by Top Tier. The attached framing analyses reflect the results of those field measurements combined with the PV solar module locations shown on the PV solar roof layout design prepared by Top Tier. Loads are calculated to combine the existing building and environmental loads with the proposed new PV array loads.

The IronRidge XR10 Rail racking and IronRidge HALO ULTRAGRIP - (QM-HUG-01-B1) along with the alternate IronRidge HALO ULTRAGRIP - (QM-HUG-01-B1) - 6 screws stanchions were selected for this project by Top Tier. The racking and support stanchions shall be placed as shown on their plans, dated 08/01/2025, and shall be fastened to the roof framing using fastener sizes indicated in this report. Rack support spacing shall be no more than that shown above. Note that support points for alternating rows shall share the same truss. Intermediate rows shall move the support points laterally to the next truss.

Google Location Map

Framing Summary

	Ex. Framing	Total Ex DL
MP 1: Truss @ 24" OC	0.79 psf	5.94 psf

** Wood species used in these calculations assumes spruce, pine or fir, #2 grade.*

Based upon the attached calculations, the existing roof's framing system is capable of supporting the additional loading for the proposed PV solar system along with the existing building and environmental loads. No supplemental roof framing structural supports are required. No further structural alterations or modifications are needed to support the system. Minimum required anchorage fastening is described above.

Wood fastener notes: 1) Fastener threads must be embedded in the side grain of a roof support structural member or other structural member integrated into the building's structure. 2) Fastener must be located in the middle third of the structural member. 3) Install fasteners with head and where required, washer, flush to material surface (no gap). Do not over-torque.

References and Codes:

1) *ASCE 7-10 Minimum Design Loads for Buildings and Other Structures*
2) *2015 IBC*
3) *2018 NC Building Code*
4) *American Wood Council, NDS 2018,Table 12.2A, 12.3.3A.*
5) *American Wood Council, Wood Structural Design, 1992, Figure 6.*

Roof Structural Calculations for PV Solar Installation **Array AR-1**
Location: MP 1 ***Roof shape: Gable / Hip***
Member: Truss - Total Length 15 ft, Unsupported 15 ft

Geometric Data			
Θ	*deg.*	30.0	*Angle of roof plane from horizontal, in degrees*
ω	*deg.*	0.0	*Angle the solar panel makes with the roof surface*
L	*ft.*	55.42	*Length of roof plane, in feet (meters)*
W	*ft.*	13.83	*Plan view width of roof plane, in feet (meters)*
h	*ft.*	14.23	*Average height of roof above grade, in feet (meters)*

Roof Wind Zone Width			
	use, a =	3.00	*ft*

Wind Velocity Pressure, q_z evaluated at the height z						
q_z=	21.93	*psf*	*Vasd* q_z=	13.17	*psf*	Basic wind pressure
V=	120	mph				

Framing Data		
Wood type	US Spruce	
Wood source, moisture content	White 0.12%	
# Framing Members / Support		1
Rafter / Truss OC	*in*	24.00
Member Total Length	*ft*	15.00

3	# Rafters / Rack Support Width
6.00	Rack Support Spacing (ft)
72.00	Max. Rack Support Spacing (in)
2	Max # of mod's / Truss top chord

Member Properties		**Member**
Name		(1) 2x4
Repetitive Member Factor (Cr)		1.15

** Mem properties based upon field measurements*

Truss top chord

Module Data			
Weight	kg	lb	psf load
Module	21.50	47.40	2.26
4 Stanchions	0.91	2.0	0.10

Existing Dead Loads	*Units*	*Value*	*Description*
Roof Deck & Surface Material*	*psf*	5.15	*Truss members' self weight added to FEA analysis*

** Roof surface: Shingles, Asphalt, Architectural (Typical)*

Rack Support Spacing and Loading				
Across rafters	*ft*	6.0		
Along rafter slope	*ft*	5.6		
Area / support point	*sf*	16.9		
Uphill gap between modules	*in*	1.0	0.08	*ft*

Member Total Length	*ft*	15.00	
Maximum member free span	*ft*	15.00	Truss top chord span

	Zones	1	2	3
	GCp	-0.94	-1.14	-1.14

Downward, Zones 1, 2 & 3
GCp 0.87

ASCE 7-10 Chapter 2 Combinations of Loads, Table 2.4, Page 8 (in psf)				
Zones	1	2	3	1, 2 & 3
2.2 SYMBOLS AND NOTATION	*Module Upward*	*Module Upward*	*Module Upward*	*Downward*
D = dead load of PV Module + Stanchion	2.35	2.35	2.35	2.35
S = snow load	15.00	15.00	15.00	15.00
W = wind load = (Vu Windload) = (Vasd Windload / 0.6)	-12.32	-14.96	-14.96	11.43

2.4 Combining Nominal Loads Using Allowable Stress Design (in psf)				
2.4.1 Basic Combinations. Loads listed herein shall be considered to act in the following combinations; whichever produces the most unfavorable effect in the building, foundation, or structural member being considered. Effects of one or more loads not acting shall be considered.				
Combination Formulae	*Upward*	*Upward*	*Upward*	*Downward*
Use this loading combination for DOWNWARD for Proposed PV Dead Load				
6. D + 0.75L - 0.75(0 or 0.7)eE + 0.75S	17.35	17.35	17.35	18.74
Module Support point load (lb)	294	294	294	318
Cr Factored Module Support point load (lb)	256	256	256	276

Use this loading combination for UPWARD for Proposed PV Dead Load				
7. 0.6D - 0.6W	-2.42	-4.00	-4.00	4.97
Module Support point load (lb)	-41	-68	-68	84

DOWNWARD

Presume loading directly over member.

Combined Dead and Wind Pressure Downward Loading					
	Truss top chord span				
PV Module Row	Point load loc's from Left support	Point Load #'s	Module Support Point Load	Comment	Module Orientation
	ft from left		*lb*		
1	2.42		276		Portrait
1	8.07			Support placed on adjoining truss	Portrait
2	8.15			Support placed on adjoining truss	Portrait
2	13.80		276		Portrait

Truss Data and Loading for MP 1

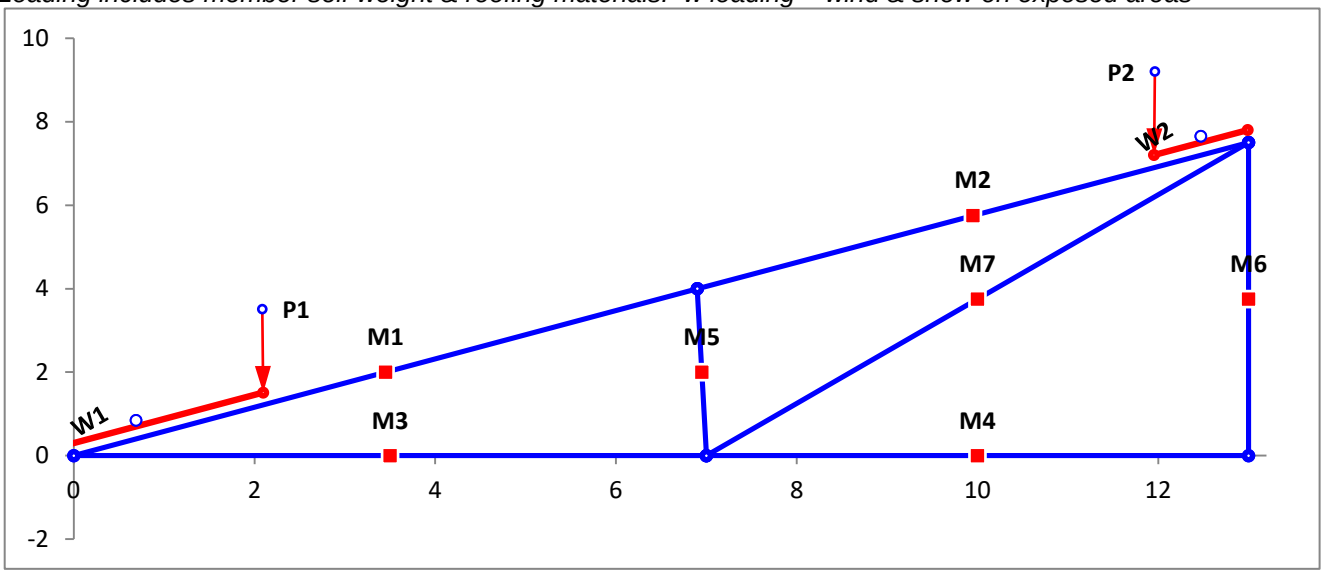
Roof slope (degrees)	30.00
Top ridge height above floor plane	7.50

	Length of roof plane	15.00
	Length of floor plane	13.00

Truss Segments

Roof Plane		*Floor Plane*		*Diagonals*		*Diagonals*	
Mem #	*Mem Type*	Mem #	*Mem Type*	Mem #	*Mem Type*	Mem #	*Mem Type*
1	2x4	3	2x4	5	2x4	7	2x4
2	2x4	4	2x4	6	2x4		

** Loading includes member self weight & roofing materials. w loading = wind & snow on exposed areas*



Snow Loading Analysis

where:

		Fully Exposed	Exposure category
Ce	=	0.9	Exposure Factor, Ce (ASCE 7-10 Table 7.3-1, Page 61)
Ct	=	1.0	Thermal Factor, Ct (ASCE 7-10 Table 7.3-2, Page 61)
Is	=	1.0	Snow Importance Factor, Is (ASCE 7-10 Table 1.5-2, Page 5)
p_g	=	15.00	Ground Snow Load pg (Over-ridden per client request. Original data from Municipality
p_f	=	**0.7CeCtIsPg**	Flat Roof Snow Load, pf (ASCE 7-10 Table 7.3-1, Page 61)
p_f	=	**9.45**	psf
			but where Pf is not less than the following:
			Minimum Snow Load pm (ASCE 7-10 Table 7.3.4, Page 62)
p_m	=	**15.00**	When Pg <=20 psf, then use Pf = Pg x Is
p_f	=	**15.00**	psf. Resultant Snow pressure to be used with Roof slope factor below
p_s	=	$C_s p_f$	Sloped Roof Snow Load ps (ASCE 7-10 Table 7.4, Page 61)
			Roof Type Warm Roofs

Roof slope factor Cs for Warm Roofs, where Ct = 1.0

Roof surface condition = Slippery Roof

C_s	=	1.00	Roof Slope Factor, Cs (ASCE 7-10 Table 7.4-1a, Page 62)

Total Snow Load

p_s	=	**15.00**	**psf**	Roof snow load

FEA Calculation Results for Roof Plane MP 1 for Top Tier Client GORDON OLESON

IDSPL - 2D Frame Analysis of a 2D frame subject to distributed loads, point loads and moments

Equilibrium check	FX	FY	0.00051
Total applied forces	0.00	1511	
Total output reactions	0.00	-1511	
Output error	-1.04E-13	-2.05E-12	

	Shear	Ax
Max (psi)	5	208
Allowable (psi)	115	5,610
# of segments/beam	1	

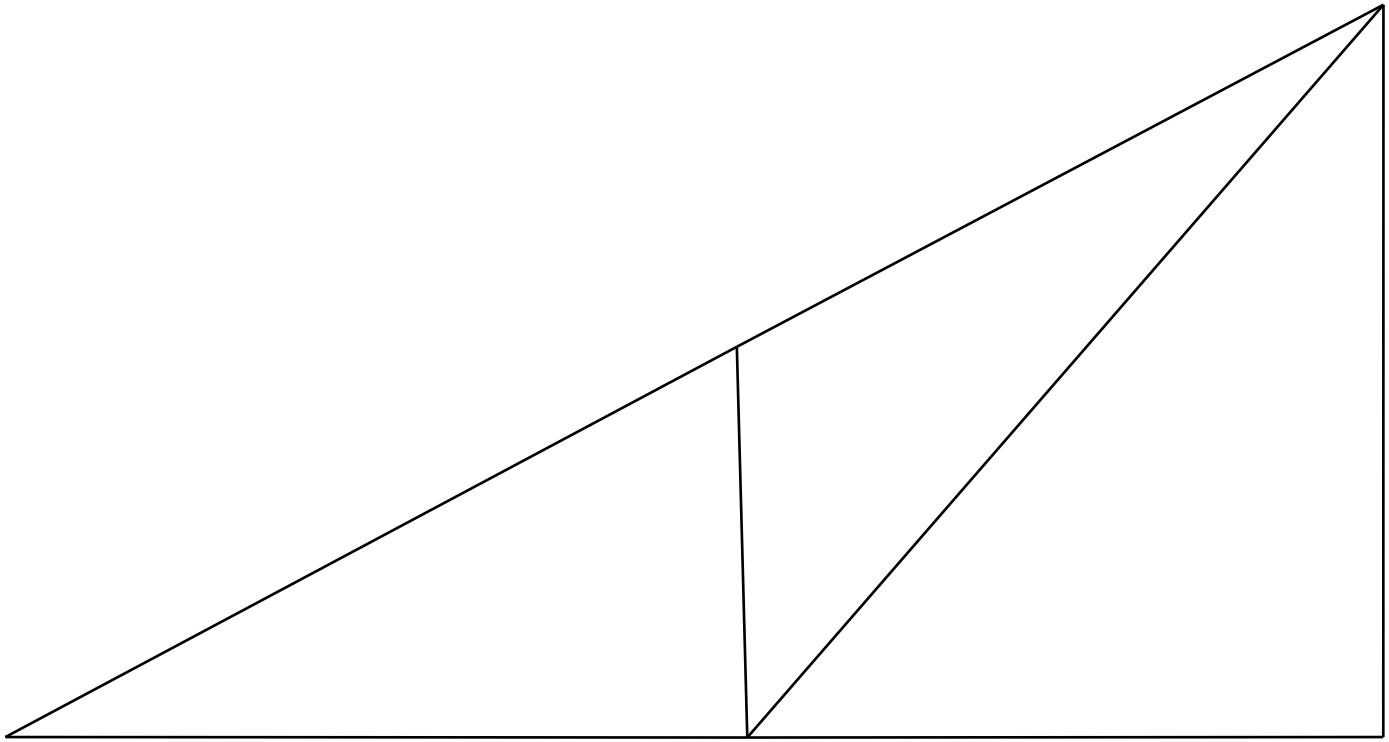
Maximum Deflections	
-1.01E-03	-2.61E-03

** vertical deflections do not take into account any supporting intermediate walls*

Node Results			Beam End Results			
Direction	Deflection	Reaction	Beam	Shear	Axial	BM
DX1	0.00E+00	0	1-1	-152	620	0
DY1	0.00E+00	-421	1-2	193	420	0
RZ1	0.00E+00	0	2-1	-169	615	0
DX2	-9.89E-04	0	2-2	617	164	0
DY2	2.61E-03	0	3-1	21	-460	0
RZ2	0.00E+00	0	3-2	125	-460	0
DX3	4.59E-04	0	4-1	0	0	0
DY3	8.62E-04	0	4-2	0	0	0
RZ3	0.00E+00	0	5-1	0	420	0
DX4	-3.43E-04	0	5-2	0	408	0
DY4	2.44E-03	0	6-1	0	1090	0
RZ4	0.00E+00	0	6-2	0	1054	0
DX5	-3.43E-04	0	7-1	-10	-706	0
DY5	0.00E+00	-1090	7-2	24	-750	0
RZ5	0.00E+00	0				
Rel1-3	5.02E-04	0				
Rel1-6	6.76E-04	0				

Beam	X	Shear	Mom	Axial	DX	DY	RZ
1	0.00	-152	0	620	0.00E+00	0.00E+00	0.00E+00
1	7.98	129	386	457	-1.01E-03	-2.59E-03	3.89E-04
2	0.00	-169	0	615	-9.89E-04	-2.61E-03	0.00E+00
2	7.03	342	4495	322	2.21E-04	-7.25E-04	-4.90E-03
3	0.00	21	0	-460	0.00E+00	0.00E+00	0.00E+00
3	7.00	61	390	-460	-3.43E-04	-2.44E-03	4.57E-04
4	0.00	0	0	0	-3.43E-04	-2.44E-03	0.00E+00
4	6.00	0	0	0	-3.43E-04	-4.34E-19	-4.07E-04
5	0.00	0	0	420	-3.43E-04	-2.44E-03	0.00E+00
5	4.00	0	0	412	-9.89E-04	-2.61E-03	1.61E-04
6	0.00	0	0	1090	-3.43E-04	0.00E+00	0.00E+00
6	7.50	0	0	1061	4.59E-04	-8.61E-04	-1.07E-04
7	0.00	-10	0	-706	-3.43E-04	-2.44E-03	0.00E+00
7	9.60	19	3	-743	4.58E-04	-8.61E-04	-1.79E-04

* vertical deflections do not take into account any supporting intermediate walls



Scaled 2X Deflected Truss Plot
Roof Plane MP 1 for Top Tier Client GORDON OLESON